

Our Ref.: DD 38 Lot 115 & VL Your Ref.: TPB/A/NE-MUP/214

The Secretary, Town Planning Board, 15/F, North Point Government Offices, 333 Java Road, North Point, Hong Kong 顧問有限公司 **盈卓物業** 

By Email 16 April 2025

Dear Sir,

#### 1<sup>st</sup> Further Information

### Proposed Temporary Warehouse with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land in "Residential (Group D)" and "Agriculture" Zones, <u>Various Lots in D.D. 38 and Adjoining Government Land, Sha Tau Kok, New Territories</u>

#### (S.16 Planning Application No. A/NE-MUP/214)

We write to submit further information in response to departmental comments of the captioned application.

Should you require more information regarding the application, please contact our Mr. Danny NG at / or the undersigned at your convenience. Thank you for your kind attention.

Yours faithfully,

For and on behalf of R-riches Property Consultants Limited

Christian CHIM Town Planner

cc DPO/STN, PlanD

(Attn.: Mr. William WONG (Attn.: Mr. Brian CHAN email: wstwong@pland.gov.hk )
email: bchchan@pland.gov.hk )

#### **Response-to-Comment**

## Proposed Temporary Warehouse with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land in "Residential (Group D)" and "Agriculture" Zones, Various Lots in D.D. 38 and Adjoining Government Land, Sha Tau Kok, New Territories

#### (Application No. A/NE-MUP/214)

#### (i) A RtC Table:

	Departmental Comments	Applicant's Responses
1. 0	Comments of the Geotechnical Engineering Offi	ce, Civil Engineering and Development
0	Department	
(a)	The subject site is overlooked by steep natural terrain and meets the alert criteria for a natural terrain hazard study (NTHS). Furthermore, Registered Slope No. 3NW- D/C41, which is steeper than 30 degrees with height greater than 6 m, is found within 6 m of the subject site.	Noted. The GPRR in support of the application is enclosed at <b>Annex 1</b> .
(b)	If the applicant wishes to proceed with the proposed development, the applicant is required to submit a Geotechnical Planning Review Report (GPRR) in support of the Planning Application. The GPRR should include a preliminary geotechnical review of slopes and natural terrain hazards, assess the geotechnical feasibility of the proposed development, including an outline of any further studies that may be required and where necessary, indicate the recommended extent of the NTHS study area and a commitment to undertake the NTHS and to carry out any necessary mitigation measures as part of the proposed development.	



#### Annex 1

Geotechnical Planning Review Report



# **Geotechnical Planning Review Report**



April 2025

Geotechnical Planning Review Report

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Geotechnical Planning Review Report

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- Appendix B SIS Records
- Appendix C Schedule Area Plan
- Appendix D Existing GI Record
- Appendix E Proposed Development Layout Plan

Geotechnical Planning Review Report

# **1** Introduction

# **1.1 Background**

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use various lots in D.D. 38 and adjoining Government Land (GL), Sha Tau Kok, New Territories (the Site) for 'Proposed Temporary Warehouse (Excluding Dangerous Goods Godown (D.G.G.)) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land'.
- 1.1.2 This Geotechnical Planning Review Report is prepared base on desk study and available documentary to support the aforementioned planning application in geotechnical aspect.

# **1.2 Objectives of this Report**

- 1.2.1 The main objectives of this report are as followings:-
  - Describe the geology of the Application Site.
  - Indicate the location of existing features within and surrounding the Application Site and the land status.
  - Review of how the Proposed Development have effects on the manmade slopes or retaining walls.
  - Assess on the geotechnical feasibility of the Proposed Development.

# **2 Site Description**

# 2.1 Site information and Topography

- 2.1.1 The development site situates beside Sha Tau Kok Road Wo Hang. It has an area of about 11,698m<sup>2</sup>. The site location plan and aerial photo view are shown in **Figure 1** and **Figure 2** respectively.
- 2.1.2 In general, the site is a flat ground with level various from approx. +28.6 mPD to + 30.7 mPD, gently falling from south to north. The site topography is relatively flat with a natural hillside near the southeast boundary
- 2.1.3 The hillside is covered in dense vegetation and the site area is mostly open area comprising paved area or shallow vegetation.

# 2.2 Existing Man-made Features

2.2.1 There is no registered man-made feature located within the Application Site. There are 6 registered slopes in vicinity of the Application Site. The locations of these 6 features are shown in **Figure 3**. Records of the slopes are retrieved from the SIS System of GEO and SIMAR of Lands Department, they are summarized in **Table 2.1**.

Geotechnical Planning Review Report

Feature No.	Location	Max. Height (m)	Length (m)	Angle (°)	Material	Consequen ce-to-life	Maintenance Parties
Adjoining to tl	he Application Site	e					
							Sub Div. 1: Private
3NW-D/C 37	Loi Tung Village, Sha Tau Kok Road -	3	24	35	Vegetated: 40	3	Sub Div. 2: Private
51100-0/0 57	Wo Hang, New Territories	5	24	55	Chunam: 60	5	Sub Div. 3: LandsD
							Sub Div. 4: Private
							Sub Div. 1: Private
	North East of Loi Tung Village, off Sha Tau Kok, Wo Hang, North	5	45		Vegetated: 100	3	Sub Div. 2: Private
				80			Sub Div. 3:
3NW-D/C 40							LandsD
							Sub Div. 4:
							Private
							Sub Div. 5:
							Private
							Sub Div. 1:
	Open storage						Lands D Sub Div. 2:
	area, north east of Loi						Private
3NW-D/C 41	Tung Village,	15	150	35	Vegetated:		Sub Div. 3:
51111 27 2 11	off Sha Tau	15	150	33	100		LICENCE
	Kok Road - Wo						N8797
	Hang, North						Sub Div. 4:
							Lands D
	East of Loi Tung East				Vegetated		
3NW-D/C 50	Village House 5 #10D, Sha Tau Kok	5	5 45	50	Vegetated: 100	3	Lands D
	East of the Loi				Vegetated: 100	3	Sub Div. 1:
	Tung Fast	5	105	50			Lands D
3NW-D/C 51							Sub Div. 2: Private

#### Table 2.1 - Existing Geotechnical Feature adjoining to the Application Site

Geotechnical	Planning	Review	Report
Geoteennieur	1 IGTUUUS		i cport

East of the Loi 3NW-D/C 52 Village, Sha Tau Kok	ed: 3 Lands D
--	---------------

2.2.2 A Copy of the SIMAR reports and Slope Information System records are attached in **Appendix A** and **B** respectively. The locations of the above features are presented in **Figure 3**.

# **3 Review of Desk Study Information**

# 3.1 Site Geology

3.1.1 The geology of the Study Area is shown on 1:20000 scale HGM20 Series Solid and Superficial Geology Map Sheet 3, Published by the GEO, HKSAR. The Application Site is generally underlain by debris flow deposits. A part print of geological map is presented in **Figure 4**.

# 3.2 Schedule Area

3.2.1 The Site is located outside the Schedule Area No. 2 (North-western New Territories) and Schedule Area 3 (The Railway Protection Zone). Plan of the Schedule Areas are enclosed in **Appendix C**.

# **3.3 Existing Ground Investigation Data**

- 3.3.1 There is no existing ground investigation (GI) information within the site. The nearest identified GI (BH20) is located approximately 50m away from the northwest of the site which is carried out by DrilTech Ground Engineering Ltd in January 2005.
- 3.3.2 The location and information of the borehole is enclosed in **Appendix D**.

#### Table 3.1 - Summary of GIU Report

GIU Report no.	Title of the Report	Done by	Date
41501	CE6/2002 (DS) Drainage Improvement in Northern NT - Package C Investigation, Design & Construction (Man Uk Ping)	DrilTech Ground Engineering Limited	2005

3.3.3 The borehole record indicated the site is coved with approx. 1m thick layer of FILL and follow by approx. 2.1m thick ALLUVIUM from 1m depth below ground level. Then it is underlain by approx. 6.6m of TUFF. The BH encounter grade III bedrock at level +16.02 mPD.

Geotechnical Planning Review Report

# 3.4 Groundwater Condition

3.4.1 Piezometer was installed in BH20. According to the water level monitoring record, the highest and lowest groundwater level recorded are +23.54 mPD and +23.52 mPD respectively, where ground level of the development site is about +29mPD.

# 3.5 Landslide History

- 3.5.1 According to the GEO's Enhanced Natural Terrain Landslide Inventory (ENTLI) data, there is neither recent nor relict relevant natural terrain landslide identified for the Application Site.
- 3.5.2 Other landslides as recorded in ENTLI are indicated in **Figure 5** for reference.

# **3.6 Boulder Field Inventory**

- 3.6.1 The boulder inventory is a territory-wide catalogue of boulder fields on natural slopes in Hong Kong based on an interpretation of the 1963/64 low level aerial photographs (Emery, K. A., 1998). Boulder fields were identified and four boulder attributes including percentage area covered, boulder type, boulder size and boulder shape were mapped.
- 3.6.2 The boulder field inventory map and summary for the adjoining boulder polygons are shown in **Figure 6**. 2 boulder polygons are identified (S3\_20 and S3\_U) covering the Application Site and the southern natural terrain. For the coverage of the 2 polygons, no boulder was observed on the ground surface.

# 3.7 Historical Landslide Catchment

3.7.1 There is no relevant Historical Landslide Catchments (HLC) (MFJV, 2007), identified adjoining to the Application Site. The HLC of nearby Area are shown in **Figure 7** for reference.

# 4 Proposed Works

# 4.1 Site formation works

4.1.1 The Application Site is proposed to be entirely filled with concrete of not more than 0.2m depth to facilitate a flat surface for maneuvering of vehicle and site formation of structures. No new slopes/ retaining walls is required to support the minor site formation works.

# **4.2 Temporary Structures**

- 4.2.1 The proposed Application Site is applying for temporary warehouse for a period of 3 years, it comprises of 6 numbers of temporary structures, 4 one-storey and 2 two-storey low rise structures. The proposed development layout plan is shown in **Appendix E**.
- 4.2.2 In view of loadings from low-rise temporary structures is comparatively general, no foundation is proposed for the temporary structures. Excavation for drainage, sewerage and utilities works are minimal, no deep Excavation and Lateral Support (ELS) is anticipated.

Geotechnical Planning Review Report

# **5 Works effect on Existing Features**

- 5.1.1 There is no existing feature within the Application Site.
- 5.1.2 In view of the above minor site formation works and low-rise temporary structures, effect on adjoining existing features is minimal.
- 5.1.3 Slope stability and integrity of the existing features, 3NW-D/C 41 and 3NW-D/C 51, that are within 6m from the proposed temporary structures shall be assessed in detail under a separate submission. After detail assessment in detail design stage, feature upgrading works such as site formation/slope stabilization are to be proposed if required.
- 5.1.4 Two features, 3NW-D/C 41 and 3NW-D/C 51, may need to raise the consequence-to-life subject to the usage of proposed structures.

# 6 Screening

6.1.1 There is no natural terrain within the Application Site, however, the site is overlooked by natural terrain at south-eastern. With reference to GEO Report No.138 (2nd edition), the Application Site does not meet the "In-principle Objection Criteria" and satisfies the "Alert Criteria" and the preliminary findings are summarized as below:

Angular Elevation ≥ 20°	Within 50m of ground sloping	Alert Criteria met?
Yes	Yes	Yes

6.1.2 The preliminary NTHS Area and section have been development and shown in **Figure 8**. A detailed natural terrain hazard study will be carried out in detailed design stage. Implementation of the mitigation measures if necessary (such as rigid barrier, flexible barrier etc.) will be conducted at construction stage. Therefore, the application site is considered geotechnically feasible from the NTH point of view.

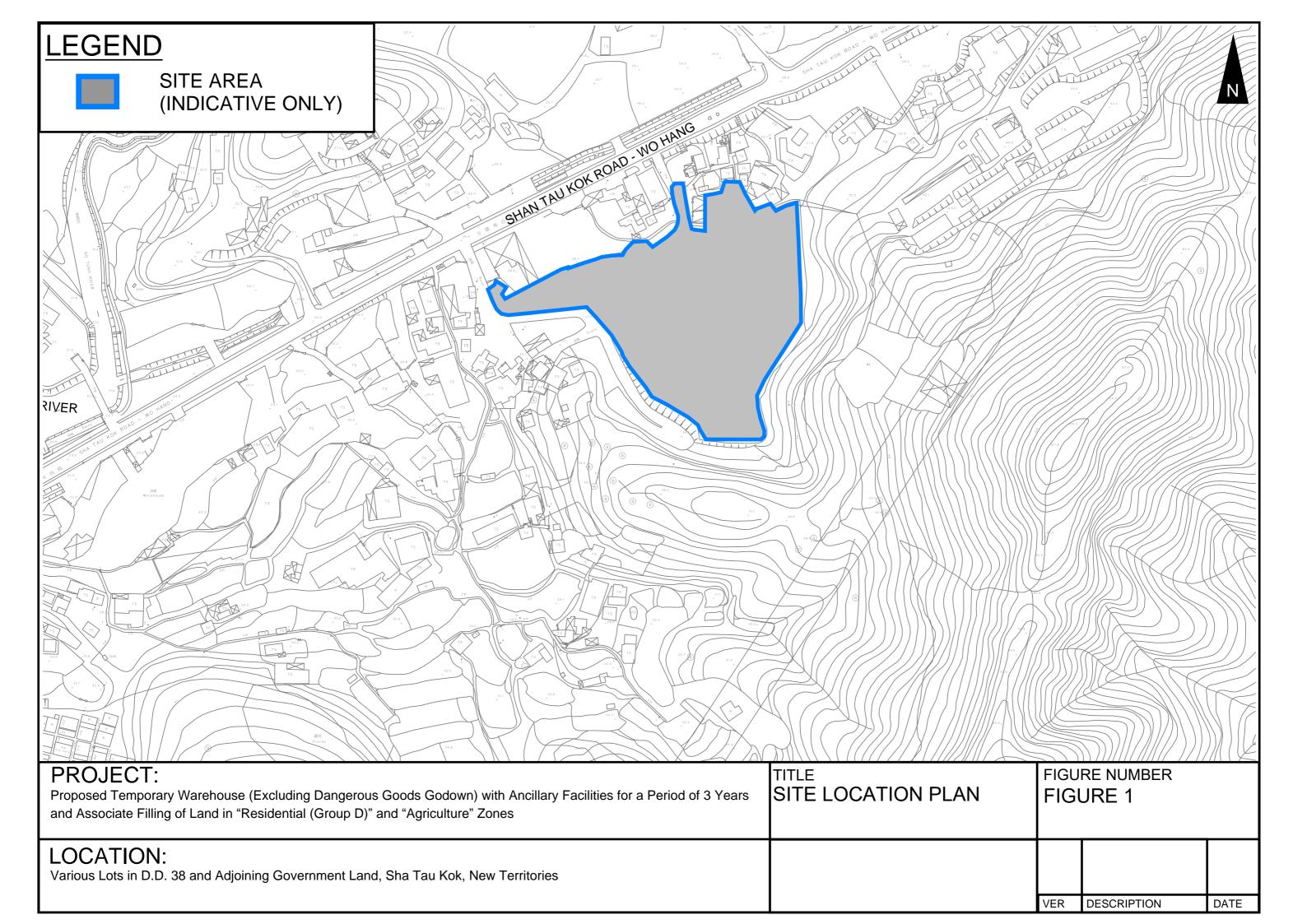
Geotechnical Planning Review Report

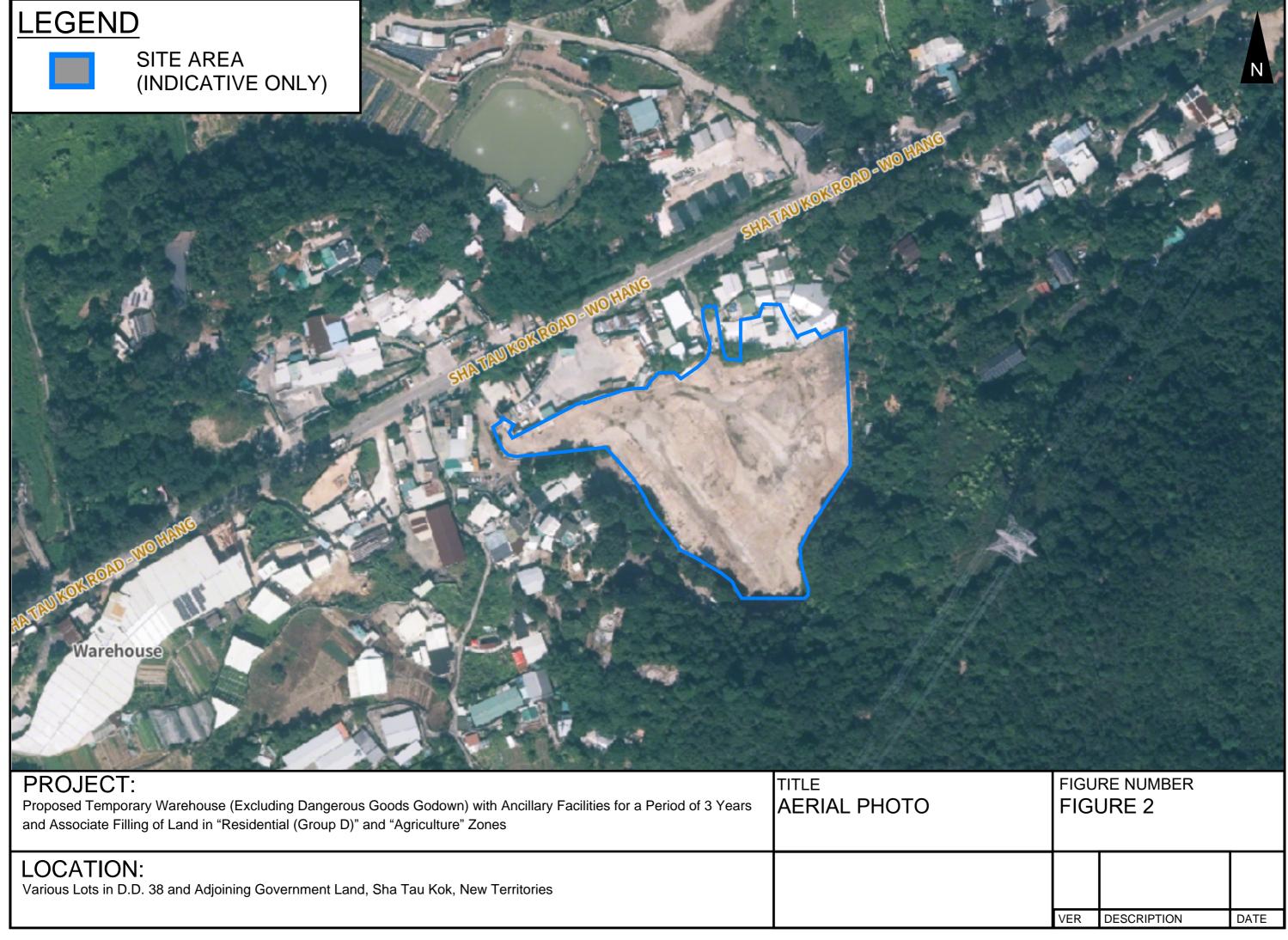
# 7 Conclusion

- 7.1.1 A geotechnical planning review has been conducted for Application Site. The physical conditions as well as the geological conditions of the Application Site have been reviewed and discussed.
- 7.1.2 3NW-D/C 41 and 3NW-D/C 51 beside the proposed development will be assessed to ensure they are complying with current geotechnical standards, if necessary, upgrading works will be carried out in detailed design.
- 7.1.3 For natural terrain hazard issues, the Site does not fall into "In-principle Objection Criteria" but falls within the "Alert Criteria. Natural Terrain Hazard Study (NTHS) is required. Base on past record there is no builder identified and relevant landslide.
- 7.1.4 In conclusion, the proposed development at the Application Site is not anticipated to case adverse geotechnical impact to the nearby area and considered geotechnically feasible.

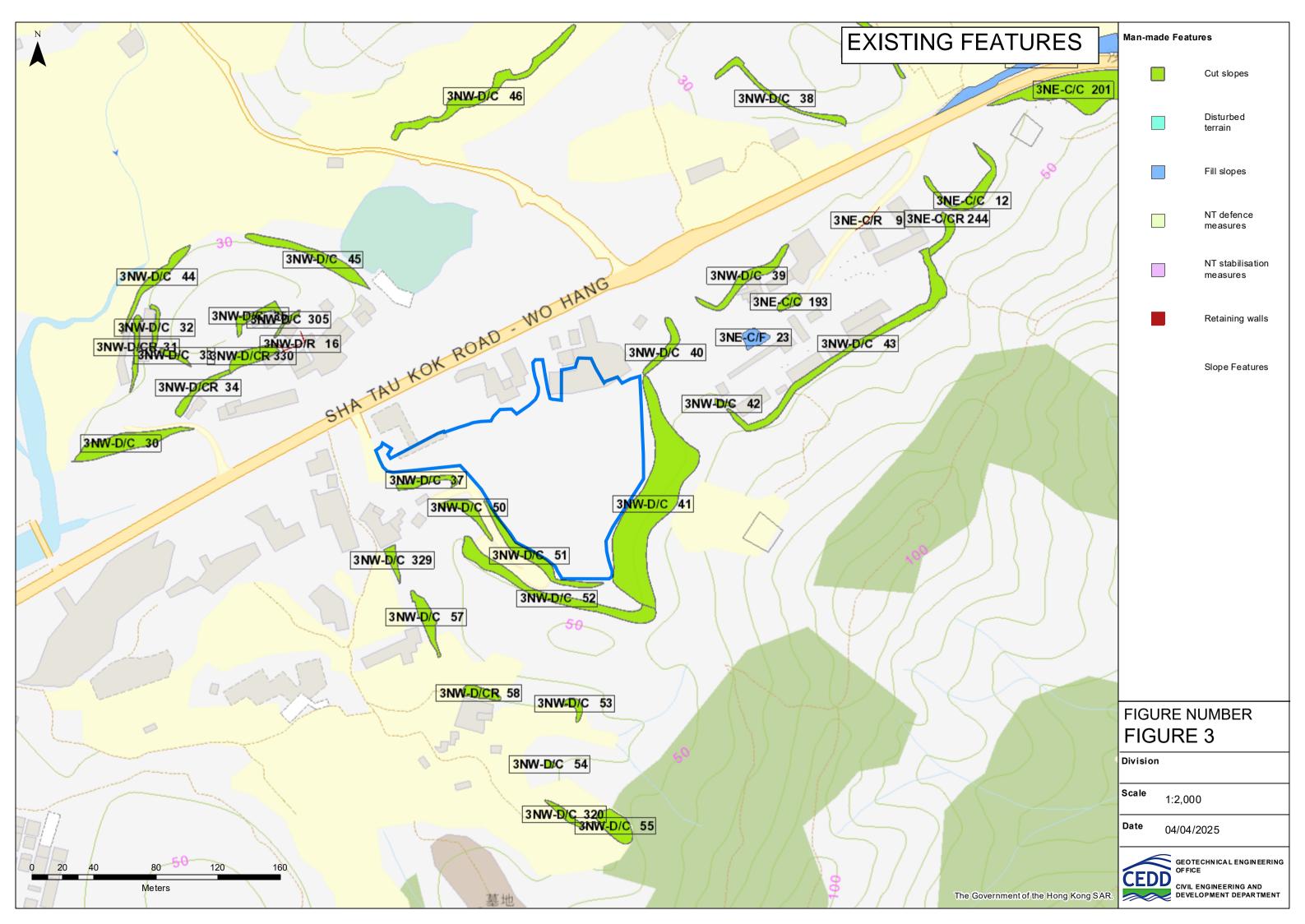
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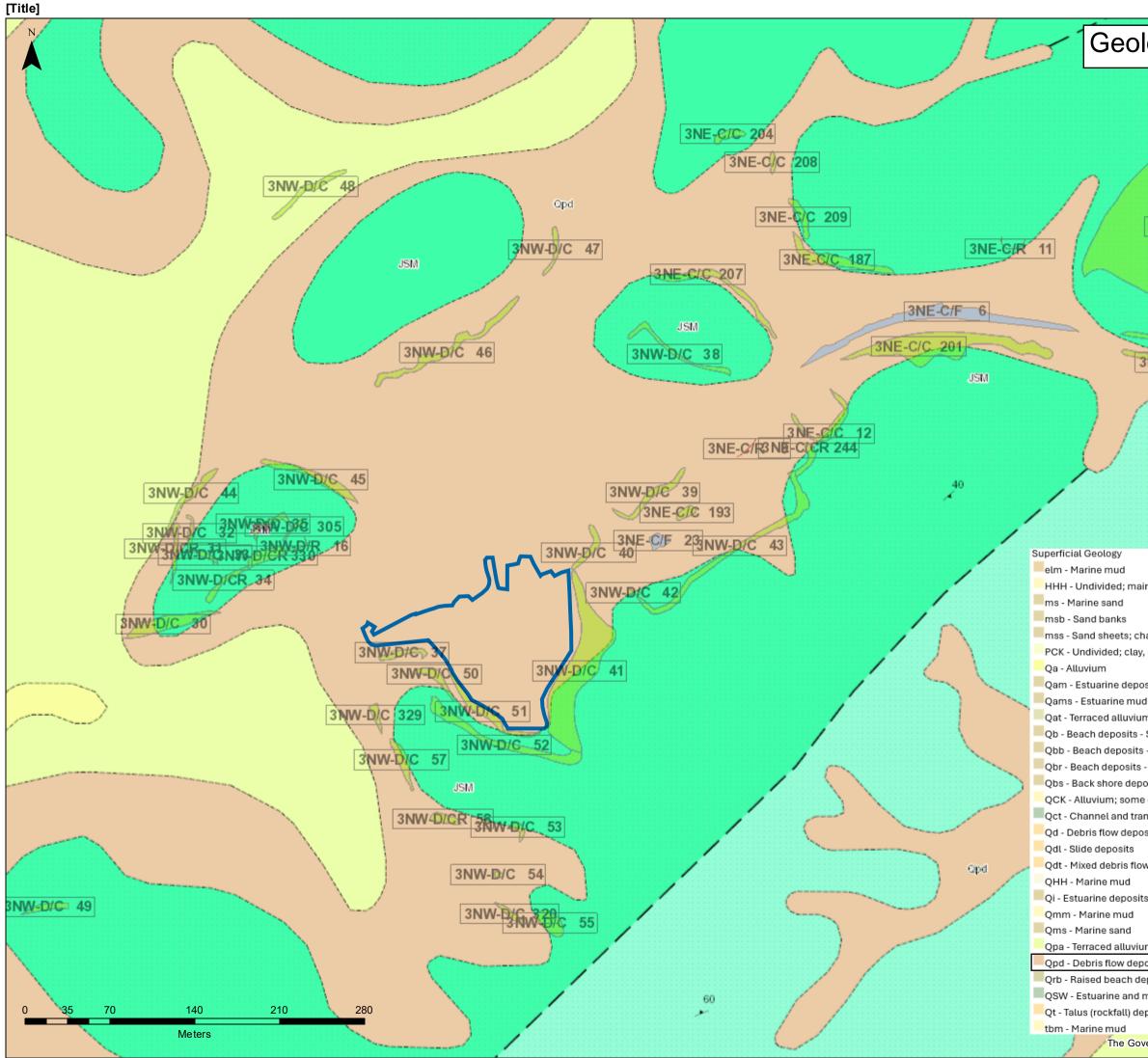
# FIGURES



