Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Facilities and Filling of Land for a Period of 3 Years at Various Lots in DD7, Kau Lung Hang, Tai Po, New Territories – S16 Planning Application

## **Appendix 2**

**Traffic Impact Assessment** 

Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Facilities and Filling of Land for a Period of 3 Years, Various Lots in DD7, Kau Lung Hang, Tai Po, New Territories

Traffic Impact Assessment Final Report September 2025

**Prepared by:** CKM Asia Limited

Prepared for: Wing Lee (Kong Shum) Transportation Limited

# Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Facilities and Filling of Land for a Period of 3 Years, Various Lots in DD7, Kau Lung Hang, Tai Po, New Territories

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# Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Facilities and Filling of Land for a Period of 3 Years, Various Lots in DD7, Kau Lung Hang, Tai Po, New Territories

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- 4.1 AADT of the Core Stations Located in the vicinity of the Application Site
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# Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Facilities and Filling of Land for a Period of 3 Years, Various Lots in DD7, Kau Lung Hang, Tai Po, New Territories

## **FIGURES**

## **NUMBER**

- 1.1 Location of the Application Site
- 2.1 Locations of Surveyed Junctions
- 2.2 Layout of Tai Wo Service Road West / Hong Lok Yuen Road
- 2.3 Layout of Lam Kam Road Interchange / Tai Po Road Tai Wo
- 2.4 Layout of Lam Kam Road Interchange
- 2.5 Existing Peak Hour Traffic Flows
- 3.1 Layout Plan of the Proposed Temporary Public Vehicle Park
- 4.1 2029 Peak Hour Traffic Flows without the Proposed Temporary Public Vehicle Park
- 4.2 2029 Peak Hour Traffic Flows with the Proposed Temporary Public Vehicle Park

## 1.0 INTRODUCTION

## **Background**

- The application site is located at various lots in D.D. 7 at Kau Lung Hang, in Tai Po. The location of the application site is shown in **Figure 1.1**.
- 1.2 CKM Asia Limited, a traffic and transportation planning consultancy firm, was commissioned by the Applicant to prepare a traffic assessment in connection with the S16 application for a temporary public vehicle park with 201 car parking spaces and 10 parking spaces shared-use for HGV and coach for a period of 3 years (the "Proposed Temporary Public Vehicle Park"). Access to the Proposed Temporary Public Vehicle Park is via its existing vehicular access, which is provided at Tai Wo Service Road West, some 600m north of its junction with Hong Lok Yuen Road.
- 1.3 This report describes the traffic assessment undertaken for the Proposed Temporary Public Vehicle Park.

## **Structure of the Report**

1.4 The report is structured as follows:

Chapter One - Gives the background of the project;

Chapter Two - Describes the existing situation;

Chapter Three - Presents the Proposed Temporary Public Vehicle Park;

Chapter Four - Describes the traffic impact analysis; and

Chapter Five - Gives the overall conclusion.

## 2.0 THE EXISTING SITUATION

## **The Application Site**

2.1 The application site is currently used as a plant nursery with some temporary shelters and ancillary storage area. It fronts onto Tai Wo Service Road West to the east, some 600m north of its junction with Hong Lok Yuen Road.

## The Road Network

- 2.2 Tai Wo Service Road West is classified as rural road, and is of single carriageway 2-lane standard. It connects with Wo Hing Road to the north, and with Hong Lok Yuen Road, Fanling Highway and Lam Kam Road Interchange to the south.
- 2.3 Lam Kam Road is classified as a rural road, and is of single carriageway 2-lane standard. It connects with the Lam Kam Road Interchange and Tolo Highway to the north and with Route Twisk, Kam Sheung Road and Kam Tin Road to the south.

## **Manual Classified Traffic Counts**

- To quantify the traffic flows in the vicinity of the application site, manual classified counts were conducted on Wednesday, 19<sup>th</sup> June 2024 during the AM and PM peak periods at the following junctions:
  - J1: Tai Wo Service Road West / Hong Lok Yuen Road;
  - J2: Lam Kam Road Interchange / Tai Po Road Tai Wo; and
  - J3: Lam Kam Road Interchange.
- 2.5 The locations of these junctions are shown in **Figure 2.1** and the layouts are shown in **Figures 2.2 2.4** respectively.
- 2.6 From the traffic survey conducted, the AM and PM peak hours are found between 0730 0830 hours and 1715 1815 hours respectively. The existing AM and PM peak hour flows are presented in **Figures 2.5**.

## **Existing Junction Performance**

2.7 The existing operating performance of the surveyed junctions is calculated based on the existing traffic flows, and the analysis was undertaken using the method found in the Transport Planning and Design Manual ("TPDM"). The results are summarised in **Table 2.1**, and detailed calculations are presented in the **Appendix A**.

TABLE 2.1 EXISTING JUNCTION PERFORMANCE

Ref	Junction	Type of Junction (Parameter)	AM Peak	PM Peak
J1	Tai Wo Service Road West / Hong Lok Yuen Road	Signal (RC)	65%	58%
J2	Lam Kam Road Interchange / Tai Po Road – Tai Wo	Priority (DFC)	0.581	0.484
J3	Lam Kam Road Interchange	RA (DFC)	0.484	0.631

Note: RA – roundabout RC – reserve capacity DFC - design flow/capacity ratio

2.8 **Table 2.1** shows that the junctions operate with capacities.

## **Public Transport Services**

2.9 At present, 10 franchised bus and 3 green minibus ("GMB") routes operate in the vicinity of the Proposed Temporary Public Vehicle Park. Details of public transport services are presented in **Table 2.2**.

TABLE 2.2 EXISTING PUBLIC TRANSPORT SERVICES OPERATING IN THE VICINITY OF THE PROPOSED TEMPORARY PUBLIC VEHICLE PARK

Route	Routing	Headway (minutes)
KMB 73	Fanling (Wah Ming) – Tai Po Industrial Estate	20 – 30
KMB 73A	Fanling (Wah Ming) – Yu Chui Court	20 – 35
KMB 73B	Chuen On Road (Nethersole Hospital) – Sheung Shui (Circular)	25 – 60
KMB 74C	Kau Lung Hang – Kwun Tong Ferry	AM Peak
KMB 74D	Kau Lung Hang – Kwun Tong Ferry	25 – 60
KMB 271P	Kau Lung Hang – Tsim Sha Tsui (Canton Road)	AM Peak
KMB 273C	Kau Lung Hang – Tsuen Wan West Station	AM Peak
KMB 373	Sheung Shui – Central (Hong Kong Station)	AM, PM Peak
KMB N373	Fanling (Luen Wo Hui) – Central (Macau Ferry)	Overnight
KMB N73	Shatin Central – Lok Ma Chau	Overnight
GMB 502	Ching Ho Estate – Nethersole Hospital	8 – 15
GMB 25A	Tai Po Market – Nam Wa Po	5 - 10
GMB 25B	Tai Po Market – Kau Lung Hang / Yuen Leng	4 – 8

Note: KMB – Kowloon Motor Bus GMB – Green Minibus

## 3.0 THE PROPOSED TEMPORARY PUBLIC VEHICLE PARK

## The Proposed Temporary Public Vehicle Park

3.1 The Proposed Temporary Public Vehicle Park provides 201 car parking spaces and 10 parking spaces shared-use for HGV and coach, and the layout plan is shown in **Figure 3.1**.

## **Swept Path Analysis**

3.2 The CAD-based swept path analysis programme, **AUTODESK VEHICLE TRACKING**, was used to check the ease of manoeuvring of vehicles within the Proposed Temporary Public Vehicle Park, and the swept path analysis drawings are found in **Appendix B**. Vehicles are found to have no manoeuvring problems.

## 4.0 TRAFFIC ANALYSIS

## **Design Year**

4.1 The Proposed Temporary Public Vehicle Park is scheduled to commence operation in 2026 and operate until 2029. Hence, the design year adopted for traffic analysis is 2029.

## **Traffic Forecasting**

4.2 Year 2029 peak hour traffic flows for the junction capacity analysis is produced (i) with reference to existing traffic flows; (ii) estimated traffic growth rate from 2024 to 2029; and (iii) expected net increase in traffic generation due to the Proposed Temporary Public Vehicle Park.

## **Estimated Traffic Growth Rate from 2024 to 2029**

4.3 Reference is made to the (i) the Annual Average Daily Traffic ("AADT") of the core stations which are located in the vicinity found in the Annual Traffic Census ("ATC") published by Transport Department, and (ii) the population projection for Tai Po District from the "Projections of Population Distribution 2023 – 2031" published by the Planning Department. The above information is presented in **Tables 4.1** and **4.2** respectively.

