Gold Rich Planners & Surveyors Ltd.

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Your Ref.: A/NE-KLH/658

Our Ref.: P25046/TL25382

11 November 2025

The Secretary

By Post and E-mail

Town Planning Board

tpbpd@pland.gov.hk

15/F., North Point Government Offices

333 Java Road, North Point, Hong Kong

Dear Sir,

Submission of Further Information (FI)

Proposed Temporary Public Vehicle Park (Excluding Container Vehicle) and Associated Filling of Land for a Period of 3 Years in "Village Type Development" and "Agriculture" Zones, Various Lots in D.D. 9, Yuen Leng, Tai Po, New Territories (Application No.: A/NE-KLH/658)

We write to submit FI in response to departmental comment(s) for the captioned application.

Yours faithfully,
For and on behalf of
Goldrich Planners & Surveyors Ltd.

Francis LAU

Encl.

c.c.

DPO/STN, PlanD

<u>Further Information for Planning Application No. A/NE-KLH/658</u> Response-to-Comments

Comments from the Water Supplies Department

Contact person: Ms. Kathy WONG (Tel.: 2152 5752)

I. Comments	Responses
1. Having reviewed the information submitted in your FI05 on 22 Oct 2025, there is still insufficient information to prove and demonstrate that the "Conditions of Working within Gathering Grounds" will be fully complied with. Therefore, our comments issued to you by email on 20 Oct 2025 remain valid.	Your comments are well noted. The applicant will employ construction contractor when commencing the proposed development. The "Conditions of Working within Gathering Grounds" will be strictly complied with when commencing the proposed development.

Comments from the Drainage Services Department

Contact person: Ms. Karen HO (Tel.: 2300 1364)

II.	Comments	Responses
1.	The drainage flow path from the drainage facilities within the Site to the public drainage system / streamcourse / sea / any recognized drainage facilities shown in LandsD map should be provided in associated with supporting site photos.	Please refer to the Viewpoint photos.
2.	Please provide topography of the adjacent areas.	Please refer to Plan 5.2 for the topography of the adjacent areas.
3.	Catchpit should be provided at intersections of u-channels.	Catchpit is provided at intersections of u-channels. Please refer to Plan 5.1a.
4.	Please specify the extent of the proposed hoarding and location of the hoarding opening on the plan.	Noted. The extent of the proposed hoarding and location of the hoarding opening is specified on Plan 5.1a. Please refer to the Legend on the Plan.
5.	The proposed drainage works, whether within or outside the application site, should be constructed and maintained by the applicant at his expense. Please specify the	Noted. Please refer to Plan 5.1a.

Daga 1 of 2

	maintenance responsibility of the proposed drainage works in the proposal.	
6.	The existing u-channel proposed for discharge of runoff from the application site is not maintained by DSD. The applicant shall resolve any conflict / disagreement arisen for discharging runoff from the application site to the proposed discharge points. Moreover, the applicant should ensure that this drainage system and the existing downstream drains/channels/streams have adequate capacity to convey the additional runoff from the application site. Regular maintenance should be carried out by the applicant to avoid blockage of the system.	Noted.
7.	The proposed drainage works, whether within or outside the application site(s), should be constructed and maintained by the applicant(s) at his/their expense.	Noted.
8.	The applicant is required to rectify/modify the drainage system if it is found to be inadequate or ineffective during operation. The applicant shall also be liable for and shall indemnify Government against claims and demands arising out of damage or nuisance caused by failure of the system.	Noted.
9.	DSD noticed that the proposed drainage connection(s) to the surrounding/downstream area(s) will run through government land and/or other private lot(s). The applicant(s) shall demonstrate that the proposed drainage construction / improvement / modification works and the operation of the drainage can be practicably implemented on site.	Noted.

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10.	The applicant should take all precautionary measures to prevent any disturbance, damage and pollution from the development to any parts of the existing drainage facilities in the vicinity of the project. In the event of any damage to the existing drainage facilities, the applicant would be held responsible for the cost of all necessary repair works, compensation and any other consequences arising therefrom.	8	Noted.	
11.	The applicant should also be advised that the limited desk-top checking by Government on the drainage proposal covers only the fundamental aspects of the drainage design which will by no means relieve his obligations to ensure that (i) the proposed drainage works will not cause any adverse drainage or environmental impacts in the vicinity; and (ii) the proposed drainage works and the downstream drainage systems have the adequate capacity and are in good conditions to receive the flows collected from his project and all upstream catchments.		Noted.	

