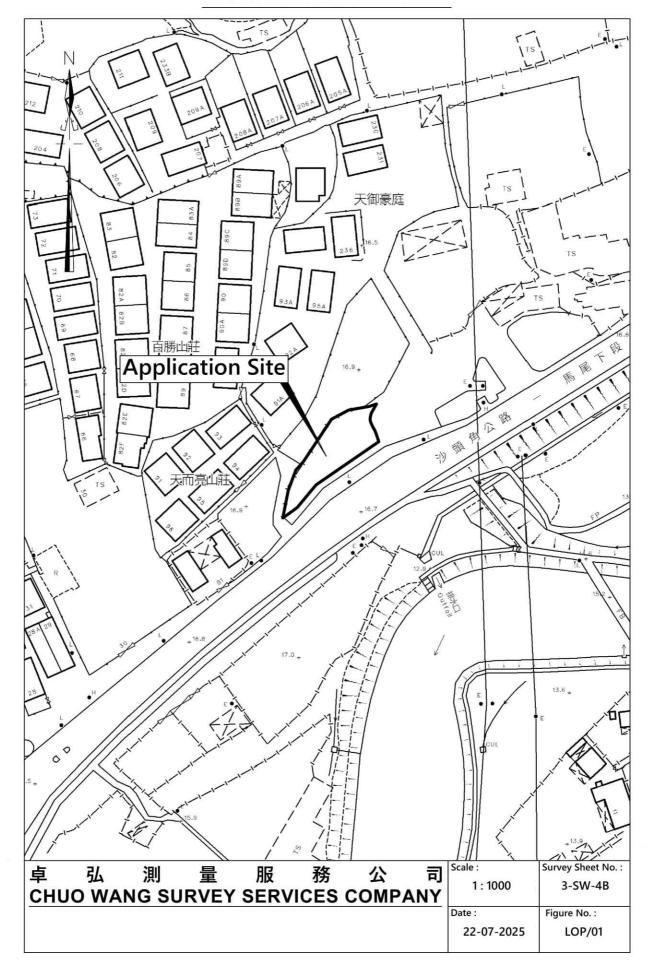
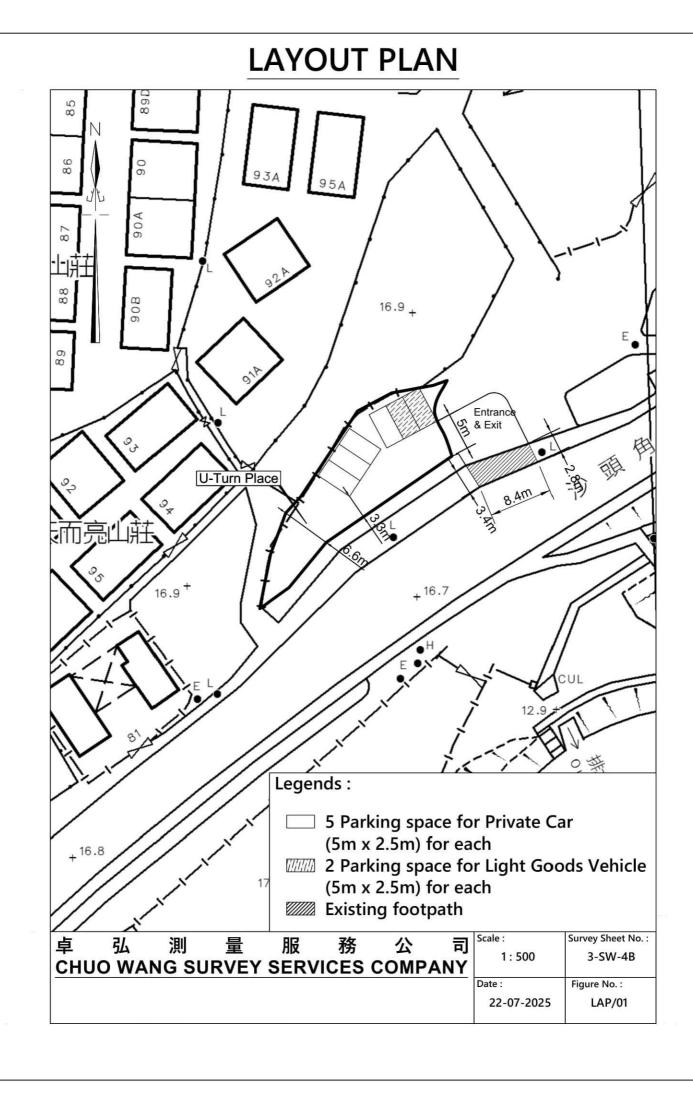
LOCATION PLAN