TABLE 4.1 AADT OF THE CORE STATIONS LOCATED IN THE VICINITY OF THE APPLICATION SITE

Station	5507	5461	Overall
Road	Tai Wo Service Rd W	Fanling Highway	-
From	Lam Kam Rd INT	Lam Kam Rd INT	_
То	Kau Lung Hang Flyover	Kau Lung Hang Lo	-
	near Kiu Tau Rd	Wai	
2017	5,540*	92,220*	97,760
2018	5,670*	95,160*	100,830
2019	4,570	95,760	100,330
2020	4,330	92,630	96,960
2021	4,500*	97,150*	101,650
2022	4,360*	92,840*	97,200
2023	4,470*	98,660*	103,130
Average Annual Growth (2017-2023)	-3.51%	1.13%	0.90%

Note: \* Estimated by Growth Factor

TABLE 4.2 POPULATION PROIECTIONS OF TAI PO DISTRICT

Year	Population in Tai Po
2024	331,800
2029	341,200
Average Annual Growth 2024 to 2029	0.56%

4.4 **Table 4.1** shows that the annual average traffic growth of 0.90%, between 2017 and 2023. **Table 4.2** shows that the annual population growth between 2024 – 2029 is 0.56%. To be conservative, an annual average traffic growth of 0.90% is adopted for year 2024 – 2029.

## Net Increase in Traffic Generation of the Proposed Temporary Public Vehicle Park

## Traffic Generation of the Existing uses

4.5 The traffic generation of the existing uses are estimated based on the traffic generation survey conducted at the application site during AM and PM peak of Wednesday, 19<sup>th</sup> June 2024. The survey result is presented in **Table 4.3**.

TABLE 4.3 TRAFFIC GENERATION OF EXISTING USES

Items	AM P	eak	PM I	Peak	
	Generation	Attraction	Generation	Attraction	
Traffic Generation of Existing Uses	17	13	18	21	
(plant nursery, ancillary storage) (pcu/hr)	30 (2-1	vay)	39 (2-way)		

**Table 4.3** shows that the existing uses generates 30 and 39 pcu (2-way) in AM and PM peak hours respectively.

## Traffic Generation of the Proposed Temporary Public Vehicle Park

4.7 The TPDM has no trip rates for temporary public vehicle park, hence, the traffic generation of the Proposed Temporary Public Vehicle Park is calculated based on the trip rates derived from the traffic generation survey conducted at a temporary car park at Ma Wo Road in Tai Po. The traffic generation survey was conducted on Wednesday, 19<sup>th</sup> June 2024, and the survey results, the derived trip rate and the calculated traffic generation of the Proposed Temporary Public Vehicle Park is presented in **Table 4.4**.

TABLE 4.4 TRAFFIC GENERATION OF THE PROPOSED TEMPORARY PUBLIC VEHICLE PARK

	Items		Parameter	AM	Peak	PM Peak		
L				GEN	ATT	GEN	ATT	
Ī	Skye Parking, Ma Wo Road, Tai	Traffic Generation (1)	pcu/hr	16	15	10	30	
	Po (246 spaces)	Derived Trip Rate (2)	pcu/space/hr	0.0650	0.0610	0.0407	0.1220	
I	The Proposed Temporary Public	Traffic Generation (1)	pcu/hr	14	13	9	26	
	Vehicle Park (Total 211 spaces:			27 (2	-way)	35 (2	-way)	
L	201 for car, 10 for HGV/Coach)							

GEN – Generation ATT – Attraction

**Table 4.4** shows that the Proposed Temporary Public Vehicle Park is expected to generate 27 and 35 pcu (2-way) in AM and PM peak hours respectively.

### Net Increase in Traffic Generation

4.9 The net increase in traffic generation between the existing uses and the Proposed Temporary Public Vehicle Park is presented in **Table 4.5**.

TABLE 4.5 NET INCREASE IN TRAFFIC GENERATION

Scheme	Traffic Generation (pcu/ hr)								
	AM	Peak	PM Peak						
	Generation	Attraction	Generation	Attraction					
The Proposed Temporary Public Vehicle Park (from Table 4.4) [a]	14	13	9	26					
Existing Uses (from Table 4.3) [b]	17	13	18	21					
Net Increase [a] – [b]:	-3	+0	-9	+ 5					
	-3 <u>(2</u> -	·way)	<b>-4</b> <u>(2</u> -	·way)					

4.10 Compared to the existing uses, the Proposed Temporary Public Vehicle Park is expected to have net increase of -3 and -4 pcu / hour (2-way) in AM and PM peak respectively.

## **Year 2029 Peak Hour Traffic Flows**

4.11 Year 2029 peak hour traffic flows for the following cases are derived:

Year 2029 Without the Proposed Temporary Public Vehicle Park [A]

Existing Traffic Flow + estimated traffic growth between 2024 and 2029

Year 2029 With the Proposed Temporary Public Vehicle Park [B] = [A] + Net Increase in traffic generation

4.12 Year 2029 peak hour traffic flows for the above two cases are shown in **Figures 4.1** and **4.2** respectively.

## **2029 Junction Capacity Analysis**

4.13 Year 2029 junction capacity analysis for the case without and with the Proposed Temporary Public Vehicle Park are summarised in **Table 4.6** and detailed calculations are found in the **Appendix A**.

TABLE 4.6 2029 IUNCTION PERFORMANCE

Ref	Junction	Type of Junction (Parameter)		osed ry Public		Proposed ry Public e Park
			AM Peak	PM Peak	AM Peak	PM Peak
J1	Tai Wo Service Road West / Hong Lok Yuen Road	Signal (RC)	57%	51%	57%	51%
J2	Lam Kam Road Interchange / Tai Po Road – Tai Wo	Priority (DFC)	0.615	0.514	0.615	0.516
J3	Lam Kam Road Interchange	RA (DFC)	0.510	0.665	0.510	0.669