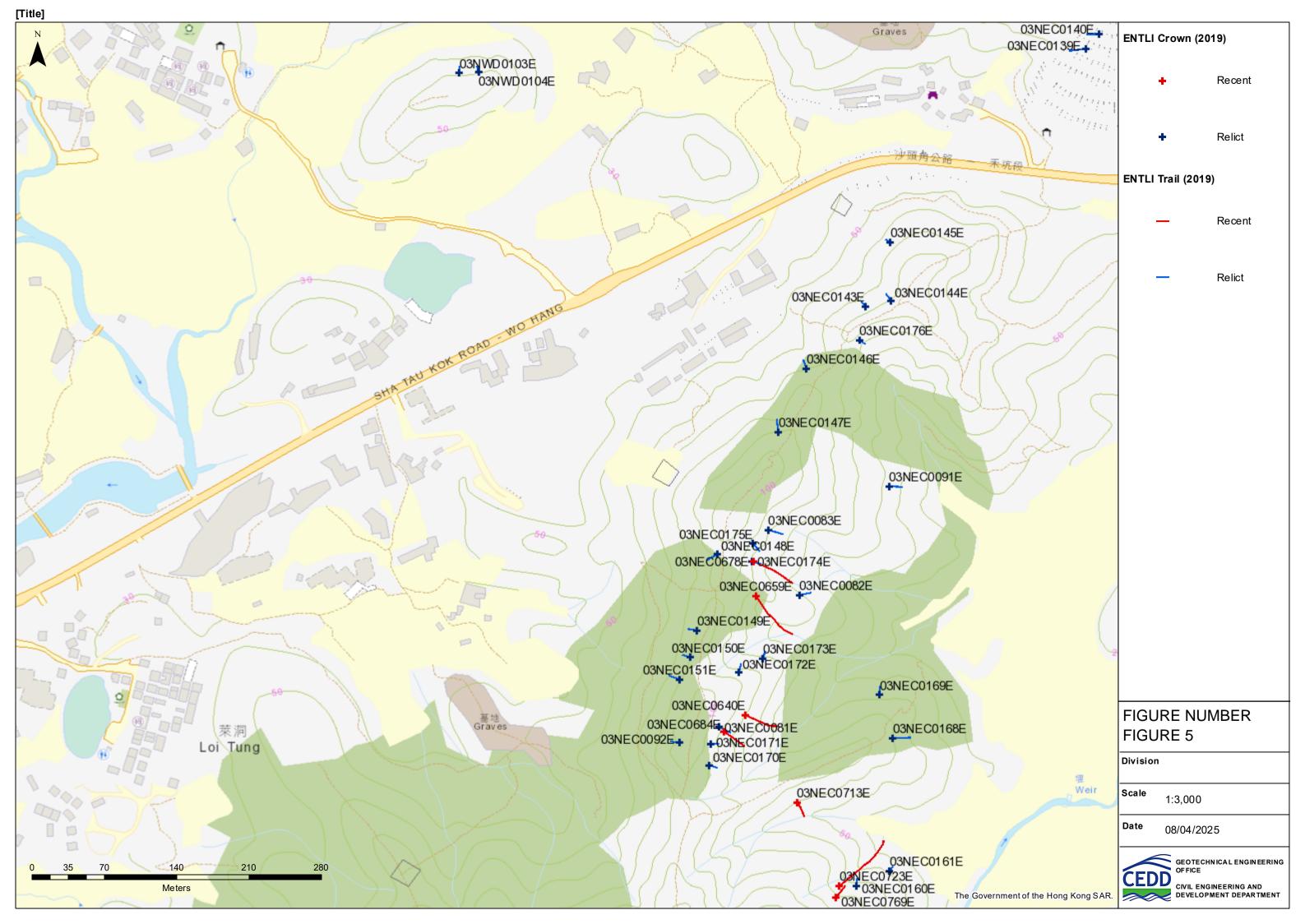


	TITLE AERIAL PHOTO
LOCATION: Various Lots in D.D. 38 and Adjoining Government Land, Sha Tau Kok, New Territories	



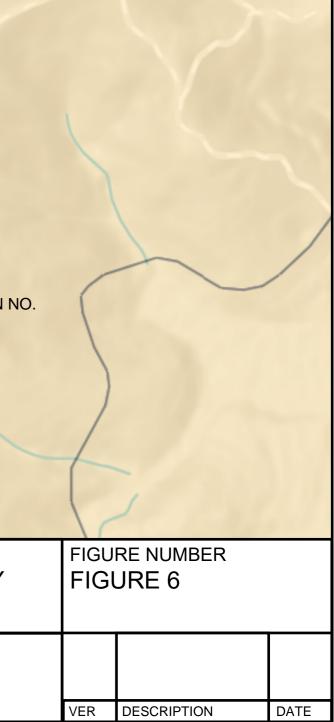


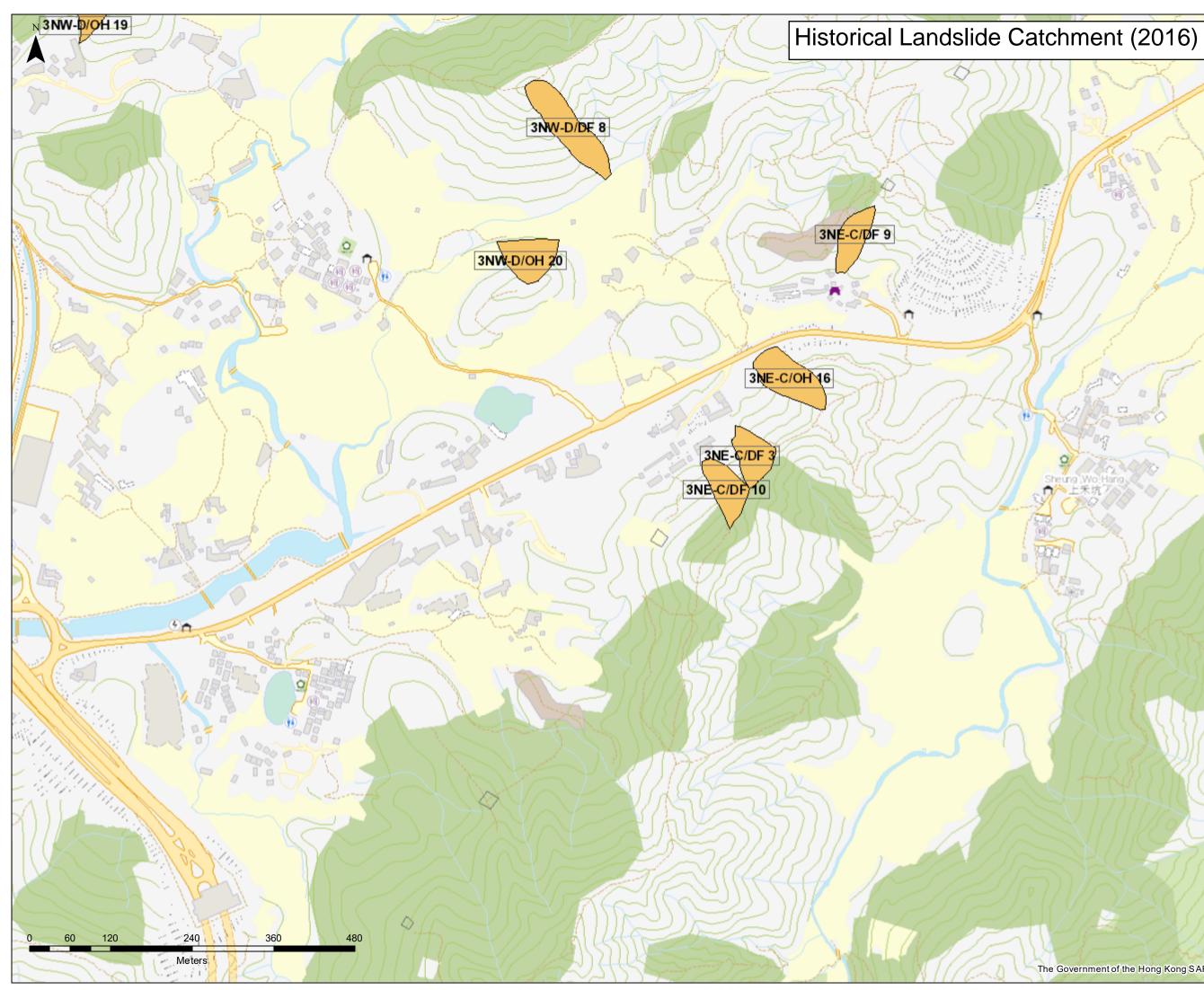
-	
Geological Map	Man-made Features
	Cut slopes
	Disturbed terrain
3NE-C/C 189	Fill slopes
	NT defence measures
3NE-C/ 3NE-C/C 790	NT stabilisation measures
3NE-C/C -200 Opd	Retaining walls
	Slope Features
JTM	
logy mud ided; mainly dark grey marine mud sand	
banks heets; channel infill ded; clay, silt, sand and gravel	
n rine deposits	
arine mud and sand d alluvium leposits - Sand	
deposits - Cobbles and boulders deposits - Beach rock hore deposits - Sand or gravel	
um; some estuarine and marine deposits	
low deposits eposits debris flow and talus deposits	FIGURE NUMBER FIGURE 4
e mud e deposits	Division
ne mud e sand ed alluvium	Scale 1:3,000
flow deposits beach deposits - Sand	Date 04/04/2025
rine and marine deposits ckfall) deposits mud The Government of the Hong Kong SAR.	GEOTECHNICAL ENGINEERING OFFICE CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT
	//~~~



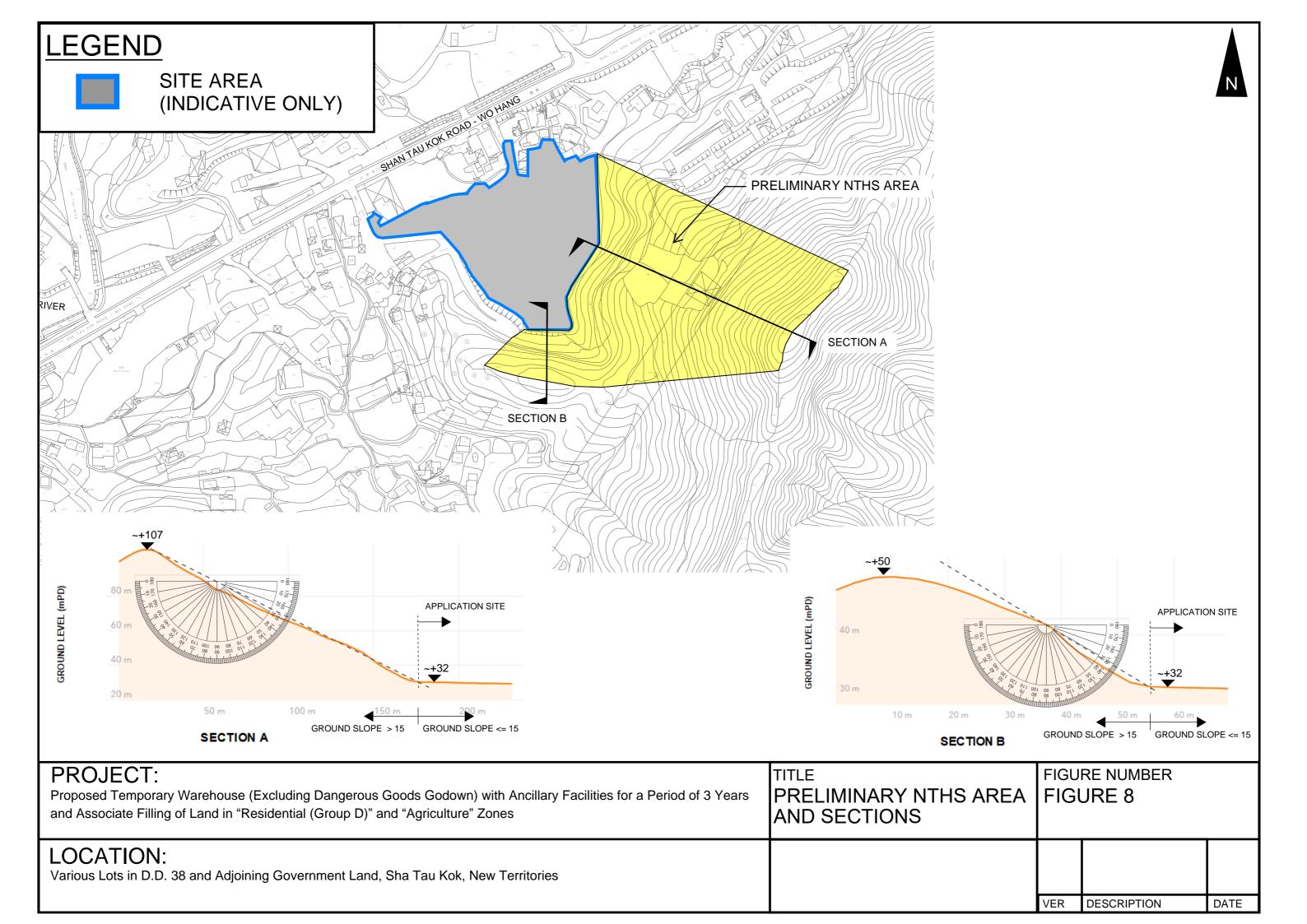
		Summary	of Boulder	Field Inve	entor
LEGEND		Polygon No.	Attribute 1	Attribute 2	
SITE AREA		S3_20	V		Cla
(INDICATIVE ONLY)		\$3_20 \$3_U	- V	-	+
			Attribute 1 - Percentage A Class 1: <10% of the sur		nit covered
		/	Class 2: 10-20% of the s Class 3: 20-50% of the s	surface area of the map u	unit cover
			Class 4: 50-75% of the s Class 5: >75% of the sur		
		uang.	Attribute 2 - Boulder Type Class 1: Corestone or To		
Ν		Noris	Class 2: Colluvial Boulde Class 3: Cliff or Rock Ou	utcrop	
	GON NO. Sha Tau Kok Road		Class 4: Scree or Talus I		
	GON NO. KOK NO		Class 1: Boulders <1m in Class 2: Boulders 1-2m i	in size	
S3_U	ha Tau		Class 3: Boulders 2-5m i Class 4: Boulders >5m ir		
	Sha	1	Attribute 4 - Boulder Shap Class 1: Rounded in Sha		
			Class 2: Angular in Shap	be	
		1			
		1			
	I m	/			
Puer					
Nor					
ad					
VOKRO					
Tauk					
Sha Tau Kok Road - Wo Hang		_			
		~		POLYG	ON
a south and				S3_20	
and send and					
2 Bend					
AG PI	1-1 MB				
			~		
PROJECT:			<b>-</b>		<b>-</b> • •
		BOULDEI	R INVE	NTOF	۲Y
and Associate Filling of Land in "Residential (Group D)"	and Agriculture Zones	MAP			
LOCATION:	d Sha Tau Kak New Tarritarian				
Various Lots in D.D. 38 and Adjoining Government Lan	U, SHA TAU KUK, INEW TEHTIUHES				

Attribute 3         Attribute 3         lass 1 (%)       Class 2 (%)       Class 3 (%)       Class 4 (%)         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         red by boulders       C: Land surface obscured by cloud       victand surface obscured by vegetation         vered by boulders       T: Land surface obscured by terrain shadow							
Attribute 4 Attrib	ory						
-     -     -     -       -     -     -     -     -       red by boulders     N: No boulders observed on the ground surface       wered by boulders     C: Land surface obscured by looud       wered by boulders     V: Land surface obscured by vegetation       red by boulders     T: Land surface obscured by vegetation       red by boulders     S: Land surface obscured by village housing       B: Beach deposits     X: Position of individual boulder(s) outcropping through vegetation       Z: Area not covered by low level aerial photography       Categories of land modification identified were:		Attribute 4					
tered by boulders wered by bo	ass 1 (%)	Class 2 (%)	Class 3 (%)	Class 4 (%)			
tered by boulders wered by bo	-	-	-	-	-		
tered by boulders wered by bo	-	-	-	-	-		
	ered by boulders overed by boulders overed by boulders overed by boulders ered by boulders		N: No boulders observed on the ground surface C: Land surface obscured by cloud V: Land surface obscured by vegetation T: Land surface obscured by virlage housing B: Beach deposits X: Position of individual boulder(s) outcropping through vegetation Z: Area not covered by low level aerial photography Categories of land modification identified were:				





# FIGURE NUMBER **FIGURE 7** Division Scale 1:5,000 Date 09/04/2025 GEOTECHNICAL ENGINEERING OFFICE CEDD CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT The Government of the Hong Kong SAR.



# Appendix A – SIMAR Report

(3NW-D/C37)



### List of Slope Maintenance Responsibility Area(s)

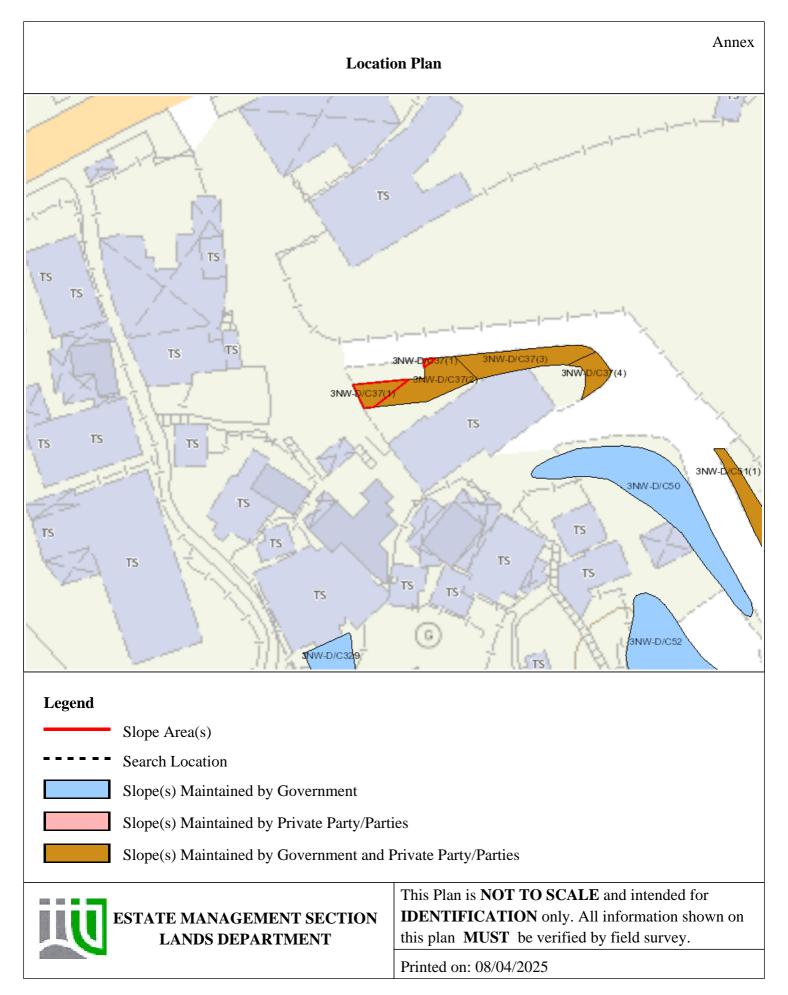
1	3NW-D/C37		Sub-Division	1
	Location	WITHIN DD38 LOTS109, 110	0,112 & GOVERNMENT LAND	
	Responsible Lot/PartyDD38 LOT109		Maintenance Agent	Not Applicable
	Remarks	Not Applicable		

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C37)



### List of Slope Maintenance Responsibility Area(s)

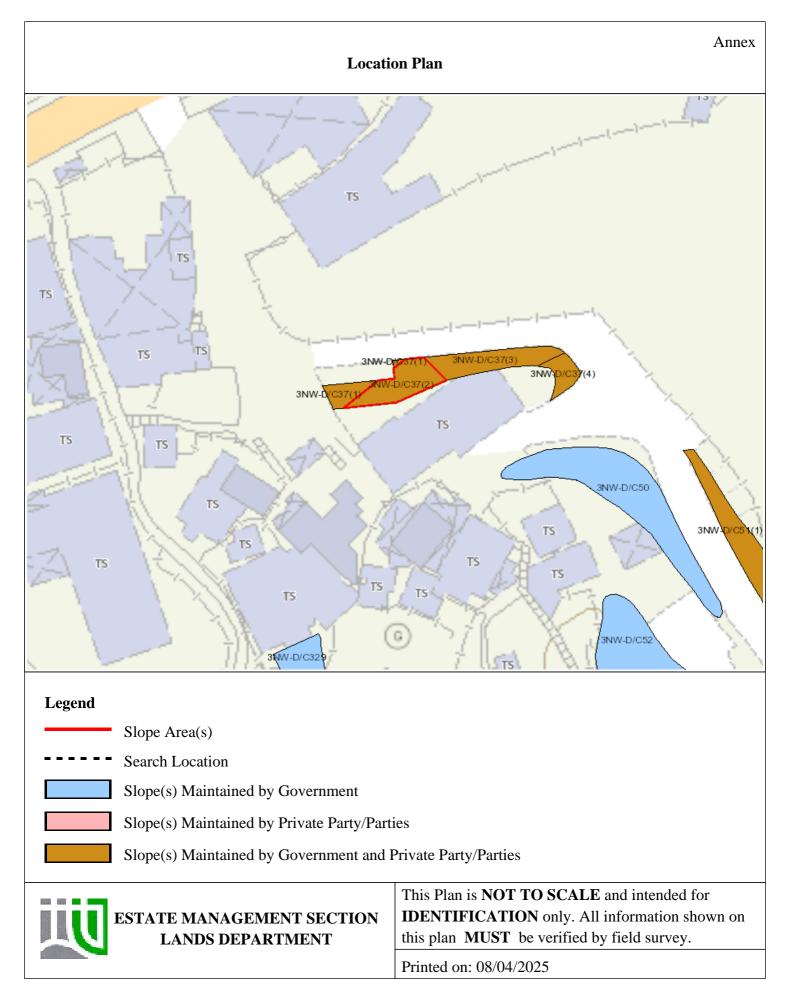
1	3NW-D/C37		Sub-Division	2
	Location	WITHIN DD38 LOTS109, 110	,112 & GOVERNMENT LAND	
	Responsible Lot/Party	DD38 LOT110	Maintenance Agent	Not Applicable
	Remarks	Not Applicable		

#### - End of Report -

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(3NW-D/C37)



### List of Slope Maintenance Responsibility Area(s)

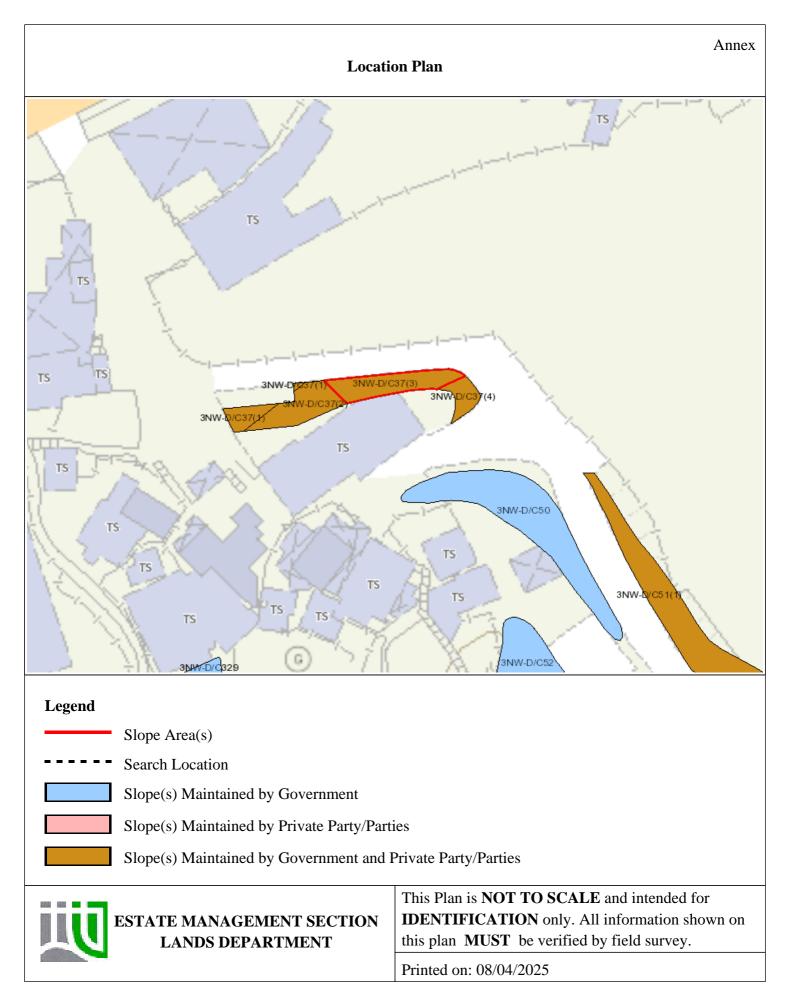
1	3NW-D/C37		Sub-Division	3
	Location WITHIN DD38 LOTS109, 110,112 & GOVERNMENT LAND			
	Responsible Lot/Party	Lands Department	Maintenance Agent	Lands Department
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the		
	Keinai KS	Maintenance Agent directly.		