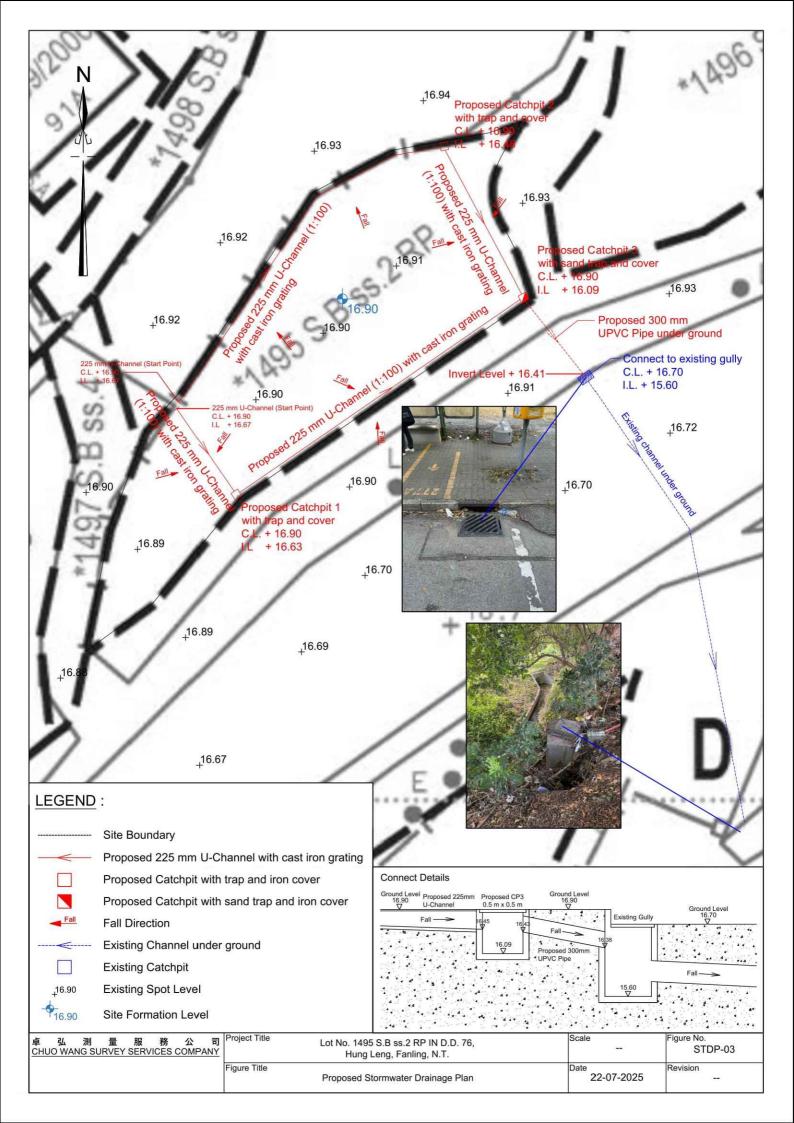
Design of
225 mm U - Channel
for
Lot 1495 S.B ss.2 RP
for use as a Temporary Private Car Park

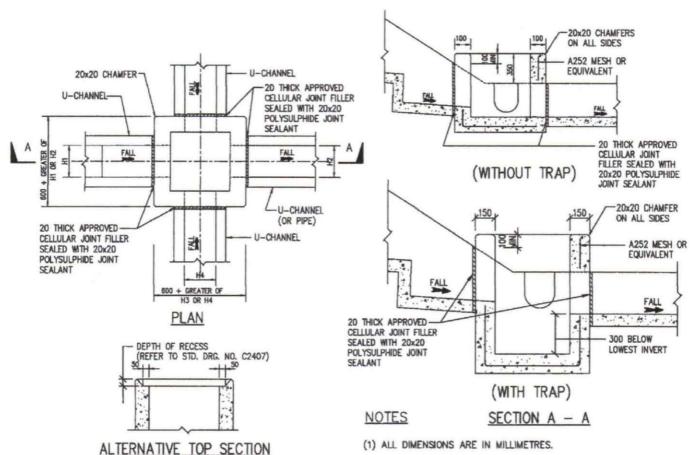
卓 弘 測 量 服 務 公 司	Project Title Lot No. 1495 S.B ss.2 RP IN D.D. 76,	Scale	Figure No.
CHUO WANG SURVEY SERVICES COMPANY	Hung Leng, Fanling, N.T.		STDP-01
	Figure Title Proposed Stormwater Drainage Plan	Date 22-07-2025	Revision

Revision History

Revision	Submission	Reply Date	Section/
	Date		Description
0	July-2025		Initial issue

