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Timothy Wai Pui WU/PLAND

寄件者: AI TSANG <[redacted]>
寄件日期: 2026年11月06日星期五 12:07
收件者: Timothy Wai Pui WU/PLAND
主旨: TKLN 129 FI
附件: TKLN 129 amended drainage proposal.pdf

類別: Internet Email

Dear Sir,

Regarding to the DSD and HYD comment , Please find attached amend drainage proposal.
This email supersedes the one sent at 11:29 today.

Thanks

Carter Tsang
[redacted]

Comment 1
The applicant should be advised that the limited desk-top checking by Government on the drainage proposal covers only the fundamental aspects of the drainage design which will by no means relieve his obligations to ensure that (i) the proposed drainage works will not cause any adverse drainage or environmental impacts in the vicinity; and (ii) the proposed drainage works and the downstream drainage systems have the adequate capacity and are in good conditions to accommodate all discharge water collected from his lot and all upstream catchments. The applicant shall effect any subsequent upgrading of these proposed works and the downstream drainage systems whenever necessary.
Response
Noted.

Comment 2
Annex 1: The applicant should indicate the cover and invert levels of the proposed 300mm UC (the one along the toe of the raised platform) at the most upstream end.
Response
Noted. Please refer to Annex 1

Comment 3
Annexes 5 and 7: From photo 2, it seems that the depth of the existing streamcourse is much less than 1m. In addition, the level difference between the invert levels of the proposed pipe and the existing streamcourse is more than 600mm, and hence a backdrop manhole should be provided. The applicant should review.
Response
Noted. The backdrop manhole will be used for CP 4 ,

Response to comment
The photo shows the measurement on site that the depth of the existing streamcourse is provided. confirming that the depth of the existing streamcourse is 0.7 m. The width of the existing streamcourse is clarified to 1.3 m The Hydraulic checking and related drawing is revised
Response to comment
Hydraulic checking of the existing streamcourse is reviewed.

General Response to Comment
The drainage consultant wishes to clarify the following details for the existing streamcourse: The invert level (I.L.) remains +7.3 mPD. The surveyed point is correct; the previously shown 1 m depth was an illustration error. The width of the existing streamcourse is confirmed as 1.3 m. The depth of the existing streamcourse is 0.7 m which comply with the survey point. The site photo is provided. The related drawing Plan is revised.
Response to comment
Hydraulic checking of the existing streamcourse is reviewed.

Drainage Proposal

1. Site particulars

1.1 The application site is located at Lots. 752 S.A,752 S.B,752 S.C,752 S.D,752 S.E,752 RP in D.D. 82. The site area is approximately 1120 m². The site location is shown in Figure 1.

1.2 The existing site is fully paved with approximate levels of +9.0 mPD. No significant site formation works are anticipated for the proposed development.

1.3 The site can access from Ping Che Road. The surrounding area is predominantly rural, comprising temporary structures Village House and government facilities.

GEOINFO MAP
地理资讯地图

Location Plan of Site Photo

前往地图: https://www.map.gov.hk/gm/geo:22.5400,114.1490?z=564



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地图列印于 2026 年 8 月 24 日

由「地理资讯地图」网站提供: https://www.map.gov.hk

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Photo 1



Photo 2



Photo 3



Photo 4

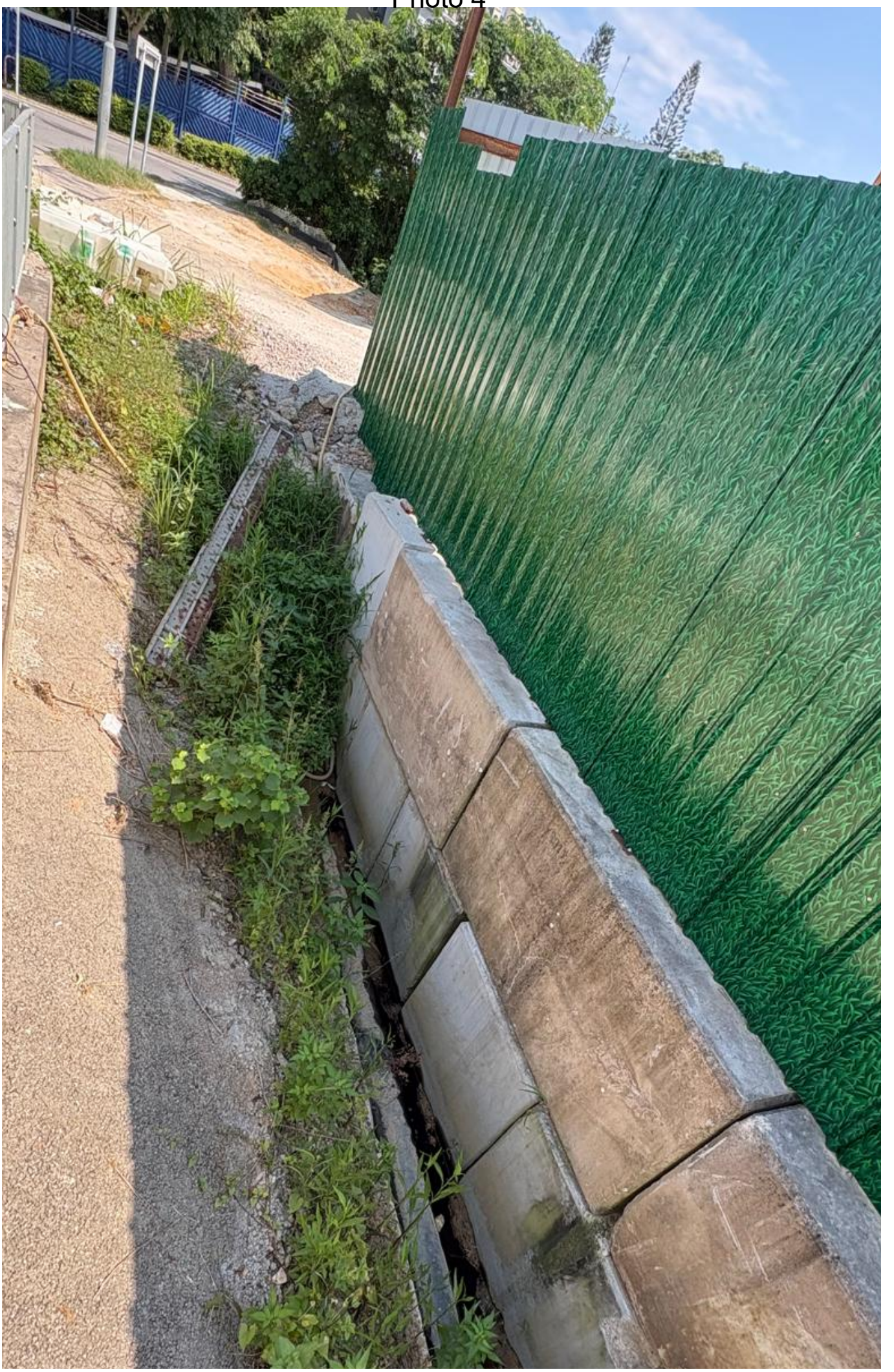
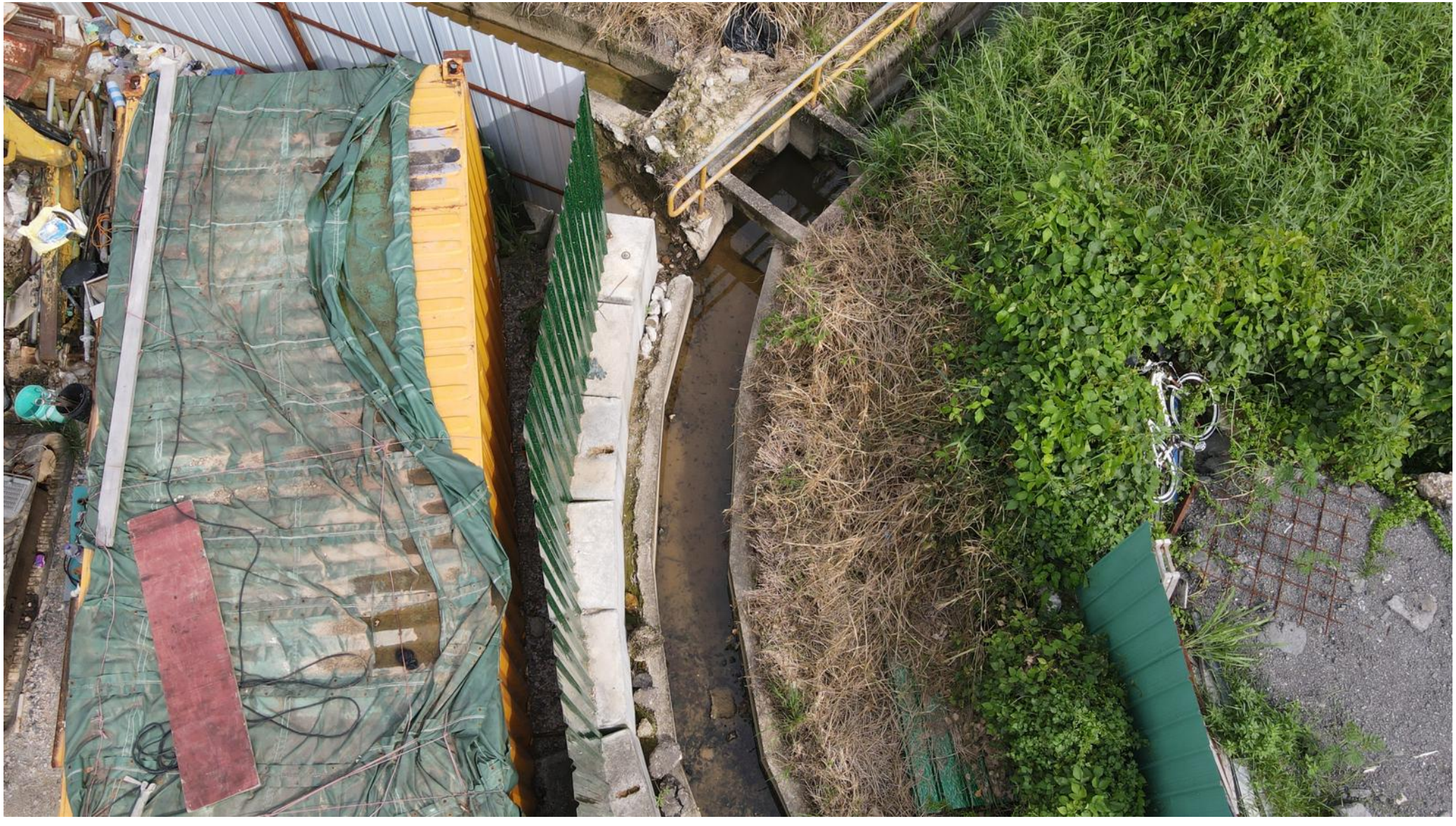


