
**Attachment 8 –
Revised Traffic Impact Assessment**

**Section 16 Planning Application for Proposed
Amendments to an Approved Comprehensive Residential
Development Scheme and Minor Relaxation of Gross Floor
Area and Building Height Restrictions at Various Lots in
D.D. 385 and Adjoining Government Land, Tai Lam Chung,
Tuen Mun**

Traffic Impact Assessment Report

September 2025

AECOM

Table of Contents

	Page
1 INTRODUCTION.....	1
1.1 Background	1
1.2 Objectives.....	1
1.3 Structure of TIA Report	1
2 PROPOSED DEVELOPMENT	3
2.1 Development Schedule.....	3
2.2 Proposed Development Access Arrangement	3
2.3 Internal Parking and Servicing Facilities	3
3 EXISTING TRAFFIC CONDITION	5
3.1 Existing Traffic Arrangement	5
3.2 Traffic Survey.....	5
3.3 Junction Assessment	5
3.4 Road Link Capacity Assessments.....	6
3.5 Existing Public Transport Facilities.....	7
4 REVIEW ON FUTURE PUBLIC TRANSPORT DEMAND.....	10
4.1 Future Public Transport Demand	10
4.2 Future Public Transport Proposal.....	10
5 TRAFFIC FORECASTING	12
5.1 Design Year	12
5.2 Traffic Forecast.....	12
5.3 Planned / Potential Future Developments	13
5.4 Trip Generation of the Proposed Development	14
6 TRAFFIC IMPACT ASSESSMENT	18
6.1 Junction Capacity Assessment	18
6.2 Further Junction Improvement for Junction of Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5)	18
6.3 Road Link Capacity Assessments.....	19
6.4 Pedestrian Assessment	20

7	REVIEW ON TRAFFIC IMPACT DURING CONSTRUCTION STAGE	22
7.1	Construction Year	22
7.2	Traffic Forecast for Construction Stage	22
7.3	Junction Assessments during Construction Stage.....	22
8	CONCLUSION	24
8.1	Summary	24
8.2	Conclusion	25

List of Tables

	Page
Table 2.1	Indicative Development Schedule of the Development Site3
Table 2.2	Parking and Servicing Facilities Provision4
Table 3.1	Surveyed Key Junctions for Assessment5
Table 3.2	Existing Junction Performance.....6
Table 3.3	Road Link Capacity Assessments for Existing Year 2023.....6
Table 3.4	Public Transport Services7
Table 4.1	Estimation on Future Public Transport Demand for the Proposed Development10
Table 5.1	Planning Data of 2019-based TPEDM12
Table 5.2	Projections of Population Distribution.....12
Table 5.3	Historical Annual Daily Traffic (AADT) Flows from ATC13
Table 5.4	Traffic Flows Extracted from 2019-Based District Traffic Model.....13
Table 5.5	Planned / Potential Future Development in the Vicinity14
Table 5.6	Estimated Traffic Flows for Planned / Potential Future Developments in the Vicinity14
Table 5.7	Estimated Traffic Flows for the Proposed Development (Approved Scheme)....15
Table 5.8	Estimated Traffic Flows for the Proposed Development for Proposed Scheme .16
Table 5.9	Comparison of Development Traffic in Reference and Design Scenarios.....16
Table 6.1	Junction Performance in 203318

Table 6.2	2033 Junction Performance with Further Improvement Scheme	19
Table 6.3	Queue Length Analysis Result of J5 with Further Junction Improvement	19
Table 6.4	Road Link Capacity Assessment in Design Year 2033	19
Table 6.5	Existing Footpath Operation Performance	20
Table 6.6	2033 Design Pedestrian Assessment Results	21
Table 7.1	Anticipated Peak Hourly Construction Traffic	22
Table 7.2	Junction Performance in 2030 during Construction Stage.....	23

Figure

Figure 1.1	Site Location
Figure 2.1	Indicative Master Layout Plan
Figure 2.2	Vehicular Access Arrangement
Figure 3.1	Critical Junctions and Road Links
Figure 3.2	Existing Junction Layout of Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam (J1)
Figure 3.3	Existing Junction Layout of Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau (J2)
Figure 3.4	Existing Junction Layout of Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road (J3)
Figure 3.5	Existing Junction Layout of Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road (J4)
Figure 3.6	Existing Junction Layout of Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5)
Figure 3.7	Existing Junction Layout of Tai Lam Chung Road / Luen Hong Lane (J6)
Figure 3.8	Year 2023 Observed Traffic Flows
Figure 3.9	Existing Public Transport Facilities
Figure 4.1	Indicative Layout for Transport Interchange (TI)
Figure 5.1	Planned Developments
Figure 5.2	Year 2033 Background Traffic Flows
Figure 5.3	Year 2033 Reference Traffic Flows
Figure 5.4	Development Traffic Flows (Proposed Scheme)
Figure 5.5	Year 2033 Design Traffic Flows
Figure 6.1	Planned Junction Improvement Scheme at Castle Peak Road – Tai Lam /

- Tai Lam Chung Road
- Figure 6.2 Proposed Further Junction Improvement Scheme at Castle Peak Road –
Tai Lam / Tai Lam Chung Road
- Figure 6.3 Year 2024 Observed Pedestrian Flows
- Figure 6.4 Pedestrian Routing to Existing Bus Stops
- Figure 6.5 Year 2033 Design Pedestrian Flows
- Figure 7.1 Year 2030 Construction Traffic Flows

Annex

- Annex A Approved Gazette Plan for Luen Hong Lane
- Annex B Junction Capacity Calculation Sheets
- Annex C Swept Path Analysis

1 INTRODUCTION

1.1 Background

- 1.1.1 The Application Site is located in Tai Lam Chung Valley between Tsuen Wan and Tuen Mun. The proposed development comprises 2,670 nos. of residential flats and site formation works for village houses and provision of public facilities. The location of Application Site is shown in **Figure 1.1**.
- 1.1.2 The Application Site is mainly zoned “Comprehensive Development Area” (“CDA”) under the Approved So Kwun Wat Outline Zoning Plan (OZP) no. S/TM-SKW/15. A previous scheme comprising 1,560 nos. of residential flats with average flat size of 49.0m² was approved in 2002 under the planning application A/TM-SKW/32 (hereinafter referred to as the “Approved Scheme”).
- 1.1.3 The Applicant is now proposing a higher domestic plot ratio of 2.107 which comprises of 2,670 nos. of residential units with an average flat size of about 40.6m² (hereinafter referred to as “Proposed Scheme”) with the site formation works for village houses and provision of public facilities remain unchanged from the Approved Scheme.
- 1.1.4 AECOM Asia Co. Ltd. was commissioned by the Applicant as the Traffic Consultant to prepare a TIA report in support of the Section 16 planning application.

1.2 Objectives

- 1.2.1 The main objectives of this report are as follows-
- Outline the proposed development parameters and internal transport facilities, internal road arrangement etc.;
 - Review the current traffic condition in the vicinity of the Application Site;
 - Estimate the future public transport demand of the proposed development and develop enhancement on public transport services if necessary;
 - Estimate the potential traffic generations and attractions of the proposed development;
 - Produce traffic forecasts on the surrounding road network at the adopted design year;
 - Assess traffic impact on the surrounding road network induced from the proposed development; and
 - Develop traffic improvement proposal(s) if necessary.

1.3 Structure of TIA Report

- 1.3.1 Following this introductory chapter, the TIA is structured as follows:
- **Chapter 2:** Proposed Development, describes the development schedule of the proposed development and its internal traffic facilities provisions, access arrangement, etc.;
 - **Chapter 3:** Existing Traffic Condition, reviews the current traffic conditions in the vicinity;

- **Chapter 4:** Future Public Transport Proposal, elaborates the anticipated public transport demand and discusses the future possible public transport proposal;
- **Chapter 5:** Traffic Forecasting, describes the traffic forecasting methodology and presents the estimated traffic flows in design year;
- **Chapter 6:** Traffic Impact Assessment, assesses the traffic impact induced on the surrounding road network and recommends improvement schemes, if considered necessary;
- **Chapter 7:** Construction Traffic Impact Assessment, assesses the traffic impact on the surround road network during constructions stage and recommends improvement schemes, if considered necessary;
- **Chapter 8:** Summary and Conclusion, summarizes the findings of the study and presents the conclusion of this TIA.

2 PROPOSED DEVELOPMENT

2.1 Development Schedule

- 2.1.1 **Table 2.1** summarizes the development schedule of the Proposed Scheme with comparison to the Approved Scheme. The proposed indicative Master Layout Plan (MLP) under the current application is illustrated in **Figure 2.1** for reference.

Table 2.1 Indicative Development Schedule of the Development Site

Development Parameters	Approved Scheme	Proposed Scheme
Development Site Area	47,070m ²	46,493m ²
Plot Ratio	1.8555	2.107
Domestic GFA	86,267	108,468
No. of Storeys	15-16	6-23
No. of Units	1,560	2,670
Average Flat Size	49m ²	40.6m ²

2.2 Proposed Development Access Arrangement

- 2.2.1 The location of vehicular run-in/out for the Proposed Development would be maintained at Luen Hong Lane near to Luen On San Tsuen. The proposed run-in/out is presented in **Figure 2.2**.
- 2.2.2 According to the approved Road Gazette 2728 Plan No. TMM4185 dated in May 2022, the road section at both ends of Luen Hong Lane would be widened to allow better vehicle manoeuvring. The approved road gazette plan is presented in **Annex A**.

2.3 Internal Parking and Servicing Facilities

- 2.3.1 The parking and loading/unloading facilities for the Proposed Development would be provided in accordance with the requirements as stipulated in the Hong Kong Planning Standards and Guidelines (HKPSG). The respective requirements and proposed provision are summarized in **Table 2.2**.
- 2.3.2 Swept path analysis for the indicative MLP was also conducted as demonstrated in **Annex C**. The results have demonstrated that sufficient manoeuvring for the critical locations of loading/unloading bay as well as carparking spaces subject to further reviewed in detail design stage.

Table 2.2 Parking and Servicing Facilities Provision

Parking/ Servicing Facilities	HKPSG Requirement		No. of Units / Blocks / GFA	Internal Transport Facilities		
				HKPSG Requirement		Proposed Provision
				Lower End	Upper End	
Proposed Residential Development (2,670 flats)						
Southern Site (1,557 flats)						
Residential Parking Spaces	Flat Size≤40m ²	1 space per 8 – 14 units ⁽¹⁾	909	65	114	91
	40m ² < Flat Size ≤70m ²	1 space per 3.33 – 5.83 units ⁽²⁾	648	112	195	156
Northern Site (1,113 flats)						
Residential Parking Spaces	Flat Size≤40m ²	1 space per 8 – 14 units ⁽¹⁾	528	38	66	53
	40m ² < Flat Size ≤70m ²	1 space per 3.33 – 5.83 units ⁽²⁾	568	98	171	137
	70m ² < Flat Size ≤100m ²	1 space per 1.67 – 2.92 units ⁽³⁾	17	6	11	9
	Total		2670	319	557	446 ⁽⁴⁾
Visitor Parking Spaces	5 spaces per block		7	35		35
Motorcycle Parking Spaces	1 space per 100 – 150 units		2670	18	27	27
Loading / Unloading Bays	1 bay per block		7	7		7
Bicycle Parking Spaces	1 bicycle parking space for every 30 flats with flat size smaller than 70m ²		2653	89		89
Retail (2000m ² GFA)						
Retail Parking Spaces	1 space per 150m ² – 300m ² GFA		2000	7	14	10
Retail Loading / Unloading Bays	1 bay per 800m ² – 1200m ² GFA		2000	2	3	3
Motorcycle Parking Space	5%-10% of the total provision for private cars		-	1	1	1

Notes:

- (1) According to the current HKPSG, Parking Requirement = Global Parking Standard (GPS) x Demand Adjustment Ratio (R1) x Accessibility Adjustment Ratio (R2) x Development Intensity Adjustment Ratio (R3), i.e. 1 car space per 4-7 units x 0.5 x 1.0 x 1.0 = 1 car space per 8.00-14.00 units.
- (2) According to the current HKPSG, Parking Requirement = Global Parking Standard (GPS) x Demand Adjustment Ratio (R1) x Accessibility Adjustment Ratio (R2) x Development Intensity Adjustment Ratio (R3), i.e. 1 car space per 4-7 units x 1.2 x 1.0 x 1.0 = 1 car space per 3.33-5.83 units.
- (3) According to the latest HKPSG, Parking Requirement = Global Parking Standard (GPS) x Demand Adjustment Ratio (R1) x Accessibility Adjustment Ratio (R2) x Development Intensity Adjustment Ratio (R3), i.e. 1 car space per 4-7 units x 2.4 x 1.0 x 1.0 = 1 car space per 1.67-2.92 units.
- (4) Taken into consideration the proximity to public transport services, availability of public car parking space, traffic conditions and the illegal parking condition in the vicinity, it is proposed to adopt a GPS of 5 for calculating the carparking provision according to HKPSG.