- End of Report -

#### Notes:

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(3NW-D/C37)



### List of Slope Maintenance Responsibility Area(s)

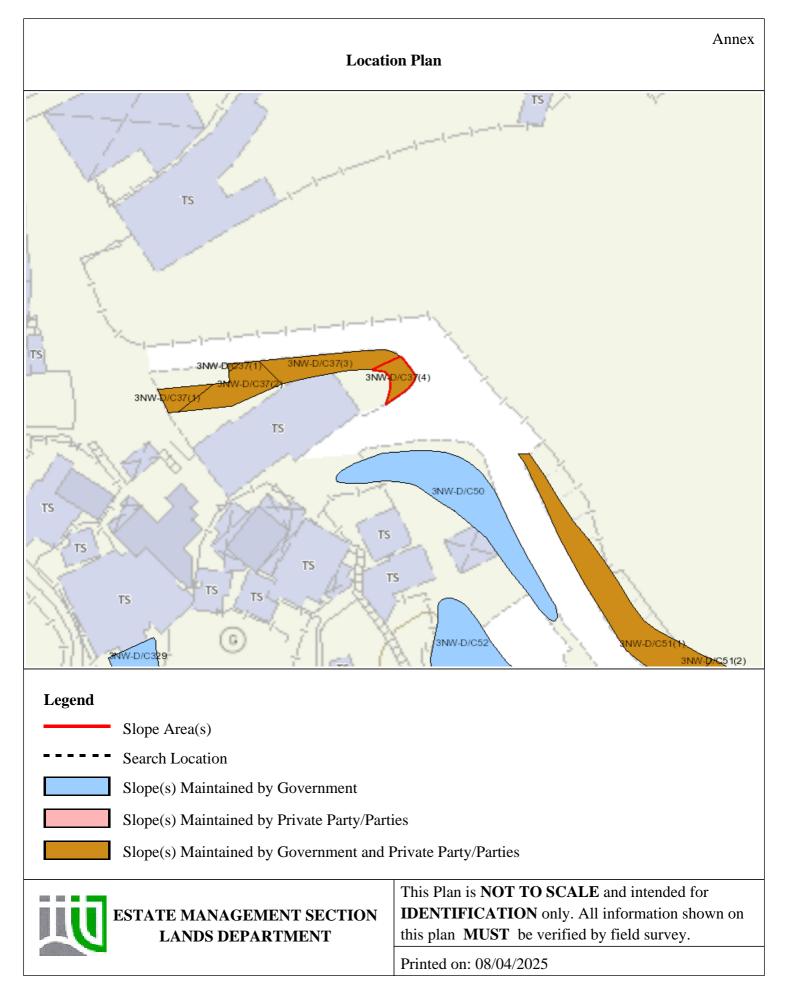
1	3NW-D/C37		Sub-Division	4
	Location	WITHIN DD38 LOTS109, 110,112 & GOVERNMENT LAND		
	Responsible Lot/Party	DD38 LOT112	Maintenance Agent	Not Applicable
	Remarks	Not Applicable		

#### - End of Report -

#### Notes:

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(3NW-D/C40)



### List of Slope Maintenance Responsibility Area(s)

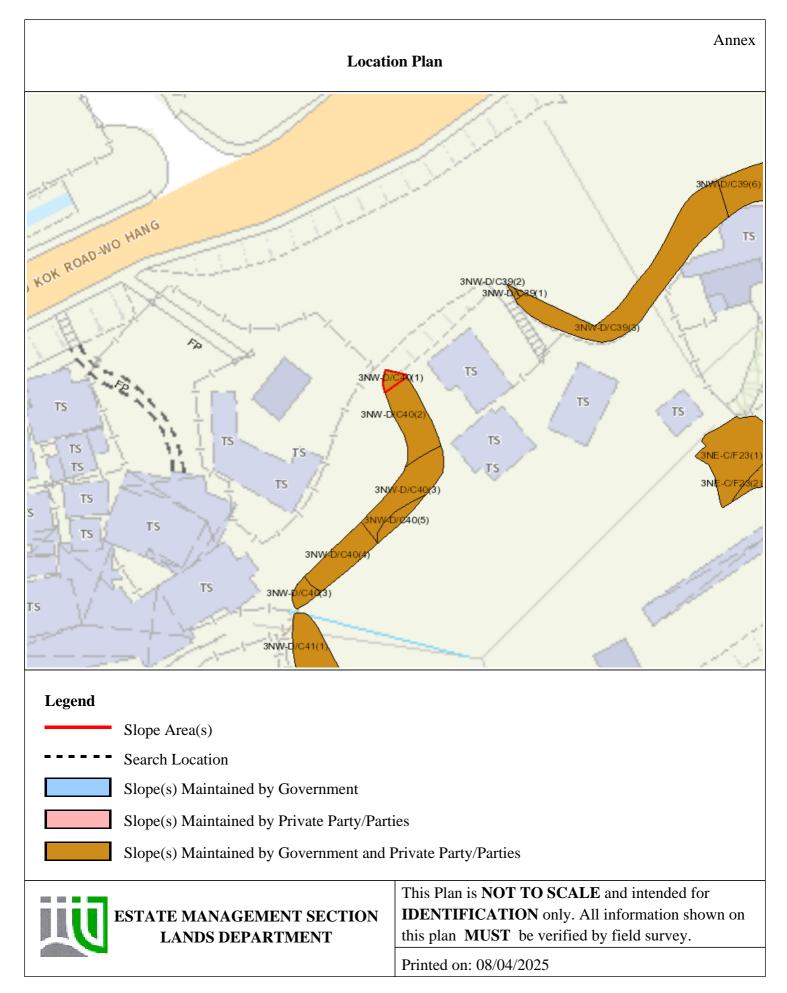
1	3NW-D/C40		Sub-Division	1
	Location	PARTLY WITHIN DD81 LOTS23, 24, 10, 19 & PARTLY ON GL		
Responsible Lot/Party         DD81 Lot24         Maintenance Agent		Maintenance Agent	Not Applicable	
	Remarks	Not Applicable		

#### - End of Report -

#### Notes:

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(3NW-D/C40)



### List of Slope Maintenance Responsibility Area(s)

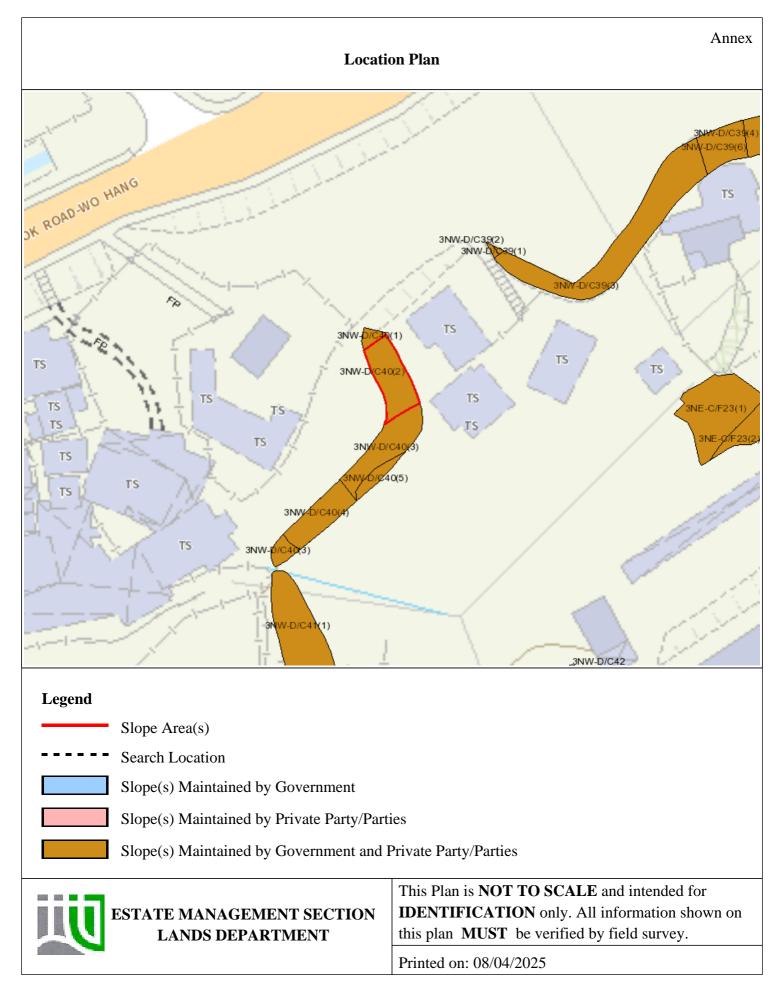
1	3NW-D/C40		Sub-Division	2
	Location	PARTLY WITHIN DD81 LOTS23, 24, 10, 19 & PARTLY ON GL		
Responsible Lot/Party         DD81 Lot23         Maintenance A		Maintenance Agent	Not Applicable	
	Remarks	Not Applicable		

#### - End of Report -

#### Notes:

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(3NW-D/C40)



### List of Slope Maintenance Responsibility Area(s)

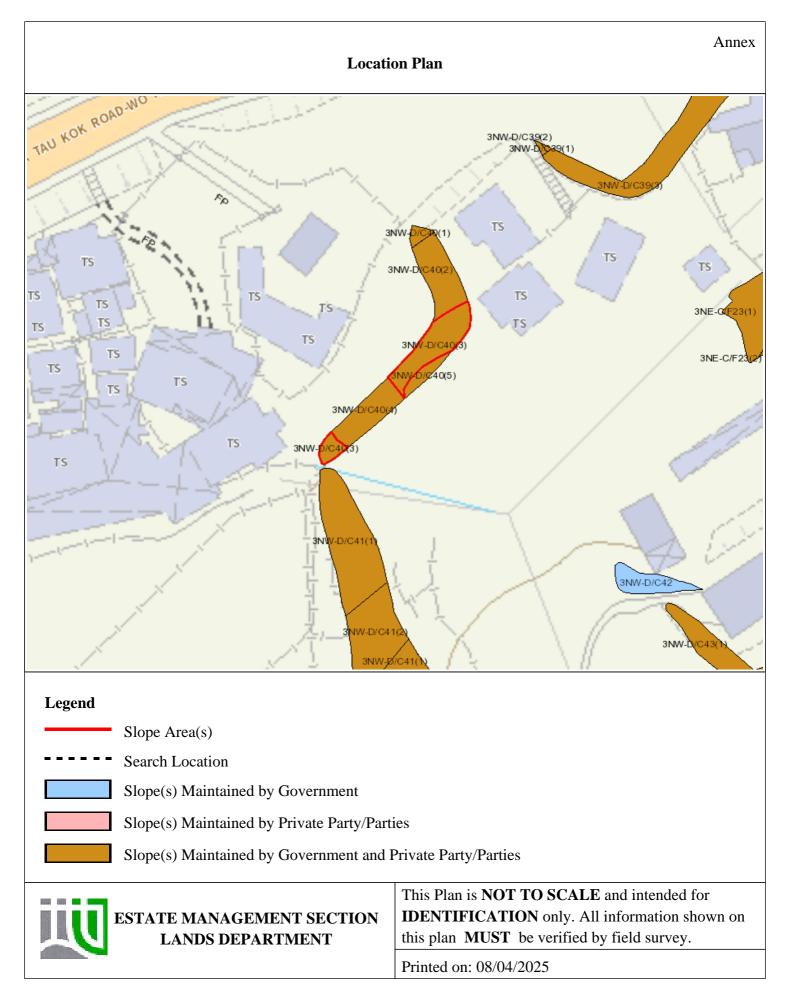
1	3NW-D/C40		Sub-Division	3
Location         PARTLY WITHIN DD81 LOTS23, 24, 10, 19 & PARTLY ON GL			GL	
	Responsible Lot/Party	Lands Department	Maintenance Agent	Lands Department
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the		
	<b>NULLAI KS</b>	Maintenance Agent directly.		

- End of Report -

#### Notes:

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(3NW-D/C40)



#### List of Slope Maintenance Responsibility Area(s)

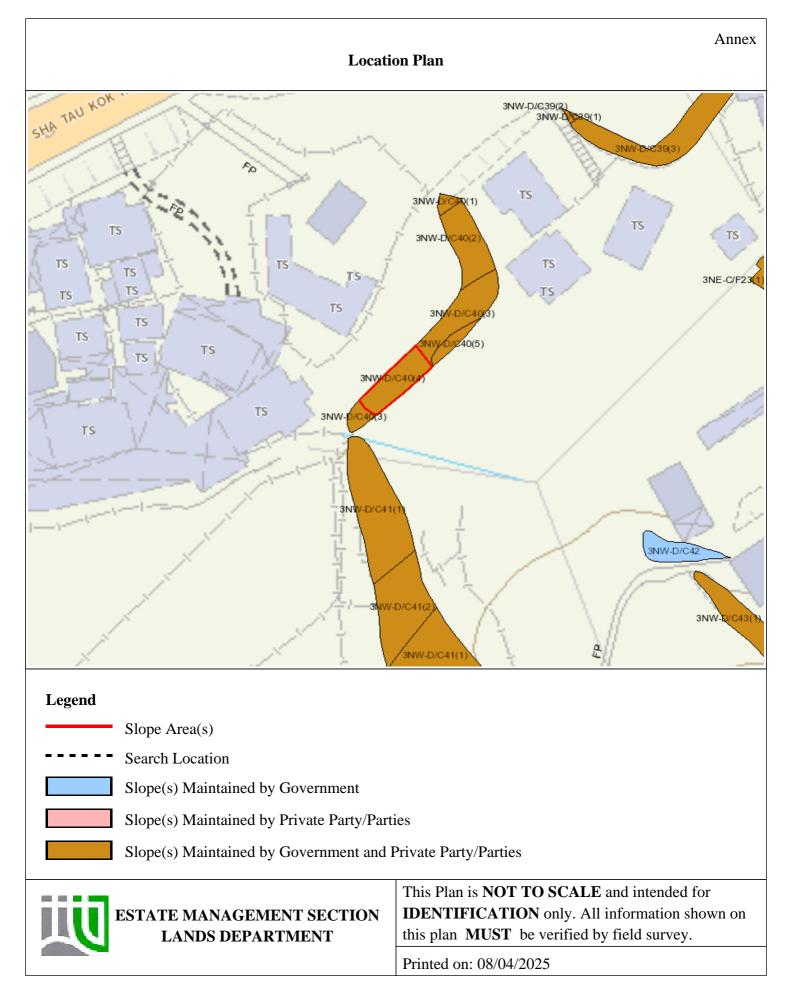
1	3NW-D/C40		Sub-Division	4
	Location         PARTLY WITHIN DD81 LOT		S23, 24, 10, 19 & PARTLY ON GL	
	Responsible Lot/Party         DD81 Lot10		Maintenance Agent	Not Applicable
	Remarks         Not Applicable			

#### - End of Report -

#### Notes:

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(3NW-D/C40)



#### List of Slope Maintenance Responsibility Area(s)

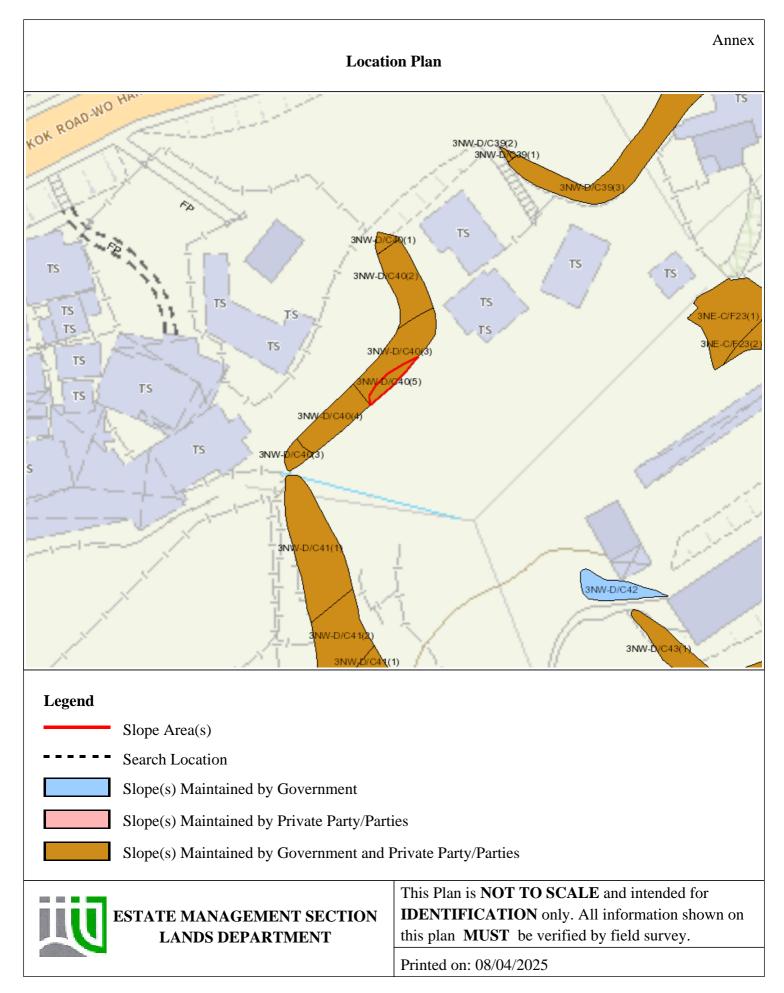
1	3NW-D/C40		Sub-Division	5
	Location         PARTLY WITHIN DD81 LOT		TS23, 24, 10, 19 & PARTLY ON GL	
	Responsible Lot/Party         DD81 Lot19		Maintenance Agent	Not Applicable
	Remarks Not Applicable			

#### - End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C41)



#### List of Slope Maintenance Responsibility Area(s)

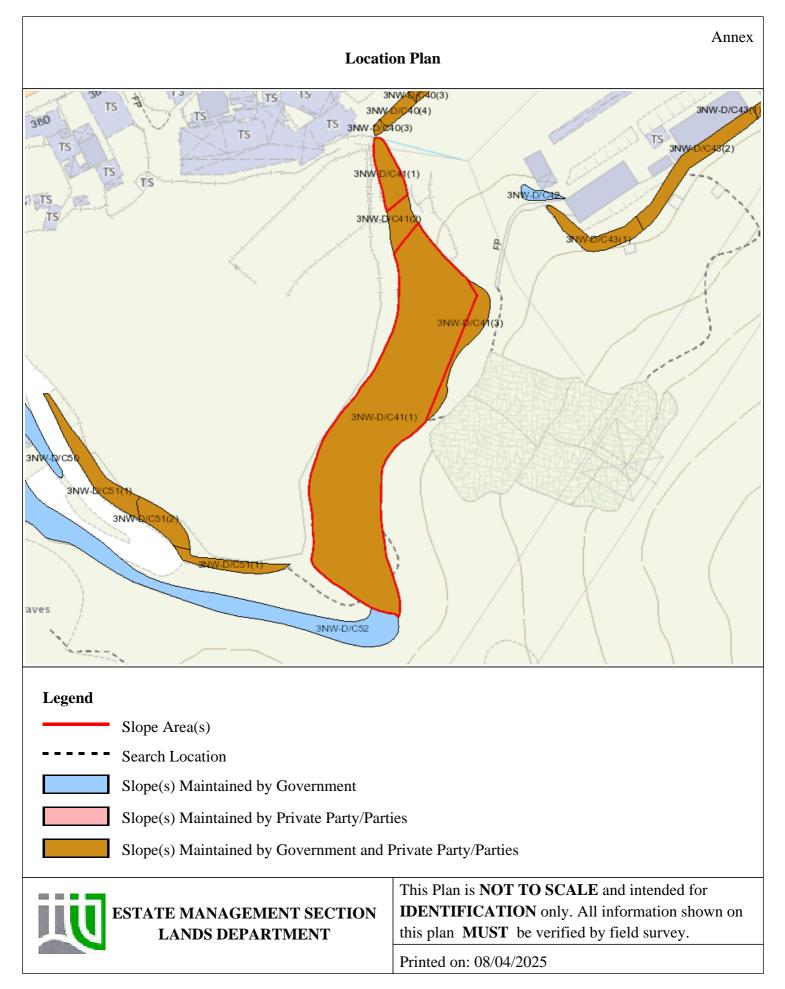
1	3NW-D/C41		Sub-Division	1	
	Location	WITHIN LICENCE N8797 & LOT NO. 135 IN DD38 AND ADJOINING GOVERNMENT			
		LAND IN DD38 & 81	LAND IN DD38 & 81		
	Responsible Lot/Party         Lands Department		Maintenance Agent	Lands Department	
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the			
	Kemarks	Maintenance Agent directly.			

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C41)



#### List of Slope Maintenance Responsibility Area(s)

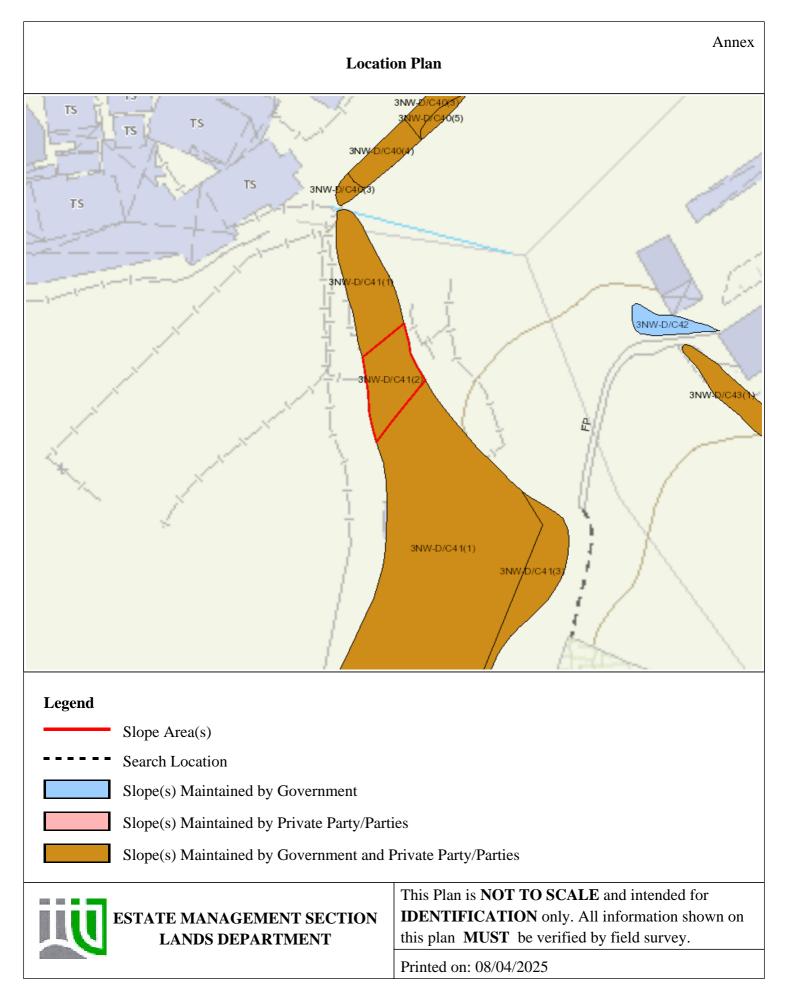
1	3NW-D/C41		Sub-Division	2
	Location	WITHIN LICENCE N8797 & LOT NO. 135 IN DD38 AND ADJOINING GOVERNMENT		
	Location	LAND IN DD38 & 81		
	Responsible Lot/Party         DD38 LOT135		Maintenance Agent	Not Applicable
	Remarks         Not Applicable			

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C41)



#### List of Slope Maintenance Responsibility Area(s)

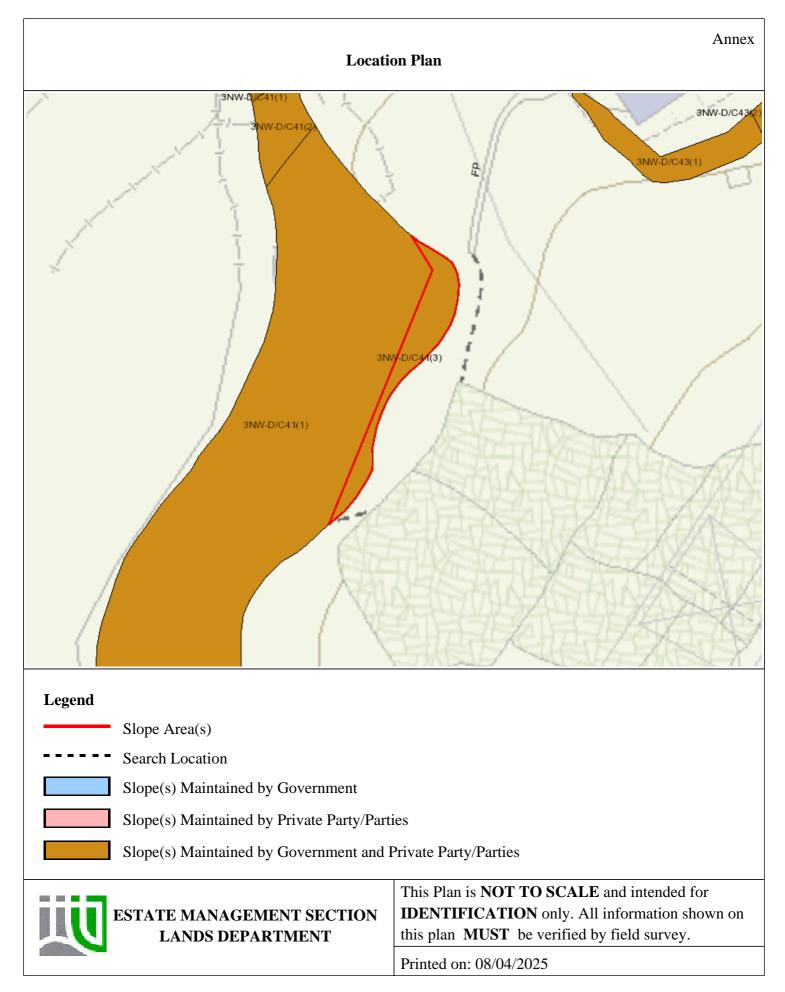
1	3NW-D/C41		Sub-Division	3	
	Location	WITHIN LICENCE N8797 & I	WITHIN LICENCE N8797 & LOT NO. 135 IN DD38 AND ADJOINING GOVERNMENT		
	Location	LAND IN DD38 & 81	LAND IN DD38 & 81		
	<b>Responsible Lot/Party</b>	LICENCE N8797	Maintenance Agent	Not Applicable	
	Remarks         Not Applicable				
2	3NW-D/C41		Sub-Division	3	
			LOT NO. 135 IN DD38 AND ADJOINING GOVERNMENT		
	Location	LAND IN DD38 & 81			
	Responsible Lot/Party Lands Department		Maintenance Agent	Lands Department	
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the			
	Nellial KS	Maintenance Agent directly.			