Photo 5

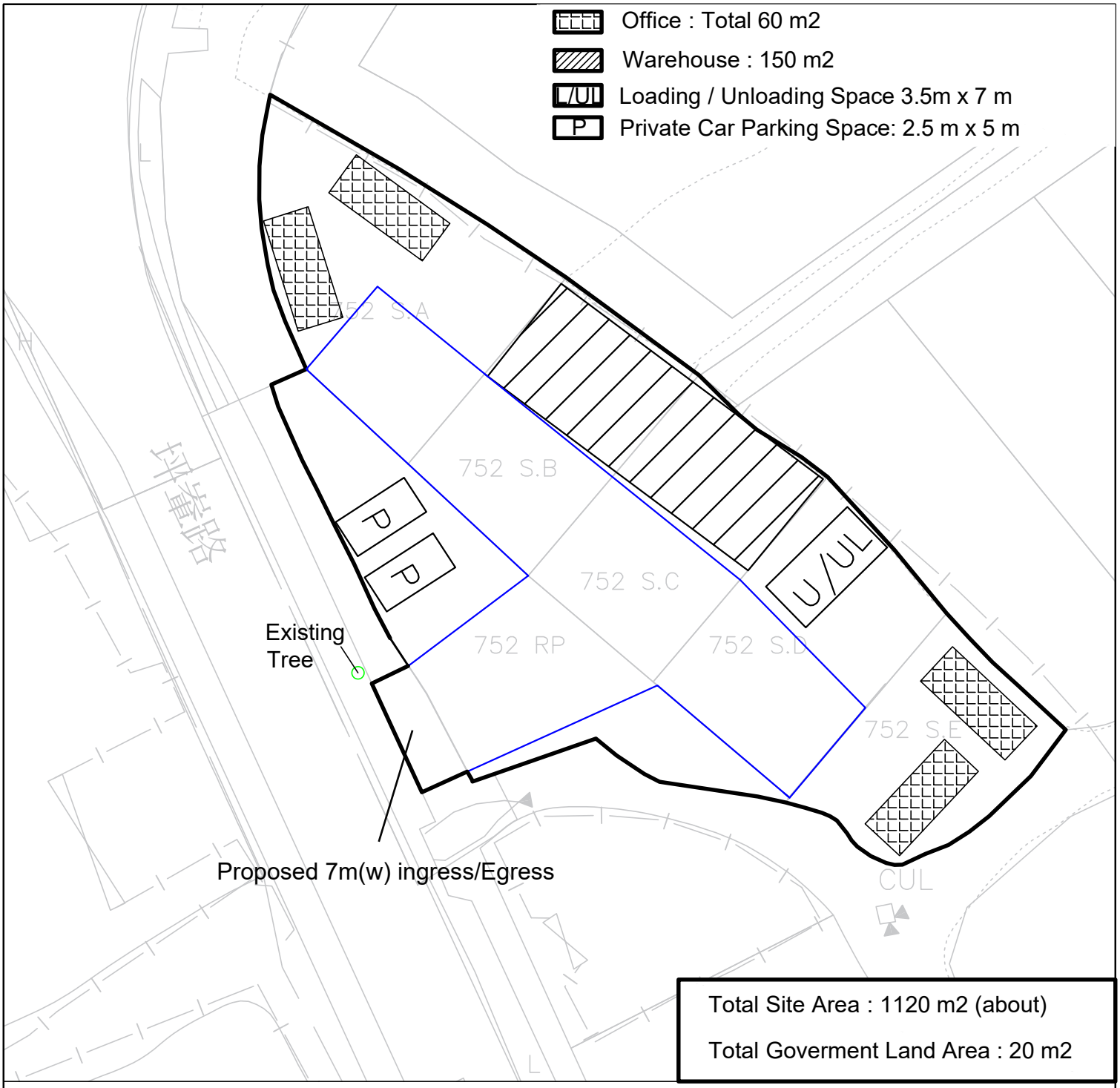


Photo 6



Site photo for measuring depth of existing streamcourse

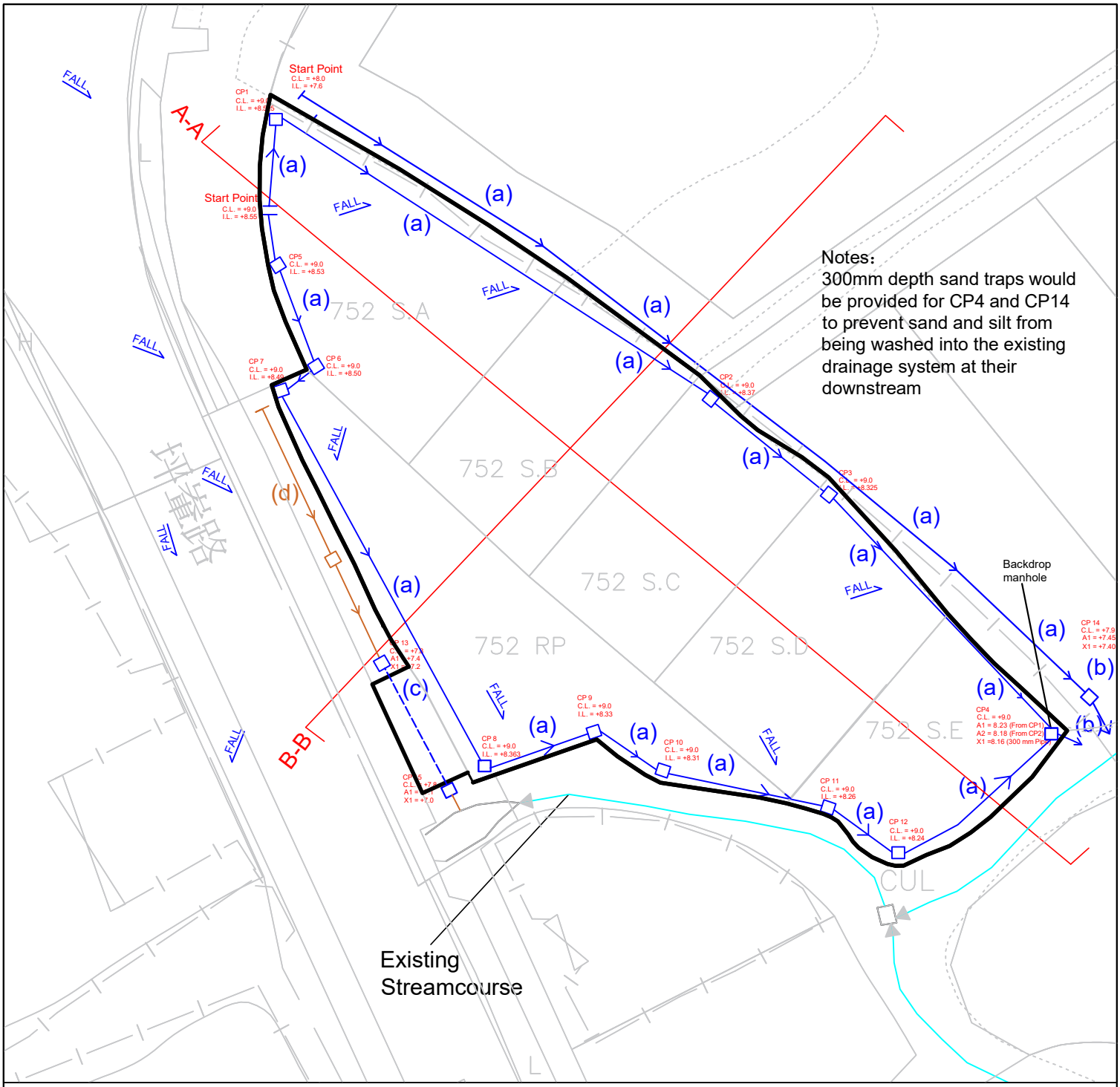




Layout Plan

YING SHING (HOPEWELL) ENGINEERING CO.LTD.

1:1000



(a) Proposed 300 mm U- Channel (1:200)

(b) Proposed 375 mm Concrete Pipe (1:200)

(c) Proposed 375 mm Concrete Pipe (1:200)