3 EXISTING TRAFFIC CONDITION

3.1 Existing Traffic Arrangement

- 3.1.1 The Application Site is located in the Tai Lam Chung Valley between Tsuen Wan and Tuen Mun as shown in **Figure 3.1**. The Tai Lam Chung Nullah runs down through the valley from the Tai Lam Chung Reservoir Main Dam and its reservoir beyond. The application site is located in a generally flat area on the east side of the river near the entrance to the valley.
- 3.1.2 Tai Lam Chung Road is a two-way single carriageway with one traffic lane at each direction. The signalised junction of Castle Peak Road – Tai Lam / Tai Lam Chung Road is the key junction connecting the development site to/from the surrounding area.
- 3.1.3 Luen Hong Lane is a single 2-lane carriageway with passing bay connecting with Luen Tai Street.

3.2 Traffic Survey

- 3.2.1 A total of 6 key junctions have been identified for assessment and listed in **Table 3.1** and shown in **Figure 3.1**.

Table 3.1 Surveyed Key Junctions for Assessment

Ref.	Junction	Type	Fig. No.
J1	Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam	Roundabout	3.2
J2	Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau	Roundabout	3.3
J3	Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road	Signal	3.4
J4	Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road	Signal	3.5
J5	Castle Peak Road – Tai Lam / Tai Lam Chung Road	Signal	3.6
J6	Tai Lam Chung Road / Luen Hong Lane	Priority	3.7

- 3.2.2 The existing layout of the above junctions are shown in **Figure 3.2** to **Figure 3.7**.
- 3.2.3 To investigate the current traffic condition of the identified critical junctions, manual classified traffic counts were conducted on 30 May 2023 during 7:30am – 9:30am and 5:00pm – 7:00pm.
- 3.2.4 The identified morning (AM) and evening (PM) peak hour are from 7:30am to 8:30am and from 5:30pm to 6:30pm respectively. The 2023 observed AM and PM peak hour traffic flows are shown in **Figure 3.8**.

3.3 Junction Assessment

- 3.3.1 Based on the 2023 observed traffic flows, capacity assessments were carried out in accordance with the methodology documented in the appendices of Transport Planning and Design Manual (TPDM) Volume 2 Chapter 4 for priority junction / roundabout. Signal junction assessments were based on TPDM Volume 4.

- 3.3.2 The existing junction performance of the critical junctions are summarized in **Table 3.2**. The junction calculation spreadsheets are enclosed in **Annex B**.

Table 3.2 Existing Junction Performance

Ref.	Junction	Indicator*	2023 Observed	
			AM Peak	PM Peak
J1	Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam	DFC	0.54	0.33
J2	Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau	DFC	0.27	0.12
J3	Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road	RC	>100%	>100%
J4	Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road	RC	>100%	>100%
J5	Castle Peak Road – Tai Lam / Tai Lam Chung Road	RC	>100%	>100%
J6	Tai Lam Chung Road / Luen Hong Lane	DFC	0.03	0.04

* RC = Reserve Capacity for signal junction; DFC = Design Flow / Capacity ratio for priority junction or roundabout

- 3.3.3 At present, the concerned junctions are operating within capacity.

3.4 Road Link Capacity Assessments

- 3.4.1 Road link capacity assessments are also conducted to assess the existing flow/capacity ratio (i.e. V/C ratio) of the key road links in the vicinity of the subject site. The assessment results are summarized in **Table 3.3**.

Table 3.3 Road Link Capacity Assessments for Existing Year 2023

Ref. ⁽¹⁾	Road Link	Direction	Link Capacity ⁽²⁾ (pcu/hr)	Year 2023 Observed Traffic Flows (pcu/hr)		Flow/Capacity Ratio (V/C Ratio)	
				AM peak	PM Peak	AM peak	PM Peak
L1	Tuen Mun Road	EB	7,560	4,905	3,490	0.65	0.46
L2		WB	5,640	3,165	4,795	0.56	0.85
L3	Castle Peak Road – Tai Lam	EB	1,320	745	805	0.56	0.61
L4		WB	1,320	180	185	0.14	0.14
L5	Tai Lam Chung Road	NB	1,020	170	100	0.17	0.10
L6		SB	1,020	185	190	0.18	0.19
L7	Luen Hong Lane	EB	480	20	20	0.04	0.04
L8		WB	480	25	30	0.05	0.06
L9	Castle Peak Road – Tai Lam	EB	3,600	1,460	745	0.41	0.21
L10		WB	3,600	520	940	0.14	0.26
L11	Castle Peak Road – Tai Lam	EB	3,600	1,020	205	0.28	0.06
L12		WB	3,600	210	305	0.06	0.08
L13	Tuen Mun Road	EB	7,560	5,585	3,970	0.74	0.53
L14		WB	5,640	3,165	4,795	0.56	0.85
L15	Slip Road of Tuen Mun Road	EB	1,320	575	300	0.44	0.23
L16		WB	1,320	445	630	0.34	0.48

Note:

- (1) Refer to **Figure 3.1**
- (2) Derived with reference to Table 2.4.1.1 in TPDM Volume 2 – Chapter 2.4

3.4.2 The above link capacity assessment results indicate that all road links are operating at within capacities.

3.5 Existing Public Transport Facilities

3.5.1 At present, there is a GMB route 43B located at Tai Lam Chung Bus Terminus near Luen Hong Lane, which is around 500m (around 7-minute walk) away from the Proposed Development.

3.5.2 The franchised bus and green minibus (GMB) routes serving at Castle Peak Road – Tai Lam surrounding of the Proposed Development are summarized in the **Table 3.4**. The location of the bus stops and minibus stops are presented in **Figure 3.9**.

Table 3.4 Public Transport Services

Public Transport Services				
Route No.	Origin / Destination			Frequency (min.) / Timetable
Franchised Bus (CTB)				
50	Yan Po Road Public Transport Interchange	↔	Kowloon Station Bus Terminus	20 - 30
55	Yan Po Road Public Transport Interchange	↔	Kwun Tong Ferry	06:30、06:50、07:05、07:20、07:40、08:05、17:40、18:00、18:20、18:45 ⁽¹⁾
950	Yan Po Road Public Transport Interchange	↔	Exhibition Centre Station Bus Terminus	07:10、07:25、18:10 ⁽¹⁾
952	Chi Lok Fa Yuen	↔	Causeway Bay (Moreton Terrace) Bus Terminus	5 - 30
952C	So Kwun Wat	→	Kornhill Plaza, Kornhill Road	07:12、07:24、07:36 ⁽¹⁾
	Sunway Gardens, King's Road	→	So Kwun Wat	18:10、18:15 ⁽¹⁾
952P	Chi Lok Fa Yuen	→	Causeway Bay (Moreton Terrace) Bus Terminus	Mon to Fri: 7:05 - 08:57 Sat: 07:13 - 08:57
955	Yan Po Road Public Transport Interchange	↔	Sai Wan Ho	Mon to Fri: 07:30 / 18:12
962	Lung Mun Oasis Bus Terminus	↔	Causeway Bay (Moreton Terrace) Bus Terminus	8 - 25
962G	Causeway Bay (Moreton Terrace) Bus Terminus	→	Yuet Wu Villa Bus Terminus	18:05、18:35 ⁽¹⁾
962P	Lung Mun Oasis Bus Terminus	→	Causeway Bay (Moreton Terrace) Bus Terminus	4 - 15
962X	Lung Mun Oasis Bus Terminus	↔	Causeway Bay (Moreton Terrace) Bus Terminus	9 - 30
N50	Yan Po Road Public Transport Interchange	↔	Kowloon Station Bus Terminus	01:15、01:45、04:35、05:05
N952	Causeway Bay (Moreton Terrace) Bus Terminus	↔	Chi Lok Fa Yuen	00:50、01:10、05:10、05:40
N962	Lung Mun Oasis Bus Terminus	↔	Causeway Bay (Moreton Terrace) Bus Terminus	25 - 45
N969	Tin Shui Wai Town Centre Public Transport Interchange	↔	Causeway Bay (Moreton Terrace) Bus Terminus	25 - 45
X962	Admiralty (West) Bus Terminus	→	Lung Mun Oasis Bus Terminus	15 - 30
Franchised Bus (KMB)				

Route No.	Origin / Destination			Frequency (min.) / Timetable
52X	Tuen Mun Central Bus Terminus	↔	Mong Kok (Park Avenue) Bus Terminus	5 - 25
52P	So Kwun Wat	→	Mong Kok (Park Avenue) Bus Terminus	08:00
53	YOHO Mall (Yuen Long)	↔	Nina Tower Bus Terminus	25 - 35
57M	Shan King Bus Terminus	↔	Lai King North Bus Terminus	10 - 30
58M	Leung King Estate Bus Terminus	↔	Kwai Fong Station Bus Terminus	3 - 15
58P	Kwai Fong Station Bus Terminus	→	Tin Yue House Tin King Estate	15 - 20
59M	Tuen Mun Ferry Pier	↔	Tsuen Wan Station Bus Terminus	3 - 20
59X	Tuen Mun Ferry Pier	↔	Mong Kok East Station Bus Terminus	3 - 20
60M	Tuen Mun Station Bus Terminus	↔	Tsuen Wan Station Bus Terminus	7 - 30
60X	Tuen Mun Central Bus Terminus	↔	Jordan (West Kowloon Station) Bus Terminus	7 - 20
61A	Yau Oi (South) Bus Terminus	→	Tuen Mun Road Bus-Bus Interchange	06:50 ⁽¹⁾
61M	Yau Oi (South) Bus Terminus	↔	Lai King North Bus Terminus	8 - 25
61P	So Kwun Wat Tsuen	↔	Tsuen Wan Station Bus Terminus	07:10、07:35、08:05 18:15、18:40、 19:05 ⁽¹⁾
61X	Tuen Mun Central Bus Terminus	↔	Kowloon City Ferry Bus Terminus	10 - 30
62X	Siu Hong Station (South)	↔	Lei Yue Mun Estate Bus Terminus	10 - 30
63X	Hung Shui Kiu (Hung Fuk Estate) Bus Terminus	↔	Jordan (West Kowloon Station) Bus Terminus	12 - 30
66M	Tai Hing Bus Terminus	↔	Nina Tower Bus Terminus	15 - 30
66X	Tai Hing Bus Terminus	↔	Olympic Station Bus Terminus	10 - 25
67A	Po Tin BBI - Po Tin Bus Terminus	↔	Kwai Tsui Estate Public Transport Interchange	20 - 30
67M	Siu Hong Court Bus Terminus	↔	Kwai Fong Station Bus Terminus	5 - 20
67X	Siu Hong Court Bus Terminus	↔	Mong Kok East Station Bus Terminus	7 - 25
68A	Long Ping Estate Bus Terminus	↔	Tsing Yi Station Public Transport Interchange	8 - 30
252	Tuen Mun Road Bus-Bus Interchange	↻	So Kwun Wat	10 - 30 ⁽²⁾
	So Kwun Wat Tsuen	→	Tuen Mun Road Bus-Bus Interchange	20
	Tuen Mun Road Bus-Bus Interchange	→	18 Kwun Chui Road	20 ⁽¹⁾
258D	Po Tin BBI - Po Tin Bus Terminus	↔	Lam Tin Public Transport Interchange	5 - 30
258X	Po Tin BBI - Po Tin Bus Terminus	↔	Kwun Tong Ferry	07:35、18:05 ⁽¹⁾
259D	Lung Mun Oasis Bus Terminus	↔	Lei Yue Mun Estate Public Transport Interchange	7 - 30
259E	Lung Mun Oasis Bus Terminus	→	Tsuen Wan Station Public Transport Interchange	40 ⁽¹⁾
259S	Lung Mun Oasis Bus Terminus	→	Kwun Tong Ferry	07:20 ⁽¹⁾
259X	Lung Mun Oasis Bus Terminus	↔	Kwun Tong Ferry	07:00、07:15、07:30 、17:50、18:20 ⁽¹⁾
260X	Po Tin BBI - Po Tin Bus Terminus	↔	Hung Hom Station Public Transport Interchange	5 - 20