Note: RA – roundabout

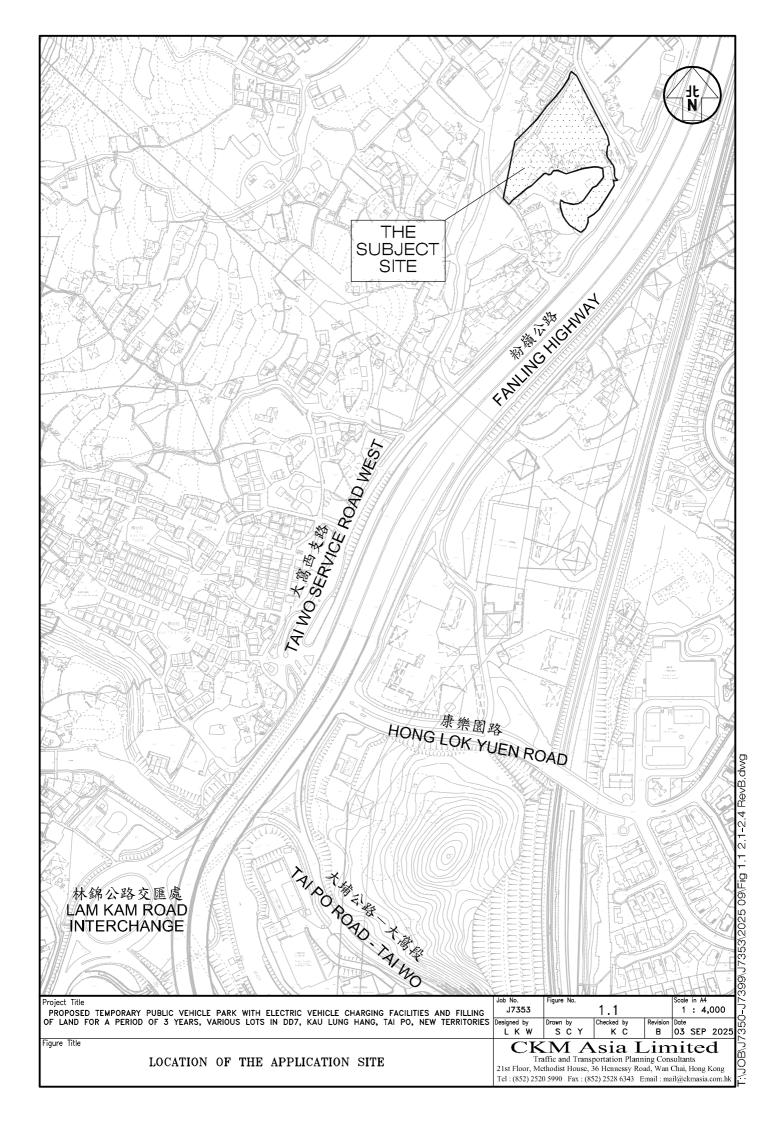
RC – reserve capacity

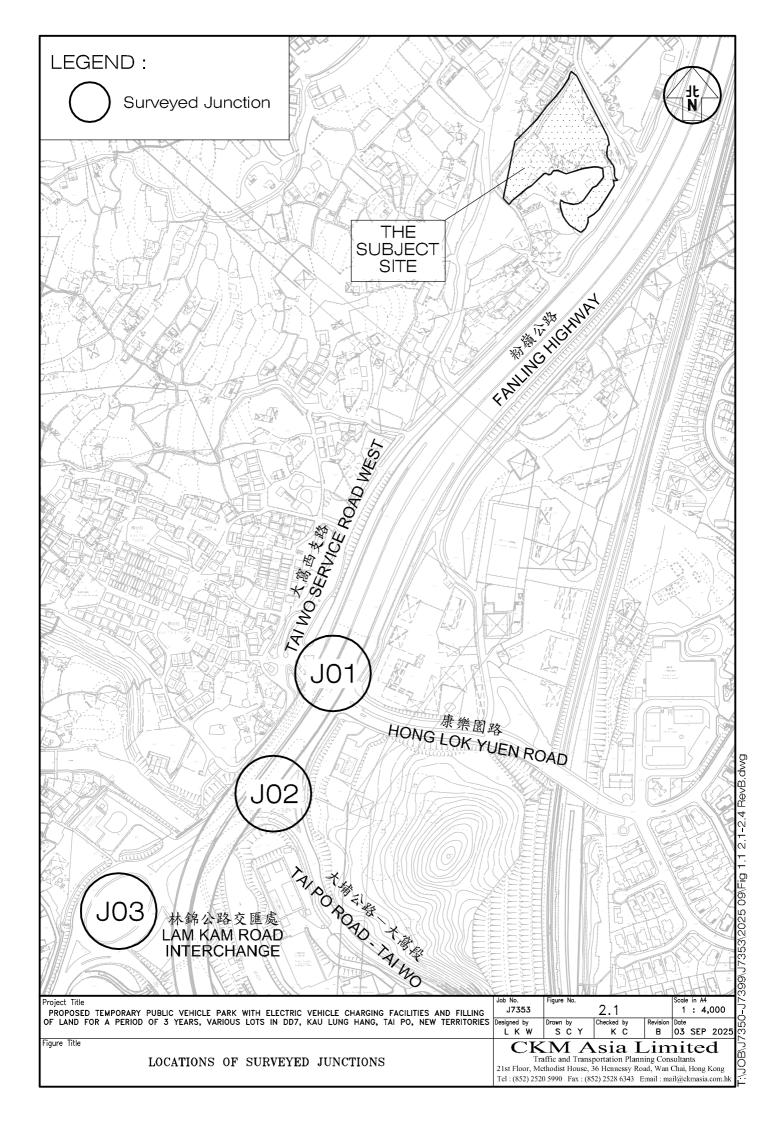
DFC - design flow/capacity ratio

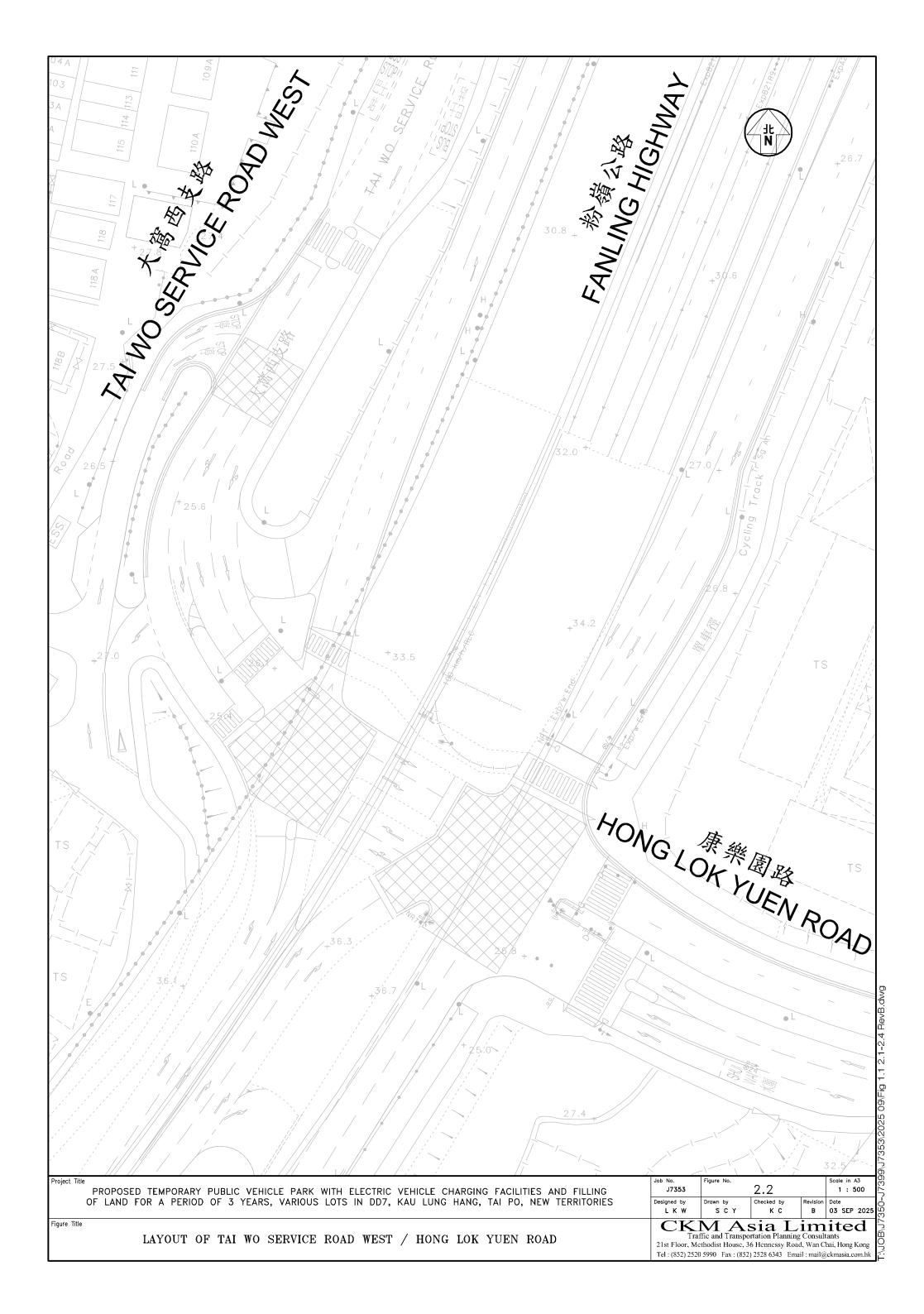
4.14 The results in **Table 4.6** indicate that the junctions analysed will operate with sufficient capacities in 2029, and the Proposed Temporary Public Vehicle Park has no adverse traffic impact.