Proposed Catch Pit

(d) Existing 300 mm U- Channel

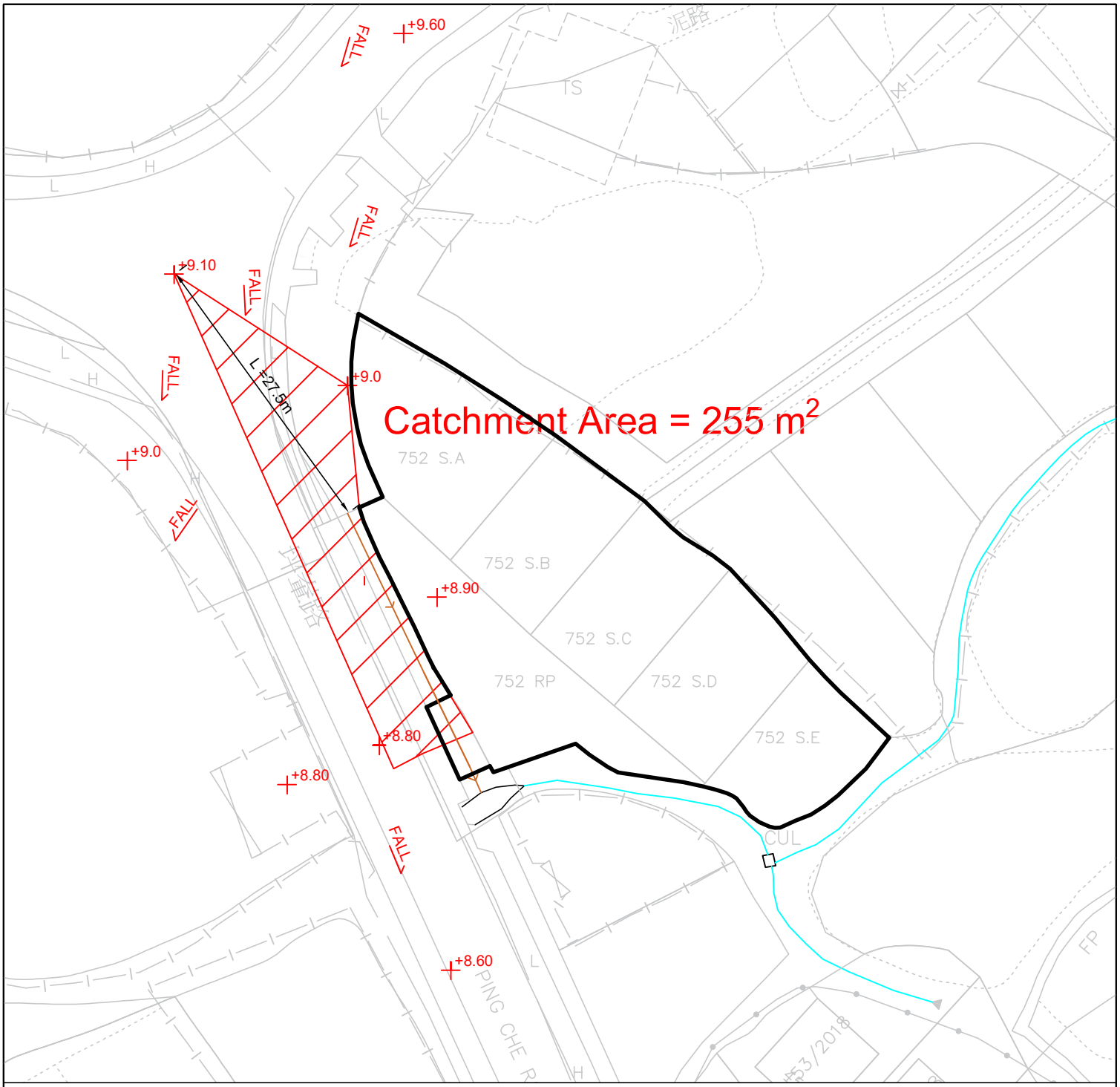
Existing Catch Pit should be demolished

1:1000

Drainage Proposal - Layout Plan

Lots. 752 S.A,752 S.B,752 S.C,752 S.D,752 S.E,752 RP In D.D. 82

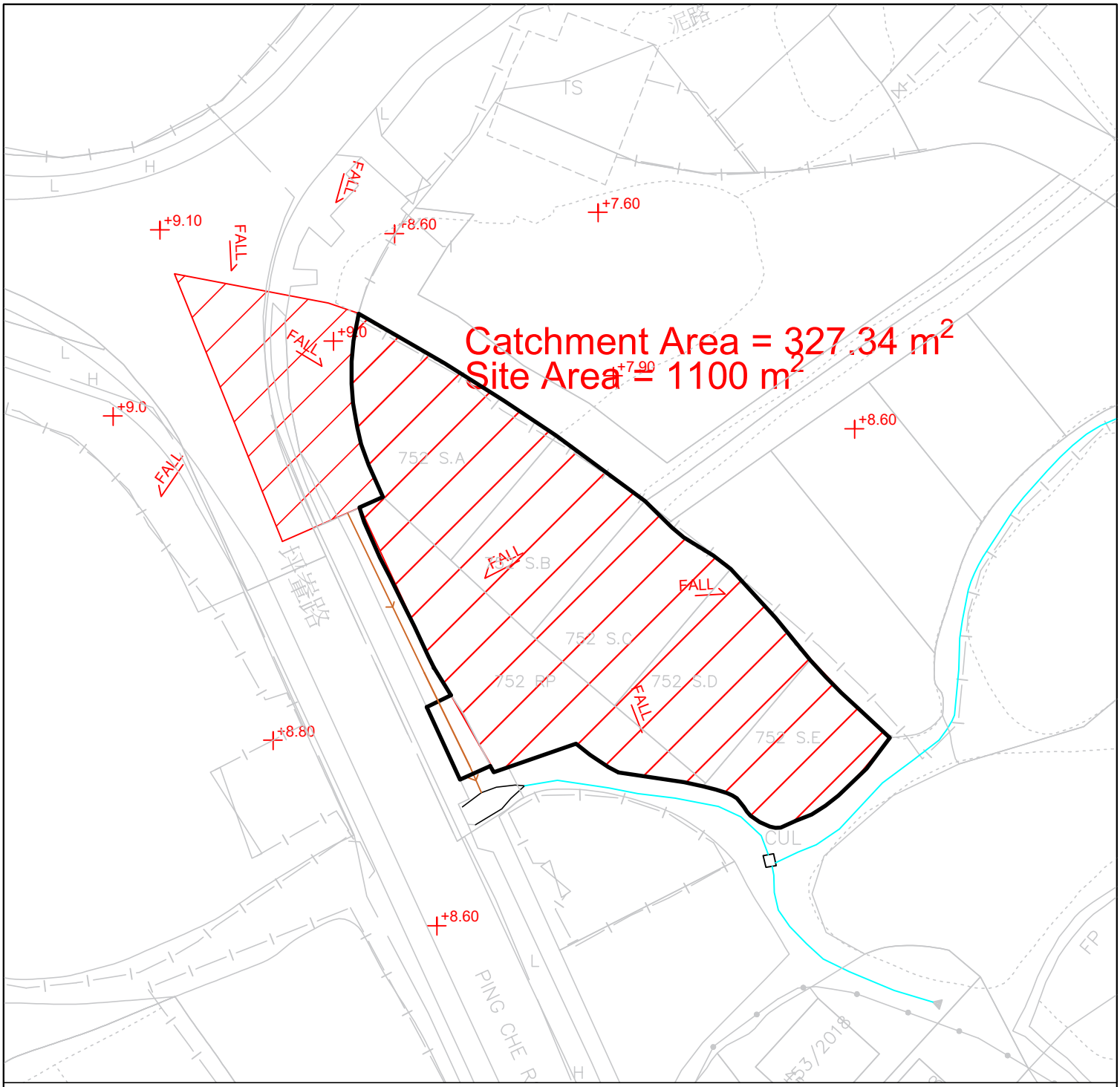
Annex 1



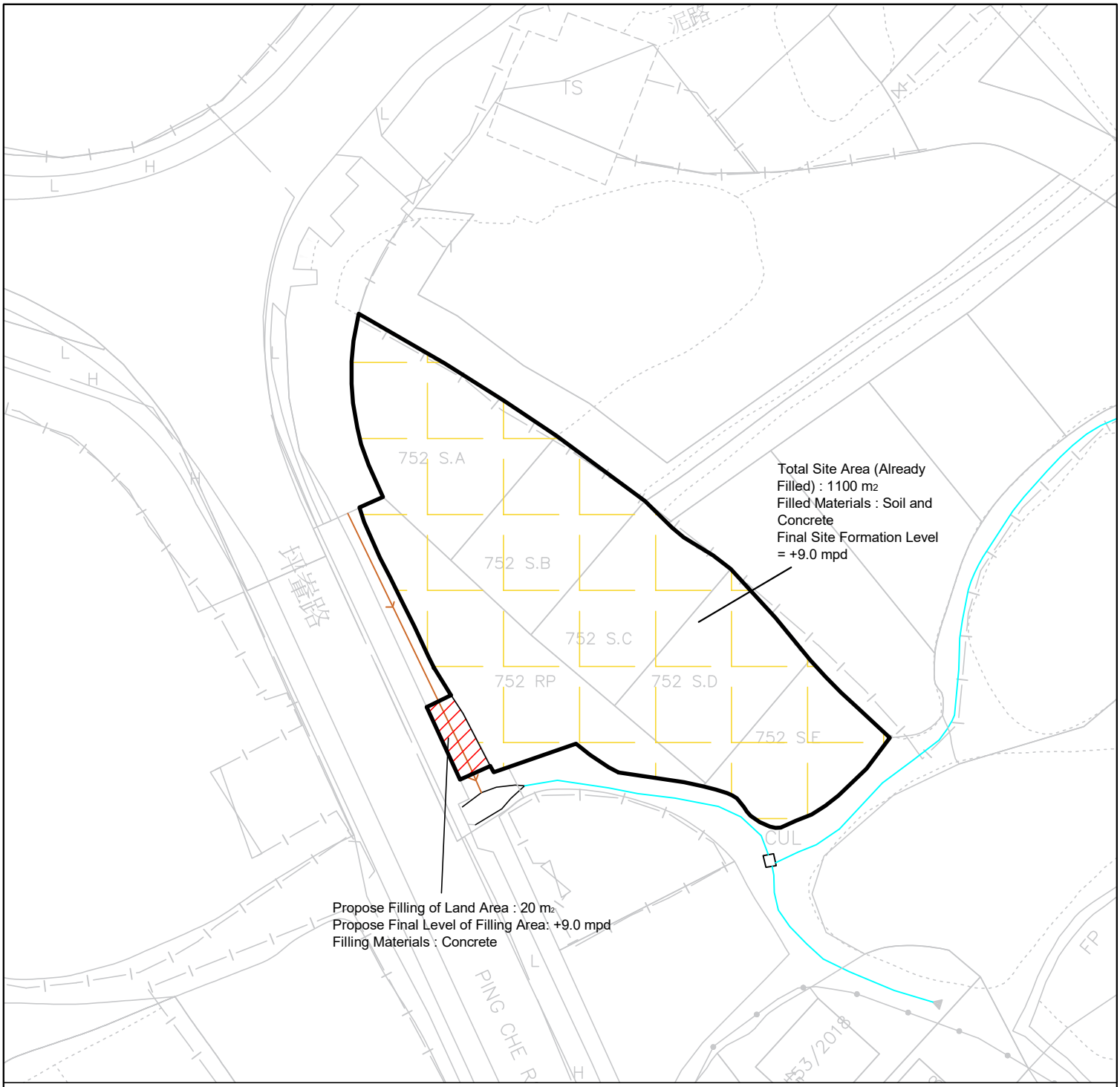
Catchment Area Plan
(Existing U-Channel)

1:1000

Annex 2



	Catchment Area Plan (Application Site)	YING SHING (HOPEWELL) ENGINEERING CO.LTD.
1:1000		Annex 3