Route No.	Origin / Destination			Frequency (min.) / Timetable
261B	Sam Shing Public Transport Interchange	→	Kowloon Station Bus Terminus	07:25 - 07:35
263	Tuen Mun Station Public Transport Interchange	↔	Sha Tin Station Public Transport Interchange	5 - 25
263A	Tuen Mun Station Public Transport Interchange	↔	Hong Kong Science Park Phase III	07:25 - 07:35 - 18:20 ⁽¹⁾
263B	Tuen Mun Station Public Transport Interchange	↔	Fo Tan (Shan Mei Street) Public Transport Interchange	07:35 - 18:15 ⁽¹⁾
263C	Tuen Mun Station Public Transport Interchange	↔	Tai Po Industrial Estate Bus Terminus	06:40 - 17:55 - 07:15 - 18:00 ⁽¹⁾
960	Kin Sang Bus Terminus	↔	Exhibition Centre Station Bus Terminus	5 - 20
960A	Pottinger Street, Connaught Road Central	→	Hung Fuk Estate Public Transport Interchange	18:30 ⁽¹⁾
960B	Kin Sang Bus Terminus	↔	Sunway Gardens, King's Road	07:00 - 07:20 - 17:55 ⁽¹⁾
960C	Fu Tai Estate Bus Terminus	↔	Victoria Park, Causeway Road	07:00 - 07:15 - 17:30 ⁽¹⁾
960P	Hung Shui Kiu (Hung Yuen Road)	↔	Victoria Park, Causeway Road	10 - 30
960S	Fu Tai Estate Bus Terminus	↔	Victoria Park, Causeway Road	10 - 15
960X	Hung Shui Kiu (Hung Yuen Road)	↔	Sunway Gardens, King's Road	9 - 15 ⁽¹⁾
961	Shan King Bus Terminus	↔	Exhibition Centre Station Bus Terminus	7 - 25
961P	Leung King Estate Bus Terminus	→	Victoria Park, Causeway Road	07:35 ⁽¹⁾
961S	Leung King Estate Bus Terminus	→	Victoria Park, Causeway Road	07:30 ⁽¹⁾
N252	Mei Foo Bus Terminus	→	Sam Shing Public Transport Interchange	01:05 - 01:35
N960	Kin Sang Bus Terminus	↔	Exhibition Centre Station Bus Terminus	01:25 - 04:45
P960	Siu Hong Station North Public Transport Interchange	↔	Exhibition Centre Station Bus Terminus	30 - 45
N260	Tuen Mun Ferry Pier	↔	Mei Foo Bus Terminus	30
Franchised Bus (LWB)				
A33	Tuen Mun Road Bus-Bus Interchange	↔	Airport (Ground Transportation Centre)	20 - 60
Franchised Bus (MTR Bus)				
K51	Fu Tai Estate Bus Terminus	↔	Tai Lam Chung	5 - 20
Green Mini-Bus (GMB)				
43B	Tuen Mun Town Centre (Ho Pong Street)	↔	Tai Lam Chung	18 - 30

Notes:

- (1) Monday to Friday only
- (2) Circular route
- (3) Saturday, Sunday and Public Holiday
- (4) No service on Sundays and public holidays

4 REVIEW ON FUTURE PUBLIC TRANSPORT DEMAND

4.1 Future Public Transport Demand

- 4.1.1 To review the appropriate public transport provision to be provided due to the population intake of the Proposed Development, the future public transport demand for the Proposed Development is reviewed with reference to the information/data as available in the Population By-Census 2021 and the Travel Characteristics Survey 2011 Final Report as available on Transport Department's website. The estimation of future public transport demand is summarized in **Table 4.1**.

Table 4.1 Estimation on Future Public Transport Demand for the Proposed Development

Parameters	Formula	Proposed Scheme
Estimated Population by Flats	2,670 Flats	7,476 ⁽¹⁾
Estimated Population by Village Housing	80 Village House	224 ⁽¹⁾
Total Estimated Population	(a)	7,700 ⁽²⁾
Average daily mechanised trips per person	(b)	1.83 ⁽³⁾
Peak hour factor (AM/PM) to daily total	(c)	12% ⁽⁴⁾
Modal Split for Public Transport	(d)	73% ⁽⁵⁾
Estimated public transport demand per hour during peak hours	(e) = (a) x (b) x (c) x (d)	1,235

Notes:

- (1) The estimated population of the application site is estimated with an assumption of 2.8 persons / flat or village house.
- (2) The total estimated population is the factor (a) of formula reflecting total population of subject site including flats and village house.
- (3) The daily mechanised trip rate per population is 1.83 trips according to the Travel Characteristics Survey 2011 Final Report.
- (4) The peak hour factor is about 12% of daily trips according to the Travel Characteristics Survey 2011 Final Report.
- (5) Modal split for public transport is made reference with Table C109 of 2021 Population Cense.

4.2 Future Public Transport Proposal

- 4.2.1 Taking into consideration that (1) franchised bus is the main mode of public transport available in the vicinity of the Proposed Development; (2) Tuen Mun Bus-bus interchange (BBI) is located approx. 800m away from the Proposed Development; (3) Tuen Mun BBI would have sufficient bus routes to various locations, it is therefore proposed to introduce additional franchised bus services travelling in between the Proposed Development and Tuen Mun BBI to facilitate the future public transport demand.

- 4.2.2 It is proposed to introduce 14 trips during peak hours to cater the public transport demand as derived in **Table 4.1**. It reflected that the total capacity of 1,260 pax/hr (i.e. 14 trips x 120 pax/bus x 75% occupancy rate) would be well sufficient to cater for the additional public transport demand generated by the Proposed Development with an utilization rate of approximately 98% (i.e. 1,235pax/hr / total capacity of 1,260pax/hr) during AM peak hour.

- 4.2.3 In order to serve the public transport demand induced by the Proposed Development, total provision of 3 nos. of bus layby (i.e. 39m in length) and 4 nos. of GMB layby (i.e. 32m in length) would be proposed in the Proposed privately-operated Transport Interchange (TI) within the Proposed Development. The indicative layout of the transport interchange is presented in **Figure 4.1**. Swept path analysis was conducted and demonstrated sufficient manoeuvring space would be allowed for 12.8m bus as presented in **Annex C**.
- 4.2.4 The detailed arrangement of the proposed enhancement on public transport services would be subject to further review and consideration by relevant government departments and/or stakeholders in detailed design stage.

5 TRAFFIC FORECASTING

5.1 Design Year

- 5.1.1 The proposed development is tentatively scheduled for completion in 2030. Year 2033 is therefore selected as a design year for assessment purpose (i.e. 3 years after the planned completion).

5.2 Traffic Forecast

- 5.2.1 Annual Growth Rate method is applied to estimate 2033 traffic forecast from the 2023 observed traffic flows. The annual growth rate is made reference to the planning data in 2019-based Territorial Population and Employment Data Matrix (TPEDM) which is available on Planning Department's website. **Table 5.1** shows the years 2019 and 2031 population planning data in Tuen Mun district.

Table 5.1 Planning Data of 2019-based TPEDM

Planning Data District	2019			2031			Annual Growth Rate
	Population	Employment	Total	Population	Employment	Total	
Tuen Mun	476,500	130,800	607,300	606,850	150,750	757,600	+1.86%

- 5.2.1 As shown in **Table 5.1**, the average growth rate as derived from TPEDM is about 1.86%.
- 5.2.2 Apart from the TPEDM, the Projections of Population Distribution in Tuen Mun district for year 2023-2031 as published by Planning Department is also reviewed. It was found that the average growth rate is about 1.46% from 2023 - 2031 and is summarized in **Table 5.2**.

Table 5.2 Projections of Population Distribution

District Council District	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031
Tuen Mun	506900	508800	539300	541600	554900	557600	573800	577700	577500	577300	586200
Average Traffic Growth Rate from 20121 to 2031 = 1.46% per annum											

- 5.2.3 Other than TPEDM and the Projections of Population Distribution from Planning Department, the historical traffic data from Annual Traffic Census (ATC) is also reviewed. The annual average daily traffic (AADT) flow and annual growths of the nearby counting stations from 2016 to 2023 as presented in the ATC reports published annually by Transport Department are summarized in **Table 5.3**.

Table 5.3 Historical Annual Daily Traffic (AADT) Flows from ATC

ATC Stn. No.	Road Name	A.A.D.T. (veh/day)							
		2016	2017	2018	2019	2020	2021	2022	2023
5012	Tuen Mun Road Expressway	123,250	129,590	129,160	133,340	127,640	127,800	121,740	124,650
5855	Tuen Mun Road Expressway	111,100	110,150	113,660	118,180	112,500	110,960	103,070	109,520
5857	Tuen Mun Road - Siu Lam	11,610	9,960	10,210	10,140	9,720	10,780	12,270	12,560
5657	Castle Peak Rd - Sham Tseng, Tsing Lung Tau & Tai Lam	12,350	12,160	12,460	12,370	12,830	13,550	13,140	13,440
6052	Castle Peak Rd - So Kwun Wat & Castle Peak Bay	19,490	19,600	20,270	20,130	19,300	20,070	20,540	22,250
Total		277,800	281,460	285,760	294,160	281,990	283,160	270,760	282,420
Average Traffic Growth Rate from 2016 to 2023 = 0.24% per annum									

5.2.4 As shown in **Table 5.3**, the average growth rate from 2016 to 2023 is about **0.24%** per annum according to the historical ATC data.

5.2.5 The Year 2019 Base District Traffic Models (BDTM) published by Transport Department was also referred to determine the traffic growth rates in Tuen Mun area. The projected traffic flows at the surrounding road network are summarized in **Table 5.4**.

Table 5.4 Traffic Flows Extracted from 2019-Based District Traffic Model

Road Links	AM		PM	
	2019	2031	2019	2031
Tai Lam Chung Road	141	775	127	665
Castle Peak Road – Tai Lam	482	1,224	1,033	1,781
Tuen Mun Road	10,622	11,504	10,586	11,076
Total	11,245	13,503	11,746	13,522
Growth Rates	1.54%		1.18%	

5.2.6 The projected traffic flows in BDTM revealed that the growth rate of traffic rate in the local road network would be increased by 1.54% and 1.18% per annum for AM and PM peak respectively.

Adopted Traffic Growth Factor

5.2.7 Based on the results given by TPEDM estimates and AADT historical data as well as BDTM, an annual growth rate of **1.86%** per annum is adopted for projecting the peak hour traffic flows from 2023 to 2033 for conservative assessments.

5.3 Planned / Potential Future Developments

5.3.1 It is noted that there are several planned / potential developments along Castle Peak Road – Tai Lam and in So Kwun Wat, and their locations are listed in **Figure 5.1** and **Table 5.5** respectively.

Table 5.5 Planned / Potential Future Development in the Vicinity

Ref.	Lot	Proposed Use	No. of Flats	Estimated Average Flat Size (sq.m)
1 ⁽¹⁾	TMTL518	Private Housing	928	50
2 ⁽¹⁾	TMTL546	Private Housing	1,586	60
3 ⁽²⁾	TMTL561	Private Housing	2,708	60
4 ⁽³⁾	TMTL463	Private Housing	672	60
5	TMTL520	Private Housing	693	60
6 ⁽⁴⁾	TMTL496	Private Housing	1326	60
7 ⁽⁵⁾	Light Public Housing at Lok On Pai	Public Housing	4200	25

Notes:

(1) Development parameters extracted from RNTPC Paper No. 9/17

(2) According to Amendment item A in RNTPC Paper No. 2/15 "Proposed Amendments to the Approved So Kwun Wat OZP No. S/TM-SKM/11" issued by Planning Department. The average flat sizes are extracted from RNTPC Paper No. 2/15

(3) Development parameters extracted from OZP

(4) Development parameters extracted from OZP

(5) Development parameters extracted from LC Paper No. CB(1)1123/2023(02)

5.3.2 Estimates of traffic generation and attraction volume are derived from the trip rates as stipulated in Annex D of Transport Planning and Design Manual (TPDM) Volume 1 Chapter 3 published by Transport Department. **Table 5.6** summarizes the estimated trip generations of the planned / potential future developments as listed in **Table 5.5**.

Table 5.6 Estimated Traffic Flows for Planned / Potential Future Developments in the Vicinity

Ref.		Estimated Trips (pcu/hr)			
		AM Peak		PM Peak	
		Generation	Attraction	Generation	Attraction
1 - TMTL518	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	67	39	27	34
2 - TMTL546	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	114	67	45	59
3 - TMTL561	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	194	115	77	100
4 - TMTL463	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	48	29	19	25
5 - TMTL520	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	50	29	20	26
6 - TMTL496	Adopted Trip Rate (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Flow (pcu/hr)	95	56	38	49
7 - Light Public Housing at Lok On Pai	Adopted Trip Rate (pcu/hr/flat) ⁽¹⁾	0.0071	0.0046	0.0112	0.014
	Estimated Flow (pcu/hr)	30	19	47	59

Notes:

(1) The adopted trip rates are referred to the Transport and Traffic Impact Assessment Report for "Proposed Light Public Housing Development and Associated Filling / Excavation of Land on a 3 Years Temporary Basis at Various Lots in D.D. 104 and Adjoining Government Land, Yau Pok Road, Yuen Long"

5.4 Trip Generation of the Proposed Development

- 5.4.1 Based on the development schedule as mentioned in **Section 2**, the adopted trip rate extracted from Annex D of TPDM Volume 1 Chapter 3 and the development trip generation and attraction for Approved Scheme is summarized in **Table 5.7**.

Table 5.7 Estimated Traffic Flows for the Proposed Development (Approved Scheme)

Application Site		Estimated Trips (pcu/hr)			
		AM Peak		PM Peak	
		Gen.	Att.	Gen.	Att.
Site (a) – 1,560 flats	Adopted Trip Rates ⁽¹⁾ (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Trips (pcu/hr)	113	67	45	58
Site (b) – 80 village house	Adopted Trip Rates ⁽²⁾ (pcu/hr/flat)	0.3012	0.2189	0.2235	0.3234
	Estimated Trips (pcu/hr)	25	18	18	26
Total (pcu/hr)		138	85	63	84

Note:

(1) TPDM mean trip rates for Private Housing: High-Density /R(A) with Ave. Flat Size of 60m²

(2) TPDM mean trip rates for Private Housing: Low-Density /R(C) with Ave. Flat Size of 240m²

Year 2033 Background Traffic Flows

- 5.4.2 The 2033 background traffic flows (without approved/proposed development) are derived by applying a growth rate of 1.86% p.a. on the 2023 observed flows upto the design year 2033 and superimpose the traffic flow as derived in **Table 5.6**. The year 2033 background traffic flows are presented in **Figure 5.2**.