卓 弘 測 量 服 務 公 司	Project Title Lot No. 1495 S.B ss.2 RP IN D.D. 76,	Scale	Figure No.
CHUO WANG SURVEY SERVICES COMPANY	Hung Leng, Fanling, N.T.		STDP-02
	Figure Title Proposed Stormwater Drainage Plan	Date 22-07-2025	Revision

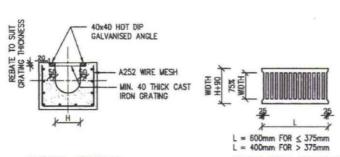




STANDARD CATCHPIT DETAILS (ACCORDING TO CEDD'S DRAWING NO. C24051 & 24061)

FOR PRECAST CONCRETE COVER

- (1) ALL DIMENSIONS ARE IN MILLIMETRES.
- (2) SIZE DEPTH ≮ 750 D 3B 4.80 as7 has F-0.5 > 4B LENGTH : L =
- (3) GRADED STONE FILTER SHALL BE CRUSHER RUN GRANITE AGGREGATE.
- (4) THE SANDTRAP SHALL BE REGULARLY DESILTED TO AVOID BLOCKAGE.



TYPICAL SECTION

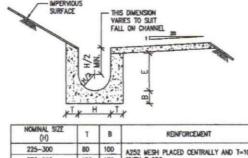
CAST IRON_GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

(ACCORDING TO CEDD'S DRAWING NO. C2412E)



NOMINAL SIZE (H)	T	В	REINFORCEMENT			
225-300	80	100	A252 MESH PLACED CENTRALLY AND T=100			
375-800	100	150	WHEN EX650			
875-900	100	150	A252 WESH PLACED CENTRALLY			

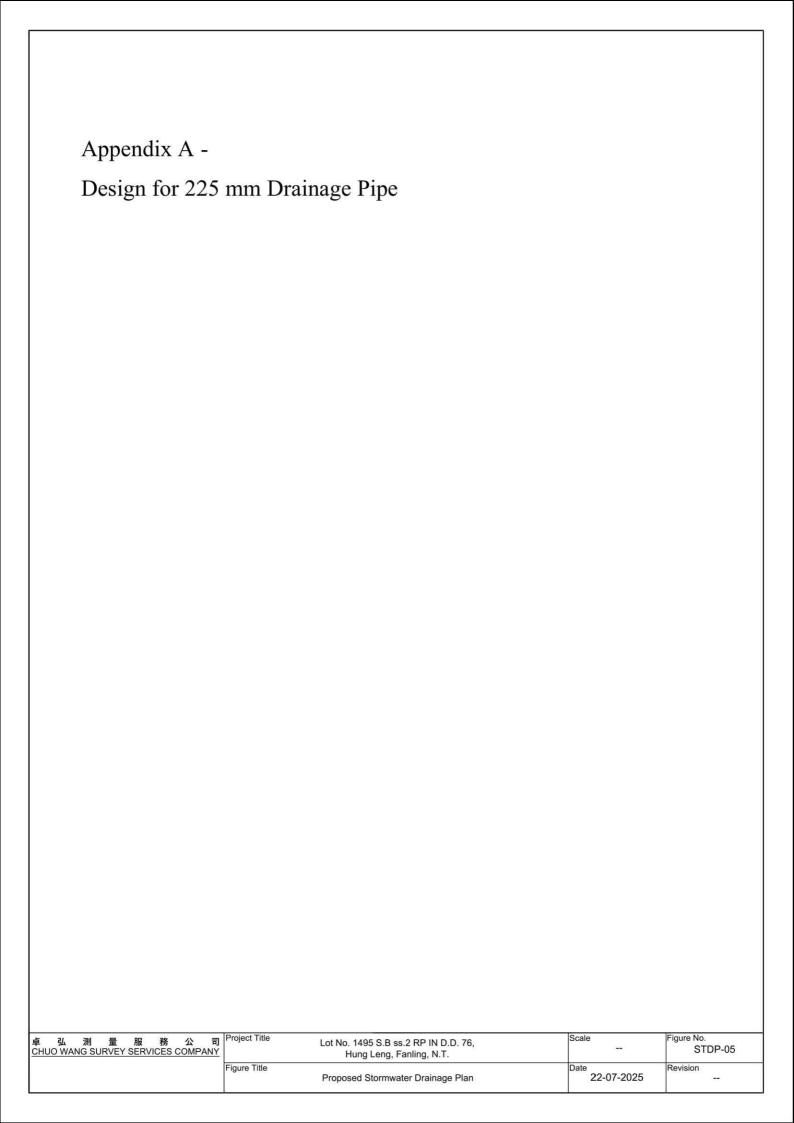
TYPICAL U-CHANNEL DETAILS (ACCORDING TO CEDD'S DRAWING NO. C2410G)

N.T.S.