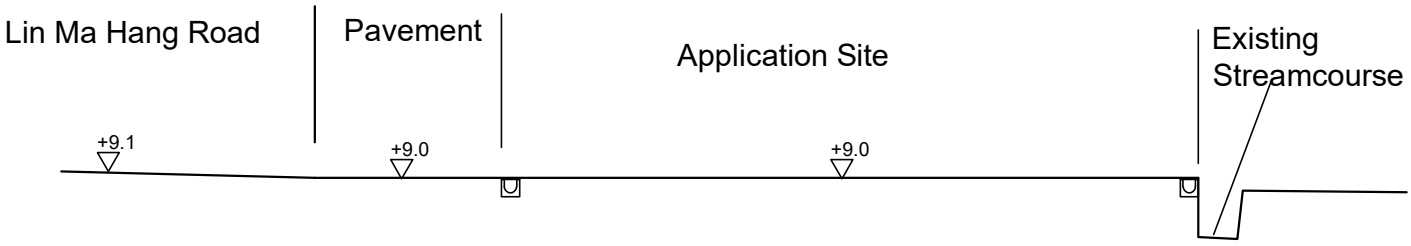


Filling of Land Plan

Lots. 752 S.A,752 S.B,752 S.C,
752 S.D,752 S.E,752 RP In D.D. 82

1:1000

Annex 4



Section A-A

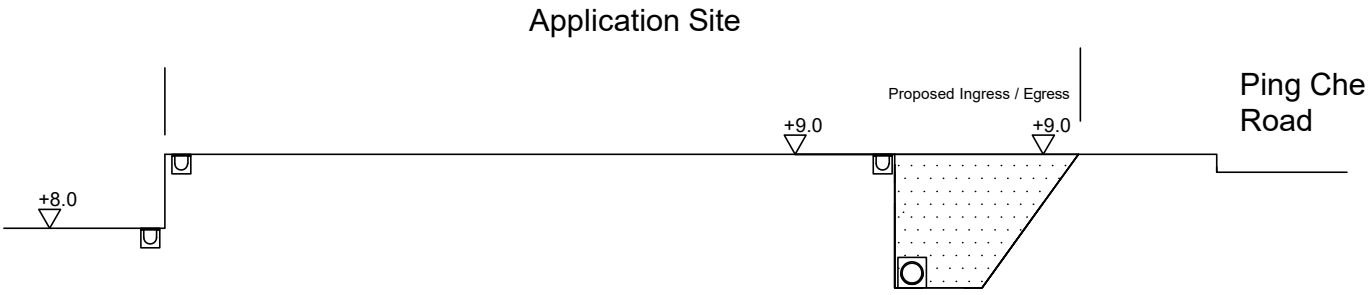
Section Plan

Lots. 752 S.A,752 S.B,752 S.C,
752 S.D,752 S.E,752 RP In D.D. 82

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Not to Scale

Annex 5



Section B-B

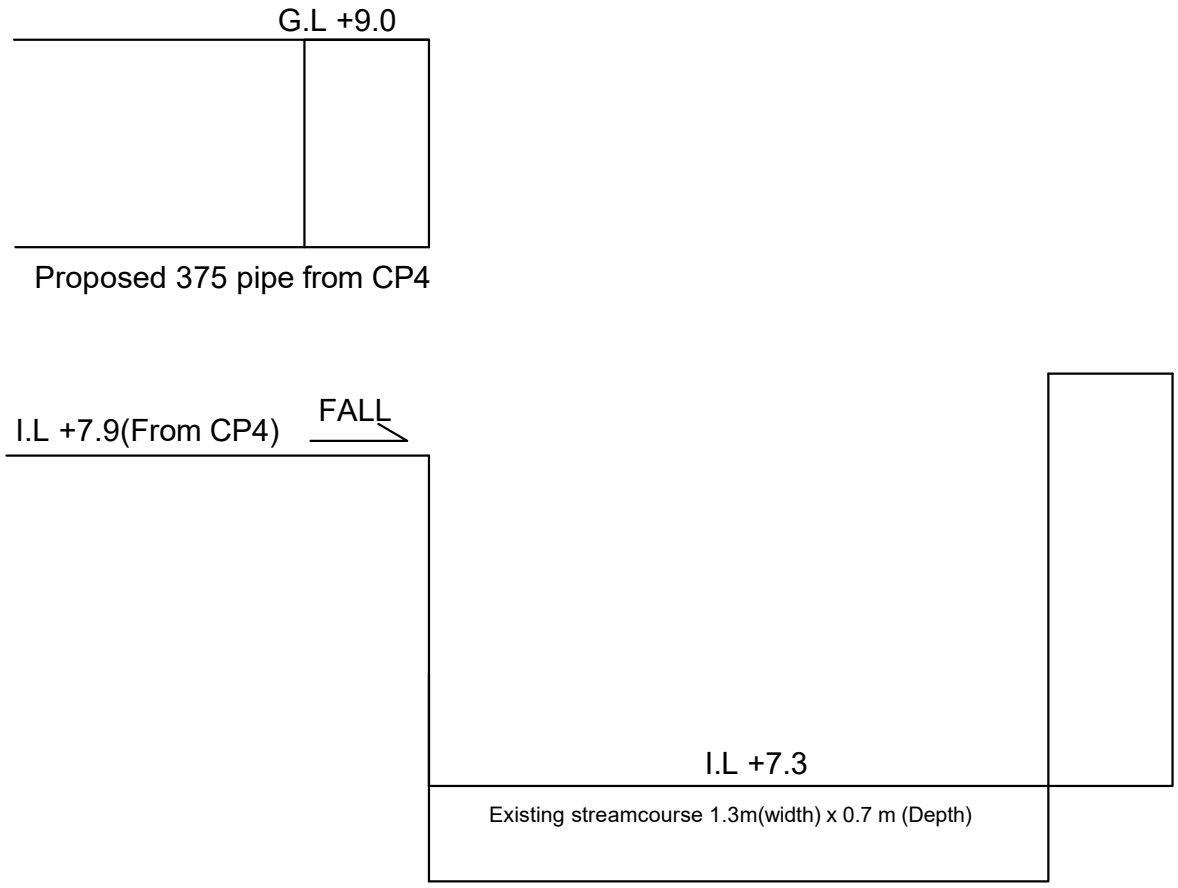
Section Plan

Lots. 752 S.A,752 S.B,752 S.C,
752 S.D,752 S.E,752 RP In D.D. 82

YING SHING (HOPEWELL) ENGINEERING CO.LTD.

1:1000

Annex 6

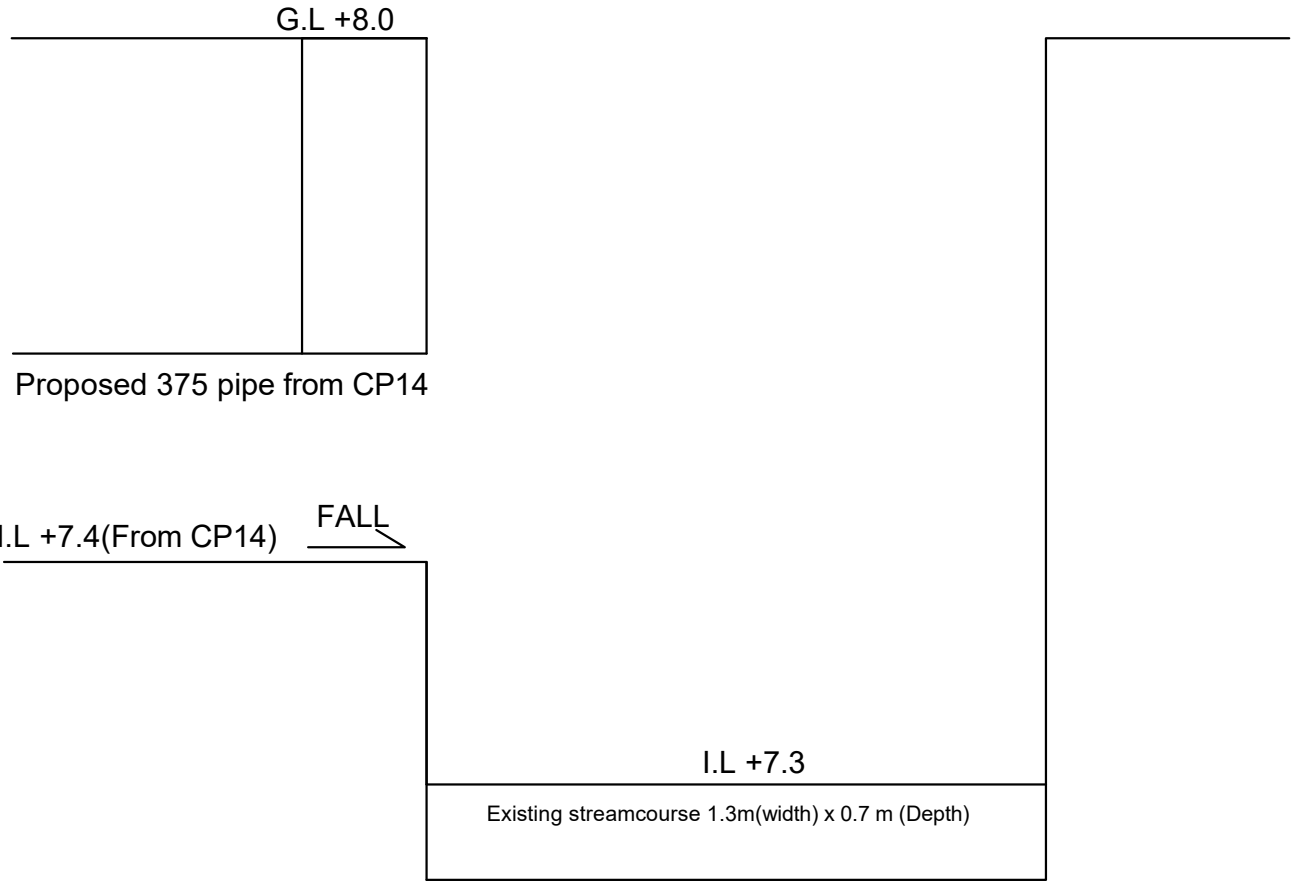


Connection Detail
(CP 4 to Existing streamcourse)

YING SHING (HOPEWELL) ENGINEERING CO.LTD.

Not to Scale

Annex 7

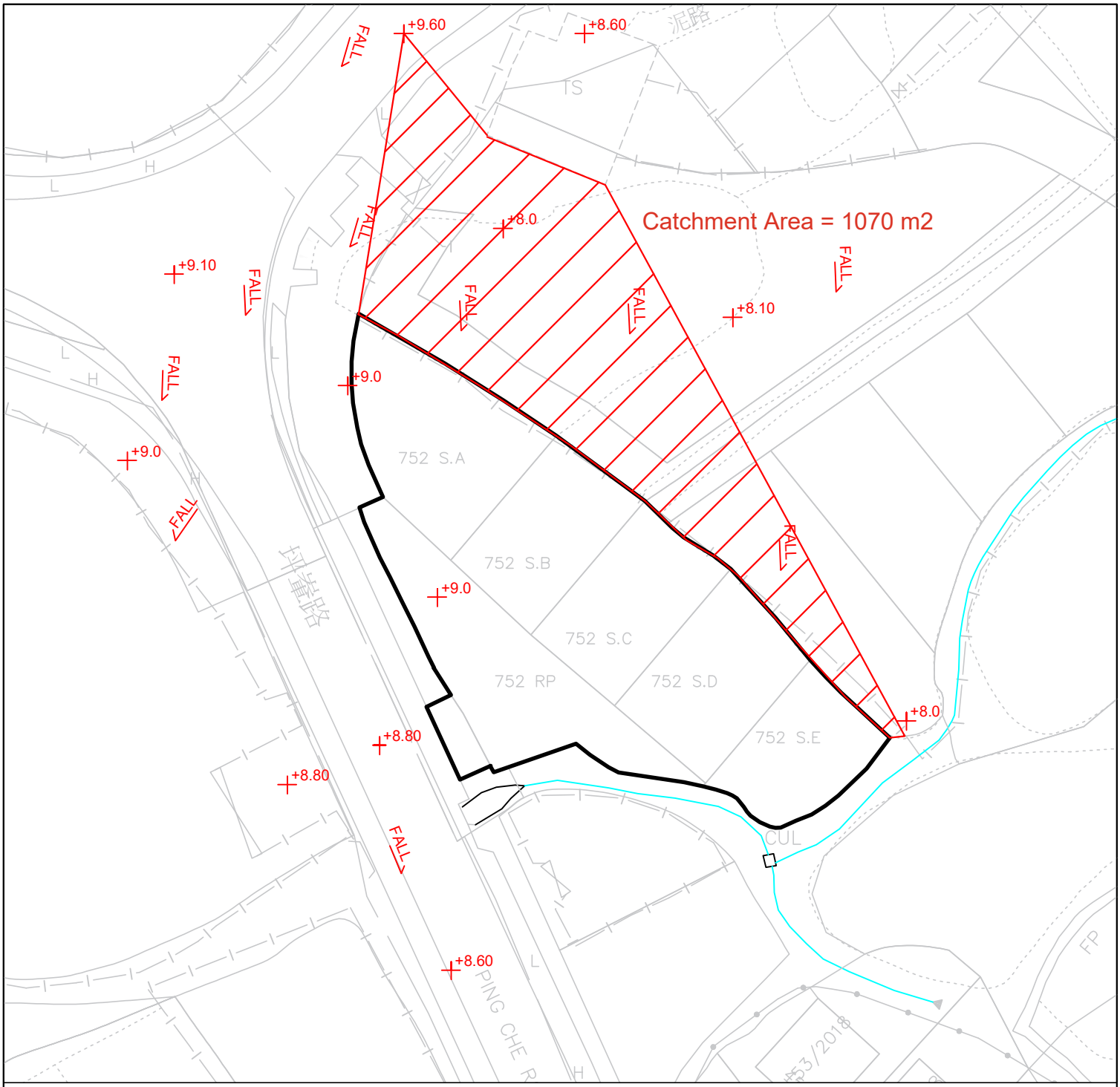


Connection Detail
(CP 14 to Existing streamcourse)

YING SHING (HOPEWELL) ENGINEERING CO.LTD.