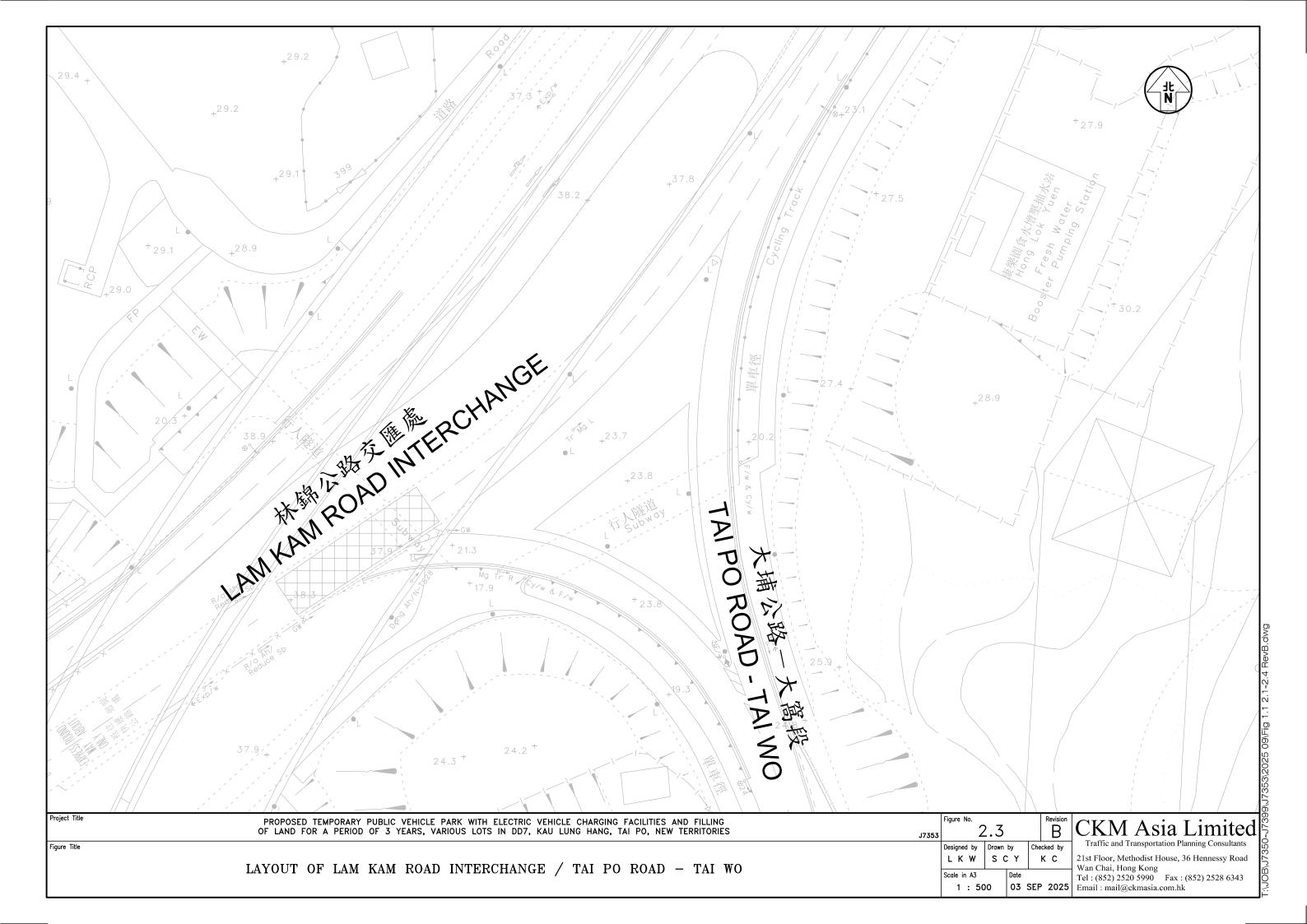
## 5.0 **SUMMARY**

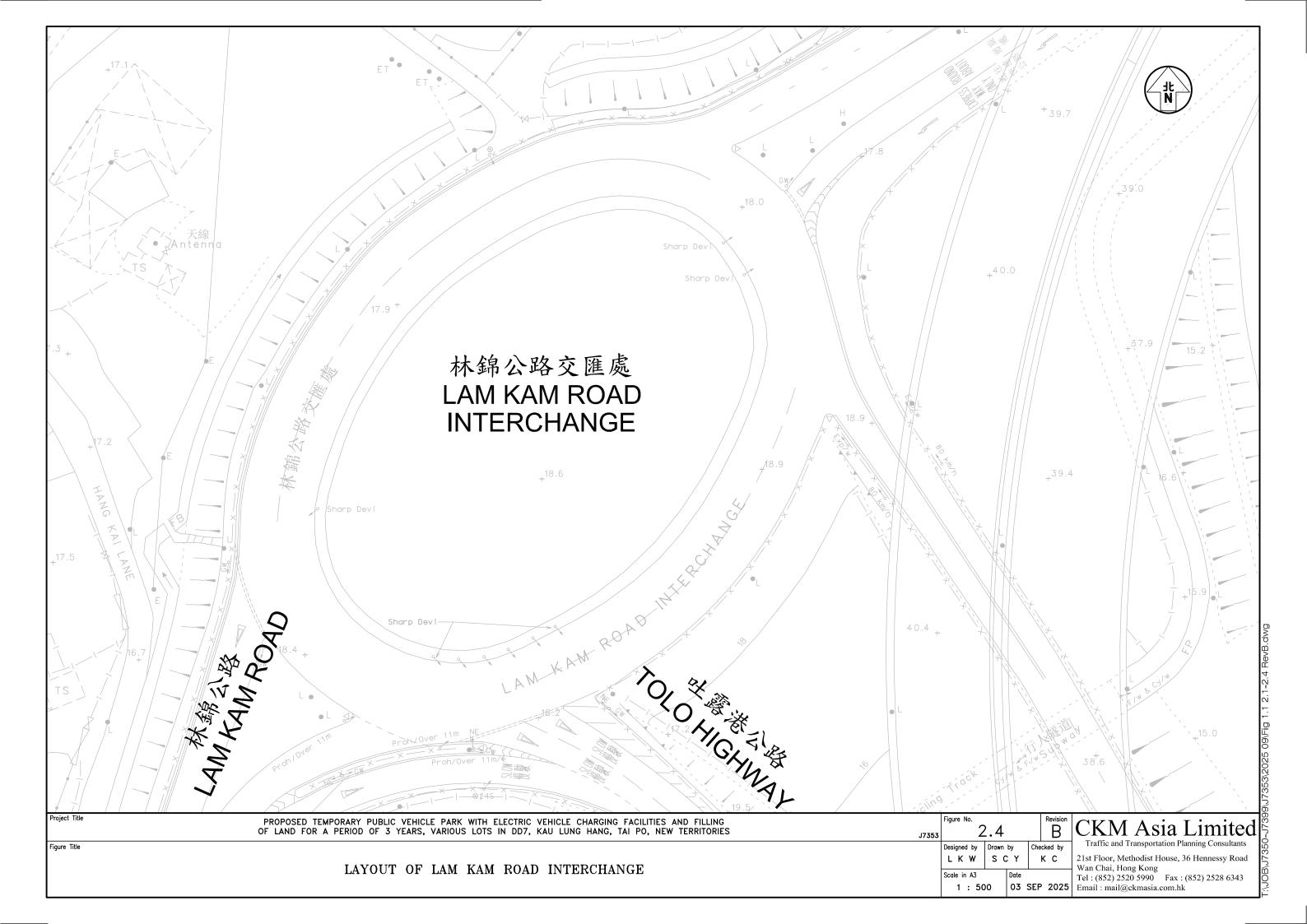
- 5.1 The application site is located at various lots in D.D. 7 at Kau Lung Hang, Tai Po. Access to the Proposed Temporary Public Vehicle Park is via its existing vehicular access which is provided at the Tai Wo Service Road West.
- 5.2 The Proposed Temporary Public Vehicle Park provides 201 car parking spaces and 10 parking spaces shared-use for HGV and coach for a period of 3 years.
- Year 2029 peak hour traffic flows for the junction capacity analysis is produced (i) with reference to existing traffic flows; (ii) estimated traffic growth rate from 2024 to 2029; and (iii) expected net increase in traffic generation due to the Proposed Temporary Public Vehicle Park.
- 5.4 A comparison is made of the performance of the junctions assessed for the cases without and with the Proposed Temporary Public Vehicle Park. The traffic analysis concluded that the junctions analysed will operate with sufficient capacities in 2029, and the Proposed Temporary Public Vehicle Park has no adverse traffic impact.