Viewpoint 1



Existing Public 800 UC

Viewpoint 2



Existing Public 800 UC to Existing Public 800 Underground UC

	1 For Catchment Area A					Ref.
ı	Area	Α	= 1080 m ²			
	Average slope, Distance on the line of natural flow,	Н	= 0.1 m per 100m			
1	Distance on the line of natural flow,	L	20 m			
			00.04			
	Time of concentraction,	t_o	$= 0.14465L / (H^{0.2}A^{0.1}) = 0.1446$	85 (20) / (0.1^0.2*	1080^0.1)	SDM 7.5.2 (d)
			= 2.3 min			
Ŀ	2 For Proposed UC in Catchmer	nt A	ea A			
ľ	2 TOT TOPOSCO GO III GALGIIII GI	1071	3471			_
L		Fro				
ı	Ground level (mPD) Invert level (mPD)	31. 30.				
ı	mivert level (iiii b)	00.	7 00.04			
ı	Width of u-channel,	W	= 350 mm			
ı	Length of u-channel, Depth of vertical part of u-channel,	L_c	= 80 m			
ı	Depth of vertical part of u-channel,	d	= 585 mm			
ı	Gradient of u-channel,	S_f	= (30.94-30.54)/80 $=$ 0.005			
ı						
	Cross-Section Area,	а	$= 0.5 \pi r^2 + w d = 0.5 \times 3.14 \times$	175^2 + 350 x 58	5	
			= 0.253 m ²		×	
ı	Wetted Perimeter,	p	$\pi r + 2 d = 3.14 \times 175 +$	2 x 585		LE.
ı			= 1.720 m			
ı	Hydralic radius,	R				SDM 8.2.1
ı			0.147 m			
1:	3 Use Manning Equation for esti	ma	ng velocity of stormwater		±	
	g against the same					
ı			= 0.016 for concrete line		#	SDM Table 13
	Allowable velocity,	٧	$= R^{1/6}x (RS_i)^{1/2}/n = (0.147)^{1/6}$	x (0.147 x 0.005)^	1/2 / 0.016	SDM Table 12
			= 1.23 m/s			
	Time of flow,	t_f	= 1.1 min			
1	4 Use "Rational Method" for calc	cula	on of design flow			
	, oco manorial montos for care	Juliu	on et design non			
ı	Design intensity,	i	$= a / (t_o + t_f + b)^c$			SDM 4.3.2
ı			= 505.5 / (2.3+1.1+3.29)^0.355 for ret	urn period T = 50	years	Corrigendum 1/2024
			= 258			SDM Table 3a
ı			D ((0) ((1) 10 0 0 11 11 11 11 11 11 11 11 11 11 11		0 4	CDM 7 F O /b)
ı	Type of surface			ment Area A (m ²) 0.0	<u>C x A</u> 0.0	SDM 7.5.2 (b)
	Flat Glassland(heavy soil) Concrete Paving		0.25 0.95	1080.0	1026.0	
l	Concrete Faving		0.00		1026.0	
	Upstream flow,	Q_{u}	= 0 m ³ /s			
	Superior and Control of the Control					
	Design flow,	Q_{d}	= 0.278i $\sum C_i A_i + Q_u$ where A_i i	s in km²		SDM 7.5.2 (a)
			= 0.278 x 258 x 1026 / 1000000 + 0			
			= 0.074 m ³ /s			
	NAME OF THE PARTY	_				
ı	Allowable flow,	Q_a				
			= 0.253 x 1.23			
			= 0.311 m ³ /s			
			Q _d (O.K.)			
			or (O.N.)			
Reference was made to Stormwater Drainage Manual (SDM) by DSD						
Goldric					Goldrich Pl	anners &
	Scale: NA Hydraulic Calculation			Surveyor		
-		-				
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1 For Catchment Area B		Ref.				
Area	= 4270 m ²					
Average slope	= 0.1 m per 100m					
Distance on the line of natural flow						
Time of concentraction	= 0.14465L / (H ^{0.2} A ^{0.1}) = 0.14465 (46 = 4.6 min	SDM 7.5.2 (d)				
2 For Proposed UC in Catchme	ea B					
2 Tot Proposed So in Satorinis		w				
	n To					
Ground level (mPD)	0 31.30 0 30.54					
Invert level (mPD)	0 30.34					
Width of u-channel	= 400 mm					
Length of u-channel		e e				
Depth of vertical part of u-channel						
	= (31.1-30.54)/111.6 = 0.005					
Gradient of a Grannon	(01.11 00.0 1)/111.0	¥				
Cross-Section Area		2 + 400 x 560				
	= 0.287 m ²					
Wetted Perimeter	$= \pi r + 2 d = 3.14 \times 200 + 2 \times 5$	560				
	= 1.748 m					
Hydralic radius		SDM 8.2.1				
	= 0.164 m	7				
3 Use Manning Equation for es	ng velocity of stormwater	*				
Take	= 0.016 for concrete lined cha	annels:- SDM Table 13				
	$= R^{1/6}x (RS_i)^{1/2}/n = (0.164)^{1/6}x (0.1$	TOTAL CONTROL OF THE PROPERTY				
Allowable velocity	= 1.33 m/s	04 X 0.003) 1/2 / 0.010 ODIN Table 12				
Time of flow						
Time of now	- 1,4 1001					
4 Use "Rational Method" for calculation of design flow						
Design intensity	$= a / (t_o + t_f + b)^c$	SDM 4.3.2				
	= 505.5 / (4.6+1.4+3.29)^0.355 for return pe	eriod T = 50 years Corrigendum 1/2024				
	= 229	SDM Table 3a				
5						
Type of surface		$\frac{\text{Area A (m}^2)}{\text{C x A}} \qquad \qquad \text{SDM 7.5.2 (b)}$				
Flat Glassland(heavy soil)	0.25					
Concrete Paving	0.95 4270 .					
		SUM = 4056.5				
	23/2					
Upstream flow	= 0 m³/s					
		2				
Design flow	= 0.278i Σ C _i A _i + Q _u where A _i is in k	m ² SDM 7.5.2 (a)				
_	= 0.278 x 229 x 4056.5 / 1000000 + 0					
n	= 0.259 m ³ /s	:** <u>.</u>				
Allowable flow						
	= 0.287 x 1.33					
1	= 0.381 m ³ /s					
	0 (0)()					
	Q_d (O.K.)					
Reference was made to Stormwater Drainage Manual (SDM) by DSD						
		Goldrich Planners &				
Scale: NA	Hydraulic Calculation	Surveyors Ltd.				
-		Surveyors Liu.				
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		(1.2000)				