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C50)



#### List of Slope Maintenance Responsibility Area(s)

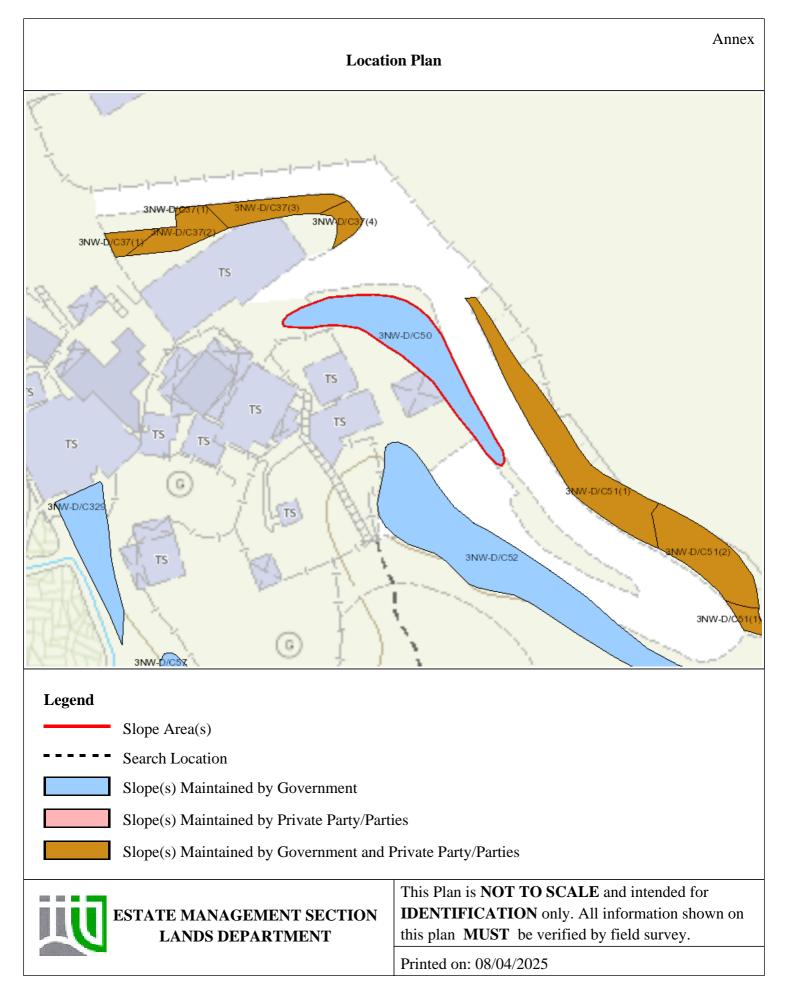
1	3NW-D/C50		Sub-Division	Not Applicable
	Location         TO THE W OF DD38 LOT121			
	Responsible Lot/Party         Lands Department		Maintenance Agent	Lands Department
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the		
	Kemarks	Maintenance Agent directly.		

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C51)



#### List of Slope Maintenance Responsibility Area(s)

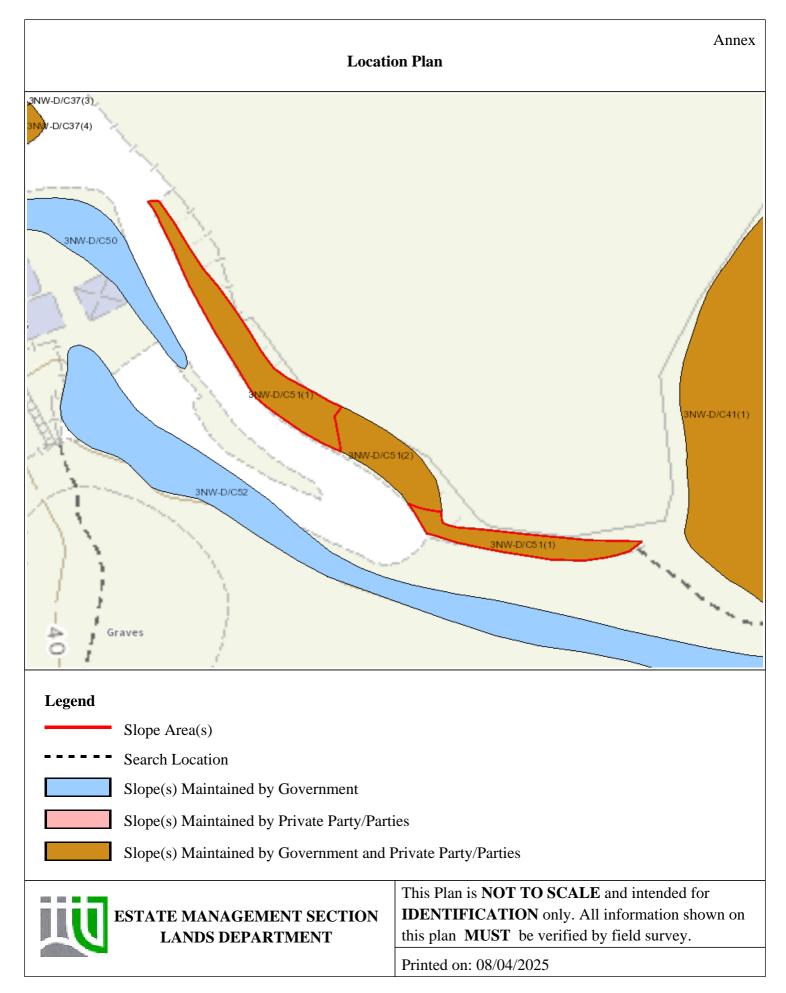
1	3NW-D/C51		Sub-Division	1
	Location         SLOPE FALLS IN GL, DD38		D38 LOT127 NEAR SPOT LEVEL 35.6	
	Responsible Lot/Party         Lands Department		Maintenance Agent	Lands Department
	Remarks	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the		
	<b>NUMATKS</b>	Maintenance Agent directly.		

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C51)



#### List of Slope Maintenance Responsibility Area(s)

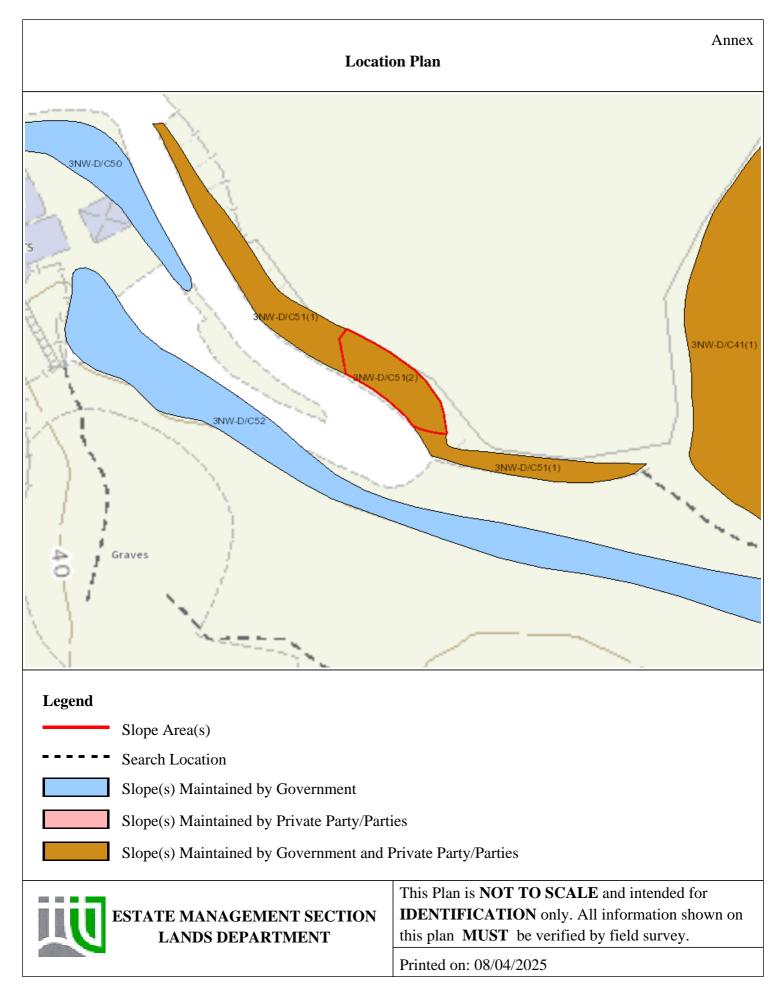
1	3NW-D/C51		Sub-Division	2
	Location SLOPE FALLS IN GL, DD38 I		LOT127 NEAR SPOT LEVEL 35.6	
	Responsible Lot/Party         DD38 Lot127		Maintenance Agent	Not Applicable
	Remarks         Not Applicable			

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



(3NW-D/C52)



#### List of Slope Maintenance Responsibility Area(s)

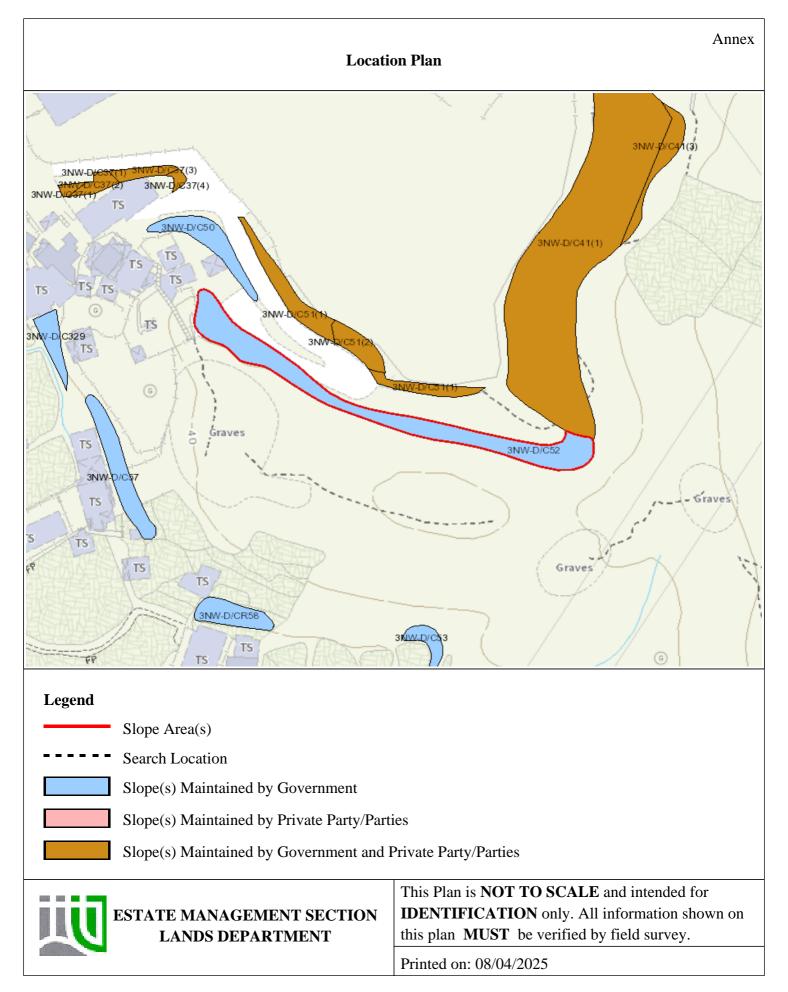
1	3NW-D/C52		Sub-Division	Not Applicable
	LocationTO THE S OF DD38 LOT127			
	Responsible Lot/Party         Lands Department		Maintenance Agent	Lands Department
	Damanlar	For enquiries about the maintenance of this slope / sub-division of the slope, please contact the		
	Remarks	Maintenance Agent directly.		

- End of Report -

#### Notes:

(i) The location plan in Annex is for identification purposes of slope(s) only.

(ii) The slope(s) as listed in the Slope Maintenance Responsibility Report may not be shown on the location plan in Annex.



# Appendix B – SIS Records

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# **BASIC INFORMATION**

Location:	Loi Tung Village, S	ha Tau Kok Road - Wo Hang, New Territories.
Registration Date:	08-07-1999	
Ranking Score (NPRS):	0 (LPMit)	
Date of Formation:	pre-1977	
Date of Construction/ Modification:		
Data Source:	EI(Lands D)	
Approximate Coordinates:	Easting : 837297	Northing : 842613

# CONSEQUENCE-TO-LIFE CATEGORY

Facility at Crest:	Densely-used open area/facilities
Distance of Facility from Crest (m):	0
Facility at Toe:	Road/footpath with very low traffic density
Distance of Facility from Toe (m):	0
Consequence-to-life Category:	2
Remarks:	N/A

# **SLOPE PART**

(1)	Max. Height (m): 3	Length (m): 24	Average Angle (deg): 35
(1)	Max. Height (m): 3	Length (m): 16	Average Angle (deg): 35

#### WALL PART

N/A

# MAINTENANCE RESPONSIBILITY

(1) Sub Div.: 1	Mixed Feature	Party: DD38 LOT109	Agent: N/A	Land Cat.: 1	Reason Code: 1	MR Endorsement Date: 19-09-2008
(2) Sub Div.: 2	Mixed Feature	Party: DD38 LOT110	Agent: N/A	Land Cat.: 1	Reason Code: 1	MR Endorsement Date: 19-09-2008
(3) Sub Div.: 3	Mixed Feature	Party: Lands D Agent	: Lands D	Land Cat.: 5b(vi)	) Reason Code:	62 MR Endorsement Date: 19-09-2008
(4) Sub Div.: 4	Mixed Feature	Party: DD38 LOT112	Agent: N/A	Land Cat.: 1	Reason Code: 1	MR Endorsement Date: 19-09-2008

#### **DETAILS OF SLOPE / RETAINING WALL**

Date of Inspection:	19-11-2010
Data Source:	EI(Lands D)
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

# **SLOPE PART**



Slope Part (1)					
Surface Protection (%):	Bare: O Veç	getated: 40	Chunam: 60	Shotcrete: O	Other Cover: O
Material Description:	Material type: S	Soil Geola	gy: N/A		
Berm:	No. of Berms: N	/A Min. B	erm Width (m): N	/Α	
Weepholes:	Size (mm): 50	Spacing (m	): 1.2		
Slope Part (2)					
Surface Protection (%):	Bare: O Veg	getated: 40	Chunam: 60	Shotcrete: O	Other Cover: O
Material Description:	Material type: S	oil Geolo	gy: N/A		
Berm:	No. of Berms: N	/A Min.B	erm Width (m): N	/Α	
Weepholes:	Size (mm): N/A	Spacing (	m): N/A		

#### WALL PART

N/A

#### **SERVICES**

N/A

# **CHECKING STATUS INFORMATION**

N/A

# **BACKGROUND INFORMATION**

GIU Cell Ref.:	3NW25B9			
Map Sheet Reference (1:1000):	3NW-25B			
Aerial Photos:	CN10451 (1995), CN	10452 (1995)		
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(NO5)			
Data Collected On:	19-11-2010			
Date of Construction, Subsequent Modification and Demolition:	Modification: Constr	ucted Before: 1978	After: 1974	
Related Reports/Files or Documents:	File/Report: PWDC File/Report: PWDC	Ref. No.: GC 4/1/2-3 f Ref. No.: GC 4/1/2-3 f		
Remarks:	N/A			
Follow Up Actions:	N/A			
DH-Order (To Be Confirmed with Buildings Department):	None			
Advisory Letter (To Be Confirmed with Buildings Department):	None			



LPMIS:

Agreement No.: CE10/2007 Report No.:

Report No.: S2R132/2008

# ENHANCED MAINTENANCE INFORMATION

From Maintenance Department: (Last Updated Date: 19/02/2025)

# **STAGE 1 STUDY REPORT**

Inspected	On:
Weather:	
District:	

**Section No:** 

Height(m):

Section No:

Type of Toe Facility:

Distance from Toe(m):

Type of Crest Facility:

Distance from Crest(m):

**Consequence Category:** 

**Engineering Judgement:** 

Type of Toe Facility:

Distance from Toe(m): Type of Crest Facility:

Distance from Crest(m):

**Consequence Category:** 

**Engineering Judgement:** 

Sign of Seepage:

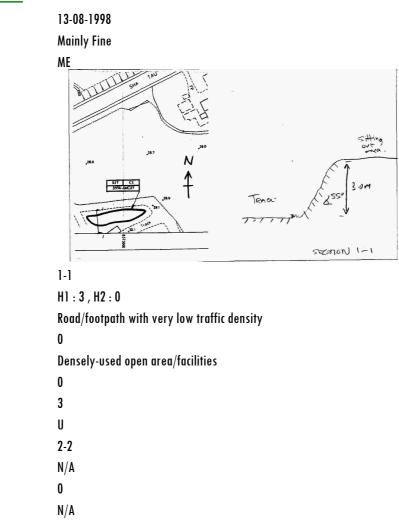
Sign of Distress:

Note:

**Criterion A satisfied:** 

**Criterion D satisfied:** 

Non-routine maintenance required:



0

3

U

N

N

N

N/A

Wall : N/A

Wall : N/A

Slope : No signs of seepage

Slope : Reasonable (near crest, mid-portion, at toe)



CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

Masonry wall/Masonry facing:	Ν
Note:	N/A
Consequence category (for critical section):	3
Observations:	N/A
Emergency Action Required:	Ν
Action By:	N/A

# ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	Ν
Action By:	N/A

# **OTHER EXTERNAL ACTION**

Check / repair Services:	Ν
Action By:	N/A
Non-routine Maintenance:	Ν
Action By:	N/A

# eLPMIS

LPM/LPMit Details Report	
LPM Study Feature No.:	3NW-D/C 37
Location:	LOI TUNG VILLAGE, SHA TAU KOK ROAD - WO HANG, NEW TERRITORIES
District Council:	North
Maintenance Responsibility (At the Time of Selection):	Mixed
Responsible Party for Maintenance of Government Portion:	Lands D
Private Lot No.:	DD38 LOT109,DD38 LOT110,DD38 LOT112
LPM/LPMit Study	
Agreement No.:	CE10/2007
Study Type:	Stage 2 Study
Consultant:	C M Wong & Associates Ltd.
GEO Managing Section / Engineer:	SS / SS <b>3</b>
Study Status:	Study completed
Design Approach:	N/A
Option Assessment Accepted:	N/A
Study Report No.:	S2R132/2008
Programme / Actual Commencement:	06-06-2008



	Programme / Actual Completion:	26-02-2010
	Report Recommendation (For Stage 2 Study):	Advisory Letter
	District Check Status:	Checked
	Checking Certificate No.:	N/A
	GEO Engineer's Remarks:	N/A
LPM/L	PMit Works	
	Works Contract No.:	N/A
	GEO Managing Section / Engineer:	N/A / N/A
	Contractor:	N/A
	Progress Status:	N/A
	Reason of Study Termination / Works Deletion (If Necessary):	N/A
	Forecast Commencement Date:	N/A
	Forecast Completion Date:	N/A
	Completion Cert. Issued:	N/A
	Site Handed Over to Maintenance Department on:	N/A
	Estimated Cost for Upgrading (HK\$M):	N/A
	Maintenance Manual No.:	N/A
	Actual Works:	N/A
	No. of Tree Felled:	N/A
	No. of Tree Planted (Incl. Transplant):	N/A
	% Bare of Slope Surfacing:	N/A
	% Vegetated of Slope Surfacing:	N/A
	% Shotcrete of Slope Surfacing:	N/A
	Other Hard Surface of Slope Surfacing:	N/A



# PHOTO

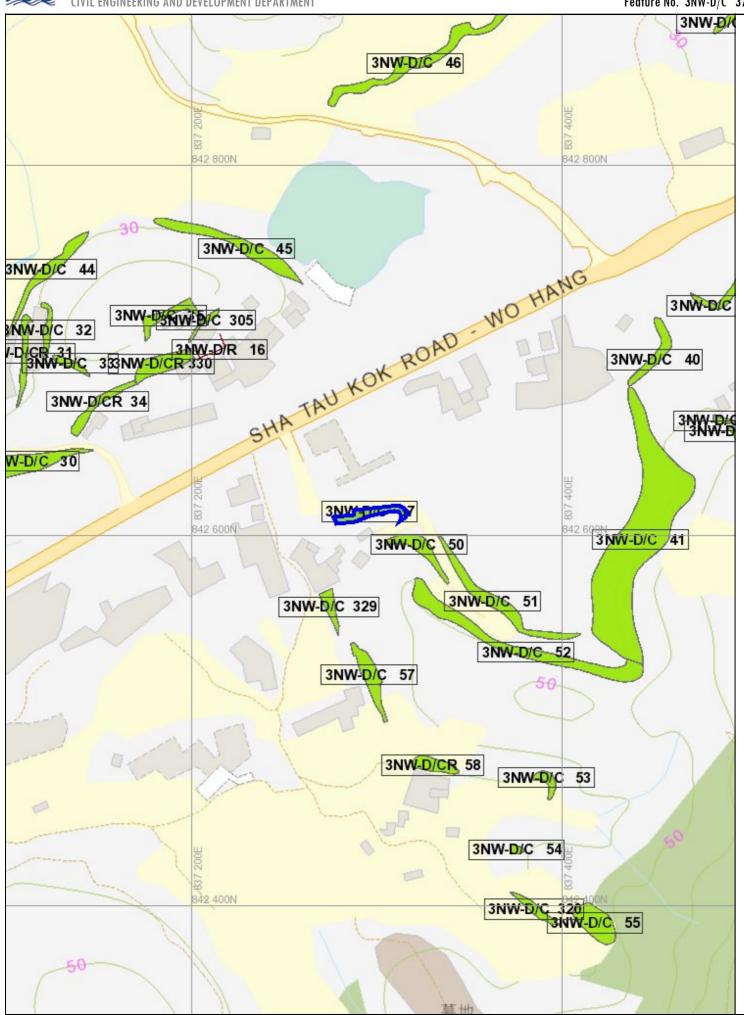








Feature No. 3NW-D/C 37



RECORD RETRIEVED FROM SIS ON 12/03/2025 20:59



# **BASIC INFORMATION**

Location:	North East of Loi T	ung Village, off Sha Tau Kok, Wo Hang, North
Registration Date:	24-10-1997	
Ranking Score (NPRS):	0 (LPMit)	
Date of Formation:	pre-1977	
Date of Construction/ Modification:		
Data Source:	EI(Lands D)	
Approximate Coordinates:	Easting : 837452	Northing : 842697

# CONSEQUENCE-TO-LIFE CATEGORY

Facility at Crest:	Remote area or abandoned facilities
Distance of Facility from Crest (m):	0
Facility at Toe:	Lightly-used open area/facilities
Distance of Facility from Toe (m):	0
Consequence-to-life Category:	3
Remarks:	N/A

# **SLOPE PART**

(1) Max. Height (m): 5 Length (m): 45 Average Angle (deg): 80

#### WALL PART

N/A

### **MAINTENANCE RESPONSIBILITY**

(1) Sub Div.: 1 (2) Sub Div.: 2	Mixed Feature Mixed Feature	Party: DD81 Lot24 Agent: N/A Party: DD81 Lot23 Agent: N/A	Land Cat.: 1,5b(vi),7 Land Cat.: 1,5b(vi),7	Reason Code: 1MR Endorsement Date: 02-04-1998Reason Code: 1MR Endorsement Date: 02-04-1998
(3) Sub Div.: 3 1998	Mixed Feature	Party: Lands D Agent: Lands D	Land Cat.: 1,5b(vi),7	Reason Code: 62,90 MR Endorsement Date: 02-04-
(4) Sub Div.: 4 (5) Sub Div.: 5	Mixed Feature Mixed Feature	Party: DD81 Lot10 Agent: N/A Party: DD81 Lot19 Agent: N/A	Land Cat.: 1,5b(vi),7 Land Cat.: 1,5b(vi),7	Reason Code: 1 MR Endorsement Date: 02-04-1998 Reason Code: 1 MR Endorsement Date: 02-04-1998

# DETAILS OF SLOPE / RETAINING WALL

Date of Inspection:	15-04-2010
Data Source:	EI(Lands D)
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

# **SLOPE PART**



Slope Part (1) Surface Protection (%):	Bare: O Veae	tated: 100	Chunam: O	Shotcrete: O	Other Cover: O
Material Description:	Material type: So			51101111010.0	
Berm:	No. of Berms: N/A		m Width (m): N	/A	
Weepholes:	Size (mm): N/A	Spacing (m)	: N/A		

#### WALL PART

N/A

#### **SERVICES**

N/A

# **CHECKING STATUS INFORMATION**

N/A

### **BACKGROUND INFORMATION**

GIU Cell Ref.: Map Sheet Reference (1:1000):	3NW25B9 3NW-25B		
Aerial Photos:	CN10450 (1995), CN10451 (1995)		
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(NO5)		
Data Collected On:	15-04-2010		
Date of Construction, Subsequent Modification and Demolition:	Modification: Constructed Before: 1964 After: N/A		
Related Reports/Files or Documents:	File/Report: PWDCRef. No.: GC 4/1/2-3 f (19) Part VIFile/Report: PWDCRef. No.: GC 4/1/2-3 f (19) Part VI		
Remarks:	N/A		
Follow Up Actions:	N/A		
DH-Order (To Be Confirmed with Buildings Department):	None		
Advisory Letter (To Be Confirmed with Buildings Department):	None		
LPMIS:	Agreement No.: CE33/2004 Report No.: S2R161/2006		

# ENHANCED MAINTENANCE INFORMATION



From Maintenance Department: (Last Updated Date: 19/02/2025)

### STAGE 1 STUDY REPORT

Inspected On:	
Weather:	
District:	ME
Section No:	]-]
Height(m):	
Type of Toe Facility:	Lightly-used open area/facilities
Distance from Toe(m):	0
Type of Crest Facility:	Remote area or abandoned facilities
Distance from Crest(m):	0
Consequence Category:	
Engineering Judgement:	
Section No:	2-2
Type of Toe Facility:	
Distance from Toe(m):	
Type of Crest Facility:	
Distance from Crest(m):	
Consequence Category:	
Engineering Judgement:	
Sign of Seepage:	
Criterion A satisfied:	
Sign of Distress:	
Criterion D satisfied:	
Non-routine maintenance required:	
Note:	
Masonry wall/Masonry facing:	
Note:	
Consequence category (for critical section):	
Observations:	N/A
Emergency Action Required:	
Action By:	N/A

# ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	
Action By:	N/A



# **OTHER EXTERNAL ACTION**

Check / repair Services:	
Action By:	N/A
Non-routine Maintenance:	
Action By:	N/A

# eLPMIS

LPM/LPMit Details Report	
LPM Study Feature No.:	3NW-D/C 40
Location:	NORTH EAST OF LOI TUNG VILLAGE, OFF SHA TAU KOK, WO HANG
District Council:	North
Maintenance Responsibility (At the Time of Selection):	Mixed
Responsible Party for Maintenance of Government Portion:	Lands D
Private Lot No.:	DD81 Lot24, DD81 Lot23, DD81 Lot10, DD81 Lot19
LPM/LPMit Study	
Agreement No.:	CE33/2004
Study Type:	Stage 2 Study
Consultant:	Maunsell Geotechnical Services Ltd.
GEO Managing Section / Engineer:	122 / 22
Study Status:	Study completed
Design Approach:	N/A
Option Assessment Accepted:	N/A
Study Report No.:	S2R161/2006
Programme / Actual Commencement:	07-07-2006
Programme / Actual Completion:	16-11-2007
Report Recommendation (For Stage 2 Study):	No action required
District Check Status:	Checked
Checking Certificate No.:	N/A
GEO Engineer's Remarks:	N/A
LPM/LPMit Works	
Works Contract No.:	N/A
GEO Managing Section / Engineer:	N/A / N/A
Contractor:	N/A
Progress Status:	N/A
Reason of Study Termination / Works Deletion (If Necessary):	N/A
Forecast Commencement Date:	N/A
Forecast Completion Date:	N/A