Not to Scale

Annex 8



Catchment Area Plan
(Proposed Peripheral Channel)

1:1000

Annex 9

2. Design Considerations

2.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 1** below:

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 50 years return period is adopted for drainage design.

2.2 Referring to the location plan and the existing ground level, the considered effective catchment area of surface runoff includes: the area of the subject lots and the adjacent area with higher cover level (including uphill and carriageway).

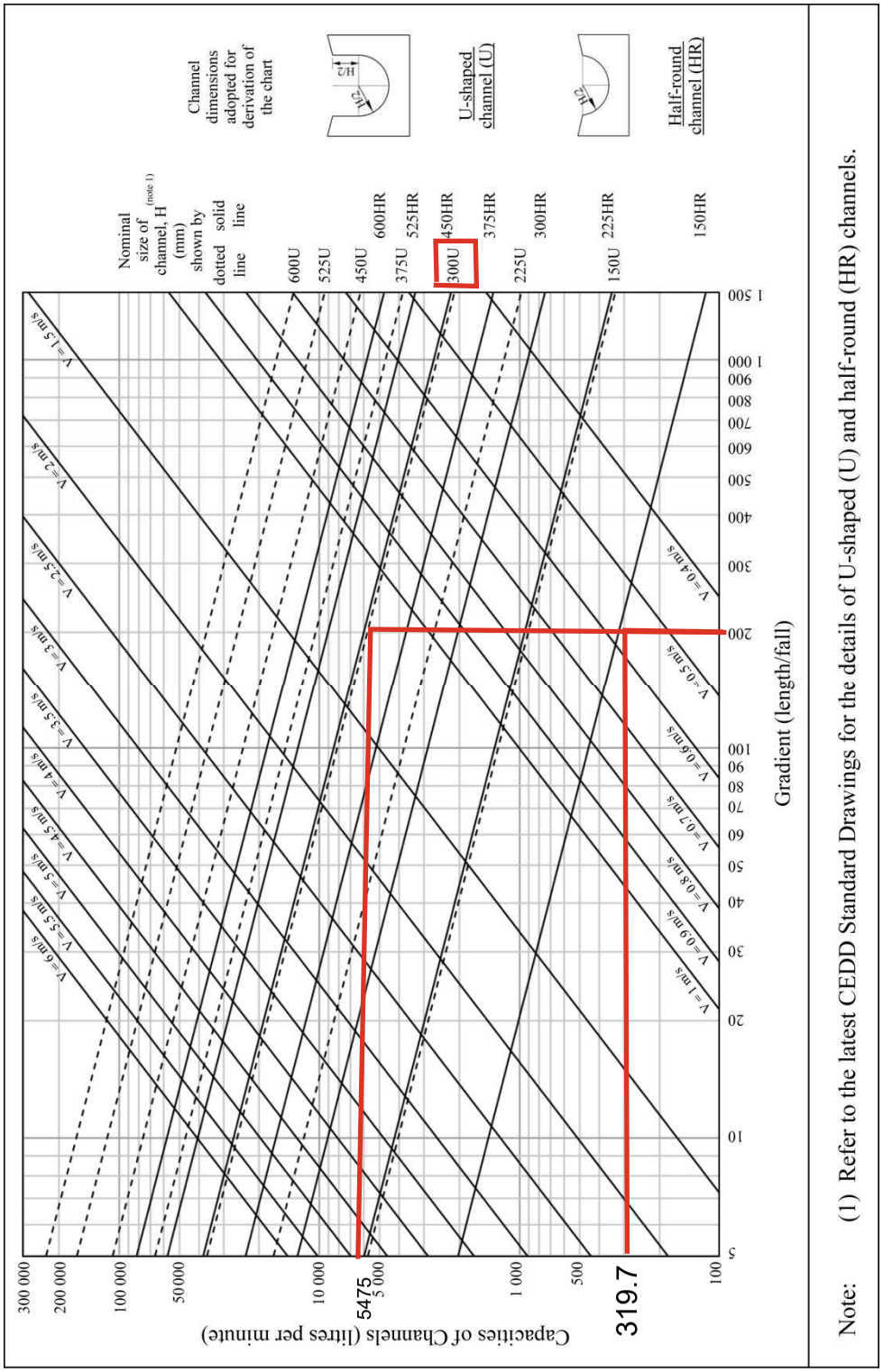


Figure 1 - Chart for the rapid design of U-shaped and half-round channels up to 600 mm

Note: (1) Refer to the latest CEDD Standard Drawings for the details of U-shaped (U) and half-round (HR) channels.

Outside Catchment Area, Area = 225.0 m² (C = 0.250)

THE SITE, Area = 30.0 m² (C = 0.950)

Calculation of Design Runoff - CP13 to CP15

$$\Sigma Q = \Sigma\ 0.278CiA$$

A = 225.0 + 30.0 = 255.0 m²

= 0.0002550 km²

H = 100 × (9.100 − 8.800) / 70.000 = 0.429 m/100m

$$t = 0.14465 \times L / (H^{0.2} \times A^{0.1})$$

= 0.14465 × 70.000 / (0.429^0.2 × 255.0^0.1)

= 6.892 min

$$i = \text{multiplier} \times a / (t + b)^{c}$$

= 1.111 × 474.600 / (6.892 + 2.900)^0.371

= 226.165 mm/hr

(50 years return period; North District, SDM Table 3d, 2024; multiplier = 1.111)

Q = 0.278 × 226.165 × (0.250 × 225.0 + 0.950 × 30.0) / 1,000,000

= 0.005329 m³/s

= 319.7 L/min

Allowable pipe capacity = 7,789.0 L/min; velocity = 1.469 m/s

D = 375 mm; gradient = 1:200; ks = 0.150 mm; capacity factor = 0.800

Provide 375 mm dia. pipe (1:200) is OK.

Check Concrete Pipe - CP13 to CP15

375 mm dia. pipe (Annex 1)

$$V = -\sqrt{(8gDS)}\ \log_{10}[ks/(3.7D) + 2.51\nu/(D\sqrt{(2gDS)})]$$

where:

V	=	1.469	m/s
g	=	9.810	m/s²
D	=	0.375	m
ks	=	0.000150	m
ν	=	1.14×10^{-6}	m²/s
S	=	0.0050	1:200
Re	=	483,295	

Therefore, full-pipe mean velocity = 1.469 m/s

$Qallow = 0.800 \times V \times \pi D^2/4$

Allowable pipe capacity	=	0.129816	m³/s
Allowable pipe capacity	**=**	**7,789.0**	**L/min**
Design runoff	**=**	**319.7**	**L/min**
Capacity utilization	=	4.1%	

Provide 375 mm dia. pipe (1:200) is OK.

Check 300 mm U-Channel - CP13 to CP15

Semicircular base with vertical sides; design water depth = H

$$V = (1/n) \times R^{(2/3)} \times S^{(1/2)}$$

where:

H	=	0.300	m
Water depth y	=	0.300	m
n	=	0.013	
S	=	0.0050	1:200
A	=	0.080343	m^2
P	=	0.771239	m
R	=	0.104174	m

$A = \pi H^2/8 + H^2/2$; $P = \pi H/2 + H$; $R = A/P$

$Q = A \times V = 0.080343 \times 1.204243$

Channel capacity	=	0.096752	m^3/s
Channel capacity	**=**	**5,805.1**	**L/min**
Design runoff	**=**	**319.7**	**L/min**
Capacity utilization	=	5.5%	

Provide 300U (1:200) is OK.

Outside Catchment Area, Area = 327.3 m² (C = 0.250)

THE SITE, Area = 1,100.0 m² (C = 0.950)

Calculation of Design Runoff - CP4

$$\Sigma Q = \Sigma\ 0.278CiA$$

A = 327.3 + 1,100.0 = 1,427.3 m²

= 0.0014273 km²

H = 100 × (9.100 − 8.900) / 27.500 = 0.727 m/100m

$$t = 0.14465 \times L / (H^{0.2} \times A^{0.1})$$

= 0.14465 × 27.500 / (0.727^0.2 × 1,427.3^0.1)

= 2.051 min

$$i = \text{multiplier} \times a / (t + b)^{c}$$

= 1.111 × 474.600 / (2.051 + 2.900)^0.371

= 291.291 mm/hr

(50 years return period; North District, SDM Table 3d, 2024; multiplier = 1.111)

Q = 0.278 × 291.291 × (0.250 × 327.3 + 0.950 × 1100.0) / 1,000,000

= 0.091249 m³/s

= 5,474.9 L/min

Allowable pipe capacity = 7,789.0 L/min; velocity = 1.469 m/s

D = 375 mm; gradient = 1:200; ks = 0.150 mm; capacity factor = 0.800

Provide 375 mm dia. pipe (1:200) is OK.

Check Concrete Pipe - CP4

375 mm dia. pipe (Annex 1)

$$V = -\sqrt{(8gDS)}\ \log_{10}[ks/(3.7D) + 2.51v/(D\sqrt{(2gDS)})]$$

where:

V	=	1.469	m/s
g	=	9.810	m/s²
D	=	0.375	m
ks	=	0.000150	m
ν	=	1.14×10^{-6}	m²/s
S	=	0.0050	1:200
Re	=	483,295	

Therefore, full-pipe mean velocity = 1.469 m/s

Qallow = 0.800 × V × πD²/4

Allowable pipe capacity	=	0.129816	m³/s
Allowable pipe capacity	**=**	**7,789.0**	**L/min**
Design runoff	**=**	**5,474.9**	**L/min**
Capacity utilization	=	70.3%	

Provide 375 mm dia. pipe (1:200) is OK.

Check 300 mm U-Channel - CP4

Semicircular base with vertical sides; design water depth = H

$$V = (1/n) \times R^{(2/3)} \times S^{(1/2)}$$

where:

H	=	0.300	m
Water depth y	=	0.300	m
n	=	0.013	
S	=	0.0050	1:200
A	=	0.080343	m^2
P	=	0.771239	m
R	=	0.104174	m

$A = \pi H^2/8 + H^2/2$; $P = \pi H/2 + H$; $R = A/P$

$Q = A \times V = 0.080343 \times 1.204243$

Channel capacity	=	0.096752	m^3/s
Channel capacity	**=**	**5,805.1**	**L/min**
Design runoff	**=**	**5,474.9**	**L/min**
Capacity utilization	=	94.3%	

Provide 300U (1:200) is OK.

Outside Catchment Area, Area = 1,070.0 m² (C = 0.250)

THE SITE, Area = 0.0 m² (C = 0.950)

Calculation of Design Runoff - CP14

$$\Sigma Q = \Sigma\ 0.278CiA$$

A = 1,070.0 + 0.0 = 1,070.0 m²

= 0.0010700 km²

H = 100 × (9.600 − 8.000) / 27.500 = 5.818 m/100m

$$t = 0.14465 \times L / (H^{0.2} \times A^{0.1})$$

= 0.14465 × 27.500 / (5.818^0.2 × 1,070.0^0.1)

= 1.392 min

$$i = \text{multiplier} \times a / (t + b)^{c}$$

= 1.111 × 474.600 / (1.392 + 2.900)^0.371

= 307.122 mm/hr

(50 years return period; North District, SDM Table 3d, 2024; multiplier = 1.111)

Q = 0.278 × 307.122 × (0.250 × 1070.0 + 0.950 × 0.0) / 1,000,000

= 0.022839 m³/s

= 1,370.3 L/min

Allowable pipe capacity = 7,789.0 L/min; velocity = 1.469 m/s

D = 375 mm; gradient = 1:200; ks = 0.150 mm; capacity factor = 0.800

Provide 375 mm dia. pipe (1:200) is OK.