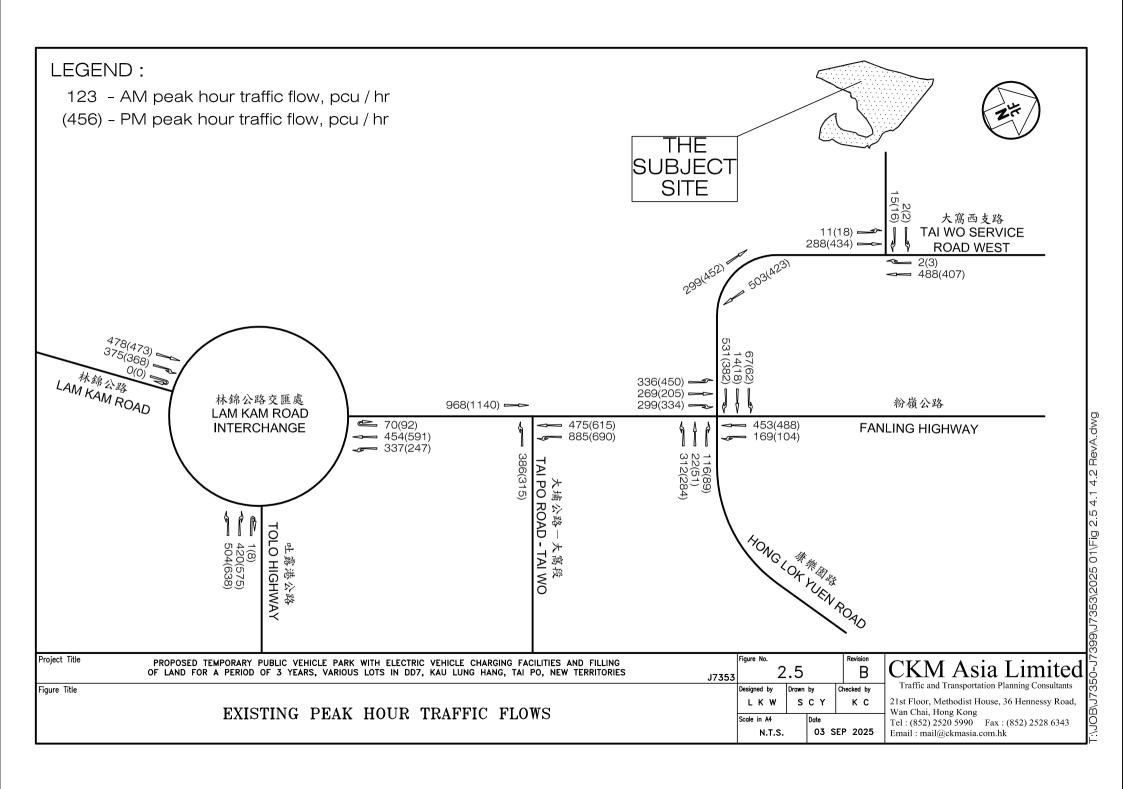


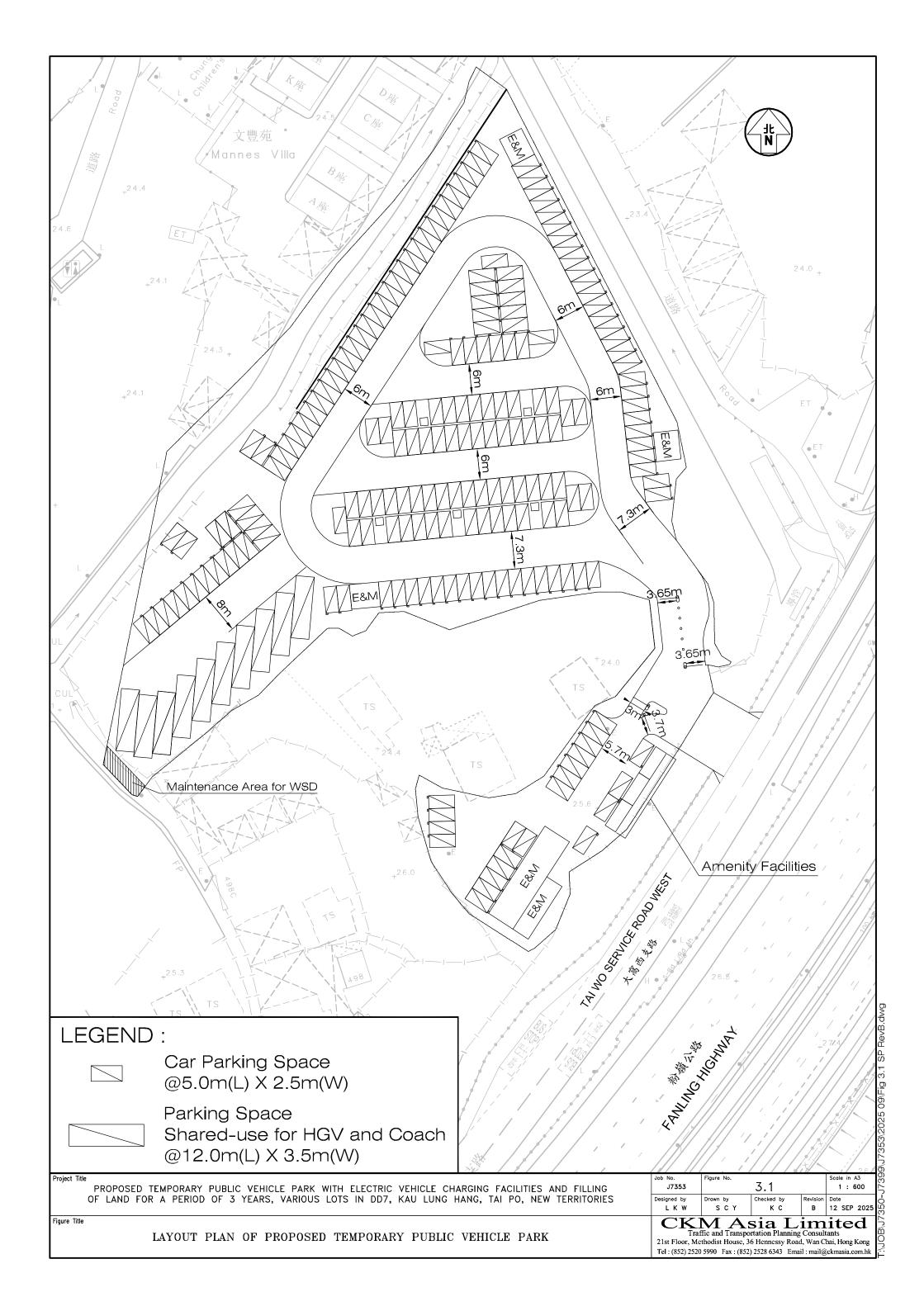


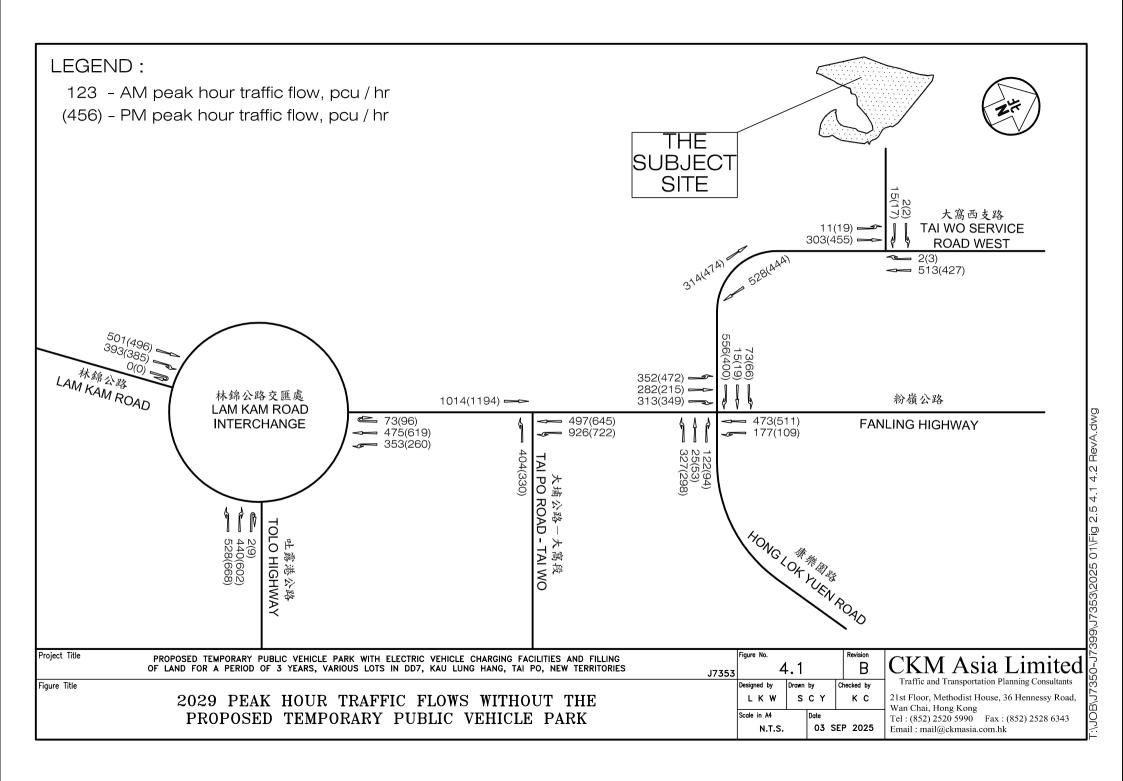


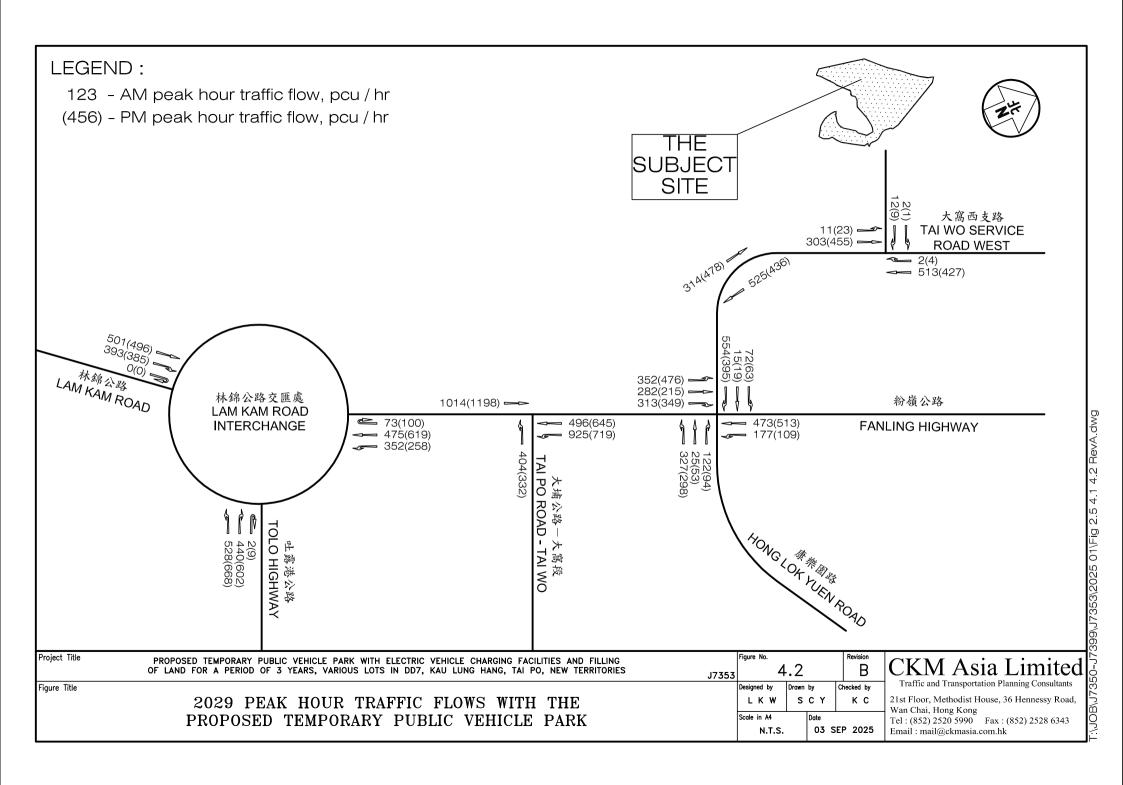














## **Signal Junction Analysis**

 Junction:
 Tai Wo Service Road West / Hong Lok Yuen Road
 Job Number:
 J7353

Scenario: Existing Condition P. 1

 Design Year:
 2024
 Designed By:
 Checked By:
 Date:
 8 Sep 2025

									AM Peak					PM Peak		
Approach Opposed Nea	rside	Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Fanling Highway NB	LT	A1	1,2	3.50	25.0		100	1854	336	0.181		100	1854	450	0.243	
1	SA	A2	1,2	3.50				1965	269	0.137			1965	205	0.104	
	RT	B1	2	3.50	30.0		100	2005	150	0.075		100	2005	168	0.084	
	RT	B2	2	3.50	25.0		100	1986	149	0.075	0.075	100	1986	166	0.084	0.084
Fanling Highway SB	LT+SA	F1	1	3.50	10.0		94	1722	181	0.105	0.105	58	1808	178	0.098	0.098
	SA	F2	1	3.50				2105	221	0.105			2105	207	0.098	
	SA	F3	1	3.50				2105	221	0.105			2105	207	0.098	
Hong Lok Yuen Road WB	LT	C1	2,3	3.50	10.0		100	1709	147	0.086		100	1709	134	0.078	
	LT	C2	2,3	3.50	15.0		100	1914	165	0.086		100	1914	150	0.078	
1	SA	D1	3	3.50				1965	138	0.070	0.070		1965	140	0.071	0.071
	SA+RT	E1	3	4.00	15.0		69	2016	70	0.035		29	2094	72	0.035	
	RT	E2	3	4.00	15.0		100	1959	68	0.035		100	1959	68	0.035	
Tai Wo Service Rd West EB	LT+SA	H1	4	4.00	10.0		24	1754	275	0.157	0.157	30	1749	207	0.118	0.118
	SA	H2	4	4.00				2155	337	0.157			2155	255	0.118	
1	SA+RT	G1	4	4.00	20.0		95	1881	267	0.142		91	1886	196	0.104	
	RT	G2	4	4.00	15.0		100	1959	278	0.142		100	1959	204	0.104	
Tai Wo Service Road West SB	SA	I1	1,2,4	4.00				2015	503	0.250			2015	423	0.210	
Tai Wo Service Road West NB	SA	12	1,2,4	4.00				2015	299	0.148			2015	452	0.224	
pedestrian phase		J <sub>(P)</sub>	4		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
		K <sub>(P)</sub>	4		min c	rossing	time =	9	sec	GM +	7	sec F	GM =	16	sec	
		L <sub>(P)</sub>	2,3,4		min c	rossing	time =	7	sec	GM +	12	sec F	GM =	19	sec	
		M <sub>(P)</sub>	3		min c	rossing	time =	14	sec	GM +	11	sec F	GM =	25	sec	
		N <sub>(P)</sub>	1,2,3		min c	rossing	time =	7	sec	GM +	9	sec F	GM =	16	sec	
		O <sub>(P)</sub>	1,4		min c	rossing	time =	7	sec	GM +	10	sec F	GM =	17	sec	
		P <sub>(P)</sub>	3		min c	rossing	time =	7	sec	GM +	6	sec F	GM =	13	sec	
<u> </u>																

AM Traffic Flow (pcu/hr)  299  N	PM Traffic Flow (pcu/hr)	452	$\longrightarrow$ N	S=1940+1	00(W-3.25	) 5	S=2080+10	0(W-3.25)	Note:
503			423	S <sub>M</sub> =S÷(1+	1.5f/r)	s	<sub>M</sub> =(S-230)	÷(1+1.5f/r)	
336	450 205 51	62 400			1+2+3+4 AM Peak	1,2+3+4 AM Peak	1+2+3+4 PM Peak	1,2+3+4 PM Peak	
299	334			Sum y	0.407	0.408	0.372	0.432	
14 138 453	140	18 48	88 🕎	L (s)	114	114	100	100	
312 4 169	284 ←		104	practical y	0.671	0.711	0.639	0.684	
				R.C. (%)	65%	74%	72%	58%	

	$ \begin{array}{c} N(p) \\ A1 \\ A2 \end{array} $ $ \begin{array}{c} O(p) \\ \bullet \cdots \bullet \end{array} $	F3 ← F1 ← F1 ← F1	N(p)  ↑ A1  ↑ A2  ↑ B1  ↑ B2  C1C2	3 11 L(p)	N(p) ← → → E1E2  C1 C2 D1 M(p) ← → →	P(p)  ↓ L(p)	J(p) H2 H1 H2 H1 G2 G1	5 11 ↓ K(p) ↓ L(p)		
AM	G =	I/G = 6	G =	I/G = 5	G =	I/G = 13	3 G=	I/G = 9	G =	
	G =	I/G =	G =	1/G = 5	G =	I/G = 13	3 G=	I/G = 9	G =	
PM	G =	I/G = 6	G =	1/G = 5	G =	I/G = 13	3 G=	I/G = 9	G =	
	G =	I/G =	G =	I/G = 5	G =	I/G = 13	3 G=	I/G = 9	G =	

## **Signal Junction Analysis**

 Junction:
 Tai Wo Service Road West / Hong Lok Yuen Road
 Job Number:
 J7353