SLOPE INFORMATION SYSTEM GEOTECHNICAL ENGINEERING OFFICE CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

Completion Cert. Issued:	N/A
Site Handed Over to Maintenance Department on:	N/A
Estimated Cost for Upgrading (HK\$M):	N/A
Maintenance Manual No.:	N/A
Actual Works:	N/A
No. of Tree Felled:	N/A
No. of Tree Planted (Incl. Transplant):	N/A
% Bare of Slope Surfacing:	N/A
% Vegetated of Slope Surfacing:	N/A
% Shotcrete of Slope Surfacing:	N/A
Other Hard Surface of Slope Surfacing:	N/A



SLOPE INFORMATION SYSTEM GEOTECHNICAL ENGINEERING OFFICE CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT





SLOPE INFORMATION SYSTEM D GEOTECHNICAL ENGINEERING OFFICE CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT Feature No. 3NW-D/C 40 3NE-C/C 2 3NW-D/C 47 3NE-C/C 3NE-C/C 207 3NW-DIC 46 3NW-D/C 38 3NE-C 842 800N 42 80 3NE-C/C 9 3NE-C/CR 3NE-C/R TAU KOK ROAD - WO HANG 3NW-D/C-39 C 45 3NE-C/C 193 3NE-C/F> 23 3NW-D/C 43 3NW 40 3NW-D/C 42 3NW-D/C-37 842 6 3NW-D/C /41 3NW-D/C 50 3NW-D/6 51 3NW-D/C 329 3NW-D/C 52 3NW-D/C 57 3NW-D/CR 58 3NW-D/C 53 3NW-D/C 5/

#### RECORD RETRIEVED FROM SIS ON 07/04/2025 16:13



# **BASIC INFORMATION**

Location:	Open storage area, north east of Loi Tung Village, off Sha Tau Kok Road - Wo Hang, North
Registration Date:	24-10-1997
Ranking Score (NPRS):	O (Notional)
Date of Formation:	pre-1977
Date of Construction/ Modification:	
Data Source:	El(Lands D)
Approximate Coordinates:	Easting : 837457 Northing : 842627

### CONSEQUENCE-TO-LIFE CATEGORY

Facility at Crest:	Road/footpath with very low traffic density
Distance of Facility from Crest (m):	3
Facility at Toe:	Non-dangerous goods storage site
Distance of Facility from Toe (m):	3
Consequence-to-life Category:	3
Remarks:	N/A

# **SLOPE PART**

(1)	Max. Height (m): 15	Length (m): 150	Average Angle (deg): 35
-----	---------------------	-----------------	-------------------------

#### WALL PART

N/A

### MAINTENANCE RESPONSIBILITY

MR Endorsement Date: 06-05-2010 (1) Sub Div.: 1 **Mixed Feature** Party: Lands D Agent: Lands D Land Cat.: 5b(vi) Reason Code: 62 Reason Code: 1 MR Endorsement Date: 06-05-2010 (2) Sub Div.: 2 **Mixed Feature** Party: DD38 LOT135 Land Cat.: 1 Agent: N/A (3) Sub Div.: 3 **Mixed Feature** Party: LICENCE N8797 Agent: N/A Land Cat.: 1,5b(vi) Reason Code: 4 MR Endorsement Date: 06-05-2010 MR Endorsement Date: 06-05-2010 (4) Sub Div.: 3 **Mixed Feature** Party: Lands D Agent: Lands D Land Cat.: 1,5b(vi) Reason Code: 59

### **DETAILS OF SLOPE / RETAINING WALL**

Date of Inspection:	11-05-2001
Data Source:	EI(Lands D)
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

#### **SLOPE PART**



Slope Part (1) Surface Protection (%):	Bare: O Veae	tated: 100	Chunam: O	Shotcrete: O	Other Cover: O
Material Description:	Material type: So			51101111010.0	
Berm:	No. of Berms: N/		m Width (m): N	/A	
Weepholes:	Size (mm): N/A	Spacing (m)	: N/A		

#### WALL PART

N/A

#### **SERVICES**

N/A

# **CHECKING STATUS INFORMATION**

N/A

### **BACKGROUND INFORMATION**

GIU Cell Ref.:	3NW25B9	
Map Sheet Reference (1:1000):	3NW-25B	
Aerial Photos:	CN10450 (1995), CN10451 (1995)	
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(NO5)	
Data Collected On:	11-05-2001	
Date of Construction, Subsequent Modification and Demolition:	Modification: Constructed Before: 1974 Aft	er: 1 <b>964</b>
Related Reports/Files or Documents:	File/Report: PWDC Ref. No.: GC 4/1/2-3 f (19) File/Ref. No.: GC 4/1/2-3 F (	
Remarks:	N/A	
Follow Up Actions:	N/A	
DH-Order (To Be Confirmed with Buildings Department):	None	
Advisory Letter (To Be Confirmed with Buildings Department):	None	
LPMIS:	None	

# ENHANCED MAINTENANCE INFORMATION

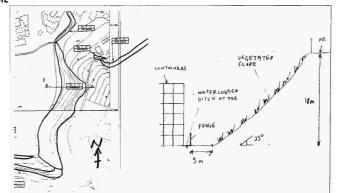


# **STAGE 1 STUDY REPORT**

Inspected On:	
Weather:	
District:	

30-05-1997 Some Rain





Section No:	1-1
Height(m):	H1 : 18 , H2 : 0
Type of Toe Facility:	Non-dangerous goods storage site
Distance from Toe(m):	3
Type of Crest Facility:	Road/footpath with very low traffic density
Distance from Crest(m):	3
Consequence Category:	3
Engineering Judgement:	Р
Section No:	2-2
Type of Toe Facility:	N/A
Distance from Toe(m):	0
Type of Crest Facility:	N/A
Distance from Crest(m):	0
Consequence Category:	3
Engineering Judgement:	Р
Sign of Seepage:	Slope : Signs of seepage Wall : N/A
Criterion A satisfied:	Ν
Sign of Distress:	Slope : Reasonable (at toe) Wall : N/A
Criterion D satisfied:	Ν
Non-routine maintenance required:	Ν
Note:	N/A
Masonry wall/Masonry facing:	Ν
Note:	N/A
Consequence category (for critical section):	3



Observations:	N/A
Emergency Action Required:	Ν
Action By:	N/A

## ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	Y
Action By:	Mixed

#### **OTHER EXTERNAL ACTION**

Check / repair Services:	Ν
Action By:	N/A
Non-routine Maintenance:	Ν
Action By:	N/A

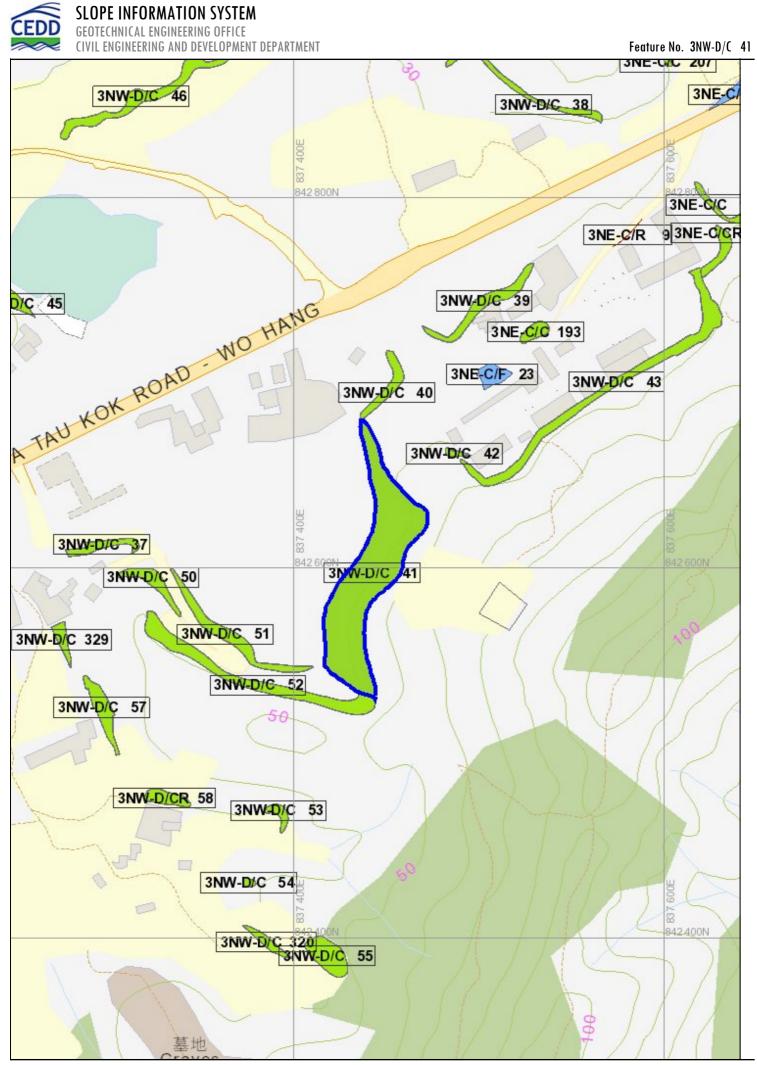












#### RECORD RETRIEVED FROM SIS ON 12/03/2025 20:55



## **BASIC INFORMATION**

Location:	East of Loi Tung Ea	ıst Village House #10D, Sha Tau Kok.
Registration Date:	24-10-1997	
Ranking Score (NPRS):	0 (Notional)	
Date of Formation:	pre-1977	
Date of Construction/ Modification:		
Data Source:	EI	
Approximate Coordinates:	Easting : 837324	Northing : 842594

## **CONSEQUENCE-TO-LIFE CATEGORY**

Facility at Crest:	District open space
Distance of Facility from Crest (m):	0
Facility at Toe:	Road/footpath with low traffic density
Distance of Facility from Toe (m):	0
Consequence-to-life Category:	3
Remarks:	N/A

## **SLOPE PART**

(1)	Max. Height (m): 5	Length (m): 45	Average Angle (deg): 50
-----	--------------------	----------------	-------------------------

#### WALL PART

N/A

#### **MAINTENANCE RESPONSIBILITY**

(1) Sub Div.: O	Government Feature	Party: Lands D	Agent: Lands D	Land Cat.: 5b(vi)	Reason Code: 62	MR Endorsement Date: 02-04-
1998						

## DETAILS OF SLOPE / RETAINING WALL

Date of Inspection:	11-05-2001
Data Source:	EI
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

## **SLOPE PART**

Slope Part (1) Surface Protection (%):	Bare: O Veget	ated: 100	Chunam: O	Shotcrete: O	Other Cover: O
Material Description:	Material type: Soil	Geology	: Other geolog	ly .	
Berm:	No. of Berms: N/A	Min. Ber	m Width (m): N	/A	
Weepholes:	Size (mm): N/A	Spacing (m)	:N/A		



#### WALL PART

N/A

## SERVICES

N/A

## CHECKING STATUS INFORMATION

N/A

## **BACKGROUND INFORMATION**

GIU Cell Ref.:	3NW25D3
Map Sheet Reference (1:1000):	3NW-25D
Aerial Photos:	CN10451 (1995), CN10452 (1995)
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(N05)
Data Collected On:	11-05-2001
Date of Construction, Subsequent Modification and Demolition:	Modification: Constructed Before: 1963 After: N/A Modification: Modified Before: 1978 After: 1974
Related Reports/Files or Documents:	File/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VIFile/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VI
Remarks:	N/A
Follow Up Actions:	N/A
DH-Order (To Be Confirmed with Buildings Department):	None
Advisory Letter (To Be Confirmed with Buildings Department):	None
LPMIS:	None

#### ENHANCED MAINTENANCE INFORMATION

From Maintenance Department: (Last Updated Date: 19/02/2025)

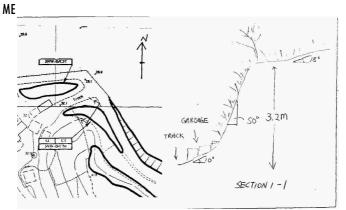


## **STAGE 1 STUDY REPORT**

Inspected	On:
Weather:	
District:	

05-06-1997

## Some Rain



Section No:	1-1
Height(m):	H1 : 3 , H2 : 0
Type of Toe Facility:	Road/footpath with low traffic d
Distance from Toe(m):	0
Type of Crest Facility:	District open space
Distance from Crest(m):	0
Consequence Category:	3
Engineering Judgement:	U
Section No:	2-2
Type of Toe Facility:	N/A
Distance from Toe(m):	0
Type of Crest Facility:	N/A
Distance from Crest(m):	0
Consequence Category:	3
Engineering Judgement:	U
Sign of Seepage:	Slope : No signs of seepage Wall : N/A
Criterion A satisfied:	Ν
Sign of Distress:	Slope : Reasonable (near crest) Wall : N/A
Criterion D satisfied:	Ν
Non-routine maintenance required:	Ν
Note:	N/A
Masonry wall/Masonry facing:	Ν
Note:	N/A
Consequence category (for critical section):	3
Observations:	N/A
Emergency Action Required:	N
Action By:	N/A

H1 : 3 , H2 : 0
Road/footpath with low traffic density
0
District open space



## ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	Ν
Action By:	N/A

## **OTHER EXTERNAL ACTION**

Check / repair Services:	Ν
Action By:	N/A
Non-routine Maintenance:	Ν
Action By:	N/A



## PHOTO





SLOPE INFORMATION SYSTEM FD **GEOTECHNICAL ENGINEERING OFFICE** CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT Feature No. 3NW-D/C 50 3NW-D/C 46 842 800N 842 800N 3NW-D/C 45 J-D/Ć 44 SHA TAU KOK ROAD - WO HANG 3NW-D/C 3 3NW-DSNW-D/C 305 3NE-C/F 3NW-D/R 16 3NW-D/C 40 V-D/C 333NW-D/CR 330 3NW-DICR 34 3NW-D/6 42 3NW-D/C D/C 30 400E 3NW-D/C 37 842 600Ň 3NW-D/C 41 31. WD 50 3NW-D/6 51 3NW-D/C 329 3NW-D/C 52 3NW-D/C 57 3NW-D/CR 58 3NW-D/C 53 3NW-D/C 54 342 400N 3NW-D/C 320 3NW-D/C 55 50 墓地 Graves

#### RECORD RETRIEVED FROM SIS ON 12/03/2025 20:59



## **BASIC INFORMATION**

Location:	East of the Loi Tun	g East Village, Sha Tau Kok
Registration Date:	24-10-1997	
Ranking Score (NPRS):	0 (Notional)	
Date of Formation:	pre-1977	
Date of Construction/ Modification:		
Data Source:	EI(Lands D)	
Approximate Coordinates:	Easting : 837363	Northing : 842564

## CONSEQUENCE-TO-LIFE CATEGORY

Facility at Crest:	Road/footpath with very low traffic density
Distance of Facility from Crest (m):	0
Facility at Toe:	Non-dangerous goods storage site
Distance of Facility from Toe (m):	0
Consequence-to-life Category:	3
Remarks:	N/A

## **SLOPE PART**

(1)	Max. Height (m): 5	Length (m): 105	Average Angle (deg): 50
-----	--------------------	-----------------	-------------------------

#### WALL PART

N/A

#### **MAINTENANCE RESPONSIBILITY**

(1) Sub Div.: 1	Mixed Feature	Party: Lands D A	gent: Lands D	Land Cat.: 5b(vi)	Reason Code: 62	MR Endorsement Date: 03-11-1998
(2) Sub Div.: 2	Mixed Feature	Party: DD38 Lot127	Agent: N/A	Land Cat.: 1	Reason Code: 1 🛛 🛚 🗛	AR Endorsement Date: 03-11-1998

## DETAILS OF SLOPE / RETAINING WALL

Date of Inspection:	11-05-2001
Data Source:	EI(Lands D)
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

## **SLOPE PART**

Slope Part (1) Surface Protection (%):	Bare: O Veaet	ated: 100	Chunam: O	Shotcrete: O	Other Cover: 0
Material Description:	Material type: Soil		: Other geolog		
Berm:	No. of Berms: N/A	Min. Ber	m Width (m): N	/A	
Weepholes:	Size (mm): N/A	Spacing (m)	: N/A		



#### WALL PART

N/A

## SERVICES

N/A

## CHECKING STATUS INFORMATION

N/A

## **BACKGROUND INFORMATION**

GIU Cell Ref.:	3NW25D3
Map Sheet Reference (1:1000):	3NW-25D
Aerial Photos:	CN10451 (1995), CN10452 (1995)
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(N05)
Data Collected On:	11-05-2001
Date of Construction, Subsequent Modification and Demolition:	Modification: Constructed Before: 1963 After: N/A Modification: Modified Before: 1978 After: 1974
Related Reports/Files or Documents:	File/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VIFile/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VI
Remarks:	N/A
Follow Up Actions:	N/A
DH-Order (To Be Confirmed with Buildings Department):	None
Advisory Letter (To Be Confirmed with Buildings Department):	None
LPMIS:	None

#### ENHANCED MAINTENANCE INFORMATION

From Maintenance Department: (Last Updated Date: 19/02/2025)



05-06-1997

80% SLOPE SURFACE GLASS

50

4.9m

SECTION 1-1

TRACK

## **STAGE 1 STUDY REPORT**

Inspected On:

Weather:	Some Rain
District:	ME
	80%
	SURFAC
	FENCE
	OPEN OPEN
	STO RAGE AREA
Section No:	]-]
Height(m):	H1 : 5 , H2 : 0
Type of Toe Facility:	Non-dangerous goods storage site
Distance from Toe(m):	0 D = = 1/6 = += = +1 = = :++ = 1 = = += + = = 66: = + = = :++ =
Type of Crest Facility:	Road/footpath with very low traffic density O
Distance from Crest(m):	2
Consequence Category: Engineering Judgement:	P
Section No:	2-2
Type of Toe Facility:	z-z N/A
Distance from Toe(m):	0
Type of Crest Facility:	N/A
Distance from Crest(m):	0
Consequence Category:	2
Engineering Judgement:	P
Sign of Seepage:	Slope : No signs of seepage
	Wall : N/A
Criterion A satisfied:	Ν
Sign of Distress:	Slope : N/A
	Wall : N/A
Criterion D satisfied:	N
Non-routine maintenance required:	N
Note:	N/A
Masonry wall/Masonry facing:	N
Note:	N/A
Consequence category (for critical section):	2
Observations:	N/A
Emergency Action Required:	Ν
Action By:	N/A



## ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	Y
Action By:	Mixed

## **OTHER EXTERNAL ACTION**

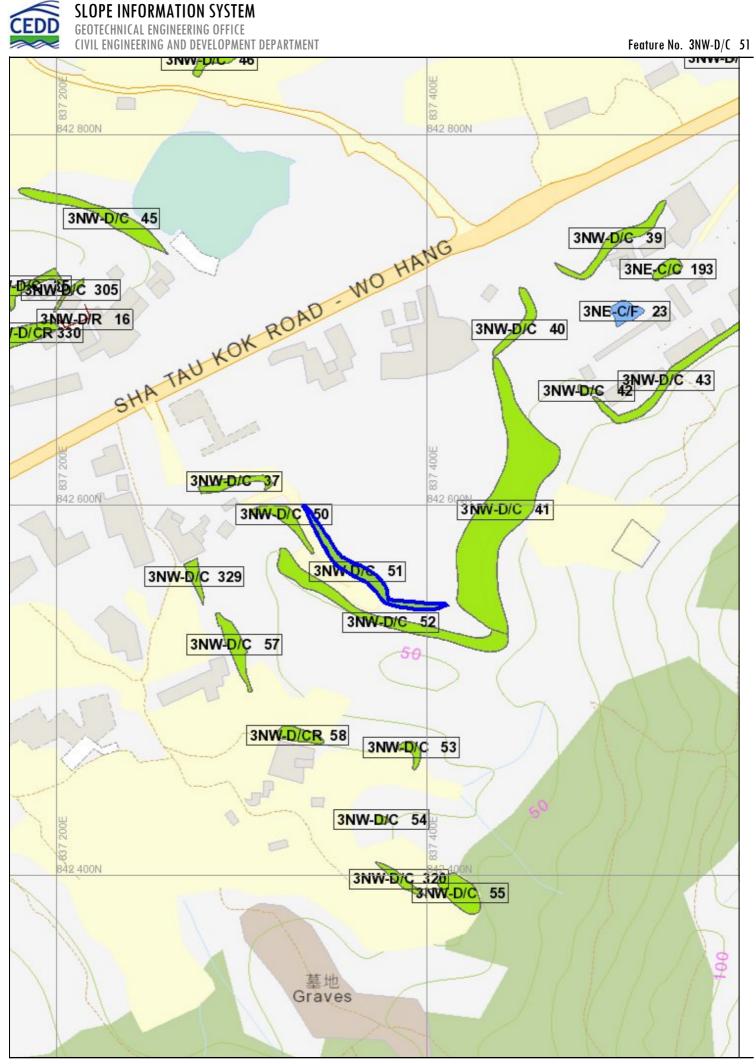
Check / repair Services:	N
Action By:	N/A
Non-routine Maintenance:	Ν
Action By:	N/A



## PHOTO







#### RECORD RETRIEVED FROM SIS ON 12/03/2025 20:58



## **BASIC INFORMATION**

Location:	East of the Loi Tun	g East Village, Sha Tau Kok.
Registration Date:	24-10-1997	
Ranking Score (NPRS):	0 (Notional)	
Date of Formation:	pre-1977	
Date of Construction/ Modification:		
Data Source:	El	
Approximate Coordinates:	Easting : 837375	Northing : 842538

#### **CONSEQUENCE-TO-LIFE CATEGORY**

Facility at Crest:	Remote area or abandoned facilities
Distance of Facility from Crest (m):	0
Facility at Toe:	Road/footpath with very low traffic density
Distance of Facility from Toe (m):	0
Consequence-to-life Category:	3
Remarks:	N/A

## **SLOPE PART**

(1)	Max. Height (m): 4.2	Length (m): 130	Average Angle (deg): 45
-----	----------------------	-----------------	-------------------------

#### WALL PART

N/A

#### **MAINTENANCE RESPONSIBILITY**

(1) Sub Div.: O	Government Feature	Party: Lands D	Agent: Lands D	Land Cat.: 5b(vi)	Reason Code: 62	MR Endorsement Date: 02-04-
1998						

## DETAILS OF SLOPE / RETAINING WALL

Date of Inspection:	11-05-2001
Data Source:	EI
Slope Part Drainage:	N/A
Wall Part Drainage:	N/A

## **SLOPE PART**

Slope Part (1) Surface Protection (%):	Bare: O Veget	ated: 100	Chunam: O	Shotcrete: O	Other Cover: O
Material Description:	Material type: Soil	Geology	: Other geolog	у	
Berm:	No. of Berms: N/A	Min. Beri	n Width (m): N	/A	
Weepholes:	Size (mm): N/A	Spacing (m):	N/A		



#### WALL PART

N/A

## SERVICES

N/A

## CHECKING STATUS INFORMATION

N/A

## **BACKGROUND INFORMATION**

GIU Cell Ref.:	3NW25D3
Map Sheet Reference (1:1000):	3NW-25D
Aerial Photos:	28359 (1979), 28360 (1979)
Nearest Rainguage Station (Station Number):	Cheung Chi House, Cheung Wah Estate(N05)
Data Collected On:	11-05-2001
Date of Construction, Subsequent Modification and Demolition:	Modification: Constructed Before: 1963 After: N/A
Related Reports/Files or Documents:	File/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VIFile/Report: PWDCRef. No.: GC 4/1/2-3 f 19 pt VI
Remarks:	N/A
Follow Up Actions:	N/A
DH-Order (To Be Confirmed with Buildings Department):	None
Advisory Letter (To Be Confirmed with Buildings Department):	None
LPMIS:	None

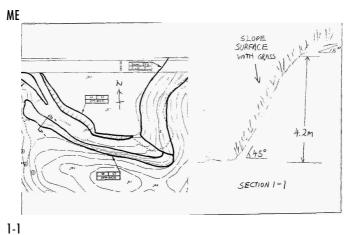
#### ENHANCED MAINTENANCE INFORMATION

From Maintenance Department: (Last Updated Date: 19/02/2025)



## **STAGE 1 STUDY REPORT**

Inspected	On:
Weather:	
District:	



Section No:	1-1
Height(m):	H1 : 4 , H2 : 0
Type of Toe Facility:	Road/footpath with very low
Distance from Toe(m):	0
Type of Crest Facility:	Remote area or abandoned f
Distance from Crest(m):	0
Consequence Category:	3
Engineering Judgement:	U
Section No:	2-2
Type of Toe Facility:	N/A
Distance from Toe(m):	0
Type of Crest Facility:	N/A
Distance from Crest(m):	0
Consequence Category:	3
Engineering Judgement:	U
Sign of Seepage:	Slope : No signs of seepage Wall : N/A
Criterion A satisfied:	Ν
Sign of Distress:	Slope : N/A Wall : N/A
Criterion D satisfied:	Ν
Non-routine maintenance required:	Ν
Note:	N/A
Masonry wall/Masonry facing:	Ν
Note:	N/A
Consequence category (for critical section):	3
Observations:	N/A
Emergency Action Required:	Ν
Action By:	N/A

1
11 : 4 , H2 : 0
coad/footpath with very low traffic density
)
lemote area or abandoned facilities
)
1
I
2-2
I/A
)
1/A



## ACTION TO INITIATE PREVENTIVE WORKS

Criterion A/Criterion D:	N/A
Action By:	N/A
Further Study:	Ν
Action By:	N/A

## **OTHER EXTERNAL ACTION**

Check / repair Services:	Ν
Action By:	N/A
Non-routine Maintenance:	Ν
Action By:	N/A