NOTES FOR U-CHANNEL

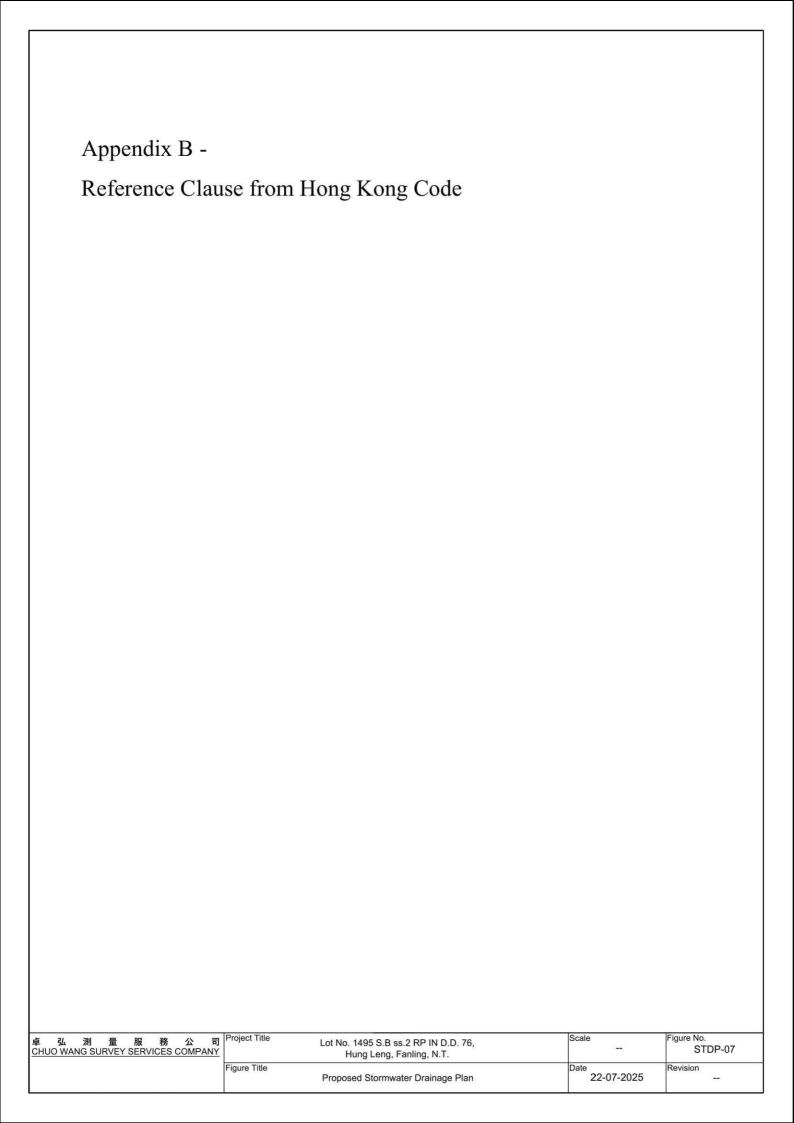
- THE COVER OF PROPOSED U-CHANNEL SHALL BE FLUSH WITH THE PATH SURFACE AND ANY HOLE IN SUCH COVER SHALL NOT EXCEED 20mm IN ONE
- 2. CAST IRON GRATINGS TO BE USED SHALL BE COMPLIANCE WITH BS 437: 2008.

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	Figure Title Proposed Stormwater Drainage Plan		Revision



			Project		Cor	nputed by	Checked by	Sheet r
			22 (20)					١.
			D.D. 76 Lot Subject	1495 S.B s	s.2 RP Dat	0	Date	1
			1.69	flow for this		е	Date	
			Design flow for this Site					
Design flow for	or Part Δ							
Design now it	AT LUICA							
Return Period	: 200 Years							
Assume flow v	elocity: (m/s)	1.5						
	Return Period T	(years)	Rainfull Inte	nsity:	$I = a / (T_c$	+ b) ^ c		
	200	(from Table 3 of DS	D Practice Not	te No. 1/20	017)			
а	1074.8							
b	12.47							
С	0.523							
5-1	! M!							
	nal Method: 0.14465*L / (H^0.2)(A	\^O 1\	Q = 0.278*C	*I*A				
Tf =		(0.1)	Q = 0.276 C	, , ,				
	T _o +T _f							
Catchment Are								
Daterinient Are	aurana							
A	rea(m²)	107.75	$T_o =$	3.62	min			
	-value	0.8	Tf =	0.47	min			
	Elev. (mPD)	16.63	Tc =	4.08	min			
	Elev.(mPD)	16.09	l =	247.66	mm/hr			
	w path,L (m)	41.98	Q=		m ³ /s)			
	H (m in 100m)	1.3	Q=	356	(l/min)			
Total Length	of Channel, L (m)	41.98						
Flow capacity	checking							
.o.r oupdony	os.aria							
	Design flow rate =	356 l/min						
	Flow capacity of							
	proposed 225mm	1000 1/1-		05	C I/mir			
	open channel =	1008 I/min	>	35	6 I/min			
			ОК					
			OK					