Check Concrete Pipe - CP14

375 mm dia. pipe (Annex 1)

$$V = -\sqrt{(8gDS)}\ \log_{10}[ks/(3.7D) + 2.51\nu/(D\sqrt{(2gDS)})]$$

where:

V	=	1.469	m/s
g	=	9.810	m/s²
D	=	0.375	m
ks	=	0.000150	m
ν	=	1.14×10^{-6}	m²/s
S	=	0.0050	1:200
Re	=	483,295	

Therefore, full-pipe mean velocity = 1.469 m/s

$Qallow = 0.800 \times V \times \pi D^2/4$

Allowable pipe capacity	=	0.129816	m³/s
Allowable pipe capacity	**=**	**7,789.0**	**L/min**
Design runoff	**=**	**1,370.3**	**L/min**
Capacity utilization	=	17.6%	

Provide 375 mm dia. pipe (1:200) is OK.

Check 300 mm U-Channel - CP14

Semicircular base with vertical sides; design water depth = H

$$V = (1/n) \times R^{(2/3)} \times S^{(1/2)}$$

where:

H	=	0.300	m
Water depth y	=	0.300	m
n	=	0.013	
S	=	0.0050	1:200
A	=	0.080343	m²
P	=	0.771239	m
R	=	0.104174	m

$A = \pi H^2/8 + H^2/2$; $P = \pi H/2 + H$; $R = A/P$

$Q = A \times V = 0.080343 \times 1.204243$

Channel capacity	=	0.096752	m³/s
Channel capacity	**=**	**5,805.1**	**L/min**
Design runoff	**=**	**1,370.3**	**L/min**
Capacity utilization	=	23.6%	

Provide 300U (1:200) is OK.

Check Widened Streamcourse by Manning Equation

Rectangular section - proposed widening to 1.300 m × 0.700 m

$$V = (1/n) \times R^{(2/3)} \times S^{(1/2)}$$

where:

Clear width b	=	1.300	m
Total depth D	=	0.700	m
Freeboard F	=	0.500	m
Water depth y = D − F	=	0.200	m
Manning n	=	0.040	
S	=	0.0050	1:200
Cross-section A	=	0.260	m²

A = b × y = 0.260 m²; P = b + 2y = 1.700 m

R = A/P = 0.152941 m; V = 0.505562 m/s

Channel capacity	=	0.131446	m³/s
Channel capacity	**=**	**7,886.8**	**L/min**

Qcapacity = 0.131446 m³/s = 7,886.8 L/min

Widened Streamcourse Flow Check

Aggregation method: arithmetic sum of individual branch peaks

Qcomparison = ΣQbranch

Case assumptions: common downstream section; catchment overlap deduction = 0 m²

QCP4 + QCP14

Q_1 = 0.091249 + 0.022839 = 0.114088 m³/s

= 6,845.3 L/min

7,886.8 > 6,845.3 L/min OK for this peak-sum case

QCP4 + QCP14 + QCP15

Q_2 = 0.091249 + 0.022839 + 0.005329 = 0.119417 m³/s

= 7,165.0 L/min

7,886.8 > 7,165.0 L/min OK for this peak-sum case

Widened Streamcourse Water Depth Calculation

Rectangular section; uniform flow; b = 1.300 m; n = 0.040; S = 1:200

Q = (by/n) × [by/(b + 2y)]^(2/3) × S^(1/2)

Flow cases: branch peak sums from Widened Streamcourse Flow Check

CP4 + CP14

0.114088 = (1.3y/0.040) × [1.3y/(1.3 + 2y)]^(2/3) × 0.005^(1/2)

y_1 = 0.182154 m

Freeboard = 0.700 − 0.182154 = 0.517846 m > 0.500 m

CP4 + CP14 + CP15

0.119417 = (1.3y/0.040) × [1.3y/(1.3 + 2y)]^(2/3) × 0.005^(1/2)

y_2 = 0.187712 m

Freeboard = 0.700 − 0.187712 = 0.512288 m > 0.500 m

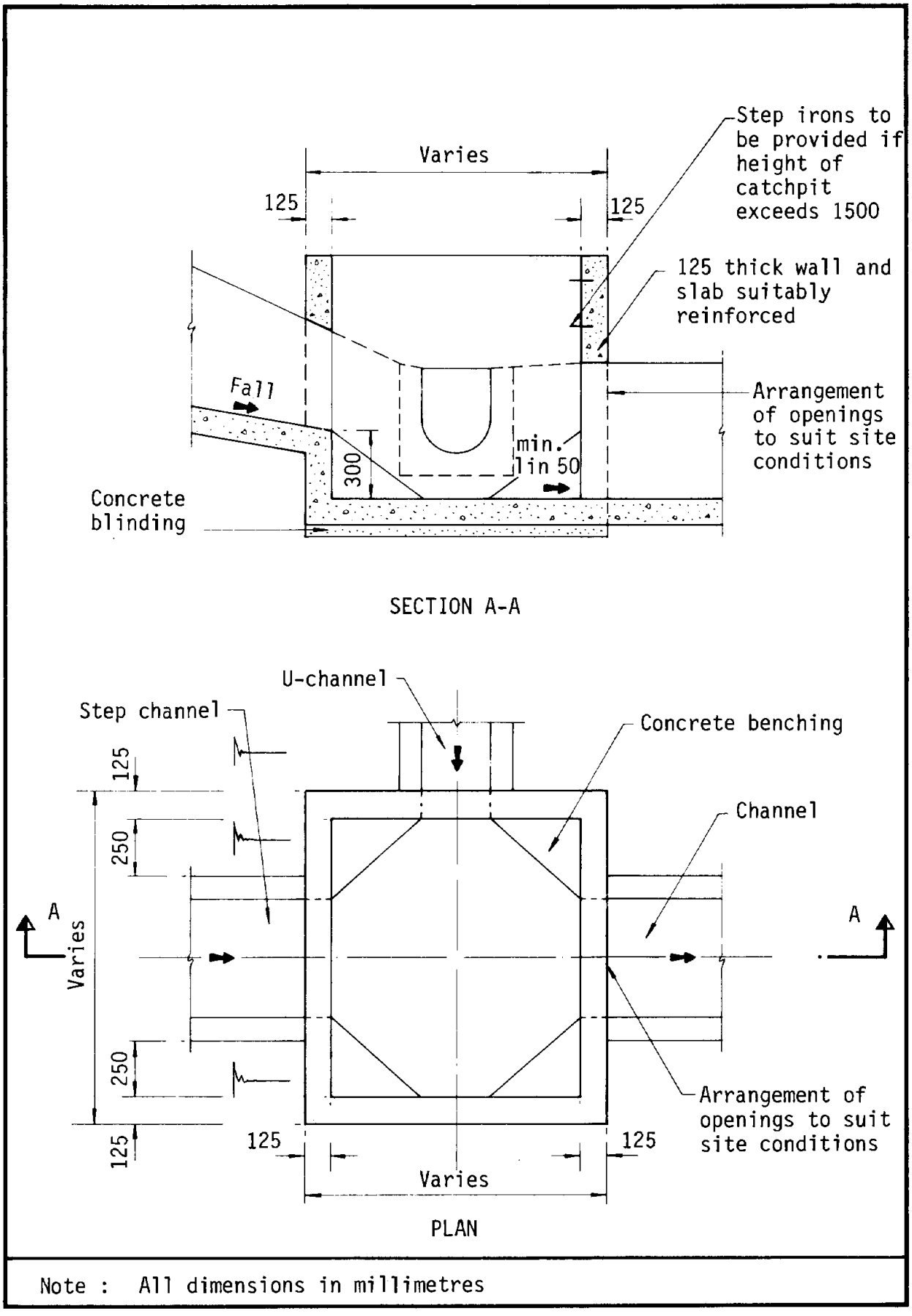
Total depth corresponding to 0.500 m freeboard:

D_1 = 0.182154 + 0.500 = 0.682154 m

D_2 = 0.187712 + 0.500 = 0.687712 m

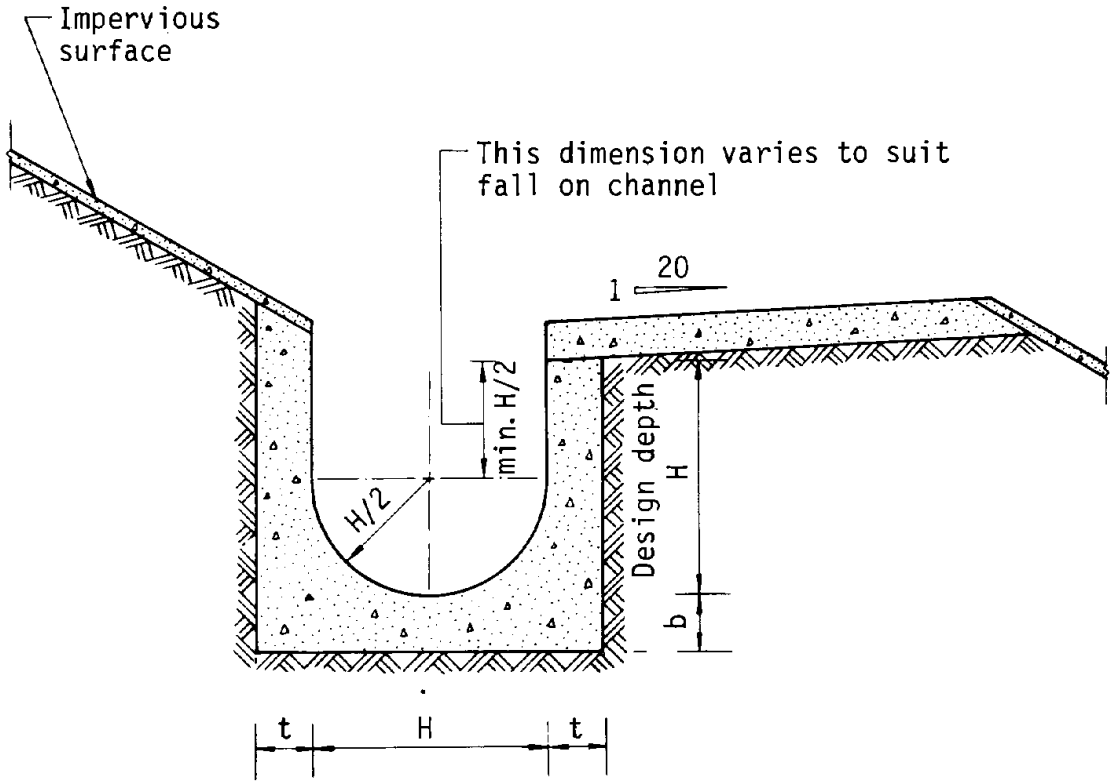
Original adopted total depth = 0.700 m; adopted water depth = 0.200 m