## PHOTO

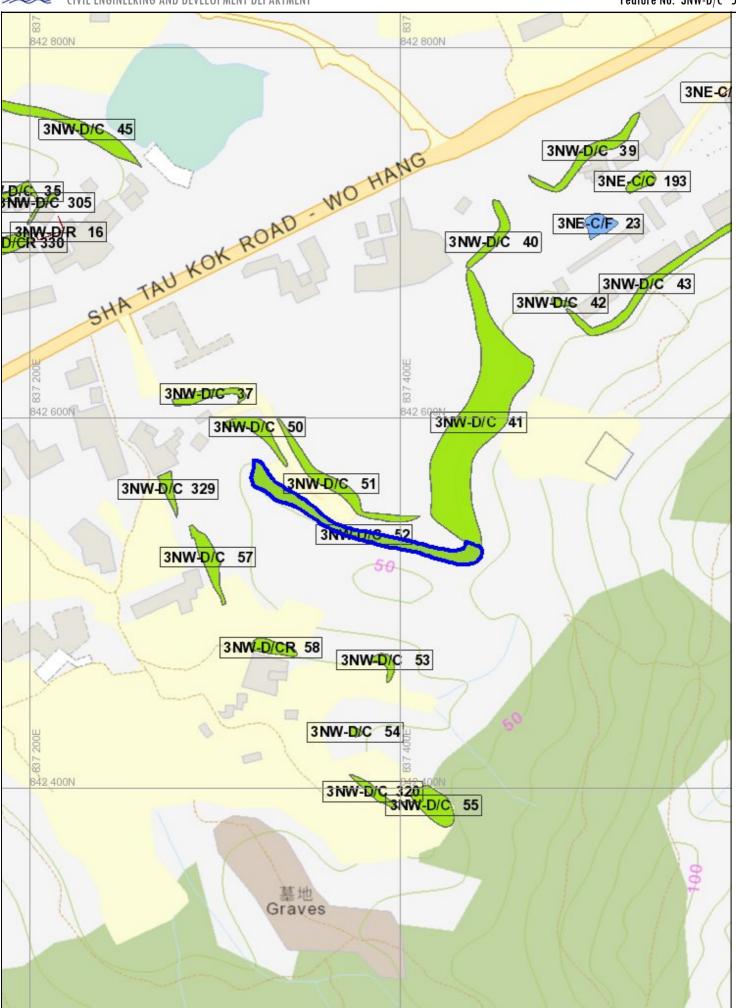




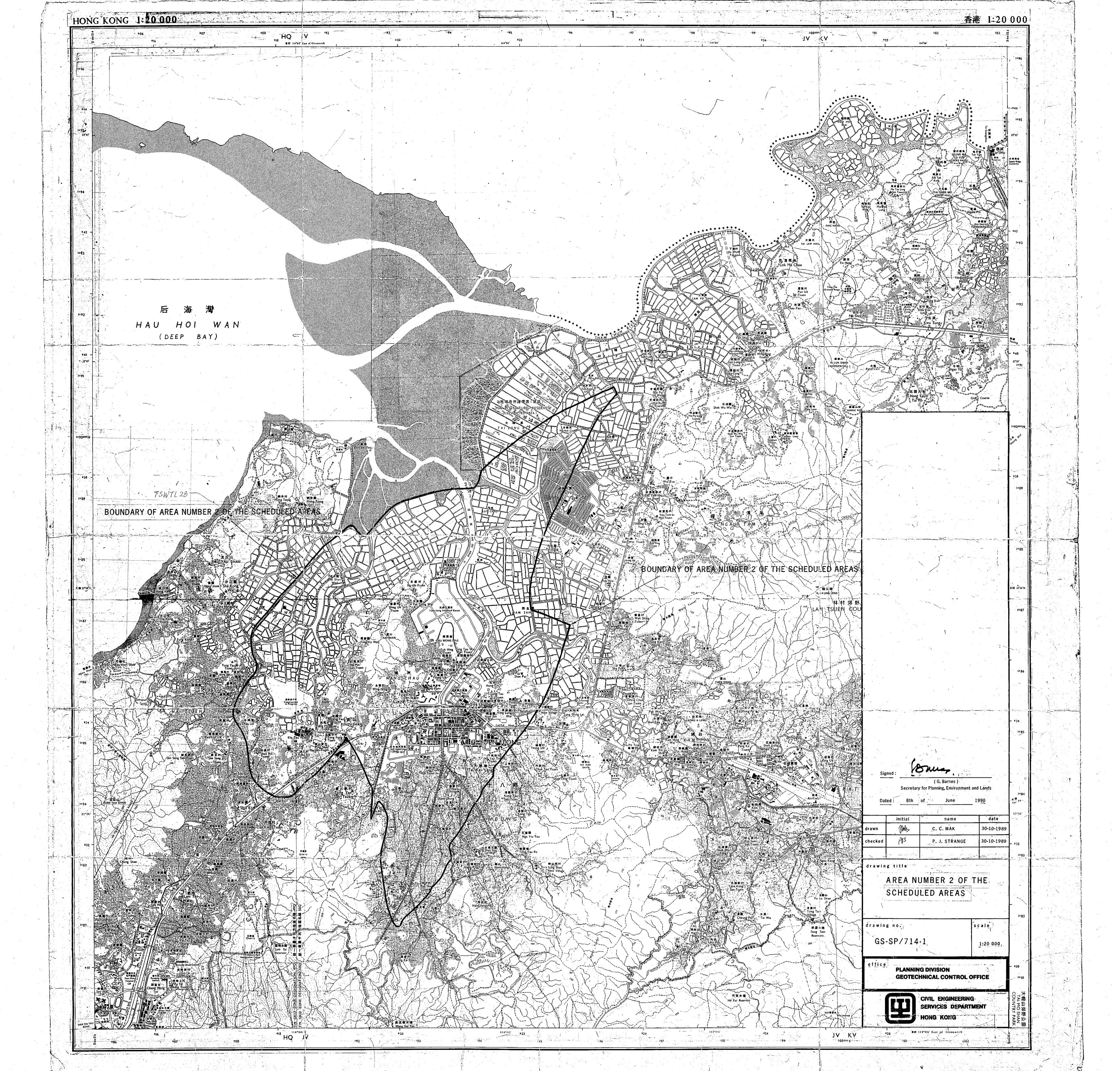
SLOPE INFORMATION SYSTEM GEOTECHNICAL ENGINEERING OFFICE

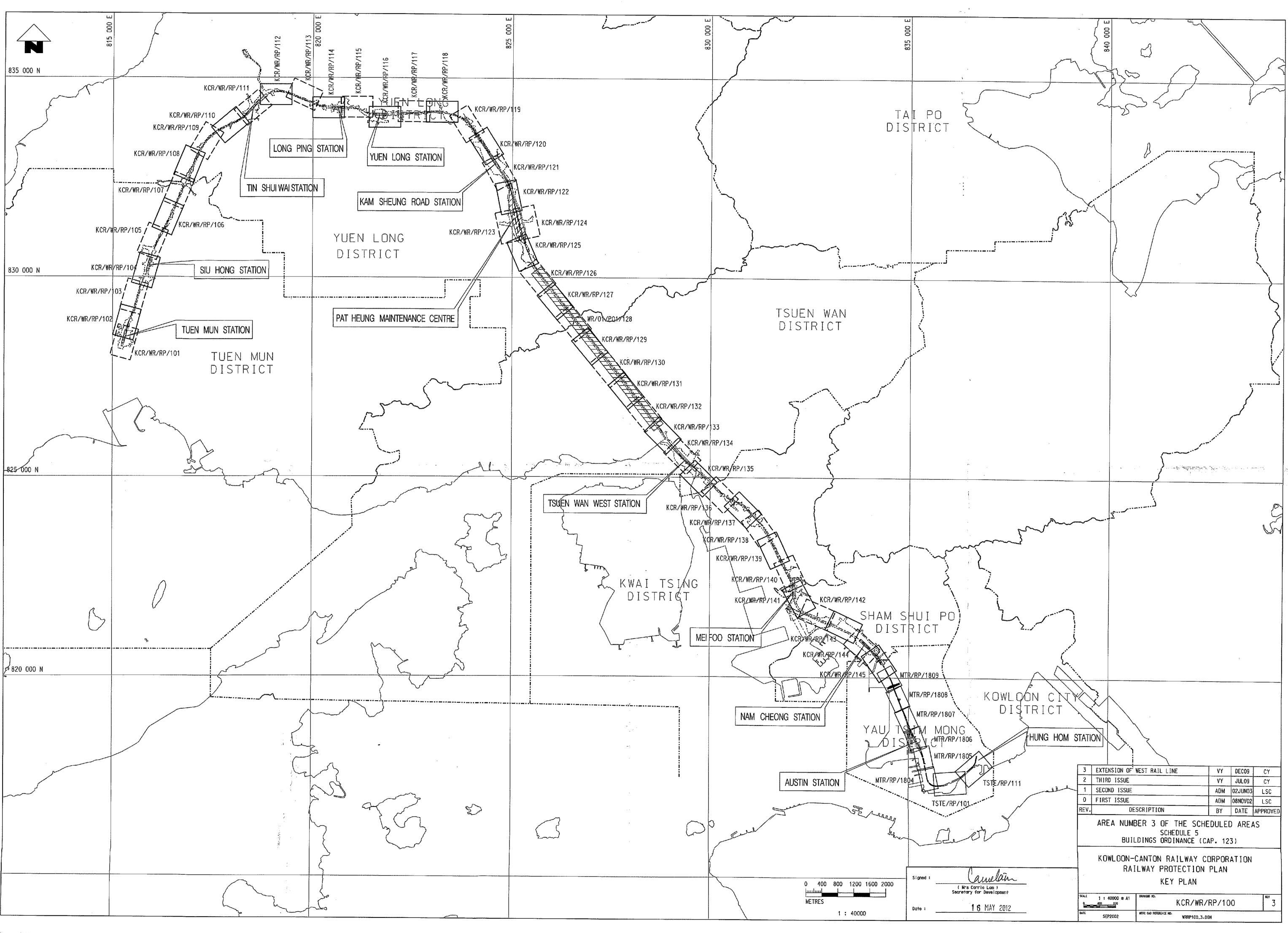
D

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

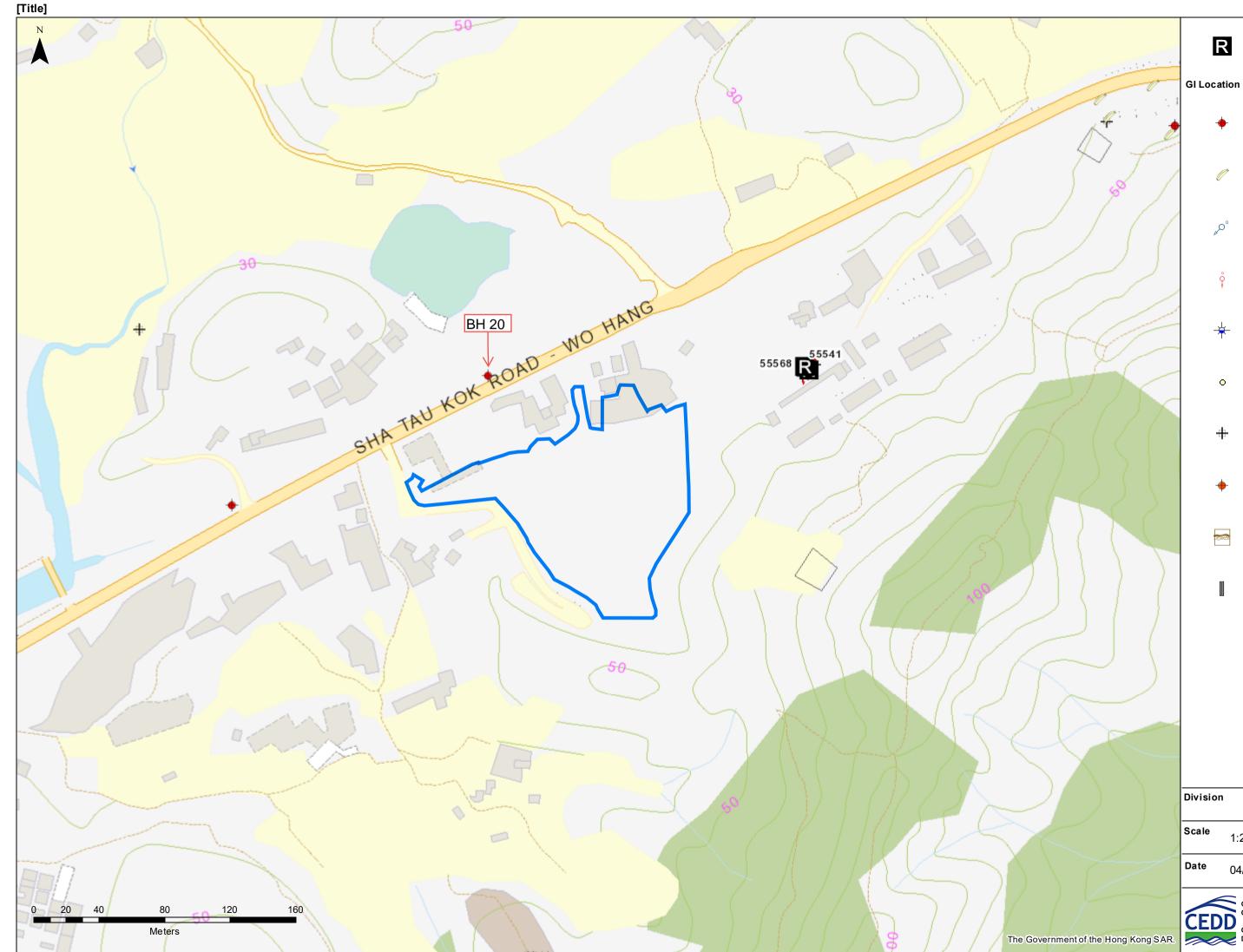


# Appendix C – Schedule Area Plan





# Appendix D – Existing GI Record



#### GIU Report

+	<all other<br="">values&gt;</all>		
Ø	Slope striping		
"P°	Cone Penetration Test		
Ŷ	GCO Probe		
+	Grab Samples		
0	Impression Packer Test		
÷	Trial pit		
+	PR		
	Rock joint survey		
	Trial trench		
on			
1:2,000			
04/04/2025			

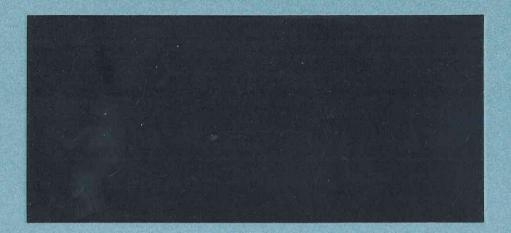
GEOTECHNICAL ENGINEERING

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

## 41501



DRILTECH DrilTech Ground Engineering Ltd. 鑽 達 地 質 工 程 有 限 公 司



















6 a	
	v # <b>n</b>

#### CONTRACT NO. GE/2003/19 GROUND INVESTIGATION – NEW TERRITORIES EAST (TERM CONTRACT)

WORKS ORDER NO. GE/2003/19.29 Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT – Package C Investigation, Design and Construction (Man Uk Ping)

#### **Ground Investigation**

#### FINAL FIELD WORK REPORT

Checked in accordance with Contract No. GE/<u>) 203 (1)</u> requirements and accepted.

GIVIL E	IGINEERING AND DEVELOPMENT DEPARTMENT
	GEOTECHNICAL
II	<b>NFORMATION UNIT</b>
Report	No. 41501
AREA	
Ref.	

Coll, Indicator
Open Section
Govt Section
Others

With ( ) 31/3" diskeltes(s) Certified as Complete by K C/Sung Contractor's Representative

Certified as Checked

Clement Lun

Geotechnical Engineer

<u>CONTRACTOR</u> DRiLTECH Ground Engineering Ltd. Blk A & B, 9/F., Hong Kong Spinners Industrial Bldg. Phase VI, 481 – 483 Castle Peak Road, Kowloon.

**CLIENT** 

Geotechnical Engineering Office, Civil Engineering and Development Department, 25/F, No. 410, Kwun Tong Road, Kowloon

24th March 2005



DRILTECH

CONTRACT DATA SUMMARY											
Project Name	& No.		Site Nam		CI DAIA SUN			Date:	07-Jan-05	to	29-Mar-05
Ground Invest			Works Order No. GE/2003/19.29				L	Official			27 114. 00
11	0									7	
11	New Territories EastAgreement No. CE 6/2002 (DS)G.E.O. Data Bank No.(Term Contract)Drainage Improvement in Northern NT - Package C										
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			(Man UK F	ring) Groi	and Investigation				· · · · ·		
G.I. Contrac			Client		hnical Engineering						
DrilTech Groi	und Engineeri	ng Ltd.			and Development I	Departme					
Contract No.			Consultin					File Ref.			
GE/2003/19			Black & I	Veatch H	ong Kong Ltd						
			F	TELD	WORK SUMM	IARY					
Drillholes:	No.	11	Method:		Rotary			Date:	10-Jan-05	to	24-Mar-05
Trial Pits:	No.	3	Trial Tre	enches:	No.	λ	Vil			-	,
Coreholes:	No.	Nil	Stripping	25:	No.	λ	Vil	Probes:		No.	Nil
<b>Piezometers:</b>	No.	7	Standpip		No.		2	Piezome	ter Buckets:	No.	18
Insitu Tests:	No.	75	Types:		Response Test (9),	Constant					
	1100	, 0	~ ) P	2	Impression Packer						
Geophysics:	No.	Nil	Types:	· · · · · ·	Nil	10010 (2)	, i anne	, 11044 1			
Geophysics.	110.	1111		DATO	RY TESTING S	TIMM	ADV		<u> </u>		
	<u> </u>		LADU	KAIUI	AT LESTING &						
No. of each ty			1				l	Date:		to	
	Physical Prop	perties	LL		PL	PSD			MC		
			SG								
SOIL	Strength Test		Cum	NC	T APPLICABLE	4			Shear Box		
	Compaction a	& CBR Tests	Stand				or		CBR		
	Oedometer &	Perm. Tests	Cv		k						
	Others		Split Maz	zier							
ROCK			Pt Load		UC	Shear	Box		US Vel.		
LOCATION	PLAN S	cale 1:	71,400		Derived from :	2001	Hong Ko	ong Guid	ebook		
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		200	素物
	G.I.	Laboratory	Geotechnical Engineering Office
Contractor	DrilTech Ground Engineering		
	Ltd.		Civil Engineering and
Works Order No.	GE/2003/19.29		Development Department



#### CONTRACT NO. GE/2003/19 GROUND INVESTIGATION – NEW TERRITORIES EAST (TERM CONTRACT)

#### WORKS ORDER NO. GE/2003/19.29 Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT – Package C Investigation, Design and Construction (Man Uk Ping)

#### **Ground Investigation**

#### FINAL FIELD WORK REPORT

#### CONTENTS

#### Contract Data Summary

- 1. Introduction
- 2. Site Location
- 3. Geology
- 4. Field Works
  - 4.1 Drillholes
  - 4.2 Field Tests
    - 4.2.1 Standard Penetration Tests
    - 4.2.2 Constant Head Permeability Tests
    - 4.2.3 Falling Head Permeability Test
    - 4.2.4 Impression Packer Surveys
    - 4.2.5 Dynamic Probing (GCO Probe) Tests
  - 4.3 Inspection Pits and Trial Pits
  - 4.4 Field Installations
    - 4.4.1 Piezometers
    - 4.4.2 Standpipes
    - 4.4.3 Piezometer (Halcrow) Buckets
- 5. Soil and Rock Descriptions
- 6. Surveying
- 7. Digital Data Record
- 8. References
- Table 1 Survey Records
- Table 2 Summary of Drillhole Records
- Table 3 Summary of Field Testing and Field Installations
- Appendix A Checklists for Soil & Rock Descriptions
- Appendix B Legends for Use on Drillhole and Trial Pit Records
- Appendix C Drillhole and Trial Pit Records
- Appendix D Photographs of Drillholes and Trial Pits
- Appendix E Constant Head Permeability Tests
- Appendix F Falling Head Permeability Test
- Appendix G Impression Packer Surveys
- Appendix H Dynamic Probing (GCO Probe) Test Records
- Appendix I Drillhole Piezometer/Standpipe Detail and Response Test Record Sheets
- Appendix J Water Level Monitoring Records
- Appendix K Piezometer Buckets Records
- Appendix L Drawing (No. D269/1929/D001)
- Appendix M Digital Data Record



#### CONTRACT NO. GE/2003/19 GROUND INVESTIGATION – NEW TERRITORIES EAST (TERM CONTRACT)

#### WORKS ORDER NO. GE/2003/19.29 Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT – Package C Investigation, Design and Construction (Man Uk Ping)

#### Ground Investigation

#### FINAL FIELD WORK REPORT

#### 1. Introduction

DRiLTECH Ground Engineering Ltd. was awarded a 2-year Term Contract by the Geotechnical Engineering Office, Civil Engineering and Development Department of the Government of Hong Kong Special Administrative Region in November 2003 to carry out ground investigation works in the Eastern New Territories.

This report presents the results of ground investigation for the Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT – Package C Investigation, Design and Construction (Man Uk Ping), under the Works Order No.GE/2003/19.29. The field work was carried out in a period between 10<sup>th</sup> January 2005 and 24<sup>th</sup> March 2005 under the supervision of Black & Veatch Hong Kong Ltd.

#### 2. <u>Site Location</u>

The site is located along Sha Tau Kok Road–Wo Hang, Man Uk Ping, N.T. bounded within the following co-ordinates.

836 493E	· 842 970N
837 314E	842 970N
836 493E	842 144N
837 314E	842 144N

The locations of investigation stations are shown in the Drawing No. D269/1929/D001 in Appendix L.

#### 3. Geology

According to the 1:20,000 scale, Sheet 3 of HGM20 Series Solid and Superficial Geology Map published by the Geotechnical Control Office (1991), the site is underlain by Terraced Alluvium, Debris Flow Deposit and bedrock of Undivided coarse ash Tuff and Undivided fine ash to coarse ash Tuffs, Tuff Breccia and Tuffite of Repulse Bay Volcanic Group, Upper Jurassic-Lower Cretaceous, Mesozoic.

Works Order No. GE/2003/19.29 Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT Package C Investigation, Design and Construction (Man Uk Ping)



#### 3. <u>Geology (Cont'd)</u>

The results of investigation reveal that the site is composed of Fill, Alluvium and Saprolite. The thickness of the Fill stratum is ranged from 1.00m to 2.10m (except ST13 and ST14), whilst the Alluvium stratum is ranged 1.00m to 2.90m (except B25, ST13 and ST14). A thin layer of Colluvium was only found in ST13 with thickness of 0.95m.

Bedrock was encountered in drillholes B20, B21, B24, B25 and B28 with rockhead level ranged from +16.02mPD to -7.58mPD. The rockhead level for other drillholes can not be defined as drilling was terminated in the Saprolite stratum before reaching the rock head.

The drillhole results are further summarized in Table 2.

#### 4. Field Works

Field works included sampling, field testing and field installation in eleven (11) drillholes (B20 to B28, ST13 and ST14) and three (3) trial pits (TP12 to TP14) were carried out at locations as shown in the Drawing No. D269/1929/D001 in Appendix L as specified in the Works Order. Piezometers and standpipes with piezometer (Halcrow) buckets were installed in designated drillholes to specified depths.

#### 4.1 <u>Drillholes</u>

The field works at the drillholes were carried out using a hydraulic rotary drilling rig with water as flushing medium. SW, PW and HW casings equipped with tungsten carbide cutting shoes were used to advance the holes. The drillholes were terminated at specified depths.

Undisturbed Mazier samples were generally taken at 2.00m intervals using a standard Mazier triple tube retractable core barrel which was fitted with a detachable 74mm I.D., 1000mm long clear ABS plastic liner, except ST13 and ST14 in which continuous U100 samples were taken. A retractable cutting shoe projecting from the tungsten core bit was used to penetrate the materials being sampled and to protect the sample from being disturbed by the drilling fluid.

The recovered samples were sealed with wax and protected with rubber cap at both ends. Small-disturbed samples were taken from the cutting shoes and were kept in airtight jars as jar samples.

Rock core samples were taken using T2-101 core barrels.

The disturbed and undisturbed samples and rock core samples are reported at relevant depths in the Drillhole Record sheets in Appendix C. Record photographs of the jar samples and core samples are included in Appendix D.

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#### 4.2 Field Tests

#### 4.2.1 Standard Penetration Tests

Standard Penetration Tests with liner samples were taken at specified depths in the drillholes B20 to B28. The tests were conducted according to BS1377 (1990 Part 9 Method 3.3) with modifications suggested in the Geoguide 2 and the Contract Specification.

The numbers of blows to drive a standard split-spoon sampler for the first 150mm penetration (seating drive) in 75mm increments and those for each 75mm penetration for the subsequent 300mm penetration were recorded. The "N" value was taken as the sum of the numbers of blows for the last 300mm penetration. Where the full penetration for seating drive was not achieved after 50 blows, the number of blows and the penetration achieved was recorded and the test continued with test drive at that point. The test was generally terminated where the total number of blows in test drive reached 100 regardless whether the full penetration of 300mm was achieved. In this case, the numbers of blows and the penetration achieved were recorded. During the test, the water level in the drillhole was maintained at or above the observed ground water level. Disturbed samples were retrieved from the cutting shoes as jar samples.

Liner samples were taken with the SPTs by including a line sample tube in the split barrel sampler in each test.

The depths of tests and the "N" values are presented in the Drillhole Record sheets in Appendix C.

#### 4.2.2 Constant Head Permeability Tests

Two (2) Constant Head Permeability Tests were carried in drillholes B23 and B24 at specified depths.

The test section was formed by surrounding a 40mm ID G.I. standpipe, which was perforated over the test section, with filter materials and sealed with bentonite pellets according to the figure 27 of Geoguide 2.

The water table in the standpipe after installation was allowed to equalize with the ambient groundwater level before commencement of the test. This water level was measured and recorded. Fresh water was then fed at a constant rate into the standpipe to raise the water level in the standpipe to about 0.3m below the top. This water level was maintained by adjusting the rate of inflowing water. The inflow rates to maintain the constant water level were recorded at the intervals as specified in the Specification. The test was terminated when the inflow rates differed less than 10% in an interval of 10 minates.

The test data and the test results are presented in Appendix E

#### 4.2.3 Falling Head Permeability Test

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A Falling Head Permeability Test was carried out in drillhole B21 between 7.70m and 8.70m below the existing ground level.

The test section was formed by surrounding a 40mm ID G.I. standpipe, which was perforated over the test section, with filter materials and sealed with bentonite pellets (Figure 27, Geoguide 2).

Before commencement of test, the ambient ground water level was allowed to equalize. For this test, the ground water level was below the test section and the water level in the standpipe was allowed to drop to the bottom of the pipe. The water level in the standpipe was then raised to the top by feeding clean water into the standpipe (the initial water head,  $h_o$ ). The water level was then allowed to drop and the distance dropped ( $h_t$ ) was measured and recorded at specified time intervals until the water level dropped to the bottom of the standpipe. The test was repeated once to ensure consistent results. The permeability of soil being test was estimated in accordance with the information given in the Figure 28 to 30 of Geoguide 2. The data and the results of the test are presented in Appendix F.

#### 4.2.4 Impression Packer Surveys

Impression packer surveys were carried out in the drillhole B24 in designated sections.

The surveys were carried out using an expandable impression packer attached with thermoplastic films of 1.5m lengths. A down hole compass was installed in an instrument house at the lower end. The packer was lowered to the specified survey section and was inflated by compressed air such that the thermoplastic films were pressed onto the drilled hole wall and traced the discontinuities of rock in the section under survey. The orientation of the thermoplastic films was recorded by the down hole compass installed at the low end of the packer; the orientation of the compass was fixed by a chemical compound which was activated when the packer was inflated by the compressed air. The direction of North was transferred to the thermoplastic films as a reference line for determination of orientations of the discontinuities recorded on the films. The traced discontinuities on the thermoplastic films were then matched with the discontinuities on the rock cores and the reference line of North was transferred to the core samples. Based on the reference line established, the orientations of discontinuities on the rock cores were measured.

The results of measurement are presented in the Discontinuity Log in Appendix G.

#### 4.2.5 Dynamic Probing (GCO Probe) Tests

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Dynamic Probing Tests were carried out at specified locations and on the trial pit before excavation and after backfilling compaction using a GCO Probe. The features of the Probe are given in the Figure 36 of Geoguide 2.