Year 2033 Reference Traffic Flows

- 5.4.3 The 2033 reference traffic flows are derived by superimposing the potential traffic as induced by the Approved Scheme in **Table 5.7** onto the traffic flows in **Figure 5.2**.
- 5.4.4 The year 2033 reference traffic flows are presented in **Figure 5.3**.

Trip Generation of the Proposed Development in Design Scenario

- 5.4.5 In the design scenario of year 2033, it is adopted that Site (a) will increase the number of flats to 2,670 flats in accordance with the development schedule as listed in **Table 2.1**.
- 5.4.6 The estimated potential traffic generation and attraction of the Proposed Development in the Design Scenario of 2033 are shown in **Table 5.8**.

Table 5.8 Estimated Traffic Flows for the Proposed Development for Proposed Scheme

Application Site		Estimated Trips (pcu/hr)			
		AM Peak		PM Peak	
		Gen.	Att.	Gen.	Att.
Site (a) – 2,670 flats	Adopted Trip Rates ⁽¹⁾ (pcu/hr/flat)	0.0718	0.0425	0.0286	0.037
	Estimated Trips (pcu/hr)	192	114	77	99
Site (b) – 80 village house	Adopted Trip Rates ⁽²⁾ (pcu/hr/flat)	0.3012	0.2189	0.2235	0.3234
	Estimated Trips (pcu/hr)	25	18	18	26
Retail – 2000m ² GFA	Adopted Trip Rates ⁽³⁾ (pcu/hr/100m ² GFA)	0.2296	0.2434	0.3100	0.3563
	Estimated Trips (pcu/hr)	5	5	7	8
Public Transport	Adopted Trip Rates ⁽⁴⁾	-	-	-	-
	Estimated Trips (pcu/hr)	36	36	36	36
Total (pcu/hr)		258	173	138	169

Note:

(1) TPDM mean trip rates for Private Housing: High-Density /R(A) with Ave. Flat Size of 60m²

(2) TPDM mean trip rates for Private Housing: Low-Density /R(C) with Ave. Flat Size of 240m²

(3) TPDM mean trip rates for Retail / Shopping Complex (Office + Retail)

(4) Details calculation of trip generate / attraction by public transport refers to TIA Section 4.2 paragraph 4.2.2

5.4.7 As shown in **Table 5.8**, it is estimated that the Proposed Scheme would potentially generate 258 pcu/hr and attract 173 pcu/hr in the morning peak hour, and generate about 138 pcu/hr and attract 169 pcu/hr in the evening peak hour.

5.4.8 As compared with the Reference Scenario, the Proposed Development with domestic plot ratio of 2.107 would induce additional 2-way traffic 208 pcu/hr and 160 pcu/hr in morning and evening peak hour respectively. The comparison of development traffic of the application site in Reference and Design Scenarios are shown in **Table 5.9**.

Table 5.9 Comparison of Development Traffic in Reference and Design Scenarios

Application Site		Estimated Trips (pcu/hr)			
		AM Peak		PM Peak	
		Gen.	Att.	Gen.	Att.
Reference Scenario (a)		138	85	63	84
Design Scenario (b)		258	173	138	169
Net Difference = (b) – (a)		+120	+88	+75	+85
Total Two-way Traffic (pcu/hr)		+208		+160	

5.4.9 In addition to the additional development traffic induced by the Proposed Development, the potential increases in bus traffic included by at the Proposed Development are also taken into account.

- 5.4.10 As discussed in **Section 4**, it is proposed to introduce 14 trips of bus services during the peak hours to cater the public transport demand as induced by the Proposed Development.

Year 2033 Design Traffic Flows

- 5.4.11 The year 2033 design flows are derived by superimposing (i) the net increases in development traffic of the Proposed Development as presented in **Table 5.9**; and (ii) the additional bus trips upon the available of the proposed transport interchange in the Application Site onto the year 2033 reference traffic flows (**Figure 5.3**).
- 5.4.12 The development traffic flows as well as the year 2033 design traffic flows are presented in **Figure 5.4** and **5.5** respectively.

6 TRAFFIC IMPACT ASSESSMENT

6.1 Junction Capacity Assessment

6.1.1 The operational performance of 6 critical junctions based on year 2033 traffic forecasts as mentioned in **Section 5** have been assessed.

6.1.2 A junction improvement at Junction of Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5) is committed by the applicant under approved planning application (No. A/TM-SKW/26) as shown in **Figure 6.1**. As presented in **Figure 6.1**, an additional flare length would be implemented at Castle Peak Road (Tai Lam) while a pedestrian stagger crossing would be introduced at Tai Lam Chung Road westbound. This committed junction improvement layout will be taken into account for the junction capacity analysis.

6.1.3 The results of junction capacity assessment are summarized in **Table 6.1**.

Table 6.1 Junction Performance in 2033

Ref.	Junction	Indicator ¹⁾	2033					
			Background Case (without Approved Scheme)		Reference Case (with Approved Scheme)		Design Case (with Proposed Scheme)	
			AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
J1	Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam	DFC	0.65	0.40	0.79	0.49	0.80	0.51
J2	Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau	DFC	0.32	0.14	0.46	0.17	0.54	0.19
J3	Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road	RC	>100%	>100%	>100%	>100%	>100%	>100%
J4	Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road	RC	>100%	>100%	>100%	>100%	>100%	>100%
J5	Castle Peak Road – Tai Lam / Tai Lam Chung Road ^{(2) (3)}	RC	88%	115%	31%	62%	3%	33%
J6	Tai Lam Chung Road / Luen Hong Lane ⁽⁴⁾	DFC	0.04	0.05	0.22	0.16	0.39	0.29

Notes:

- (1) RC = Reserve Capacity for signal junction; DFC = Design Flow / Capacity ratio for priority junction or roundabout
- (2) Committed junction improvement layout refers **Figure 6.1**.
- (3) Cycle time of 120s to be adopted
- (4) The road layout of approved Road Gazette 2728 refers to **Annex A**

6.1.4 As shown in **Table 6.1**, all junctions will be operating within capacity in 2033 except J5.

6.2 Further Junction Improvement for Junction of Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5)

6.2.1 To enhance junction capacity of J5, a left turn lane at Castle Peak Road – Tai Lam southbound will be proposed. Also, according to the latest requirement as stipulated in TPDM, the use of staggered crossing should be avoided. Therefore, the straight crossing at Tai Lam Chung Road will be adopted in the junction improvement scheme as illustrated in **Figure 6.2**. The junction performance is reassessed by taking into consideration the further junction improvement and the junction would operate with sufficient capacity as shown in **Table 6.2**.

Table 6.2 2033 Junction Performance with Further Improvement Scheme

Ref.	Junction	Indicator*	2033 Design Case	
			AM Peak	PM Peak
J5	Castle Peak Road – Tai Lam / Tai Lam Chung Road	RC	24%	44%

Notes:

- (1) RC = Reserve Capacity for signal junction
- (2) Cycle time of 90s to be adopted for design purpose

6.2.2 Apart from the junction assessments, queue length analysis for J5 with further improvement is also conducted for Design case of 2033. The analysis results are summarized in Table 6.3.

Table 6.3 Queue Length Analysis Result of J5 with Further Junction Improvement

Critical Arm	Available Length for Queuing (m)	2033 Design Case	
		AM Peak	PM Peak
Castle Peak Road – Tai Lam SB	36m ⁽¹⁾ / 280m ⁽²⁾	30m ⁽¹⁾ / 36m ⁽²⁾	18m ⁽¹⁾ / 24m ⁽²⁾
Castle Peak Road – Tai Lam NB	86m	12m	18m
Tai Lam Chung Road WB	95m	54m	36m

Notes:

- (1) Traffic queue for flared lane (Castle Peak Road – Tai Lam straight and right turn traffic)
- (2) Traffic queue for nearside lane (Castle Peak Road – Tai Lam left turn traffic)

6.3 Road Link Capacity Assessments

6.3.1 Road link capacity assessments are also conducted to assess the V/C ratio of the key road links in both the Reference and Design Cases of year 2033. The assessment results are summarized in Table 6.4.

Table 6.4 Road Link Capacity Assessment in Design Year 2033

Ref. ⁽¹⁾	Road Link	Direction	Link Capacity ⁽²⁾ (pcu/hr)	Year 2033 Traffic Flows (pcu/hr)				Flow/Capacity Ratio (V/C Ratio)			
				Reference Case		Design Case		Reference Case		Design Case	
				AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
L1	Tuen Mun Road	EB	7,560	5,245	3,730	5,265	3,750	0.69	0.49	0.70	0.50
L2		WB	5,640	3,385	5,125	3,415	5,155	0.60	0.91	0.61	0.91
L3	Castle Peak Road – Tai Lam	EB	1,320	990	1,045	1,050	1,110	0.75	0.79	0.80	0.84
L4		WB	1,320	300	260	365	300	0.23	0.20	0.28	0.23
L5	Tai Lam Chung Road	NB	1,020	295	210	380	290	0.29	0.21	0.37	0.28
L6		SB	1,020	365	290	485	360	0.36	0.28	0.48	0.35
L7	Luen Hong Lane	EB	1,020	110	110	200	195	0.11	0.11	0.20	0.19
L8		WB	1,020	165	95	290	175	0.16	0.09	0.28	0.17
L9	Castle Peak Road – Tai Lam	EB	3,600	2,085	1,030	2,105	1,050	0.58	0.29	0.58	0.29
L10		WB	3,600	725	1,275	745	1,295	0.20	0.35	0.21	0.36



L11	Castle Peak Road – Tai Lam	EB	3,600	1,580	390	1,595	395	0.44	0.11	0.44	0.11
L12		WB	3,600	270	395	295	420	0.08	0.11	0.08	0.12
L13	Tuen Mun Road	EB	7,560	5,970	4,245	5,990	4,265	0.79	0.56	0.79	0.56
L14		WB	5,640	3,385	5,125	3,415	5,155	0.60	0.91	0.61	0.91
L15	Slip Road of Tuen Mun Road	EB	1,320	810	415	880	445	0.61	0.31	0.67	0.34
L16		WB	1,320	665	910	695	940	0.50	0.69	0.53	0.71

Note:

- (1) Refer to **Figure 3.1**
- (2) Derived with reference to Table 2.4.1.1 in TPDM Volume 2 – Chapter 2.4

6.3.2 The assessment results presented that Tuen Mun Road will be overloaded during PM peak period. However, the results reflected the effects on V/C ratios of the road links due to the Proposed Scheme is negligible. It should also be noted that an investigation study on Route 11 (section between Yuen Long and North Lantau) had commenced 2023 with a target to commissioning this major road by 2033. Subsequent to the commissioning of Route 11, the V/C ratio of the concerned road link are expected to improve.

6.3.3 Apart from Tuen Mun Road, all other road links will be operating with adequate capacities in both Reference and Design Case.

6.4 Pedestrian Assessment

Existing Pedestrian Condition

6.4.1 To review the existing pedestrian situation, pedestrian head count surveys have been conducted at the footpath sections in the vicinity of the subject site on 17 January 2025 during AM and PM peak period. The observed AM and PM peak hour fall within 8:10am to 9:10am and 5:10pm to 6:10pm respectively. The observed pedestrian flows are shown in **Figure 6.3**.

6.4.2 The footpath sections are assessed by the observed peak pedestrian flows with reference to the criteria of Level-Of-Service (LOS) from HCM 2000 as exhibited in TPDM. The LOS assessment results are summarized in **Table 6.5**.

Table 6.5 Existing Footpath Operation Performance

Location ⁽¹⁾	Clear Width (m)	Effective Width ⁽²⁾ (m) (a)	Observed 2-way Pedestrian (Ped/hr) (b)		Level-of-Services ⁽³⁾ (c) = (b) / (a) / 60mins	
			AM	PM	AM	PM
FP1	1.6	0.6	10	20	A	A
FP2	2.0	1.0	40	125	A	A
FP3	3.6	2.6	45	120	A	A
FP4	2.7	1.7	30	95	A	A

Note:

- (1) Refer to **Figure 6.3**
- (2) Effective width = Clear width – 0.5m dead width on both sides.
- (3) Based on the criteria of LOS from HCM 2000 as exhibited in TPDM. In general, LOS A & B provide a good walking environment; LOS C & D are acceptable values; and LOS E & F reflect the design volume has approached or over the limit of walking capacity.

6.4.3 As shown in **Table 6.5**, it is indicated that the footpath sections in the vicinity of the subject site are currently operating at acceptable LOS during peak hours.