OK



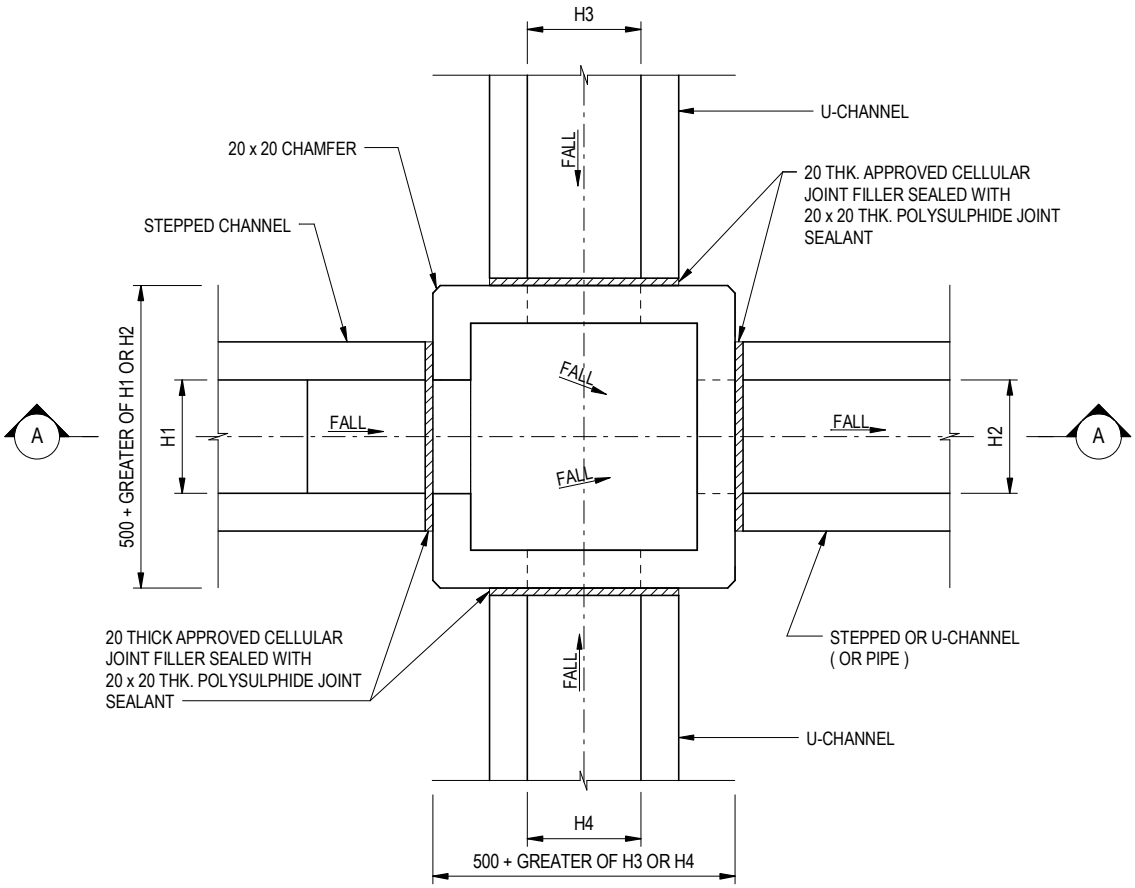
Note : All dimensions in millimetres

Figure 8.10 - Typical Details of Catchpits



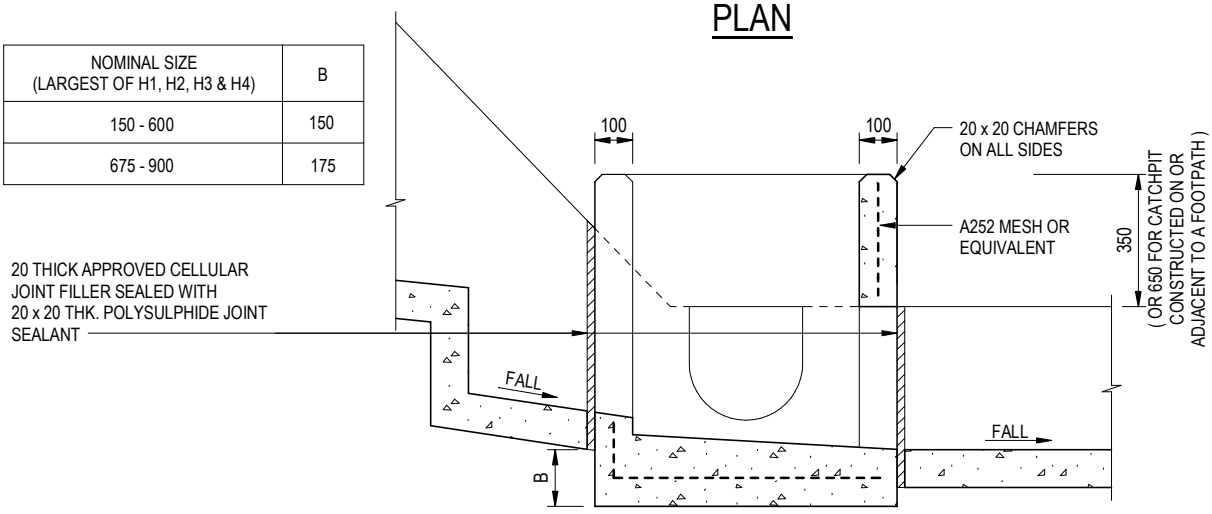
Dimensions of U - channel		
Nominal size of channel H (mm)	Thickness t (mm)	Thickness b (mm)
225 to 600	150	150
675 to 1200	175	225

Figure 8.11 - Typical U-channel Details

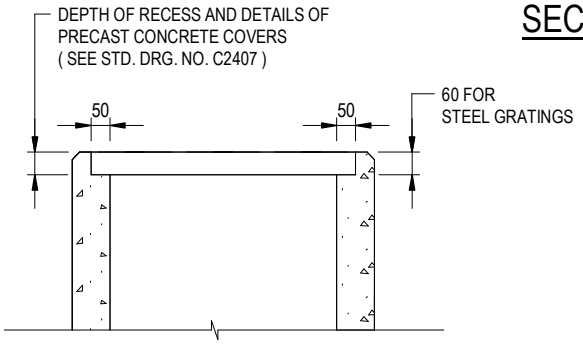


PLAN

NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
150 - 600	150
675 - 900	175



SECTION A - A



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. REFER TO SHEET 5 FOR OTHER NOTES.

REF.	REVISION	SIGNATURE	DATE
A	BIM OBJECT REVISION.	Original Signed	12.2025
–	FORMER DRG. NO. C2405J.	Original Signed	03.2015

STANDARD CATCHPIT DETAILS (SHEET 1 OF 5)

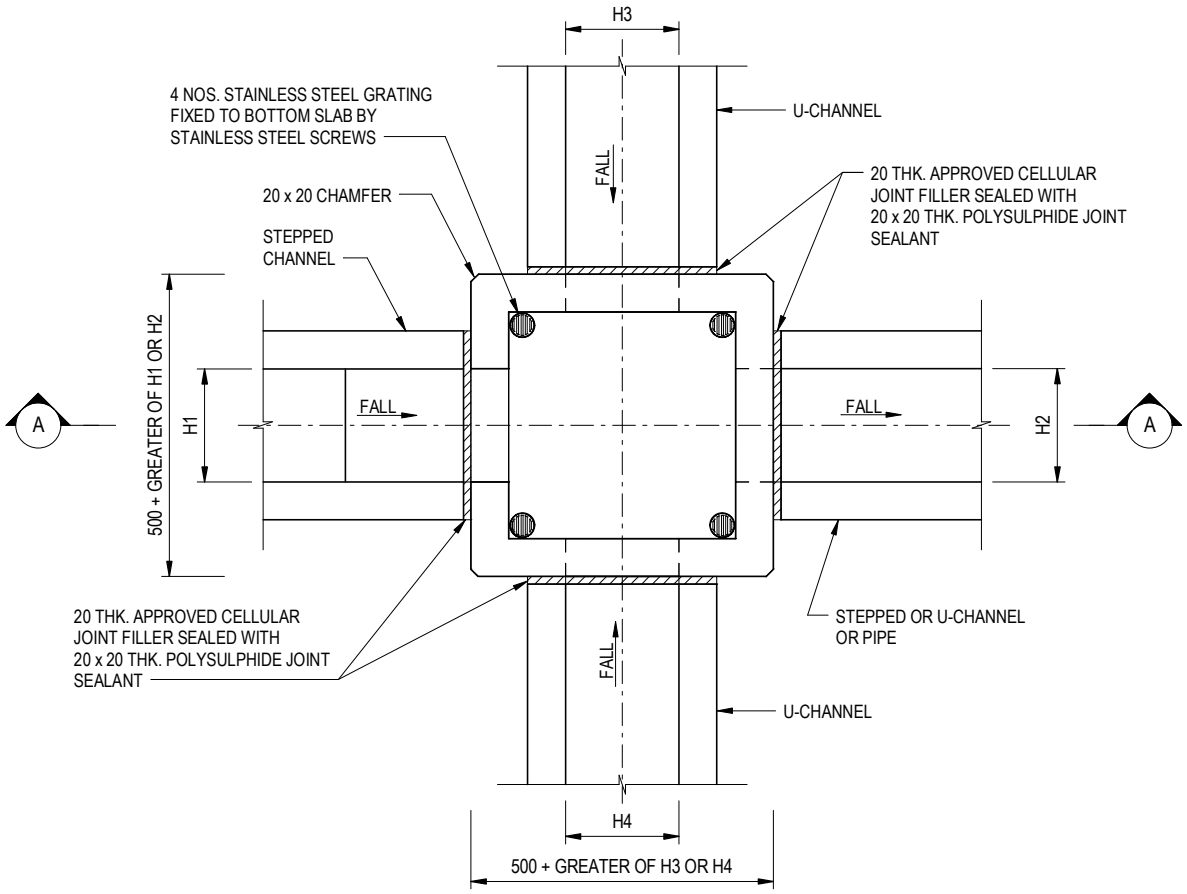
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1 : 20

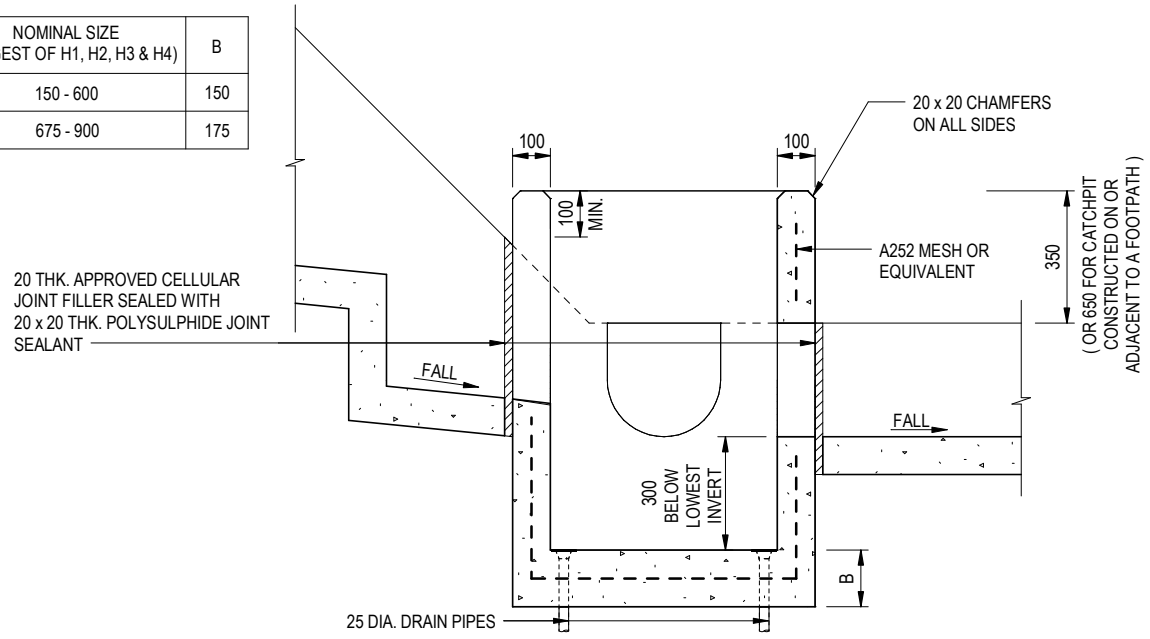
DATE JAN 1991

DRAWING NO. C2405 /1A



PLAN

NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
150 - 600	150
675 - 900	175



SECTION A - A

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. REFER TO SHEET 2 FOR OTHER NOTES.