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Estimated Flow Capacity of the proposed 225mm dia. Open channel

According to Colebrook-White Equation in Table 12 of Stormwater Drainage Manual)

Length = 41.98 m Inlet I.L. = 16.63 mPD Outlet I.L. = 16.09 mPD

Gradient = 0.01286 (for conservative; fall is taken 1:100)

k_s = 14.0000 mm (Table 14 of Stormwater Drainage Mannual) normal classification

Colebrook-White $\overline{V} = -\sqrt{32gRS_f} \log \left[\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$

		Drain					
Drain Size	Gradient	Capacity	Velolcity	Hydraulic.	Coeff.	Viscosity	Roughness
Dia	S_f	Area x \overline{V}	\overline{V}	radius R	$\sqrt{32gRS_f}$	V	1è
	Z.j	Area x V	V	(D/4 for circular pipe)	√32gk3 _f	V	k _s
(mm)		(m^3/s)	(m/s)	(m)		(m^2/s)	(m)
225	0.01286	0.017	0.845	0.0563	0.48	1.14E-06	0.0140

Pipe capacity = $0.017 \text{ m}^3/\text{s}$ = 1008 l/min

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routing through drainage channels. The same consideration shall also be applied when ground gradients vary greatly within the catchment.

(b) Runoff Coefficient. C is the least precisely known variable in the Rational Method. Proper selection of the runoff coefficient requires judgement and experience on the part of the designer. The value of C depends on the impermeability, slope and retention characteristics of the ground surface. It also depends on the characteristics and conditions of the soil, vegetation cover, the duration and intensity of rainfall, and the antecedent moisture conditions, etc. In Hong Kong, a value of C = 1.0 is commonly used in developed urban areas.

In less developed areas, the following C values may be used but it should be checked that the pertinent catchment area will not be changed to a developed area in the foreseeable future. Particular care should be taken when choosing a C value for unpaved surface as the uncertainties and variability of surface characteristics associated with this type of ground are known to be large. It is important for designer to investigate and ascertain the ground conditions before adopting an appropriate runoff coefficient. Designers may consider it appropriate to adopt a more conservative approach in estimation of C values for smaller catchments where any consequent increase in cost may not be significant. However, for larger catchments, the designers should exercise due care in the selection of appropriate C values in order to ensure that the design would be fully cost-effective.

Surface Characteristics	Runoff coefficient, C*
Asphalt	0.70 - 0.95
Concrete	0.80 - 0.95
Brick	0.70 - 0.85
Grassland (heavy soil**)	
Flat	0.13 - 0.25
Steep	0.25 - 0.35
Grassland (sandy soil)	
Flat	0.05 - 0.15
Steep	0.15 - 0.20

^{*} For steep natural slopes or areas where a shallow soil surface is underlain by an impervious rock layer, a higher C value of 0.4 - 0.9 may be applicable.

- (c) Rainfall intensity. i is the average rainfall intensity selected on the basis of the design rainfall duration and return period. The design rainfall duration is taken as the time of concentration, t_c. The Intensity-Duration-Frequency Relationship is given in Section 4.3.2.
- (d) Time of concentration. t_c is the time for a drop of water to flow from the remotest point in the catchment to its outlet. For an urban drainage system,

$$t_c = t_o + t_f t_f = \sum_{j=1}^n \frac{L_j}{V_j}$$

where t_o = inlet time (time taken for flow from the remotest point to reach the most upstream point of the urban drainage

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^{**} Heavy soil refers to fine grain soil composed largely of silt and clay

Return Period T (years)	2	5	10	20	50	100	200	500	1000
a	499.8	480.2	471.9	463.6	451.3	440.8	429.5	414.0	402.1
b	4.26	3.36	3.02	2.76	2.46	2.26	2.05	1.77	1.55
С	0.494	0.429	0.397	0.369	0.337	0.316	0.295	0.269	0.251