Future Pedestrian Condition

6.4.4 As discussed in **Section 4**, a transport interchange will be provided within the

Proposed Development to cope with the public transport demand induced by the subject site. It is anticipated that the proposed transport interchange would serve most of the public transport demand to various destinations via Tuen Mun Bus-Bus interchange.

- 6.4.5 However, it is understood that the future public transport proposal as discussed in **Section 4.2** is still under reviewed by the relevant government departments as well as local stakeholders. To review the future pedestrian condition surrounding the Proposed Development, it is assumed that all the future public transport demand as derived in **Table 4.1** would use the Tai Lam Chung Bus Terminus to Tuen Mun BBI for conservative assessments.
- 6.4.6 Therefore, a set of future pedestrian flows on the key routes to Tai Lam Chung Bus Terminus in design year 2033 are derived by applying a growth rate of 1.86% p.a. (refers to **Section 5.2**) onto the observed pedestrian flows (**Figure 6.3**) upto the design year 2033 and superimposing the potential future public demand induced by the Proposed Development as derived in **Table 4.1** with anticipated pedestrian routing as demonstrated in **Figure 6.4**. The future year 2033 pedestrian flows are shown in **Figure 6.5**.
- 6.4.7 The LOS assessments for design year 2033 pedestrian flows are conducted, and the results are summarized in **Table 6.6**.

Table 6.6 2033 Design Pedestrian Assessment Results

Location ⁽¹⁾	Clear Width (m)	Effective Width ⁽²⁾ (m) (a)	Observed 2-way Pedestrian (Ped/hr) (b)		Level-of-Services ⁽³⁾ (c) = (b) / (a) / 60mins	
			AM	PM	AM	PM
FP1	1.6	0.6	145	160	A	A
FP2	2.0	1.0	1,115	1,215	B	B
FP3	3.6	2.6	1,255	1,345	A	A
FP4 ⁽⁴⁾	2.0	1.0	1,240	1,315	B	B

Note:

- (1) Refer to **Figure 6.5**
- (2) Effective width = Clear width – 0.5m dead width on both sides.
- (3) Based on the criteria of LOS from HCM 2000 as exhibited in TPDM. In general, LOS A & B provide a good walking environment; LOS C & D are acceptable values; and LOS E & F reflect the design volume has approached or over the limit of walking capacity.
- (4) The road layout of approved Road Gazette 2728 refers to **Annex A**

- 6.4.8 The LOS assessment results suggested that all the footpath sections will still be operating at acceptable LOS in design year 2033.

7 REVIEW ON TRAFFIC IMPACT DURING CONSTRUCTION STAGE

7.1 Construction Year

- 7.1.1 Considering the completion of the Proposed Development is scheduled in 2030, the traffic impact induced by the construction traffic in year 2030 is assessed for conservative approach.

7.2 Traffic Forecast for Construction Stage

Growth Factor

- 7.2.1 As discussed in **Section 5**, an annual growth factor of 1.86% per annum as derived in **Table 5.1** is applied onto the traffic flows as observed in year 2023 to project the peak hour traffic flows from year 2023 to 2030.

Anticipated Construction Traffic

- 7.2.2 Taking into account of the site area of the Proposed Development, it is preliminary estimated that the construction traffic to/from the Application Site would be approximately 30 pcu/hr. The anticipated peak hourly construction traffic demand are summarized in **Table 7.1**.

Table 7.1 Anticipated Peak Hourly Construction Traffic

Anticipated Peak Hour Construction Traffic	
Generation	Attraction
30 pcu/hr	30 pcu/hr

- 7.2.3 The ingress and egress routes of the construction traffic are subject to approval of the Waste Management Plan and the future contactors/suppliers to be awarded. Nevertheless, it is assumed that construction trucks would mainly for delivery to/from Tuen Mun Area 38 fill bank.
- 7.2.4 The traffic forecasts are derived by applying a growth factor onto the observed background traffic flows and superimposing the traffic of the planned future development as listed in **Table 5.4** as well as the construction traffic as derived in **Table 7.1**. The future year 2030 traffic flows are presented in **Figure 7.1**.

7.3 Junction Assessments during Construction Stage

- 7.3.1 The performance of the road junctions are assessed with the derived 2030 traffic flows for the construction stage are summarized in **Table 7.2** respectively.

Table 7.2 Junction Performance in 2030 during Construction Stage

Ref.	Junction	Indicator ⁽¹⁾	2030 Construction Stage	
			AM Peak	PM Peak
J1	Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam	DFC	0.74	0.44
J2	Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau	DFC	0.41	0.16
J3	Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road	RC	>100%	>100%
J4	Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road	RC	>100%	>100%
J5	Castle Peak Road – Tai Lam / Tai Lam Chung Road ⁽²⁾	RC	81%	108%
J6	Tai Lam Chung Road / Luen Hong Lane ⁽³⁾	DFC	0.08	0.09

Notes:

- (1) RC = Reserve Capacity for signal junction; DFC = Design Flow / Capacity ratio for priority junction or roundabout
- (2) Existing road layout refers to **Figure 3.6**
- (3) Existing road layout refers to **Figure 3.7**

7.3.2 The assessment results in **Table 7.2** showed that the road junctions in the vicinity of the Proposed Development will be operating within capacities during the peak hours in year 2030 with the construction works at the subject site. The corresponding junction calculation sheets are attached in **Annex B**.

8 CONCLUSION

8.1 Summary

- 8.1.1 The Application Site covers various lots and adjacent Government land in DD 385, Tai Lam Chung, Tuen Mun, New Territories. The Site is bounded by Luen Hong Lane to the west with a development site area for private residential development of about 46,493m².
- 8.1.2 Compared to the Approved Scheme, the Applicant now proposed a higher domestic plot ratio of 2.107 which comprises of 2,670 nos. of residential units with an average flat size of about 40.6m².
- 8.1.3 The parking and loading/unloading facilities of the proposed development would be provided in accordance with the requirements as stipulated in the HKPSG.
- 8.1.4 In order to review the existing traffic condition, traffic count surveys were conducted at the following 6 identified critical junctions to investigate the traffic condition during commuting peak hours. At present, all the critical junctions are operating within capacity.
- Castle Peak Road – Tai Lam / Castle Peak Road – New Tai Lam (J1)
 - Castle Peak Road – New Tai Lam / Castle Peak Road – Tsing Lung Tau (J2)
 - Castle Peak Road – Tai Lam / Slip Road from Tuen Mun Road (J3)
 - Castle Peak Road – Tai Lam / Slip Road to Tuen Mun Road (J4)
 - Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5)
 - Tai Lam Chung Road / Luen Hong Lane (J6)
- 8.1.5 To serve the additional public transport demand as induced by the Proposed Development, additional bus trips will be introduced during the peak hours. Also, 3 nos. of bus layby and 4 nos. of GMB layby are proposed at proposed privately-operated transport interchange to cater for the potential transport demand arising from the proposed development.
- 8.1.6 By comparing the trip generation/ attractions of the proposed development under Approved Scheme and Proposed Scheme, the Proposed Scheme will induce additional 208 pcu/hr (two-way) during AM peak hour and 160 pcu/hr (two-way) during PM peak hour.
- 8.1.7 The proposed development is tentatively scheduled for completion in 2030. According to Guidelines and Requirements of TIA Studies, the TIA should assess at least 3 years after the planned completion of the Proposed Development. Hence, 2033 is adopted as the design year for this TIA.
- 8.1.8 In order to carry out traffic forecast and examine traffic impact due to the Proposed Development in year 2033, Annual Growth Rate method is applied to estimate the traffic forecast year 2033. The annual growth rate is made reference to 2019-based TPEDM, Projection of Population Distribution from Planning Department and the historical traffic data from ATC which is available on Transport Department's website. It is proposed to adopt an annual growth rate of 1.86% per annum for projecting the peak hour traffic flow from 2023 to 2033.

- 8.1.9 The 2033 reference traffic flows were derived based on the observed traffic demands and circulation pattern by adopting an appropriate growth rate with consideration of the planned developments within the vicinity and the anticipated trips generated by the Approved Scheme.
- 8.1.10 The additional trip due to the Proposed Scheme have been superimposed onto the anticipated 2033 reference traffic flows to produce 2033 design traffic flows.
- 8.1.11 Junction capacity assessment was conducted for both 2033 reference and design scenarios, taking into consideration the committed junction improvement at Junction of Castle Peak Road – Tai Lam / Tai Lam Chung Road (J5) under previously approved planning application and the approved gazette plan of Luen Hong Lane (J6). The results revealed that all junctions would be operating within capacity under the design case in 2033 except for J5. In light of this, further junction improvement scheme at J5 is formulated for improving the junction performance and will be carried out by the project proponent prior to the completion of the Proposed Development. With the said further junction improvement scheme, J5 would operate with sufficient capacity in 2033 design case.
- 8.1.12 The performance of footpath surrounding the subject site will also operate in adequate capacity together with the proposed public transport demand induced by the Proposed Development.
- 8.1.13 The traffic impact during the construction stage is also reviewed. Taking into consideration the site area of the subject site, it is preliminary estimated that the construction to/from the Application Site would be approximately 30 pcu/hr, and is assumed that the construction traffic would be mainly for delivery to/from Tuen Mun Area 38 fill bank subject to further construction arrangement by the future contractor. The results revealed that all junctions will be operating within capacity in year 2030 (i.e. completion year of the proposed development).

8.2 Conclusion

- 8.2.1 In light of the findings of this TIA, it is concluded that there is no insurmountable traffic impact imposed onto the local road network due to the Proposed Development. With the proposed mitigation measures in place, the Proposed Development is technically feasible in traffic terms.

Figure

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PATH C:\Users\chanowa\OneDrive - AECOM\OneDrive - AECOM\JOB BACKUP\060281828\TAIFigure 1.1.dgn



LEGEND:
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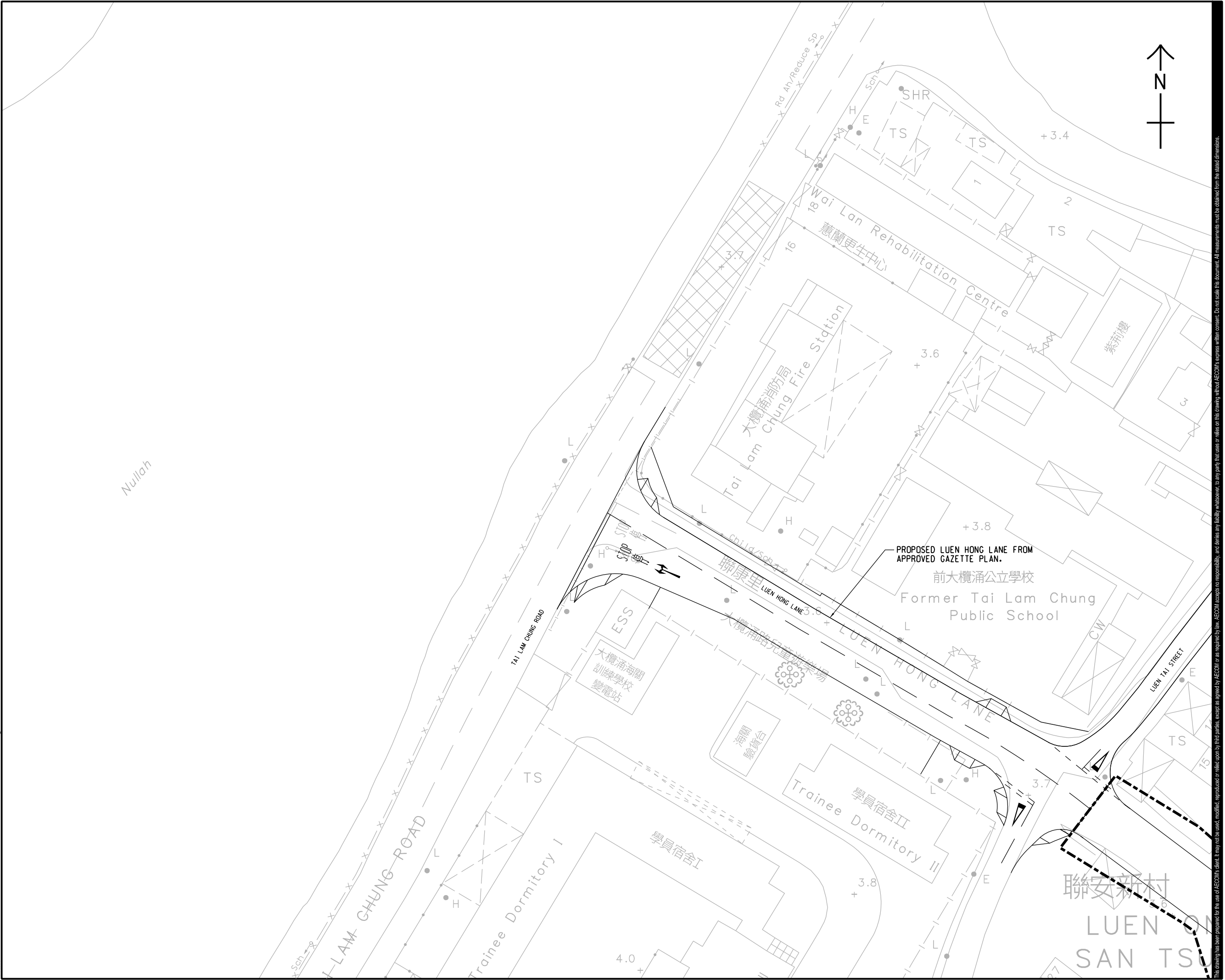
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FIGURE 1.1