REF.	REVISION	SIGNATURE	DATE
A	BIM OBJECT REVISION.	Original Signed	12.2025
–	FORMER DRG. NO. C2406J.	Original Signed	03.2015

CATCHPIT WITH TRAP (SHEET 1 OF 2)

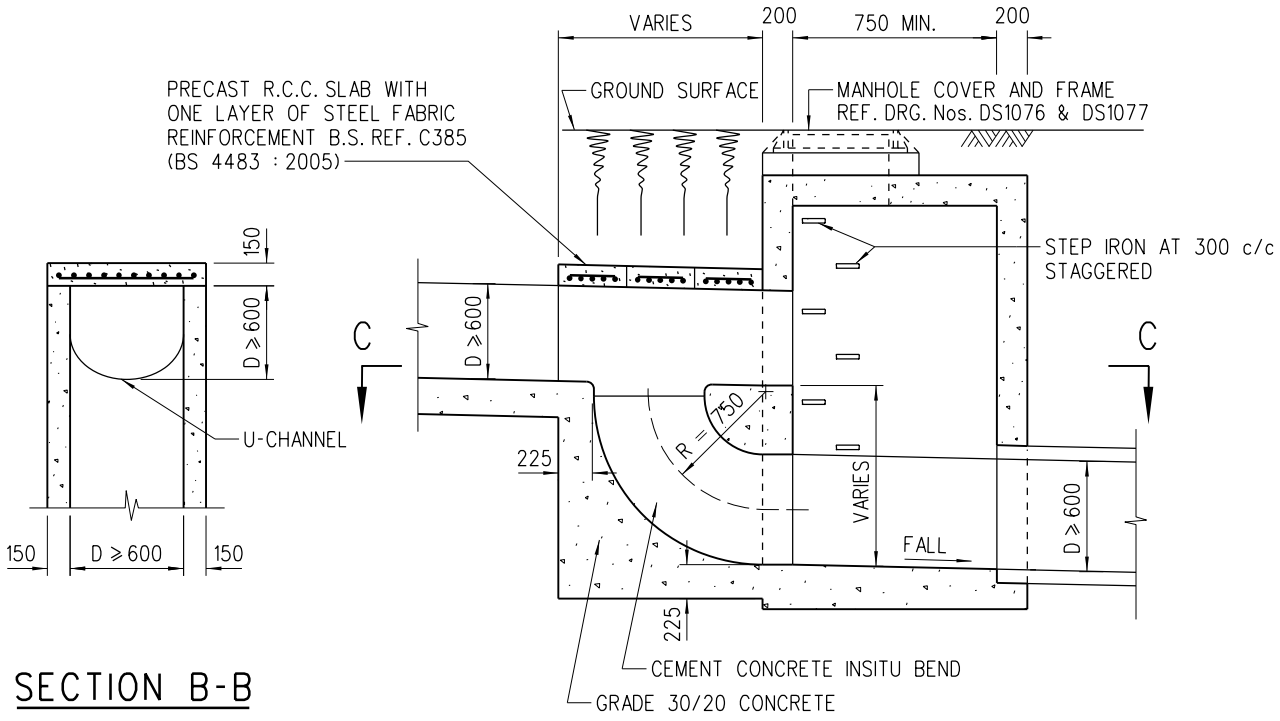
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CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1 : 20

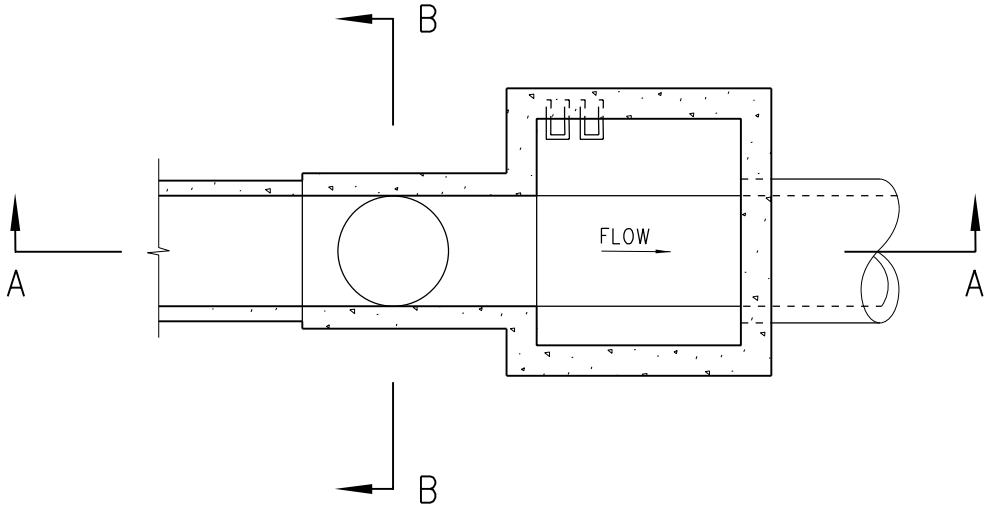
DATE JAN 1991

DRAWING NO. C2406 /1A



SECTION B-B

SECTION A-A



SECTION PLAN C-C

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. RECESS WITH SQUARE STEEL ROD SHALL BE PROVIDED AT TOP OF MANHOLE CHAMBER FOR INSTALLING MONITORING DEVICE(S). DETAILS REFER TO DSD STANDARD DRAWING NO. DS 1099.

REV.	DESCRIPTION	SIGNATURE	DATE
D	NOTE 2 ADDED	ORIGINAL SIGNED	2.8.2022
C	MANHOLE COVER & FRAME UPDATED	ORIGINAL SIGNED	10.5.2010
B	REDRAWN BY CAD	ORIGINAL SIGNED	8.8.2001
A	GENERAL REVIEW	ORIGINAL SIGNED	2.2.2001

BACKDROP MANHOLE TYPE 1

DRAINAGE SERVICES DEPARTMENT

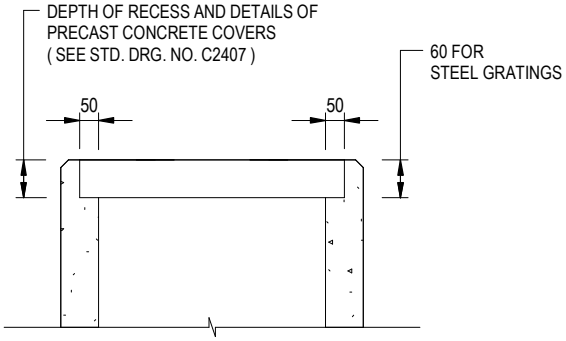
REFERENCE

DRAWING No.

DS 1018D

SCALE

1 : 50



ALTERNATIVE TOP SECTION
FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.

2. ALL CONCRETE SHALL BE GRADE 20 / 20.

3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.

4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.

5. CONCRETE TO BE COLOURED AS SPECIFIED.

6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.

7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.

8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.

9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.

10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.

11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.

12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

REF.	REVISION	SIGNATURE	DATE
B	BIM OBJECT REVISION.	Original Signed	12.2025
A	MINOR AMENDMENT.	Original Signed	04.2016
–	FORMER DRG. NO. C2406J.	Original Signed	03.2015

CATCHPIT WITH TRAP
(SHEET 2 OF 2)

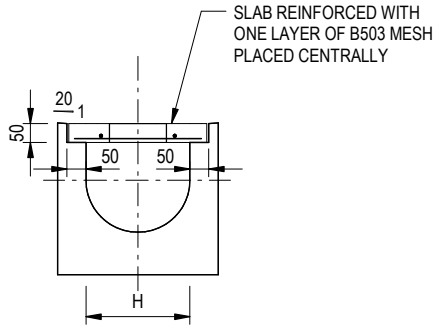
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

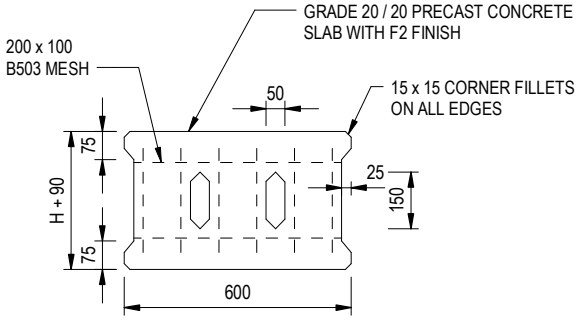
SCALE 1 : 20

DATE JAN 1991

DRAWING NO.
C2406 /2B



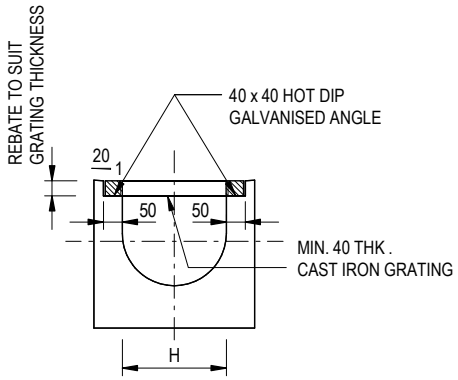
TYPICAL SECTION



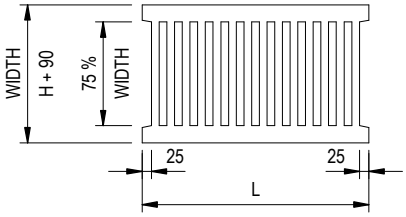
PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)



TYPICAL SECTION



L = 600 mm FOR H ≤ 375 mm
L = 400 mm FOR H > 375 mm

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.

2. H = NOMINAL CHANNEL SIZE.

3. ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.

4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

REF.	REVISION	SIGNATURE	DATE
F	BIM OBJECT REVISION.	Original Signed	12.2025
E	NOTES 3 & 4 AMENDED.	Original Signed	12.2014
D	NOTE 4 ADDED.	Original Signed	06.2008
C	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
B	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
A	CAST IRON GRATING AMENDED.	Original Signed	12.2002

COVER SLAB AND CAST IRON GRATING FOR CHANNELS

CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1 : 20

DATE JAN 1991

DRAWING NO. C2412F