Table 3b - Storm Constants for Different Return Periods of Tai Mo Shan Area

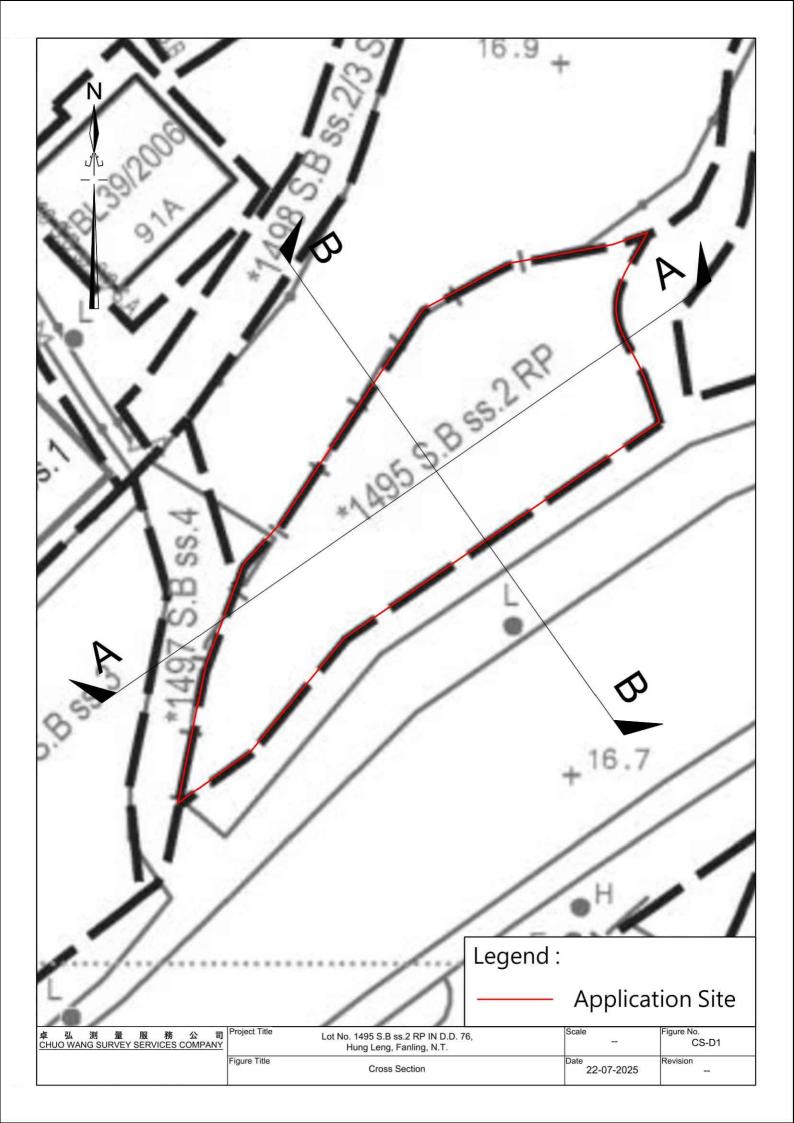
Return Period T (years)	2	5	10	20	50	100	200
a	1743.9	2183.2	2251.3	2159.2	1740.1	1307.3	1005.0
b	22.12	27.12	27.46	25.79	19.78	12.85	7.01
c	0.694	0.682	0.661	0.633	0.570	0.501	0.434

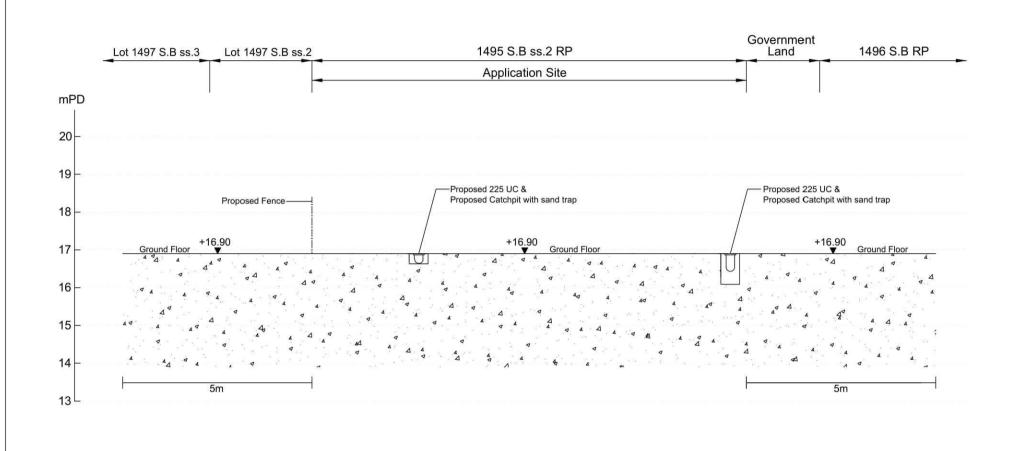
Table 3c – Storm Constants for Different Return Periods of West Lantau Area