Scenario: Without the the Proposed Temporary Public Vehicle Park P. 2

Design Year: 2029 Designed By: \_\_\_\_\_ Checked By: \_\_\_\_\_ Date: 8 Sep 2025

Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critica
Approach		rnase	Stage	vviatn (m)	radius (m)	% Up-hill Gradient	rurning %	Sat. Flow (pcu/hr)	(pcu/hr)	y value	Critical y	rurning %	Sat. Flow (pcu/hr)	(pcu/hr)	y value	Critic
anling Highway NB	1 LT	A1	1,2	3.50	25.0		100	1854	352	0.190		100	1854	472	0.255	
	1 SA	A2	1,2	3.50				1965	282	0.144			1965	215	0.109	
	RT	B1	2	3.50	30.0		100	2005	157	0.078		100	2005	175	0.087	
	RT	B2	2	3.50	25.0		100	1986	156	0.078	0.078	100	1986	174	0.087	0.08
anling Highway SB	1 LT+SA	F1	1	3.50	10.0		94	1722	189	0.110	0.110	59	1805	186	0.103	0.10
	SA	F2	1	3.50				2105	231	0.110			2105	217	0.103	
	SA	F3	1	3.50				2105	231	0.110			2105	217	0.103	
Hong Lok Yuen Road WB	1 LT	C1	2,3	3.50	10.0		100	1709	154	0.090		100	1709	141	0.082	
	LT	C2	2,3	3.50	15.0		100	1914	173	0.090		100	1914	157	0.082	
	1 SA	D1	3	3.50				1965	147	0.075	0.075		1965	147	0.075	0.07
	SA+RT	E1	3	4.00	15.0		67	2020	75	0.037		30	2092	76	0.036	
	RT	E2	3	4.00	15.0		100	1959	72	0.037		100	1959	71	0.036	
Tai Wo Service Road West EB	1 LT+SA	H1	4	4.00	10.0		25	1752	289	0.165	0.165	30	1740	217	0.125	0.12
	SA	H2	4	4.00				2155	355	0.165			2155	268	0.125	
	1 SA+RT	G1	4	4.00	20.0		95	1881	280	0.149		91	1886	206	0.109	
	RT	G2	4	4.00	15.0		100	1959	291	0.149		100	1959	213	0.109	
Tai Wo Service Road West SB	1 SA	11	1,2,4	4.00				2015	528	0.262			2015	444	0.220	
Tai Wo Service Road West NB	1 SA	12	1,2,4	4.00				2015	314	0.156			2015	474	0.235	
			,,_,													
pedestrian phase		J <sub>(P)</sub>	4		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
F		K <sub>(P)</sub>	4			rossing		9		GM +	7		GM =	16	sec	
		L <sub>(P)</sub>	2,3,4			rossing		7		GM +	12		GM =	19	sec	
		M <sub>(P)</sub>	3			rossing		14		GM +	11		GM =	25	sec	
		N <sub>(P)</sub>	1,2,3			rossing		7		GM +	9		GM =	16	sec	
		O <sub>(P)</sub>	1,4			rossing		7		GM +	10		GM =	17	sec	
		P <sub>(P)</sub>	3			rossing		7		GM +	6		GM =	13	sec	
		1 (P)	3		HIIII	iossirig	ume –	- 1	360	JIVI T	0	3601	GIVI -	13	360	
AM Traffic Flow (pcu/hr)  31	4	→ N	PM Traffic I	Flow (pcu/hr)		474		→ N	S=1940+1	00(W-3.25	) ;	S=2080+10	00(W-3.25)	Note:		
	528	_					444	_	S <sub>M</sub> =S÷(1+	1.5f/r)	s	S <sub>M</sub> =(S-230)	÷(1+1.5f/r)			
352	73		472 <b>•</b>			→ 66 419				1+2+3+4	1,2+3+4	1+2+3+4	1,2+3+4			
→ 282 <u>2</u> 5			┝	215	53	419				AM Peak	AM Peak	PM Peak	PM Peak			
313			<b>♦</b> 349		→ <sup>94</sup>	٦			Sum y	0.428	0.430	0.390	0.454			
556 <b>↓</b> 15					400	19			L (s)	29	24	29	24			
147	473 ◀				147		511		C (s)	114	114	100	100			
327	<b>↓</b> 177			298	<b>←</b>		<b>↓</b> 109		practical y	0.671	0.711	0.639	0.684			
									R.C. (%)	57%	65%	64%	51%			
1	2				3				4	•		•	5			
$N \longrightarrow N \longrightarrow N$			12 N(n)	<b>←</b> I1			N(n)	P(p)	•		12	<b>←</b> 11				
N(p) 13	`   <u> </u>	A1	N(p) <b>◄·-·►</b>	11		E1 E2	N(p) ▼			√ <b>&gt;</b>	$\downarrow \downarrow$	11				
→ A2		A2 B1				14					H2 H1	K(p)				
F3 <b>←</b> F2 <b>←</b>	_   →	B2		<b>↑</b> L(p)		1.1		<b>↑</b> L(p)			<b>↓</b>   ← G2 G1	<b>1</b> L(p)				
O(p) F1	-	C1C2 <b>↑</b>		∳ <sub>Γ</sub> (h)		C1 C2 D1	<b>VI(p)</b>	<b>∳</b> <sup>μ(μ)</sup>		O(p) <b>◆·····</b> ▶		<b>★</b> -(b)				
						111										
AM G = I	/G = 6	G =		I/G =	5	G =		I/G =	13	G =		I/G =	9	G =		
G = I	/G =	G =		I/G =	5	G =		I/G =	13	G =		I/G =	9	G =		