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FIGURE 2.1



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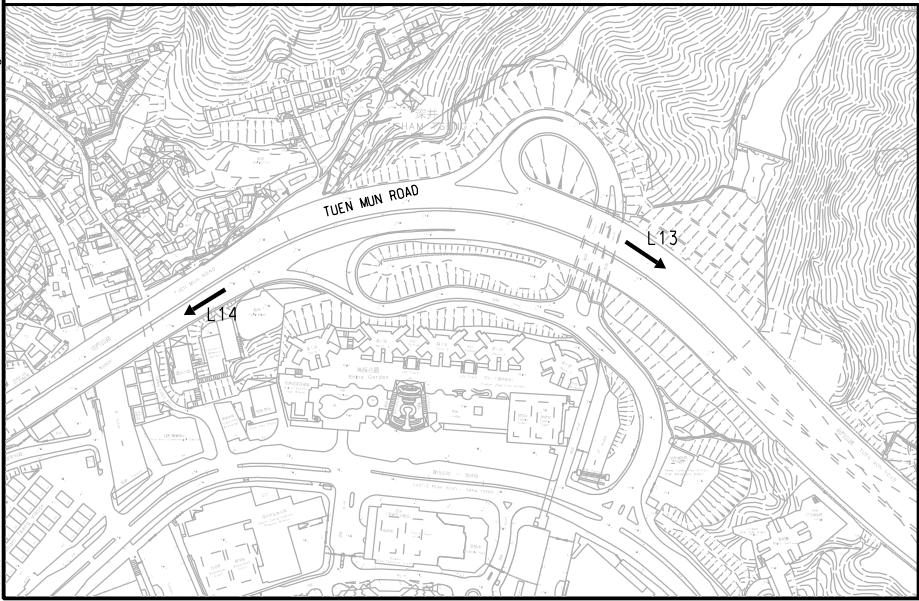
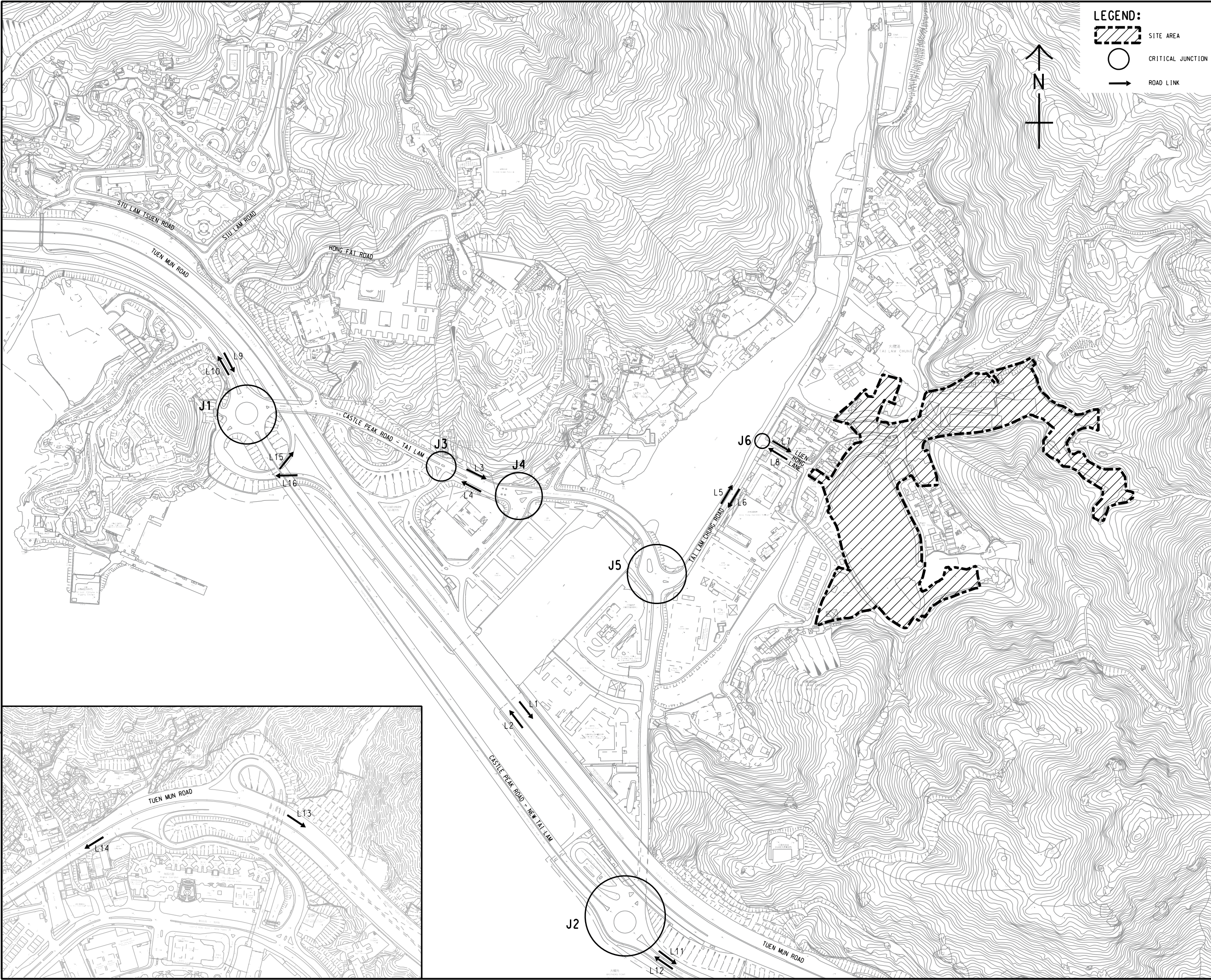
圖紙名稱

VEHICULAR ACCESS ARRANGEMENT

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FIGURE 2.2



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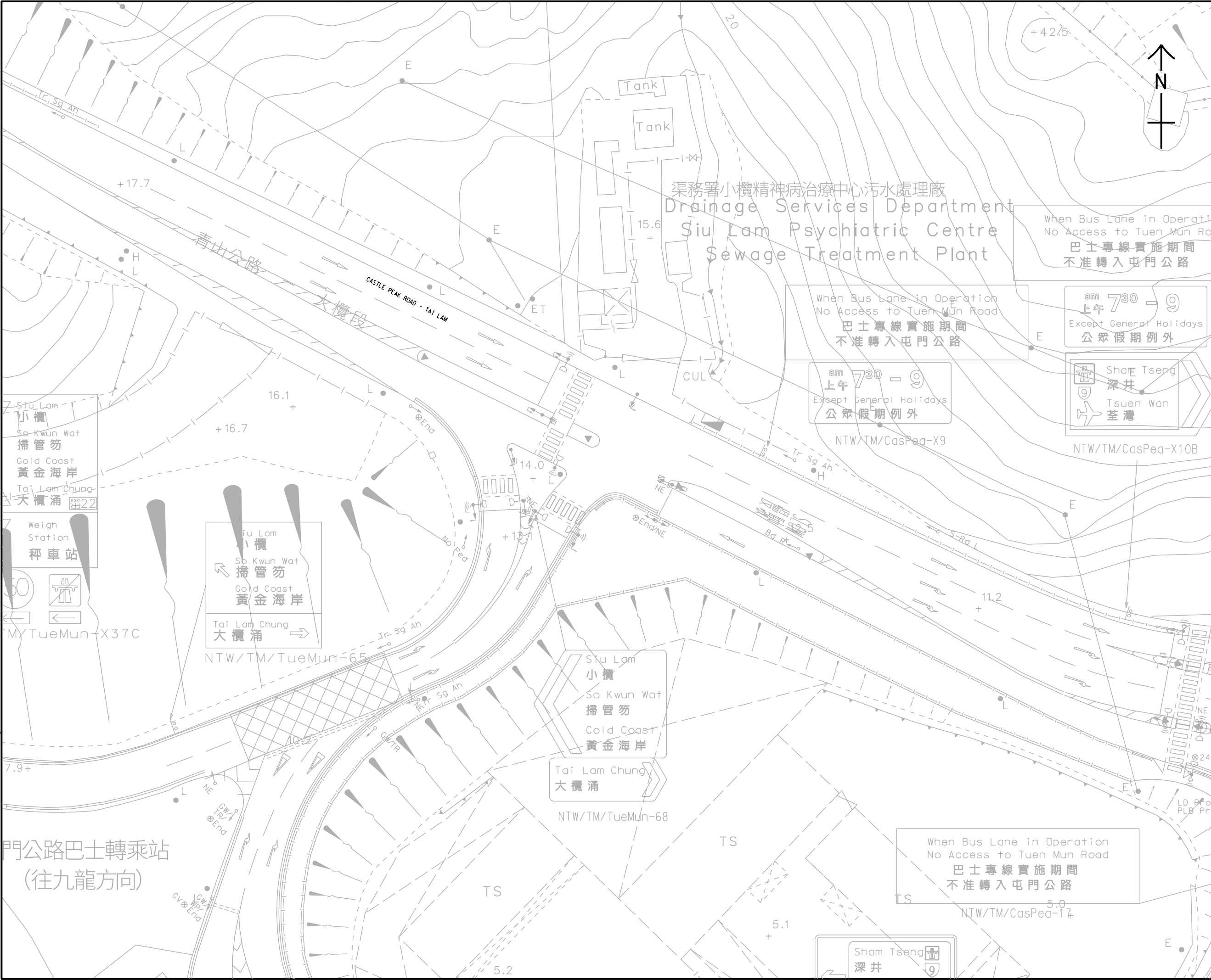
SHEET TITLE
圖紙名稱
CRITICAL JUNCTIONS AND ROAD LINK

SHEET NUMBER
圖紙編號

FIGURE 3.1

FIGURE 3.3

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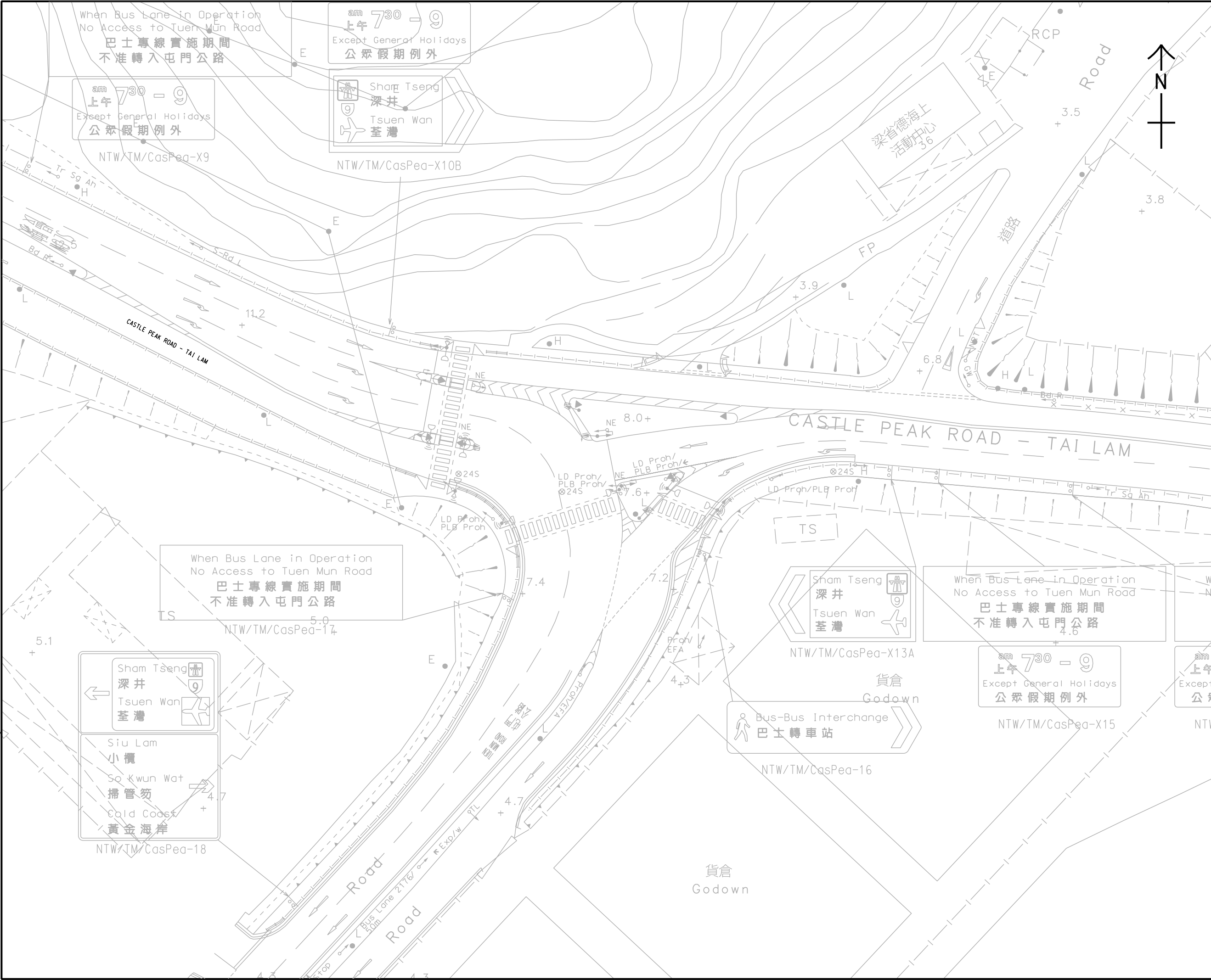
EXISTING JUNCTION LAYOUT OF CASTLE PEAK ROAD - TAI LAM / SLIP ROAD FROM TUEN MUN ROAD (J3)