Return Period T (years)	2	5	10	20	50	100	200
a	2047.9	1994.1	1735.2	1445.6	1107.2	909.1	761.8
b	24.27	24.23	21.82	18.36	13.01	8.98	5.40
С	0.733	0.673	0.619	0.561	0.484	0.428	0.377

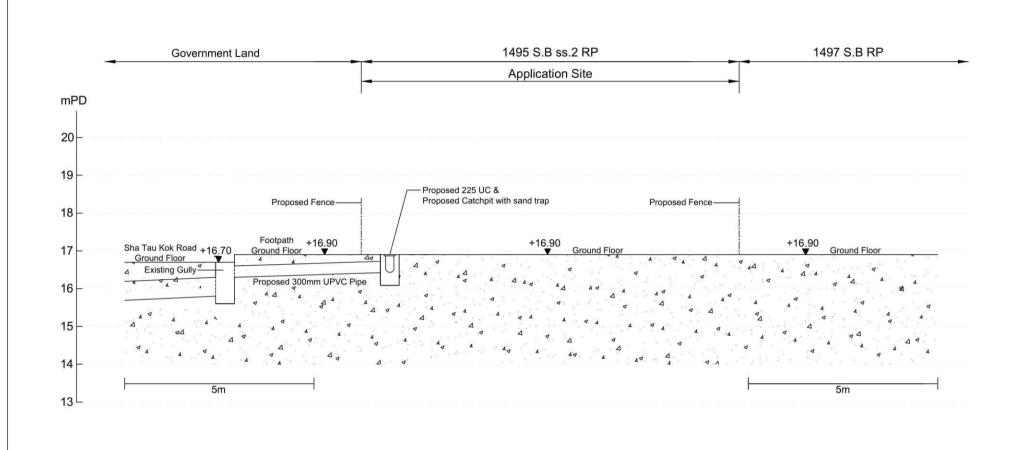
Return Period T (years)	2	5	10	20	50	100	200
a	1004.5	1112.2	1157.7	1178.6	1167.6	1131.2	1074.8
b	17.24	18.86	19.04	18.49	16.76	14.82	12.47
c	0.644	0.614	0.597	0.582	0.561	0.543	0.523

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	Figure Title Proposed Stormwater Drainage Plan	Date 22-07-2025	Revision