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G =

G =

I/G = 13

1/G = 13

G =

G =

I/G = 9

I/G = 9

G =

G =

G =

I/G = 6

I/G =

G =

G =

I/G = 5

I/G = 5

## **Signal Junction Analysis**

 Junction:
 Tai Wo Service Road West / Hong Lok Yuen Road
 Job Number:
 J7353

Scenario: With the the Proposed Temporary Public Vehicle Park P. 3

Design Year: 2029 Designed By: \_\_\_\_\_ Checked By: \_\_\_\_\_ Date: \_\_\_\_ 8 Sep 2025

Design Year: 2029	Design	ed By:				-	Checke	d By:				-	Date:	8	Sep 202	25
		ı	1	ı	1	1	ī		AM Peak			ī		PM Peak		
Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critica
anling Highway NB	1 LT	A1	1,2	3.50	25.0		100	1854	352	0.190		100	1854	476	0.257	
	1 SA	A2	1,2	3.50				1965	282	0.144			1965	215	0.109	
	RT	B1	2	3.50	30.0		100	2005	157	0.078		100	2005	175	0.087	
	RT	B2	2	3.50	25.0		100	1986	156	0.078	0.078	100	1986	174	0.087	0.08
anling Highway SB	1 LT+SA	F1	1	3.50	10.0		94	1722	189	0.110		58	1808	187	0.103	0.10
<u> </u>	SA	F2	1	3.50				2105	231	0.110			2105	218	0.103	
	SA	F3	1	3.50				2105	231	0.110			2105	218	0.103	
long Lok Yuen Road WB	1 LT	C1	2,3	3.50	10.0		100	1709	154	0.090		100	1709	141	0.082	
	LT	C2	2,3	3.50	15.0		100	1914	173	0.090		100	1914	157	0.082	
	1 SA	D1	3	3.50				1965	147	0.075	0.075		1965	147	0.075	0.07
	SA+RT	E1	3	4.00	15.0		67	2020	75	0.037		30	2092	76	0.036	
	RT	E2	3	4.00	15.0		100	1959	72	0.037		100	1959	71	0.036	
Γai Wo Service Road West EB	1 LT+SA	H1	4	4.00	10.0		25	1752	287	0.164	0.164	30	1740	213	0.122	0.12
	SA	H2	4	4.00				2155	354	0.164			2155	264	0.122	
-	1 SA+RT	G1	4	4.00	20.0		95	1881	279	0.148		91	1886	203	0.108	
	RT	G2	4	4.00	15.0		100	1959	290	0.148		100	1959	211	0.108	
Tai Wo Service Road West SB	1 SA	11	1,2,4	4.00			T	2015	525	0.261		T	2015	436	0.216	
Tai Wo Service Road West NB	1 SA	12	1,2,4	4.00				2015	314	0.156			2015	478	0.237	
Tal Wo Colvide Hoad West ND	<u> </u>		1,2,1	1.00				2010	011	0.100			2010	110	0.201	
pedestrian phase		J <sub>(P)</sub>	4		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
F		K <sub>(P)</sub>	4			rossing		9		GM +	7		GM =	16	sec	
		L <sub>(P)</sub>	2,3,4			rossing		7		GM +	12		GM =	19	sec	
		M <sub>(P)</sub>	3			rossing		14		GM +	11		GM =	25	sec	
		N <sub>(P)</sub>	1,2,3			rossing		7		GM +	9		GM =	16	sec	
		O <sub>(P)</sub>	1,4			rossing		7		GM +	10		GM =	17	sec	
		P <sub>(P)</sub>	3			rossing		7		GM +	6		GM =	13	sec	
		- (F)											···			
AM Traffic Flow (pcu/hr)		I	PM Traffic I	low (pcu/hr	\				·			1		Note:		
3	14	→ N				478	_	→ N		00(W-3.25		S=2080+10				
I	525					1	436		S <sub>M</sub> =S÷(1+	1.5f/r)	S	S <sub>M</sub> =(S-230)	÷(1+1.5f/r)			
352	72		476 <b>†</b>			63				1+2+3+4 AM	1,2+3+4 AM	1+2+3+4 PM	1,2+3+4 PM			
282 25			ightharpoons	215	53 <b>↑</b> 04					Peak	Peak	Peak	Peak			
313			349		305	-			Sum y	0.427	0.429	0.388	0.454			
15					1 393	19			L (s)	29	24	29	24			
147	473	_			147		513 ←		C (s)	114	114	100	100			
327	177			298	$\leftarrow$		109		practical y	0.671	0.711	0.639	0.684			
									R.C. (%)	57%	66%	65%	51%			
$1 \longrightarrow N \longrightarrow$	2				3		•	~/ \	4				5			
N(p)	1		I2 N(p) <b>4···</b> ►	I1	•		N(p)	P(p)		J(p)	12 	<b>■</b> I1				
A1 A2				E1 E2				4	↓ <b>↓</b> H2 Ḥ1 •	· v/						
F2 4	F3 ← B1 B2					*					<b>↓</b> ↓ ↓	K(p)				
O(p) F1				<b>♦</b> L(p)		C1 C2 D1	(a)N	<b>♦</b> L(p)		O(p)	G2 G1	<b>‡</b> L(p)				
4		77				7714	·····			<b>◆·····</b> ▶						
AM C -	I/G = 6	-		I/G =	5			I/G =	13			I/G =	<u>α</u>	2		
		G =				G =				G =			_	G =		
	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	I/G = 6	G =		I/G =	S	G =		I/G =	13	G =		I/G =	9	G =		