SHEET NUMBER

圖紙編號

FIGURE 3.4

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A3 1: 500

KEY PLAN
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項目編號
60281828

SHEET TITLE
圖紙名稱
EXISTING JUNCTION LAYOUT OF CASTLE PEAK ROAD – TAI LAM / SLIP ROAD TO TUEN MUN ROAD (J4)

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FIGURE 3.5

FIGURE 3.6



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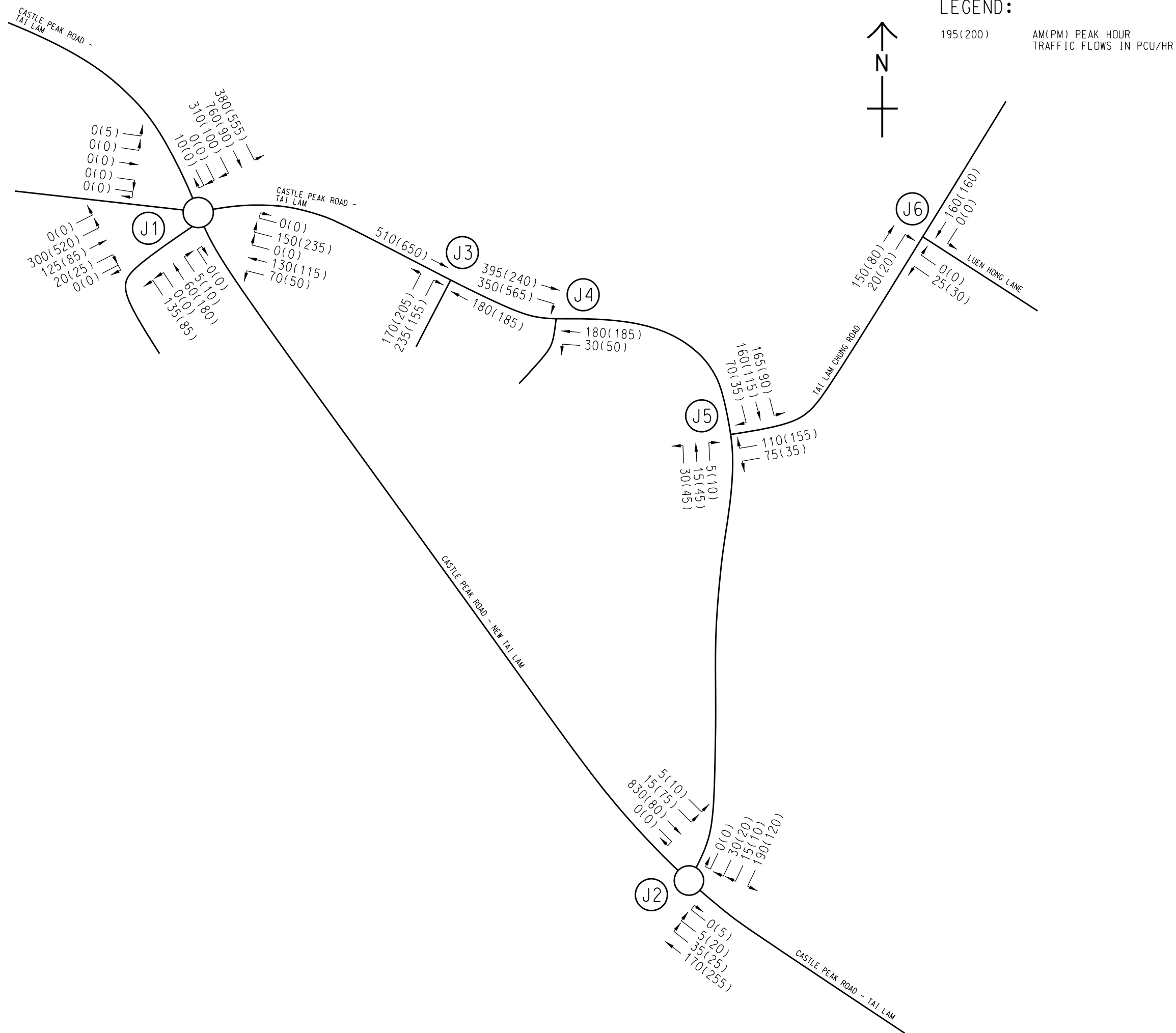
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SHEET TITLE
圖紙名稱

EXISTING JUNCTION LAYOUT OF
TAI LAM CHUNG ROAD /
LUEN HONG LANE (J6)

SHEET NUMBER
圖紙編號

FIGURE 3.7

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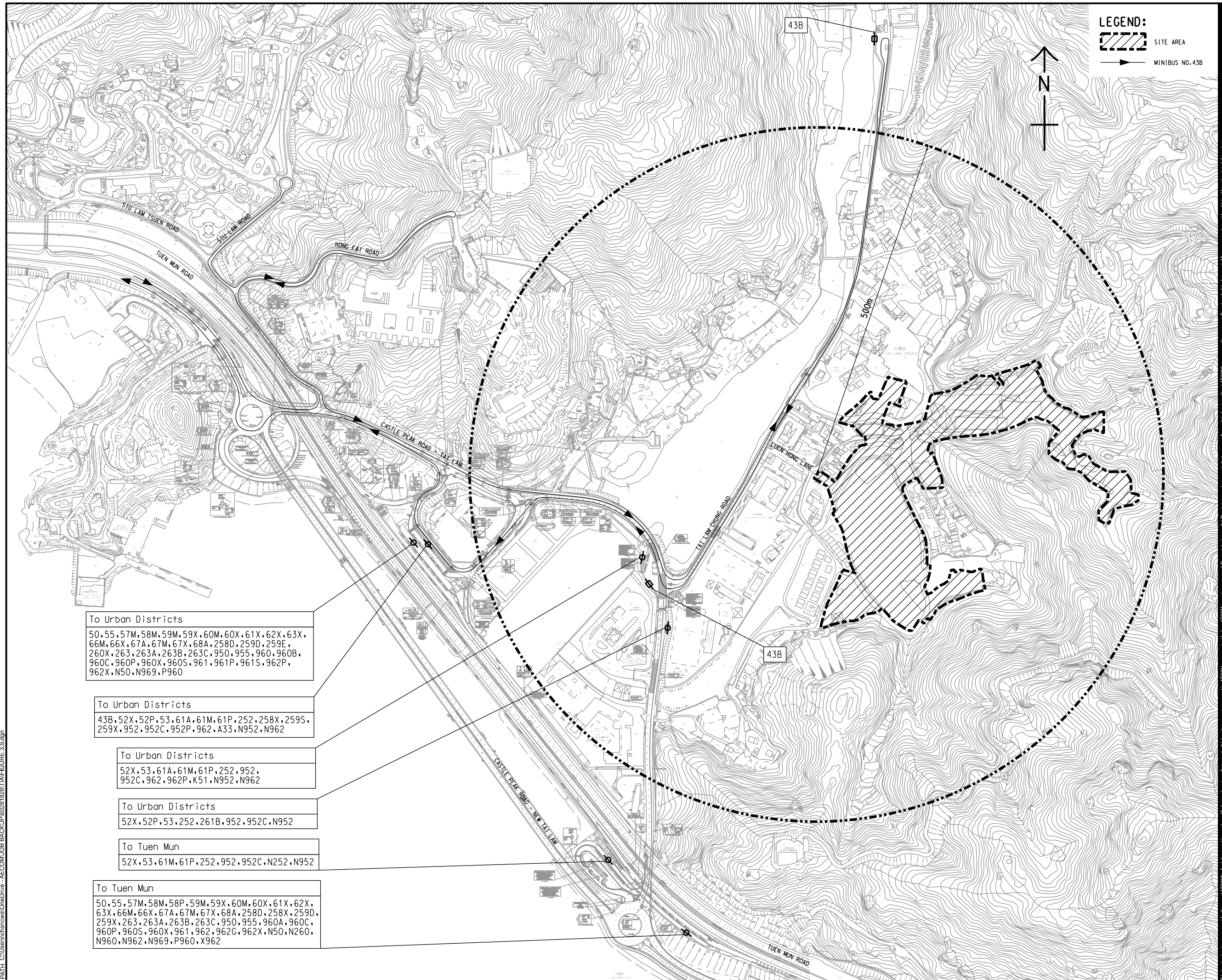
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YEAR 2023 OBSERVED TRAFFIC FLOWS

SHEET NUMBER

FIGURE 3.8

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EXISTING PUBLIC TRANSPORT FACILITIES

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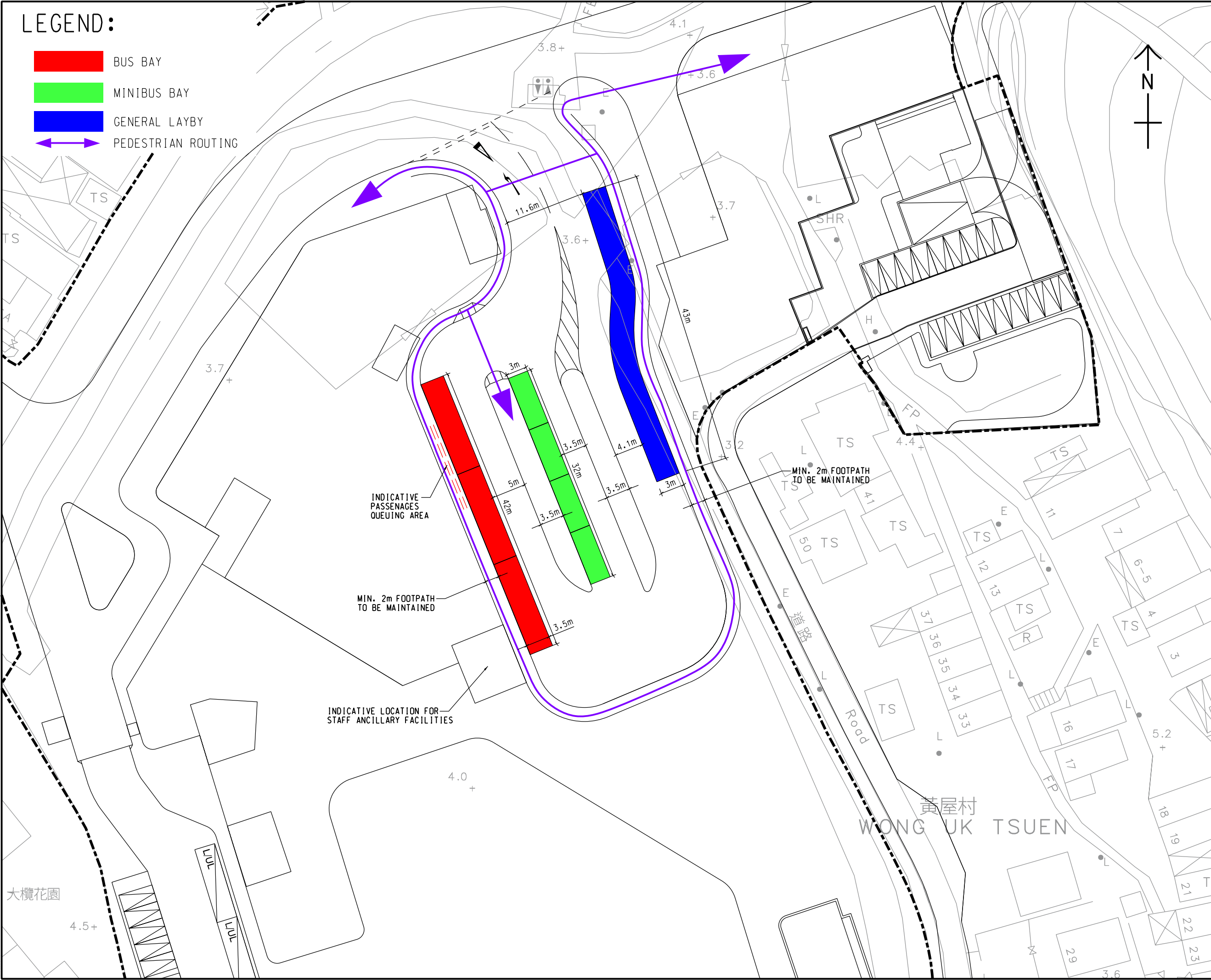
FIGURE 3.9