1 For Connection between CP2	and E	existing Public 800	UC		Ref.
	A =		m^2		
Average slope,		0.1	m per 100m		
Distance on the line of natural flow,	L =	U	m		
Time of concentraction,	t _o ====================================		= 0.14465 (0) / (0.1^0.2*0^ min	0.1)	SDM 7.5.2 (d)
2 For Proposed UC in between	CP2 a	and Existing Public	800 UC		
	From	То	¢.		
Ground level (mPD)	31.30	31.10	-		
Invert level (mPD)	30.54	30.52	•		
Width of u-channel,	w =	450	mm		
Length of u-channel,			m		
Depth of vertical part of u-channel,			mm		
Gradient of u-channel,	$S_{l} =$	(30.54-30.52)/2	= 0.010		
Cross-Section Area,	a =		= 0.5 x 3.14 x 225^2 + 450 x 355	i	
Wetted Perimeter,			= 3.14 x 225 + 2 x 355		
Trouble Tollinoid	=				
Hydralic radius,			m		SDM 8.2.1
	=	0.169	.m		
3 Use Manning Equation for est					
	n =		for concrete lined channels:- = (0.169)^1/6 x (0.169 x 0.01)^1/	2/0.016	SDM Table 13 SDM Table 12
Allowable velocity,	V =			270.016	SDIVI TABLE 12
Time of flow,			min		
4 Use "Rational Method" for calc	culatio	n of design flow			
Design intensity,	j =	$a/(t_o + t_f + b)^c$			SDM 4.3.2
	=	505.5 / (0+0+3.29)^	0.355 for return period $T = 50$ y	/ears	Corrigendum 1/2024
	=	331			SDM Table 3a
Type of surface		Runoff Coefficient C	Catchment Area A (m²)	CXA	SDM 7.5.2 (b)
Flat Glassland(heavy soil)		0.25	0.0	0.0	Control Contro
Concrete Paving		0.95	0.0 SUM =	0.0	
			30W -	0.0	
Upstream flow,	Q _u =	0.333	m³/s		
Design flow,			where A _i is in km ²		SDM 7.5.2 (a)
	=	0.278 x 331 x 0 / 10			
	=	0.333	m³/s		
Allowable flow,	Q. =	axv	9		
7		0.239 x 1.91			
^	=	0.457	m ³ /s		
	>	Q _d (O.K.)			
Reference was made to Stormwate	er Draii	nage Manual (SDM)	by DSD		
	Π	24		Goldrich Pl	anners &
Scale: NA	Scale: NA Hydraulic Calculation Surveyor		2000 TWO 201-2000 FEE		
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SOCIAL CONTROL		(1 230	10)		