A rod with diameter of approximately 12mm was driven into the ground by a hammer of 10kg in weight dropping freely from a height of 300mm. The rod was equipped at its lower end with a driving point, which was 25mm in diameter with a cylindrical portion of about 25mm long and a conical tip with an apex angle of approximately 45°.

The numbers of blows required for the hammer to drive the rod into the ground for each 100mm were recorded. The tests were carried out to specified depths or to the refusal where penetration for 100 blows was less than 100mm. Where refusal was encountered above the anticipated depth, one replacement probing test was attempted at an adjacent location.

The results of the probing tests are presented in Appendix H. The results of post compaction backfilling probing were submitted for approval of backfilling compaction and are not included again in this report.

#### 4.3 Inspection Pits and Trial Pits

Trial pits and inspection pits were excavated manually using hand tools.

An inspection pit was excavated at each of the drillhole locations prior to drilling commencement to ensure that no underground utility would be damaged by the investigation works.

The trial pits were excavated to expose the underground materials for inspection and sampling. Undisturbed U100 sample and large disturbed samples were taken at specified locations and depths using hand tools. Small disturbed samples were taken at 0.50m intervals starting from the ground level.

The trial pits were inspected and logged and the results are reported in the Trial Pit Records in Appendix C. Record photographs taken on each side of the trial pits are presented in Appendix D.

The trial pits were subsequently backfilled with excavated materials and were compacted using portable tools.



#### 4.4 Field Installations

#### 4.4.1 <u>Piezometers</u>

Piezometers of Casagrande type were installed with 25mm I.D. PVC riser pipes in all drillholes, except B23, B25, ST13 and ST14 at specified depths. The piezometer tips were surrounded by clean sand of grading between 1,200 and 210 microns and were sealed with bentonite pellets to form response zones of specified lengths.

#### 4.4.2 <u>Standpipes</u>

Standpipes were installed in drillholes B23 and B25 to specified depths. The standpipes comprised 25mm I.D. PVC riser pipes surrounded by clean inert aggregate filter of size between 10mm and 16mm. The standpipe tube was capped at the base and was perforated about 5% of the surface area from 0.5m below the top to the bottom of the tube and was protected by nylon mesh.

Response tests were carried out on the piezometers and standpipes after completion of installation. The details of installations and the response test results are included in Appendix I.

Readings of water levels in the piezometers and standpipes were taken daily for 7 days following the completion of response tests. The results are presented in Appendix J.

The details of installation are summarized in Table 3.

#### 4.4.3 <u>Piezometer (Halcrow) Buckets</u>

Halcrow type piezometer buckets were installed in the selected piezometers and standpipe at specified depths. The bucket strings were fabricated in accordance with the Figure 23 of Geoguide 2.

A summary of installations is presented in Table 3.

#### 5. <u>Soil and Rock Descriptions</u>

The soils and rocks encountered in the investigation have generally been described according to the Geoguide 3, Guide to Rock and Soil Descriptions, except for the following terms which are used for the secondary constituents other than clay, silt and sand, in composition of common ground:

"with occasional" for less than 5%, and "with some" for between 5% and 20%; and "with much or many" for greater than 20%

The classification and definitions of the descriptive terms are presented in Appendix A.



### 5. <u>Soil and Rock Descriptions (Cont'd)</u>

The delineation of various strata was primary based on examination of disturbed samples and core samples recovered from the drillholes and the exposed faces of trial pits. The results are presented in Appendix C in form of Drillhole and Trial Pit Records, which have been finalized by incorporating comments provided by Black & Veatch Hong Kong Limited.

The legends used in these records are summarized in Appendix B.

### 6. Surveying

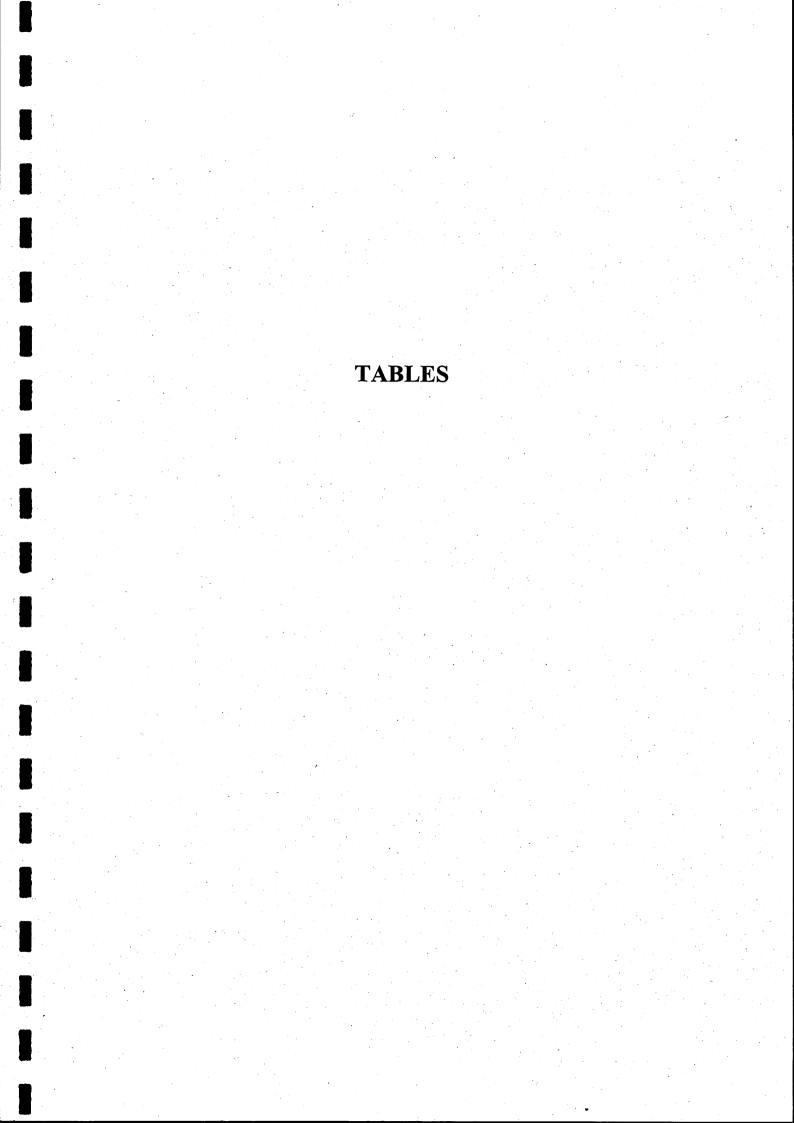
The locations of investigation stations were surveyed using theodolite and the results are related to the Hong Kong Grid System. The co-ordinates and levels of these investigation stations are presented on the relevant records and are summarized in Table 1.

### 7. Digital Data Record

The data of the ground investigation works are also provided in an electronic format on a 3.5" floppy disk. The format complies with the 3rd edition of the Association of Geotechnical and Geoenvironmental Specialists (AGS) Publication 'Electronic Transfer of Geotechnical and Geoenvironmental Data'. The record is included in Appendix M.

### 8. <u>References</u>

- 1. Geotechnical Control Office (1991), Geological Map of Hong Kong HGM20, Sheet 3 (Edition 1) 1:20,000
- 2. Geotechnical Engineering Office (2<sup>nd</sup> Reprint, 1994), Guide to Rock and Soil Descriptions (Geoguide 3)
- 3. Geotechnical Engineering Office (4<sup>th</sup> Reprint, 2000), Guide to Site Investigation (Geoguide 2)
- 4. Association of Geotechnical & Geoenvironmental Specialists (1999), Electronic Transfer of Geotechnical and Geoenvironmental Data, 3rd Edition
- 5. BS 5930: 1981, the "Code of Practice for Site Investigation"





### Contract No. GE/2003/19 Ground Investigation - New Territories East (Term Contract)

### Works Order No. GE/2003/19.29 Agreement No. CE 6/2002 (DS) Drainage Improvement in Northern NT - Package C Investigation, Design and Construction (Man Uk Ping)

### **Final Field Work Report**

### Table 1 - Survey Records

Station No.	Ground Level/ Reference Level (mPD)	Easting	Northing
B20	+ 25.74	837313.12	842697.16
B21	+ 24.88	837156.68	842618.35
B22	+ 25.83	837111.26	842932.32
B23	+ 26.30	836857.40	842969.76
B24	+ 21.90	836956.72	842718.12
B25	+ 18.69	836911.61	842516.07
B26	+ 18.44	836713.44	842442.50
B27	+ 16.94	836493.01	842431.77
B28	+ 20.47	836836.91	842332.00
ST13	+ 17.68	836850.41	842489.59
ST14	+ 16.12	836629.33	842432.79
TP12	+ 21.93	837100.49	842725.69
TP13	+ 25.09	836900.10	842910.78
TP14	+ 25.28	836960.47	842144.75

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### **Final Field Work Report**

# **Table 2 - Summary of Drillhole Record**

<b>-</b>										
Remarks					With completely decomposed zone				With completely and highly decomposed zones	
End of Hole (mPD)	+10.51	-5.06	+10.73	+11.25	-3.13	-11.31	+3.64	+1.84	+5.35	+14.78
Rock Type	Coarse ash crystal TUFF	Coarse ash crystal TUFF	Coarse ash crystal TUFF	Coarse ash crystal TUFF	Coarse ash crystal TUFF	Coarse ash crystal TUFF				
Moderately Decomposed or Less Decomposed Rock (Top Level, mPD)	+16.02	+0.68	8	•	+4.98	-7.58	1	ŧ	+13.42	ı
Residual Soil to Highlty Decomposed Rock (mPD)	+16.02	+0.68	+21.83 to "*"	+21.87 to "*"	+4.98	-7.58	+16.44 to "*"	+14.94 to "*"	+13.42	+16.73 to "*"
Colluvium Bottom Level (mPD)	L	•	1	•	•	1	۰.	•	1	16.73
Alluvium Bottom Level (mPD)	+22.64	+ 20.88	+ 21.83	+ 21.87	+ 19.40	ł	+ 16.44	+ 14.94	+ 15.57	
Fill Bottom Level, (mPD)	+24.74	+ 22.78	+ 23.83	+ 24.30	+ 20.70	+ 16.99	+ 17.44	+ 15.94	+ 18.47	1
Existing Ground Level (mPD)	+25.74	+24.88	+25.83	+26.30	+21.90	+18.69	+18.44	+16.94	+20.47	+17.68
Drillhole No.	B20	B21	B22	B23	B24	B25	B26	B27	B28	ST13

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### **Final Field Work Report**

## **Table 2 - Summary of Drillhole Record**

Remarks	
End of Hole (mPD)	+ 12.77
Rock Type	Coarse ash crystal TUFF
Residual Soil to Highlty Decomposed Rock (Top Level, (mPD) mPD)	
Residual Soil to Highlty Decomposed Rock (mPD)	+16.12 to "*"
DrillholeExistingFillAlluviumColluviumNo.Ground LevelBottomBottomLevelBottom LevelImPD)No.(mPD)Level, (mPD)(mPD)(mPD)(mPD)	
Alluvium Bottom Level (mPD)	
Fill Bottom Level, (mPD)	1
Existing Ground Level (mPD)	+16.12
Drillhole No.	ST14

1. Where stratum descriptions straddle two decomposition grades, the most decomposed grade is reported in the above table. Remarks:

2. For ease of reference, the strata less than 0.5m are not included in the table. Please refer to the Drillhole Records for detailed geology description.

3. <sup>1\*1</sup> Bottom level of the stratum cannot be determined.



Ground Investigation - New Territories East (Term Contract) Contract No. GE/2003/19

Investigation, Design and Construction (Man Uk Ping) Drainage Improvement in Northern NT - Package C Works Order No. GE/2003/19.29 Agreement No. CE 6/2002 (DS)

### **Final Field Work Report**

# Table 3 - Summary of Field Testing and Field Installation

Station	Type of	Test Zone/ Test	Type of	Installation Tip/	Response Zone	Install Halcı	Install Halcrow Buckets	Remarks
No.	Test	Depth (m bgl)	Installation	End Depth (m bgl)	(m bgl)	Level (m bgl)	Spacing (m)	
B20			Piezometer	9.20	8.20 to 9.70	0.50 to 2.50	0.5	
B21			Piezometer	23.20	22.20 to 23.70	1.00 to 3.00	0.5	
B21	Falling	7.70 to 8.70						
B22			Piezometer	14.10	13.10 to 14.60	0.50 to 1.50	0.5	
B23			Standpipe	14.60	4.00 to 15.05	0.50 to 2.50	0.5	
B23	Constant	9.50 to 10.50						
B24			Piezometer	16.50	15.50 to 17.00			
B24	Constant	9.00 to 10.00				-		
B24	IP	20.00 to 21.50 and 21.20 to 22.70						
B25	•		Standpipe	29.50	1.00 to 30.00			
B26			Piezometer	13.80	12.80 to 14.30			
B27			Piezometer	14.10	13.10 to 14.60			
B28			Piezometer	6.50	5.50 to 7.00			
Notes:	IP - Impression Packer Survey		<b>Rising - Rising Head Permeability Test</b>	meability Test		Pressuremeter - Pressuremeter Test	ssuremeter Test	

**Constant - Constant Head Permeability Test Rising - Rising Head Permeability Test** Packer - Water Absorption Test **IP - Impression Packer Survey** Sand - In-Situ Density Test Vane - Vane Shear Test

.

**Pressuremeter - Pressuremeter Test** 

Falling - Falling Head Permeability Test GCO - Dynamic Probing Test

### **APPENDIX A**

### **Checklists for Soil & Rock Descriptions**

### CHECKLIST FOR SOIL DESCRIPTION

### GEOTECHNICAL ENGINEERING OFFICE, HKSAR

6. SOIL NAME

### 1. STRENGTH (Compactness & Consistency)

Soil Type	Term	Identification
Very Coarse	۲ Loose	
(COBBLES &	{	By inspection of voids and particle packing in the field.
BOULDERS)	Dense	
	/ Very loose	SPT 'N' value 0-4.
	Loose	SPT 4-10; can be excavated with spade; 50 mm peg easily
Coarse		driven.
(SANDS &	Kedium dense	SPT 10-30.
GRAVELS)	Dense	SPT 30-50; requires pick for excavation; 50 mm peg hard to drive.
	Very dense	SPT > 50.
•	Very soft	Undrained shear strength (USS) < 20 kPa; exudes between fingers when squeezed in hand.
Fine	Soft	USS 20-40 kPa; moulded by light finger pressure.
	Firm	USS 40-75 kPa; can be moulded by strong finger pressure.
(CLAYS & SILTS)	Stiff	USS 75-150 kPa; cannot be moulded by fingers; can be indented by thumb.
	Very stiff	USS > 150 kPa; can be indented by thumbnail.
	or hard	
Organic	Compact	Fibres already compressed together.
(ORGANIC CLAYS, SILTS	Spongy	Very compressible and open structure.
SANDS & PEAT	S) Plastic	Can be moulded in hand and smears fingers.

Terms applicable only to transported soils. For soils derived from insitu rock weathering, record

actual values of quantitative tests (e.g. SPT 'N' value) as part of the description, where appropriate.

### 2. COLOUR

Parameter	Terms
Value	Light, Dark
Chroma	Pinkish, Reddish, Yellowish, Orangish, Brownish, Greenish, Bluish, Purplish, Greyish
Hue	Pink, Red, Yellow, Orange, Brown, Green, Blue, Purple, White, Grey, Black

For uniform colour distribution, choose a hue, supplemented by a value and/or chroma if necessary.

For non-uniform distribution, repeat this procedure using one of the following descriptors: spotted, mottled, dappled, streaked, striped (e.g. light yellowish brown mottled with red).

State whether sample was wet or dry when described.

### 3. PARTICLE SHAPE & COMPOSITION

Characteristic	Terms
Form	Equidimensional, Flat, Elongate, Flat & Elongate
Angularity	Angular, Subangular, Subrounded, Rounded
Surface Texture	Smooth, Rough, Glassy, Honeycombed, Pitted, Striated

Describe composition of coarse particles where appropriate. Gravel and larger particles are usually rock fragments (e.g. granite, tuff); sand particles are usually individual minerals (e.g. quartz, feldspar).

### 4. STRUCTURE

1	Soil Type	Term	Identification
		Homogenous	Deposit consists essentially of one type.
(	Coarse &	Interstratified	Alternating layers of varying types or with bands or lenses of other
1	Fine	) (Interbedded or	materials.
		(Interlaminated)	
(	Coarse	Heterogenous	A mixture of types.
	Fine	∫ Fissured	Breaks into polyhedral fragments along fissures.
		L Intact	No fissures.
	Organic	∫ Fibrous	Plant remains recognizable & retain some strength.
	organic	L Amorphous	No recognizable plant remains,

escribe spacing of bedding planes, fissures, shell bands, etc using the spacing terms given in items 6 & 7 for rock description (see other side).

Above terms applicable only to transported soils. For soils derived from insitu rock weathering, describe relict structures in accordance with item 6 of rock description (see other side).

### 5. WEATHERING

### Soils Derived from Insitu Weathering of Rocks

There are two main types: saprolites (rock texture/structure retained) and residual soils (rock texture/structure completely destroyed). Describe state of weathering in accordance with items 4 & 8 for rock description (see other side).

### Sedimentary (Transported) Soils

Coarse soils: Describe overall discolouration of soil and degree of decomposition of gravel and larger particles (see item 4, other side). Also note any signs of disintegration of large particles where apparent.

Fine Soils: Describe overall discolouration of soil where apparent.

A Desi- 0-8	<b>T</b>		
A. <u>Basic Soil</u> Soil Type		lizes (mm)	Idon tifantia a
BOULDERS	Farucie c	> 200	Identification Only seen complete in pits or exposures.
COBBLES		60 - 200	Often difficult to recover from boreholes.
COBBLES	-	00 - 200	
	r Coarse	20 - 60	<ul> <li>Easily visible to naked eye; particle shape and grading can be described.</li> </ul>
GRAVELS	{ Medium	6-20 {	Well-graded: wide range of grain sizes.
	L <sub>Fine</sub>	2 - 6	Poorly-graded: not well-graded (split further into uniform or gap-graded).
			Visible to naked eye; very little or no cohesion; grading
	Coarse	0.6 - 2	can be described.
SANDS	{ Medium Fine		May be well-graded or poorly-graded (uniform or
	< Fine	0.06 - 0.2	gap-graded) as for gravel.
		ſ	Only coarse silt barely visible to naked eye; exhibits
	Coarse	0.02 - 0.06	little plasticity and marked dilatancy; slightly granular
SILTS	⊀ Medium	0.006 - 0.02	or silky to the touch. Disintegrates in water; lumps
	ر Fine	0.002 - 0.006	dry quickly; possesses cohesion but can be
			powdered easily between fingers.
		(	Dry lumps can be broken by hand but not powdered
			between the fingers. Disintegrates in water more
			slowly than silt; smooth to the touch; exhibits
CLAYS		< 0.002	plasticity but no dilatancy; sticks to the fingers and
			dries slowly; shrinks appreciably on drying, usually
			showing cracks. These properties more noticeable
		l	with increasing plasticity.
ORGANIC			
CLAYS,		varies	Contains much organic vegetable matter; often has a
SILTS OR	-	varies	noticeable smell and changes colour on oxidation.
SANDS	-		
		ſ	Predominantly plant remains; usually dark brown or
PEATS		varies -	black in colour, often with distinctive smell; low bulk
		, t	Predominantly plant remains; usually dark brown or black in colour, often with distinctive smell; low bulk density.
B. <u>Composit</u>	<u>e Soil Types</u> (	Mixtures of Bas	

Principal	Terminology	Torm for Secondary	W - 10 1
	••	Term for Secondary	% of Secondary
Soil Type	Sequence	Constituent	<u>Constituent</u>
Very coarse (BOULDERS &	Secondary constituents	With a little	< 5
COBBLES) (> 50% of	(finer material) 🛦	{ With some	5 - 20
soil > 60 mm)	after principal	With much	20 - 50
		/ Slightly (silty, clayey	
		or silty/clayey) *	< 5
		<ul> <li>(silty, clayey</li> </ul>	
		or silty/clayey) *	5 - 15
Coarse	Secondary	Very (silty, clayey	
(GRAVELS &	constituents	or silty/clayey) *	15 - 35
SANDS)	before principal	AND/OR	
(> 65% graveł	(excluding cobbles	Slightly (gravelly	
& sand sizes)	& boulders) +	or sandy) *	< 5
		<ul> <li>(gravelly</li> </ul>	
		or sandy) *	5 - 20
		Very (gravelly	
		or sandy) *	20 - 50
Fine (SILTS	Secondary	Slightly (gravelly	
& CLAYS)	constituents	or sandy or	
(> 35% silt &	before principal	ץ both) א	< 35
clay sizes)	(excluding cobbles	- (gravelly	
only sizes/	& bouiders) +	└ orsandy) ₩	35 - 65

Full name of finer material should be given (see examples below). ٠ Secondary soil type as appropriate; use 'silty/clayey' when a distinction cannot be made

between the two if cobbles or boulders are also present in a coarse or fine soil, this can be indicated by using one of the following terms relating to the very coarse fraction after the principal: 'with occasional' (< 5), 'with some' (5-20), 'with many' (20-50), where figures in brackets are % very coarse material expressed as a fraction of the whole soil (see examples below).

Examples: Slightly silty/clayey, sandy GRAVEL. Slightly gravelly, sandy SILT. Very gravelly SAND. Sandy GRAVEL with occasional boulders. BOULDERS with much finer material (silty/clayey, very sandy gravel).

For fine soils, plasticity terms should also be described where possible, viz: 'non-plastic' (generally silts), 'intermediate plasticity' (lean clays), 'high plasticity' (fat clays).

### 7. DISCONTINUITIES

Full description of discontinuities, where necessary, should be made using the methods and terms given in item 7 for rock description (see other side).

### 8. ADDITIONAL GEOLOGICAL INFORMATION

Record geological name which indicates geological origin or soil type (e.g. Alluvium, Colluvium, Marine sand etc.). Refer to HKGS maps & memoirs for further information

NOTES:

- Mass characteristics of soils (i.e. structure, weathering, discontinuities) can only be described satisfactorily in undisturbed field exposures or large undisturbed samples. 1. For full descriptions of soils derived from insitu rock weathering: 2.
  - (a) saprolites describe as rocks, supplemented by soil strength and soil name terms in brackets,
  - (b) residual soils describe as soils, supplemented by name of parent rock where apparent from field evidence.

### CHECKLIST FOR ROCK DESCRIPTION

### GEOTECHNICAL ENGINEERING OFFICE, HKSAR

### 1. STRENGTH

### Identification Term Extremely weak Easily crumbled by hand; indented deeply by thumbnail. Sedimentary Crumbled with difficulty; scratched easily by thumbnail; peeled easily by pocket Igneous, Pyroclastic Verv weak Metamorphic knife. Weak Broken into pieces by hand; scratched by thumbnail; peeled by pocket knife; Spacing of Planar Structures deep indentations (to 5 mm) by point of geological pick; hand-held specimen Very thick (> 2 m), Thick (0.6-2 m), Medium (200-600 mm), easily broken by single light hammer blow. Thin (60-200 mm), Very thin (20-60 mm), Broken with difficulty in two hands; scratched with difficulty by thumbnail; Moderately weak Thickly-laminated (Sedimentary) (6-20 mm) or Narrow (Igneous, Metamorphic) (6-20 mm), difficult to peel but easily scratched by pocket knife; shallow indentations Thinly-laminated (Sedimentary) (< 6 mm) or Very narrow (Igneous, Metamorphic) (< 6 mm). easily made by point of pick; hand-held specimen usually broken by single light hammer blow. Examples: Thickly-bedded SANDSTONE. Narrowly flow-banded RHYOLITE. Scratched by pocket knife; shallow indentations made by firm blow with point of Moderately strong pick; hand-held specimen usually broken by single firm hammer blow. Point 7. DISCONTINUITIES load strength (PLS) 0.5 - 2 MPa. Strong Firm blows with point of pick cause only superficial surface damage: hand-held N specimen requires more than one firm hammer blow to break. PLS 2 - 4 MPa. Very strong Many hammer blows required to break specimen. PLS 4 - 8 MPa. Specimen only chipped by hammer blows. PLS > 8 MPa. Extremely strong Location and Orientation 2. COLOUR Record location as co-ordinates or relative position along datum line, preferably on map or plan. Record orientation as dip direction/dip in degrees (e.g. 032/55). Parameter Terms Light, Dark Value Spacing Pinkish, Reddish, Yellowish, Orangish, Brownish, Greenish, Bluish, Purplish, Chroma Extremely widely-spaced (> 6 m), Very widely-spaced (2-6 m), Widely-spaced (0.6-2 m), Medium-spaced (200-600 mm), Grevish Pink, Red, Yellow, Orange, Brown, Green, Blue, Purple, White, Grey, Black Hue Closely-spaced (60-200 mm), Very closely-spaced (20-60 mm), Extremely closely-spaced (< 20 mm). For uniform colour distribution, choose a hue, supplemented by a value and/or chroma if necessary. In exposures, supplement spacing with description of rock block shape where possible. For non-uniform distribution, repeat this procedure using one of the following descriptors: spotted, Descriptors: Blocky, Tabular, Columnar, Polyhedral. mottled, dappled, streaked, striped (e.g. light pinkish grey spotted with black). Persistence (Areal extent or size of a discontinuity within a plane) Measured maximum persistence dimension should be used where possible (e.g. the discontinuity State whether sample was wet or dry when described. 3. TEXTURE/FABRIC sets, relative terms should be used. Roughness Texture Terms (Applicable Mainly to Igneous Rocks) Waviness (large-scale): Estimate/measure wavelength and amplitude in metres. Equigranular, Inequigranular, Megacrystic, Porphyritic, Crystalline, Cryptocrystalline, Aphanitic nall-scale) use one term from the Ur

### Fabric

Describe preferred orientation of grains/crystals where apparent.

Describe intensity, spacing, continuity and any preferred orientation of microfractures where apparent.

### 4. MATERIAL WEATHERING/ALTERATION

Decomposition	Grade	
Term	Symbol	Typical Characteristics
Residual Soil	VI	Original rock texture completely destroyed; can be crumbled by hand and finger pressure into constituent grains.
Completely Decomposed	V	Original rock texture preserved; can be crumbled by hand and finger pressure into constituent grains; easily indented by point of geological pick; slakes in water; completely discoloured compared with fresh rock.
Highly Decomposed	IV	Can be broken by hand into smaller pieces; makes a dull sound when struck by hammer; not easily indented by point of pick; does not slake in water; completely discoloured compared with fresh rock.
Moderately Decomposed	111	Cannot usually be broken by hand; easily broken by hammer; makes a dull or slight ringing sound when struck by hammer; completely stained throughout.
Slightly Decomposed	11	Not broken easily by hammer; makes a ringing sound when struck by hammer; fresh rock colours generally retained but stained near joint surfaces.
Fresh Rock	· I	Not broken easily by hammer; makes a ringing sound when struck by hammer; no visible signs of decomposition (i.e. no discolouration).