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G =

I/G = 13

G =

I/G = 9

G =

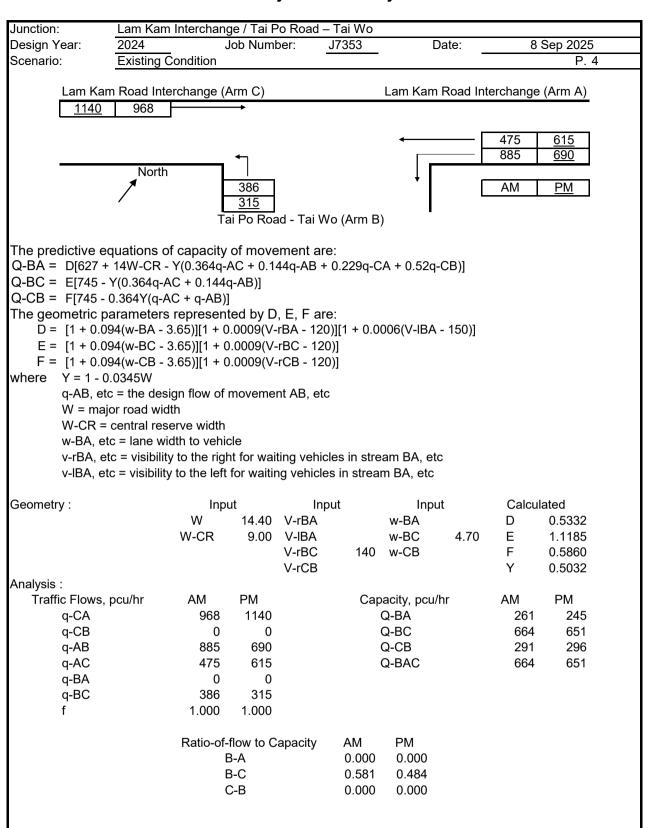
I/G = 5

G =

I/G =

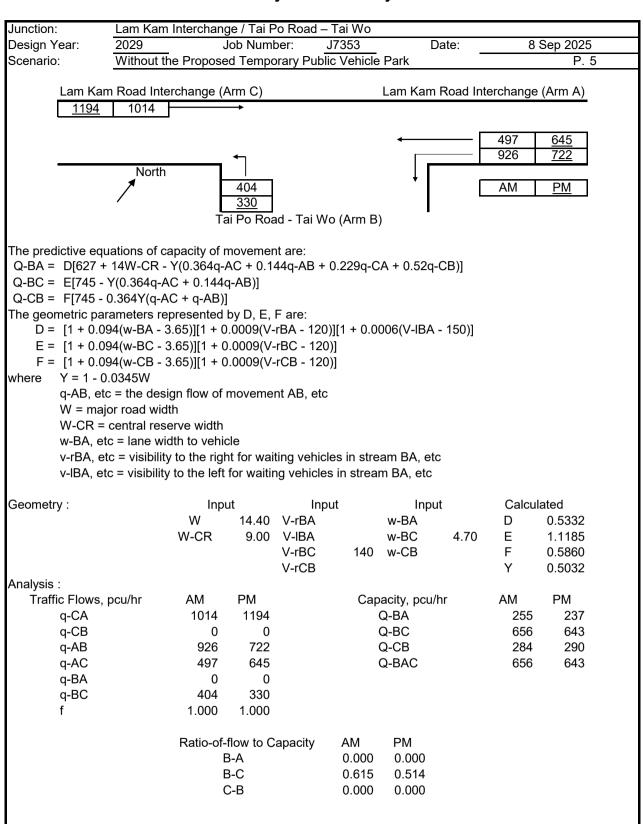
G=

## **Priority Junction Analysis**



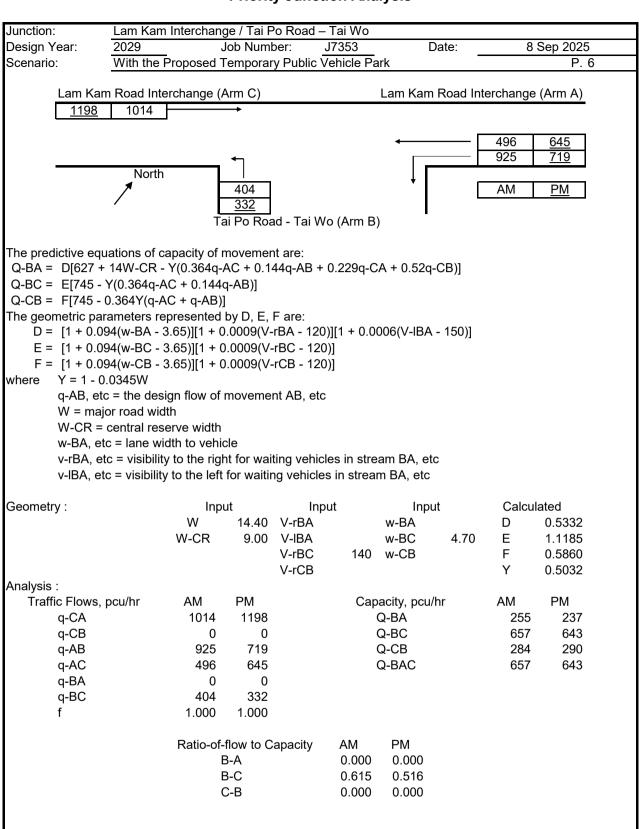
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## **Priority Junction Analysis**



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## **Priority Junction Analysis**



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## **Roundabout Analysis**

Location Lam Kam Interchange

Scenario Existi	ng Condition				Page	7
Design Year	2024	Job Number	J7353	Date	08 Sep	2025

## AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	478	375						853	491
From B	454	70	0						524	376
From C	0	420	1						421	524
From D										
From E										
From F										
From G										
From H										
Total	454	968	376						1798	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	473	368						841	675
From B	591	92	0						683	376
From C	0	575	8						583	683
From D										
From E										
From F										
From G										
From H										
Total	591	1140	376			·		·	2107	

## Legend

Arm	Road (in clockwise order)
Α	Lam Kam Road
В	Lam Kam Road Interchange
С	Slip Road to Tolo Highway
D	
Е	
F	
G	
Н	

#### **Geometric Parameters**

Geometri	L Paramett	#15					
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
From A	7.0	6.0	100.0	14.5	78	18	0.1
From B	4.5	4.0	33.5	9.5	78	60	0.1
From C	8.5	7.0	42.3	9.5	78	22	0.3
From D							
From E							
From F							
From G							
From H							

## Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_E$	Entry Capacity
$q_{c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_{D}$	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
<b>x</b> <sub>2</sub>	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

### Limitation

е	Entry Width	4.0 - 15.0 m
V	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

## Ratio-of-Flow to Capacity (RFC)

							$Q_{E}$		Entry Flow		RI	-C
Arm	<b>x</b> <sub>2</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	6.819	6.050	1.071	1.081	2066	0.532	1951	1845	853	841	0.437	0.456
From B	4.428	6.050	1.071	0.916	1342	0.424	1082	1082	524	683	0.484	0.631
From C	7.997	6.050	1.071	1.055	2423	0.585	2234	2136	421	583	0.188	0.273
From D												
From E												
From F												
From G												
From H												

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## **Roundabout Analysis**

Location Lam Kam Interchange

Scenario Witho	Page	8				
Design Year	2028	Job Number	J7353	Date	08 Sep	2025

## AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	501	393						894	515
From B	475	73	0						548	395
From C	0	440	2						442	548
From D										
From E										
From F										
From G										
From H										
Total	475	1014	395						1884	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	496	385						881	707
From B	619	96	0						715	394
From C	0	602	9						611	715
From D										
From E										
From F										
From G										
From H										
Total	619	1194	394						2207	

### Legend

Arm	Road (in clockwise order)
Α	Lam Kam Road
В	Lam Kam Road Interchange
С	Slip Road to Tolo Highway
D	
Е	
F	
G	
н	

#### **Geometric Parameters**

Ocometin	o i urumott	,13					
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
From A	7.0	6.0	100.0	14.5	78	18	0.1
From B	4.5	4.0	33.5	9.5	78	60	0.1
From C	8.5	7.0	42.3	9.5	78	22	0.3
From D							
From E							
From F							
From G							
From H							

## Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{\rm c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_{D}$	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
X <sub>2</sub>	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

## Limitation

е	Entry Width	4.0 - 15.0 m
V	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

## Ratio-of-Flow to Capacity (RFC)

							C	) <sub>E</sub>	Entry	/ Flow	RF	-C
Arm	<b>x</b> <sub>2</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	6.819	6.050	1.071	1.081	2066	0.532	1937	1827	894	881	0.461	0.482
From B	4.428	6.050	1.071	0.916	1342	0.424	1075	1075	548	715	0.510	0.665
From C	7.997	6.050	1.071	1.055	2423	0.585	2219	2116	442	611	0.199	0.289
From D												
From E												
From F												
From G												
From H												

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## **Roundabout Analysis**

Location Lam Kam Interchange

Scenario With th	Page	9				
Design Year	2028	Job Number	J7353	Date	08 Sep	2025

## AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	501	393						894	515
From B	475	73	0						548	395
From C	0	440	2						442	548
From D										
From E										
From F										
From G										
From H										
Total	475	1014	395						1884	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	$q_c$
From A	0	496	385						881	711
From B	619	100	0						719	394
From C	0	602	9						611	719
From D										
From E										
From F										
From G										
From H										
Total	619	1198	394						2211	

## Legend

Arm	Road (in clockwise order)
Α	Lam Kam Road
В	Lam Kam Road Interchange
С	Slip Road to Tolo Highway
D	
Е	
F	
G	
Н	

#### **Geometric Parameters**

Geometri	c i arainett	71 <b>3</b>					
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
From A	7.0	6.0	100.0	14.5	78	18	0.1
From B	4.5	4.0	33.5	9.5	78	60	0.1
From C	8.5	7.0	42.3	9.5	78	22	0.3
From D							
From E							
From F							
From G							
From H							

## Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{\rm c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_D$	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
X <sub>2</sub>	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

## Limitation

е	Entry Width	4.0 - 15.0 m
V	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

## Ratio-of-Flow to Capacity (RFC)

							$Q_{E}$		Entry Flow		RFC	
Arm	<b>x</b> <sub>2</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	6.819	6.050	1.071	1.081	2066	0.532	1937	1825	894	881	0.461	0.483
From B	4.428	6.050	1.071	0.916	1342	0.424	1075	1075	548	719	0.510	0.669
From C	7.997	6.050	1.071	1.055	2423	0.585	2219	2113	442	611	0.199	0.289
From D												
From E												
From F												
From G												
From H												

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