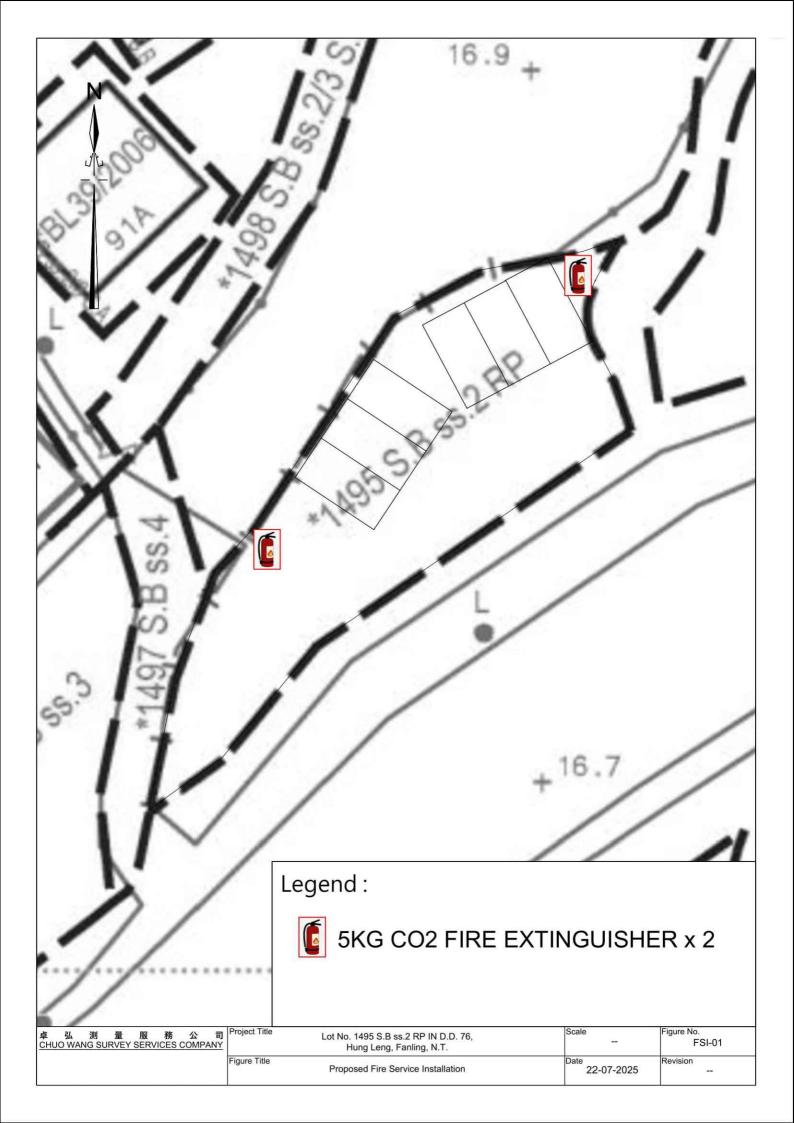


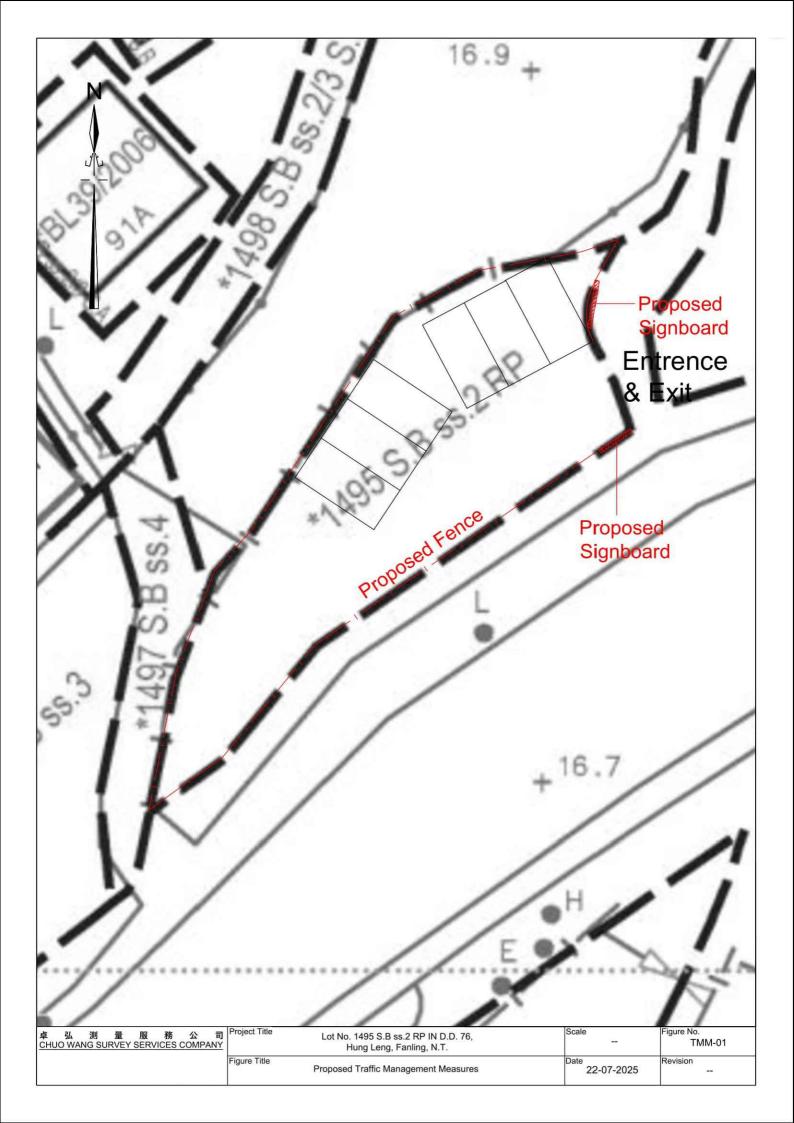


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CHUO WANG SURVEY SERVICES COMPANY		Not to scale	CS/D2
	Figure Title SECTION A - A	Date 22-07-2025	Revision



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CHUO WANG SURVEY SERVICES COMPAN		Not to scale	CS/D3
	Figure Title SECTION A - A	Date 22-07-2025	Revision





FILLING PLAN SIDE VIEW Filling Area (Lot 1495 S.B ss.2 RP) Ground Level 16.75 mPD Legend: Filling Area Survey Sheet No. : 弘 測 服 務 1:500 3-SW-4B **CHUO WANG SURVEY SERVICES COMPANY** Date: Figure No. : 22-07-2025 FLP/01