This classification is applicable to igneous and volcanic rocks and other rocks of equivalent strength in fresh state.

### Disintegration

Describe small-scale cracking and fracturing caused by mechanical weathering, where apparent.

Alteration

Describe state of alteration (e.g. mineralised, kaolinised) where apparent.

### 5. ROCK NAME (Including Grain Size)

Igneous	: Coarse- (6-20 mm), Medium- (2-6 mm) & Fine- (0.06-2 mm) grained
	GRANITE; GRANODIORITE. Very Fine-grained (< 0.06 mm) RHYOLITE;
	BASALT. (Common types only, see Geoguide 3 for others).
Pyroclastic	: PYROCLASTIC BRECCIA (> 60 mm), Lapiili TUFF (2-60 mm), Coarse ash
	TUFF (0.06-2 mm), Fine ash TUFF (< 0.06 mm).
Metamorphic	: Foliated - SCHIST (> 0.06 mm), PHYLLITE (< 0.06 mm). Non-foliated -
	MARBLE, QUARTZITE, FAULT BRECCIA.
Sedimentary	: CONGLOMERATE, BRECCIA (> 2 mm), SANDSTONE (0.06-2 mm),
	MUDSTONE (< 0.06 mm) = SILTSTONE (0.002-0.06 mm) + CLAYSTONE
	(< 0.002 mm). (Common types only).

If rock name cannot be identified, describe grain size quantitatively, including textural term where appropriate.

### 6 STRUCTURE

Rock Type

Nature (Type of	Discontinuity)		
Fault zone	Cleavage	Fissure	Bedding
Fault	Schistocity	Tension crack	
Joint	Shear plane	Foliation	

trace length on the surfaces of rock exposures). For general descriptions of different discontinuity

onevenness (sman soale), e	ise one term norr the lonowing.	
Rough stepped	Smooth stepped	Slickensided stepped
Rough undulating	Smooth undulating	Slickensided undulating
Rough planar	Smooth planar	Slickensided planar

### Aperture Size

Wide (> 200 mm), Moderately wide (60-200 mm), Moderately narrow (20-60 mm), Narrow (6-20 m), Very narrow (2-6 mm), Extremely narrow (> 0-2 mm), Tight (zero).

Infilling (Nature)			
Clean	Surface staining	Decomposed/	
Non-cohesive soil	Cohesive soil	disintegrated rock	
Calcite	Manganese	Quartz	
Other (Specify)		Kaolin	

Give full description of infill materials/minerals where appropriate.

### Seepage

Drv Damp/wet Seepage present (estimate quantity in 1/sec or 1/min)

### Fracture State

In borehole cores, measure the following: Total Core Recovery (TCR), Solid Core Recovery (SCR), Rock Quality Designation (RQD), Fracture Index (FI). See Geoguide 3 for definitions.

### 8 MASS WEATHERING

Term	Zone Symbol	Typical Characteristics
Residual Soil	RS	Residual soil derived from insitu weathering; mass structure and material texture/fabric completely destroyed: 100% soil
	( PW	Less than 30% rock
	0/30	Soil retains original mass structure and material texture/fabric (i.e. saprolite)
		Rock content does not affect shear behaviour of mass, but relict discontinuities in soil may do so.
Partially		Rock content may be significant for investigation and construction.
Weathered	{ PW	30% to 50% rock
Rock	30/50	Both rock content and relict discontinuities may affect shear behaviour of mass.
	PW	50% to 90% rock
	50/90	Interlocked structure.
	PW	Greater than 90% rock
	<b>`90/100</b>	Small amount of the material converted to soil along discontinuities.
Unweathered	UW	100% rock
Rock		May show slight discolouration along discontinuities.

### 9. ADDITIONAL GEOLOGICAL INFORMATION

Record geological formation name if known. Avoid conjecture. Refer to HKGS maps & memoirs for further information.

### NOTES:

- Rock material description normally includes: strength, colour, texture/fabric, material weathering/alteration and ROCK NAME.
- 2 Rock mass description normally includes: strength, colour, structure, mass weathering , ROCK NAME, discontinuities and additional geological information. Can be supplemented with more detailed information on texture/fabric and material weathening/alteration of different materials within the mass where necessary.

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### **APPENDIX D**

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Photographs of Drillholes and Trial Pits

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DD	20.4		
IVN		l fan V	

### **DRILLHOLE RECORD**

CONTRACT NO. GE/2003/19

HOLE NO.

SHEET 1 of 2

**B20** 

ETH	HOD	ROT	ARY						CO-OF					WORKS ORDER NO. GE/2003/19.29
ACł	HINE	SD-8									137313 142697			DATE 20.01.2005 to 21.01.2005
US	HIN	G ME	DIUM	WA	TER				ORIEN	ITATIO	ON N	/ERTIC	AL	GROUND LEVEL +25.74 mPD
Progress	Casing Size	Water Level (m) Shift Start/ End	Water Return%	TCR%	SCR%	RQD%	Fracture Index	Tests	Samples	Reduced Level	Depth (m)	Legend	Grade	Description
2005	sw		80	100					$1 \pm 0.45$ $2 \pm 0.95$ $3 \pm 1.45$ $4 \qquad 2.00$	+25.74 +25.24 +24.74 +23.74	0.50 			Light yellowish brown (2.5Y6/4), dry, clayey silty sandy subangular GRAVEL of moderately strong tuff and quartz fragments. (FILL) Soft, moist, brown (10YR4/3), sandy clayey SILT with occasional subangular gravel and rootlets. (FILL) Soft, moist, light yellowish brown (2.5Y6/3), slightly sandy, silty CLAY with occasional subangular fine gravel of quartz and rootlets. (ALLUVIUM) Soft, light greenish grey (10Y8/1), silty sandy CLA' with some subangular gravel of quartz. (ALLUVIUM)
-	SW PW	Dry at 1800 1.60 at 0800	80	100				1,1 2,3,3,4 N=12 1,27,19,17 N=84	$ \begin{array}{c} 5 \\ 6 \\ 7 \\ 7 \\ 9 \\ 10 \\ 11 \\ 11 \\ 11 \\ 11 \\ 11 \\ 11 \\ 11$	+22.64	3.10	$ \begin{array}{c c} - & - & - & - & - & - & - & - & - & - $	V	Extremely weak, occasionally very weak, light grey (2.5Y7/2) to light yellowish brown (2.5Y6/4), completely decomposed coarse ash crystal TUFF with iron and manganese oxide stained relict joints (Firm, slightly sandy, clayey SILT with occasional subangular gravel, occasional cobble sized rock fragments)
-	PW HW			100			1	4,13 20.23,35,22/45mm 100bls/270mm	$\begin{array}{c} 12 \\ 13 \\ 13 \\ 14 \\ 7 \\ 7 \\ 16 \\ 15 \\ 16 \\ 16 \\ 16 \\ 9 \\ 10 \\ 9 \\ 10 \\ 9 \\ 9 \\ 3 \\ 10 \\ 9 \\ 9 \\ 3 \\ 10 \\ 9 \\ 9 \\ 3 \\ 10 \\ 9 \\ 9 \\ 3 \\ 10 \\ 19 \\ 9 \\ 9 \\ 3 \\ 10 \\ 19 \\ 9 \\ 9 \\ 3 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 $	+18.74	7,00	$\begin{array}{c} -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -0 & -1 & -1$	V-IV	Very weak to weak, light yellowish brown (2.5Y6/4) completely to highly decomposed coarse ash crystal TUFF. (Very stiff, sandy clayey SILT with much gravel and occasional cobbles)
ł	w		-	-	-	_	NI		9.72 T2-101	+16.02	9.72 9.86		111	Moderately strong, light grey to brownish grey,
ARGE 100 S ISTO IAZIE	e dis Ampl Samp N Sa R / P		6mm) -E			I-SITU IPRES ERME RESSI EZON	VANE SION ABILIT		LOGGEI DATE CHECKE DATE	-	26.0 C	Zhang 01.2005 . Lun <sup>V</sup> 01.2005	h	REMARKS 1. An inspection pit was excavated to 1.50m deep by hand tools. 2. One piezometer was installed with tip at 9.20m. 3. 5 nos. of piezometer Halcrow Buckets were installed from 0.50m to 2.50m with 0.50m intervals.

(		R			-/			DI	RILLH	10	LE	RE	CO	RD	HOLE N	0.		B2(	)
	Ľ						IJ		CONTRACT NO. GE/2003/19						SHEET		2	of	2
PRO	JEC	T Agre Ping	eme ), Gr	nt No	. CE	6/20 estig	02(DS), ation	Drainage I	improveme	ent in	North	ern NT	- Pac	ckage C Ir	vestigatio	n, Desig	gn and (	Constru	iction (Man Uk
		ROT							CO-OR					1	ORDER			/2003/1	
MAC	HINE	E SD-8	3						-		342697			DATE		20.0	01.2005	to	21.01.2005
FLU	SHIN	g met	DIUM	WA	TER				ORIEN	TATI	ON V	ERTICA	AL.	GROUN	D LEVEL		+2	2 <b>5.74</b> m	PD
Progress Progress Progress Avalter Return% Return% Return% Return% Return% TCR% Return% Return% TCR% Return% TCR% Return% TCR% Return% Return% Return% TCR% Return% Return% TCR% Return% Return% Return% Return% Return% Return% Return% Return% TCR% Return% Retur								Tests	Samples	Reduced Level	Depth (m)	Legend	Grade			Des	scription		
11			80	85	52	33	12.2 >20 5.9	Ĩ	T2-101		- 10,15		III	closely	spaced ro	nugh n	anar v	erv na	crystal TUFF. nally very rrow to ained, dipping
12			80	100	90	85	7.1		T2-101										
13			80	100	93	87	≥20 7.7		T2-101		- - - - - - - - - - - - - - - - - - -								
14 15		1.96 at 1800	80	100	97	62	>20 5.6 18.6 5.3		T2-101		13.87								
<u>01.2005</u> 16		1000								+10.51	- 15.23			End of I	nole at 15	.23 m.			
17											ليتبابينا								
18																			
19																			
LARO U76 U100 PIST	GE DIS SAMP O SAMI ON SA		D SAM 76mm) PLE	PLE		N-SITU MPRE PERME PRESS	J VANE SH	R TEST	LOGGE DATE CHECKI DATE		26.0	Zhang 01.2005 . Lun 01.2005	W	REMAR	KS				

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### **APPENDIX I**

### Drillholes Piezometer / Standpipe Detail and Response Test Record Sheets

Project     RESPONSE TEST RECORD SHEET     B20       Contract No.: CF2003/19.0     Date of Installation:     22-Jan-2005       Works Order No.: CF2003/19.20     Date of Installation:     22-Jan-2005       Froject:     Agreement No.: CF2003/19.20     Date of Installation:     22-Jan-2005       Ground Level:     -2.6.7     mPD       (Man Uk Ping)     2.00     mbelow C.L.     Fizometer: Tip Level:     +16.54       (Man Uk Ping)     D.20     mbelow C.L.     Fizometer: Tip Level:     +16.54       (Man Uk Ping)     D.20     mbelow C.L.     Fizometer: Tip Level:     +16.54       (Man Uk Ping)     D.7044033     Checked Date:     C.L. Hand 55       (Minates)     (m)     0.00     0.20     m       (Minates)     (m)     0.20     m     Coordinates :       (Minates)     (m) <t< th=""><th></th><th></th><th>DRILLHOLF PIFZO</th><th>METER DETAIL</th><th></th><th>illhole No. :</th></t<>			DRILLHOLF PIFZO	METER DETAIL		illhole No. :
Contract No.:         GE/2003/19         Date of Installation :         22-Jan-2005           Works Order No.:         GE/2003/19.29         Date of Est:         24-Jan-2005           Project:         Agreement No.:         Geround Level:         24-Jan-2005           In Norther NT - Package Clavestigation, Design and Construction         Ground Level:         +25.74         mPD           (Man Uk Ping)         D.00         D.0         B Chan         Checkkel by :         -14.54         mPD           Dip meter, LD:         D'D'D'24003         Checkkel Date :         24-Jan-2005         mDt         -4.54.64         mPD           0         0.00         Dopt meter, LD:         D'D'D'24003         Checkkel Date :         24-Jan-36         mD'D'D'D'D'D'D'D'D'D'D'D'D'D'D'D'D'D'D'						
Works Order No.: GE/2003/19:29         Date of test :         24-Jan-2005           Project:         Agreement No. CE6/2002(DS) Drainage Improvement in Northern NT - Package C Investigation, Design and Construction         Co-ordinates :         +25.74         mTD           Initial Water Level :         2.00         m below G.L.         Plezometer Tip Level :         +16.54         mVD           Dip meter LD :         Chan         Checked Date :         24-Jan-05         Checked Date :         24-Jan-05           Time         Depth of pipe         Depth above Ground Level         Checked Date :         24-Jan-05           0         0.00         0.20         m         Order Note of pipe         Depth above Ground Level         PVC cap with vent 1           0         0.00         0.20         m         Order Note of pipe         Depth below Ground Level         Projec dia.: 25mm           0.20         2.16         3.00         2.20          Order Note of pipe         Pipe dia.: 25mm           1.00         1.86          Pipe dia.: 25mm         Order Note of pipe         Pipe dia.: 25mm           0.20         2.16         3.00         2.20         m         Estonitie sell         Centent benonite grout           1.00         1.86          Pipe di	Contract No ·	GE/2003/19		1	h	
Project:       Agreement No. CE6/2002(DS) Drainage Improvement in Northern NT- Package C Investigation, Design and Construction (Man Uk Ping)       Ground Level : +25.74 mtD Co-ordinates :       mtD         Chan Uk Ping)       E 837313.12 N 842697.16       N 842697.16         Titid Water Level :       2.00 m below GL.       Plezometer Tip Level : +16.54 mtPD         Tested / Supervised by :       U/ B. Chan       Checked by :       C. Lan Ud         Dip meter LD.       Depth of Water       Depth above Ground Level       Calcelade over         0       0.00       0.00       0.20 m       PVC cap with venth         0.25       0.55       0.50       1.50       Depth below Ground Level       Depth below Ground Level       Pipe dia : 25mn         1.00       1.86       1.50       2.10       Depth below Ground Level       Pipe dia : 25mn         3.00       2.20       2.16       State of the seal       Pipe dia : 25mn         Material Surrounding Response Zone:       Response Zone:       Completaly to highly decomposed TUFF       9.20 m       Image: Material State of dillicole         9.20 m       3.00       19.20 m       Image: Material State of dillicole       Image: Material State of dillicole						
In Northern NT - Package C Investigation, Design and Construction (Man Uk Ping)  E 37313.12  N 842697.16  B Chordinates: B 37313.12  N 842697.16  B 3731  B 373  B 373  B 373  B 37  B 373  B 37  B 37 B 37			2(DS) Drainage Improvement			
Man Uk Ping)         F 837313.12         N 842697.16           Initial Water Level :         2.00         m below G.L.         Pizzometer Tip Level :         +16.54         mtD           Dip meter LD. :         D1-024-003         Checked by :         24-Jan-05         Checked by :         24-Jan-05           Time         Depth of Water         D1-024-003         Checked Date :         24-Jan-05         Checked Date :         24-Jan-05           0         0.00         0.00         Checked Date :         24-Jan-05         Concrete surface box           0.100         1.50         Cape Mith Vent1         Depth babox Ground Level         PVC cap with Vent3           0.75         1.70         Depth below Ground Level         Ground Level         Pipe dia : 25mm           3.00         2.20         2.16         Material Sand         Pipe dia : 25mm           3.00         2.20         2.16         Material Sand         Pipe dia : 25mm           1.100         1.86	1 -					III D
Initial Water Level :         2.00         melow G.L.         Piazometer Tip Level :         +16.54         mPD           Tested / Supervised by :         .         D. Chan         Checked Dy :         C. Lam but         Checked Dy :         C. Lam but           Dip meter L0:         .         D'D'24/003         Checked Dy :         C. Lam but         Checked Dy :         C. Lam but           Inite         Depth of Water         I.od/24/003         Checked Dy :         C. Lam but         Checked Dy :         C. Lam but           0         0.000         0.000         0.000         0.000         Dot habove Ground Level         Prove dia : 25mm         Drain pipe           0.20         2.16         .         Depth below Ground Level         Pipe dia : 25mm         Comeent           1.00         1.86         .         .         .         .         .         .           1.00         2.16         .		0 0 ,	8		3.12	N 842697.16
Tested / Supervised by :       Dr.       B. Chan       Checked by :       C. Lun W         Dip metr LD. :       Dr. 1024-003       Checked Date :       24-Jan-05         Time       Depth of Water       Finance       Lockable cover         Remarks :       Depth of Water       Finance       Lockable cover         0       0.00       0.00       Depth above Ground Level       Concrete surface box         0.25       0.55       0.55       Depth below Ground Level       PVC cap with vent I         0.20       2.16       Depth below Ground Level       Pipe dia: 25mm         0.20       2.16       Pipe dia: 25mm       Cement bentonite grout         0.20       2.16       Pipe dia: 25mm       Cement bentonite grout         0.20       2.16       Sand       Pipe dia: 25mm       Cement bentonite grout         0.20       2.16       Sand       Pipe dia: 25mm       Cement bentonite grout         0.101       2.200       m       Response zone       Cement bentonite grout       Cement bentonite grout         0.1131       2.20       m       Response zone       Cement bentonite grout       Cement bentonite grout         0.1132       2.20       m       Cement bentonite grout       Cement bentonite grout <td></td> <td>el :</td> <td>2.00 m below G.L.</td> <td>Piezometer Tip Level :</td> <td>+16.54</td> <td></td>		el :	2.00 m below G.L.	Piezometer Tip Level :	+16.54	
Dip meter ID:         To Dt 04 Vater         Lacktable cover           Filme         Depth of Water         Lacktable cover         Lacktable cover           (minutes)         (m)         0         Lacktable cover         Lacktable cover           (minutes)         (m)         0         Lacktable cover         Lacktable cover           0         0.00         0         0         Drain pipe           0.25         0.55         0.55         Drain pipe           0.50         1.50         2.10         Drain pipe           2.00         2.16         Depth below Ground Level         Pipe dia: : 25mm           0.00         2.20         2.16         Drain pipe         Cement           0         0.00         2.20         Pipe dia: : 25mm         Cement           0         0.00         2.20         m         Pipe dia: : 25mm           1.00         1.52         m         Pipe dia: : 25mm         Cement           0         0.00         0.00         Pipe dia: : 25mm         Cement           0         0         0         0         Pipe dia: : 25mm         Cement           0         0         0         0         0         Cement         Cement <td>Tested / Supervise</td> <td>ed by:</td> <td>B. Chan</td> <td></td> <td><b>C.</b> 2</td> <td></td>	Tested / Supervise	ed by:	B. Chan		<b>C.</b> 2	
Elapsed (minutes)         from top of pipe (m)         Depth above Ground Level         Concrete aufLee box (concrete aufLee bo	Dip meter I.D. :	1.1	DT-024-003			
(minutes)         (m)         0.20         m         PVC cap with vent filloot           0         0.00         0.55         0.55         0.55         0.75         1.70           0.00         1.50         0.20         1.60         1.86         0.20         0.216           1.50         2.10         0.20         2.16         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.216         0.20         0.	Time	Depth of Water		· · · · · · · · · · · · · · · · · · ·		Lockable cover
0       0.00         0.25       0.55         0.50       1.50         0.775       1.70         1.00       1.86         1.50       2.10         2.00       2.16         3.00       2.20	Elapsed	from top of pipe	Depth above Ground Level	· · · ·	Conc	rete surface box
0       0.00         0.25       0.55         0.50       1.50         0.775       1.70         1.00       1.86         1.50       2.10         2.00       2.16         3.00       2.20	(minutos)	(m)	0.20			~
0.25       0.55       Ground Level       Image: product of the second			<u> </u>		PVC	
0.50       1.50         0.75       1.70         1.00       1.86         1.50       2.10         2.00       2.16         3.00       2.20						Drain pipe
0.75       1.70         1.00       1.86         1.50       2.10         2.00       2.16         3.00       2.20	0.25	0.55	Ground	d Level	2004.027-2004	Statistic 2 Note
1.00       1.86         1.50       2.10         2.00       2.16         3.00       2.20	0.50	1.50				
1.50       2.10         2.00       2.16         3.00       2.20	0.75	1.70	Depth below Ground Level			
2.00       2.16         3.00       2.20	1.00	1.86				
3.00       2.20	1.50	2.10				
3.00       2.20	· 2.00	2.16			n N	ine dia · 25mm
Image: Comparison of the comparison						ipe dia 2511111
	5.00	2.20				
	· · · · · · · · · · · · · · · · · · ·					Cement
Image: marks :						bentonite
						grout
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.23 m       (1:3)						(1:3)
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.20 m       (1:3)						
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.20 m       (1:3)			•			
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.23 m       (1:3)	-					
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.23 m       (1:3)						
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.20 m       (1:3)						
Filter Material:       Sand         Material Surrounding Response Zone:       8.20 m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Bentonite Grout         15.23 m       Cement bentonite Grout         13.23 m       (1:3)						
Filter Material:       Sand         Material Surrounding Response Zone:       m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Cement bentonite Grout         15.23 m       Base of drillhole	• .		<u>7.20</u> m			
Filter Material:       Sand         Material Surrounding Response Zone:       m         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Cement bentonite Grout         15.23 m       Base of drillhole	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·				
Filter Material:       Sand         Material Surrounding Response Zone:       Page 1         Completely to highly decomposed TUFF       9.20 m         9.20 m       9.70 m         9.70 m       Bentonite seal         10.70 m       Cement bentonite Grout         15.23 m       (1:3)         Remarks :       Base of drillhole					///	Bentonite seal
Material Surrounding Response Zone:       Response Zone:         Completely to highly decomposed TUFF       9.20 m         9.70 m       Bentonite seal         10.70 m       Cement bentonite Grout         15.23 m       (1:3)         Remarks :       Base of drillhole			<u>8.20</u> m			
Completely to highly decomposed TUFF       9.20     m       9.70     m       9.70     m       10.70     m       15.23     m       Remarks :     Base of drillhole	Filter Material:	Sand	- - 			
9.20 m       (Filter Sand)         9.70 m       Bentonite seal         10.70 m       Cement bentonite Grout         15.23 m       (1:3)         Base of drillhole	Material Surroundin	ng Response Zone:		p d -		
9.20 m     (Filter Sand)       9.70 m     Bentonite seal       10.70 m     Cement bentonite Grout       15.23 m     (1:3)       Remarks :     Base of drillhole	Completely to high	ly decomposed TUFF		b d .	R	esponse zone
9.70     m       9.70     m       10.70     m       15.23     m       15.23     m       Base of drillhole		-	<b>9.20</b> m	bd	1 1	
10.70 m     Bentonite seal       10.70 m     Cement bentonite Grout       15.23 m     (1:3)       Remarks :     Base of drillhole			·			(a mor Durid)
10.70 m     Cement bentonite Grout       15.23 m     (1:3)       Remarks :     Base of drillhole			<u> </u>			
15.23     m     Cement bentonite Grout       Remarks :     Base of drillhole					(/ <u>)</u>	Bentonite seal
15.23 m     (1:3)       Remarks :     Base of drillhole			<u> </u>			
Remarks : Base of drillhole			,	<i>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</i>	Cement	bentonite Grout
			<u> </u>			(1:3)
	Remarks :		й. -	Base of drill	hole	
DGEL\Site-F11b_03/02 STD	DGEL\Site-F11b_03/02_STD			·		

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### **APPENDIX J**

### Water Level Monitoring Records

DRiLT	'ECU	WATER	LEVEL	Station No.						
		MONITORI	MONITORING RECORD							
Contract No. :	GE/2003/19	· ·	Standpipe/Piezometer*							
Works Order No. :	GE/2003/19.29		Co-ordinates :							
Project: Agreement No.	CE6/2002(DS) Drainage	e Improvement in Northern	E 837313.12	N 842697.16						
NT - Package C Investigati	on, Design and Constru	ction (Man Uk Ping)	Ground Level :	+25.74mPJ						
Date of Installation :	22-Jan-05		Tip Level :	+16.54mP						
110	B. Chan		Dip Meter I.D. :	DT-010-018						
Date	Time	Depth of Ground Water Level/below G.L. (m)	Depth of Ground Water/Reduced Level (mPD)	Weather						
25-Jan-05	10:00 AM	2.20	+23.54	Fine						
26-Jan-05	11:00 AM	2.20	+23.54	Fine						
27-Jan-05	4:00 PM	2.20	+23.54	Fine						
28-Jan-05	3:00 PM	2.20	+23.54	Fine						
29-Jan-05	12:00 PM	2.20	+23.54	Fine						
31-Jan-05	1:35 PM	2.21	+23.53	Fine						
1-Feb-05	12:50 PM	2.22	+23.52	Fine						
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·····										
			·							
	· · · · · · · · · · · · · · · · · · ·		, , ,							
Remarks :		· ·	. ,							

\* Delete as appropriate DGEL\Site-F12 03/02 STD

WL\_19.29

### APPENDIX K

### **Piezometer Buckets Records**

DF	RiLTI	EC	H			T		IEZ				<b>`</b>	Drillhole /Trial		
						1	suc	KE	15 F	EC.	UKI			B20	
Contract No		GE/20										Buckets :	Number: 5		
Works Orde		GE/20										4	Depth : 0.5		m
Project: NT - Packag	Agreement e C Investig									thern		-	Spacing : 0.5 Date Installed :	50 m 24-Mar-05	
Ground Lev	el :		+25.74	ļ	mPD			· · · · ·				Depth of Ti	D:	9.20 m below G	.L.
Top of Pipe			0.20			above	below	G.L.				Tip Level o		-16.54 mPD	
	Ground Water Level,	Buckets Found to Contain Water el, Buckets No. t													
Date	Depth*		t No.: 2	3	4	5		1	1	1		Level, Depth*	Commo	nts/Weather	Recorde
Date	measured	1	2	3								indicated	Comme	its/ weather	by
	by Dipmeter		t Dept									by buckets			
	_	0.50	1.00	1.50	2.00	2.50						- ()			
	(m)		· · ·							+		(m)			-
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		•													
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Remarks :		<u> </u>		1		I	1			1	<u> </u>	L	I	Checked by :	
€														C. Lun	
<sup>f</sup> Delete as appr * Depth measu	-	nd level											· .	Date : 24-Mar-0	5

BLK A & B, HONG KONG SPINNERS INDUSTRIAL BLDG, PHASE VI. 481-483 CASTLE PEAK ROAD, KOWLOON, HONG KONG 香港九龍青山道481-483號香港紗廠工業大廈第六期九樓 A,B座 TEL: 2371 0008 FAX: 2744 1037 E-Mail: driltech@driltech.com.hk WEBSITE: driltech.com.hk

### Appendix E – Proposed Development Layout Plan