FIGURE 12-10

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PATH \$FILES
SDATES

LEGEND:

- BUS BAY
- MINIBUS BAY
- GENERAL LAYBY
- PEDESTRIAN ROUTING



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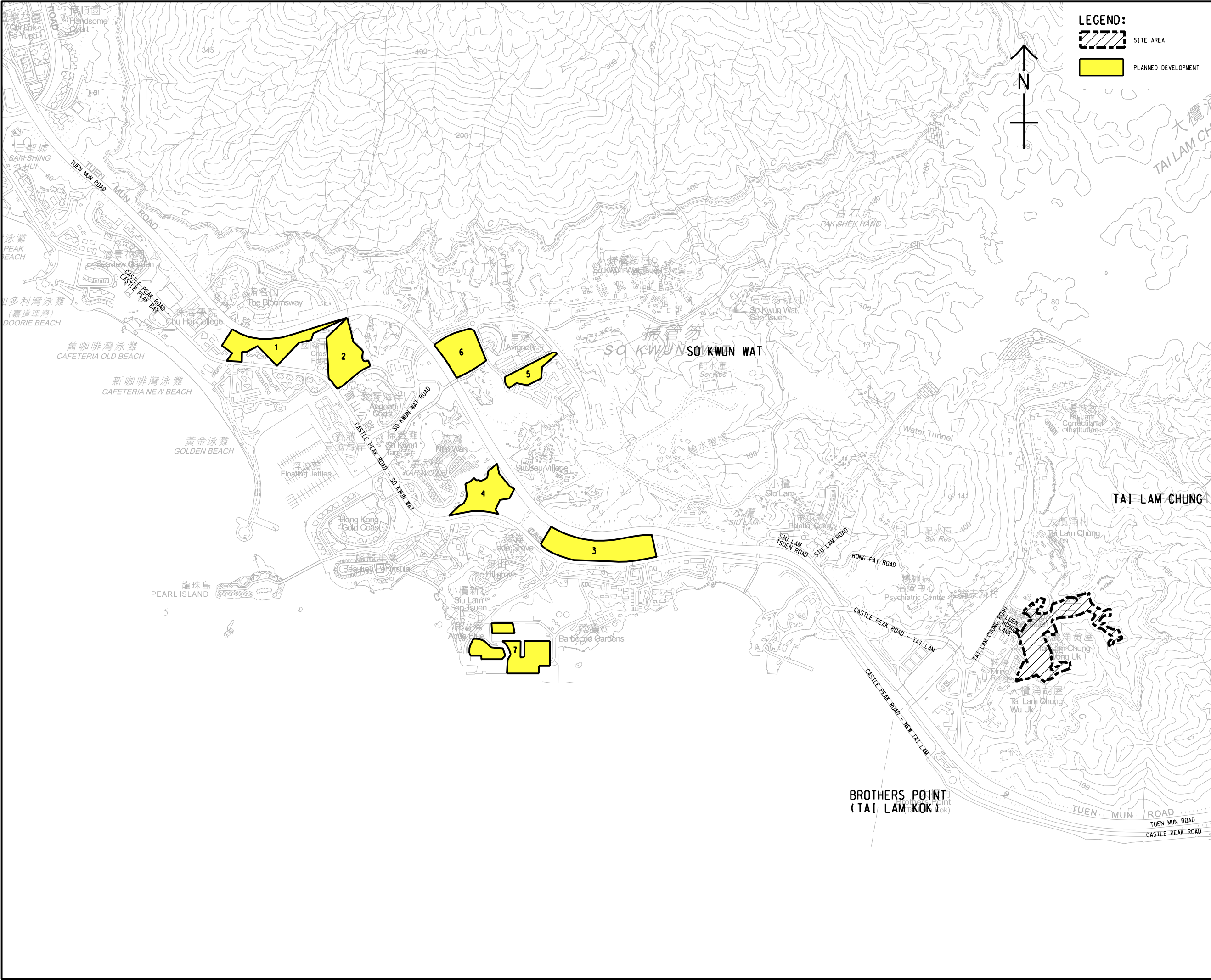
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SHEET TITLE
圖紙名稱

INDICATIVE LAYOUT FOR TRANSPORT INTERCHANGE (TI)

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FIGURE 4.1



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圖紙名稱

PLANNED DEVELOPMENTS

SHEET NUMBER
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FIGURE 5.1

CONTRACT NO.
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LEGEND:

195(200) AM(PM) PEAK HOUR TRAFFIC FLOWS IN PCU/HR



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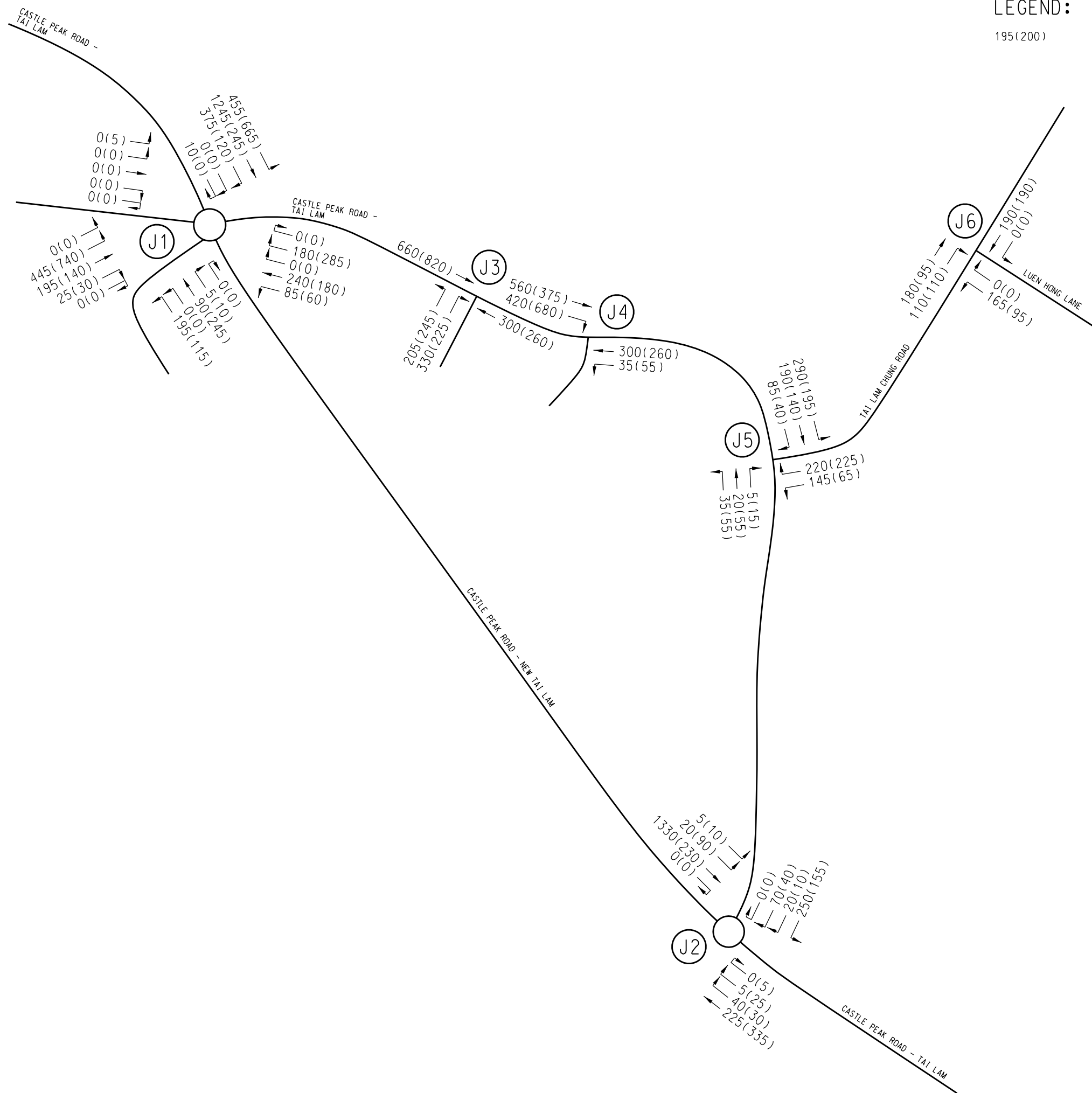
SHEET TITLE
圖紙名稱

YEAR 2033 BACKGROUND TRAFFIC FLOWS

SHEET NUMBER
圖紙編號

FIGURE 5.2

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LEGEND:

195(200)

AM(PM) PEAK HOUR
TRAFFIC FLOWS IN PCU/HR



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APPROVED COMPREHENSIVE RESIDENTIAL
DEVELOPMENT SCHEME AND MINOR
RELAXATION OF GROSS FLOOR AREA AND
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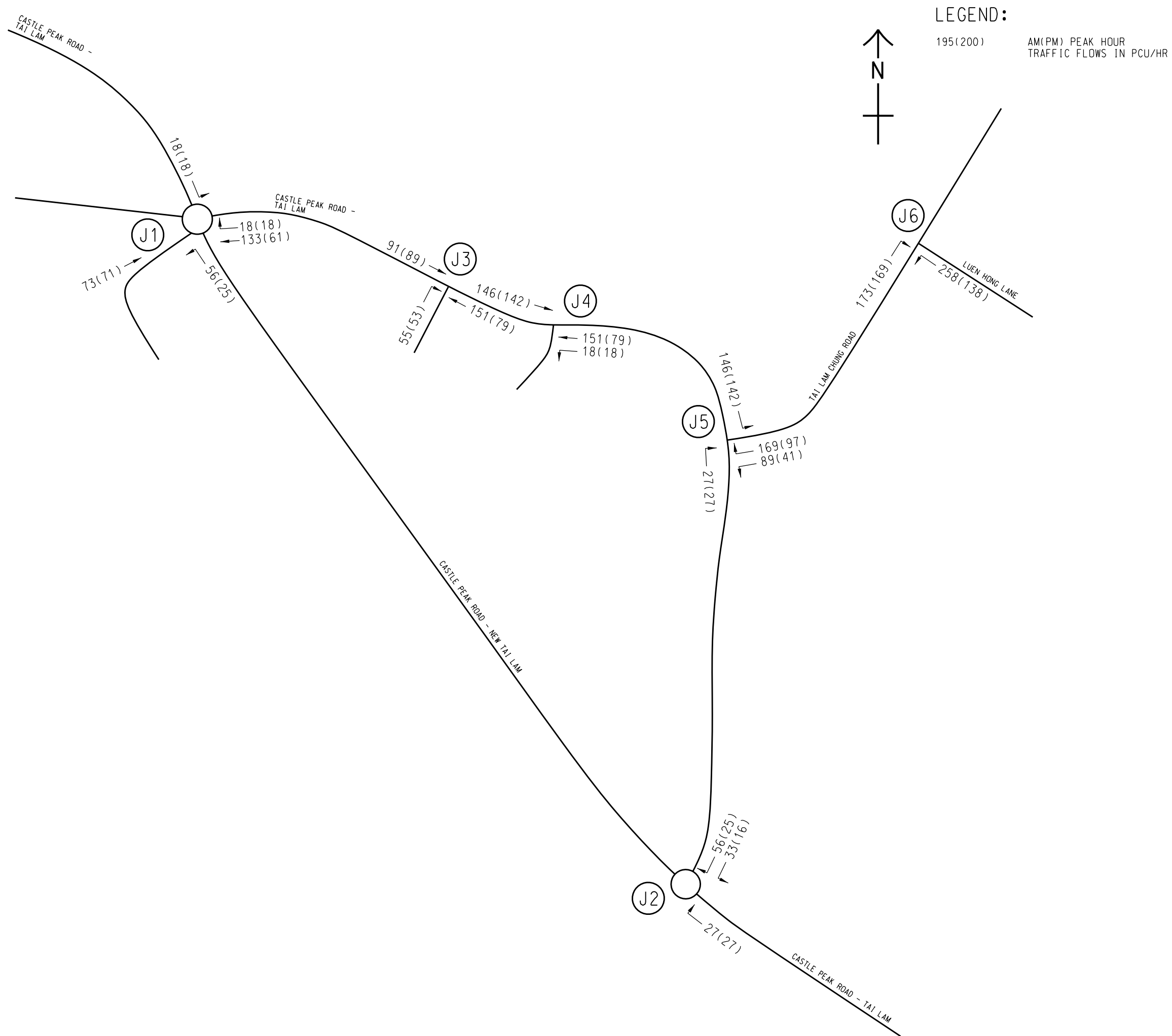
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SHEE
四新名報

YEAR 2033 REFERENCE
TRAFFIC FLOWS

SHEET NUMBER

FIGURE 5.3

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SHEET TITLE
[73] 94C 1 x 80

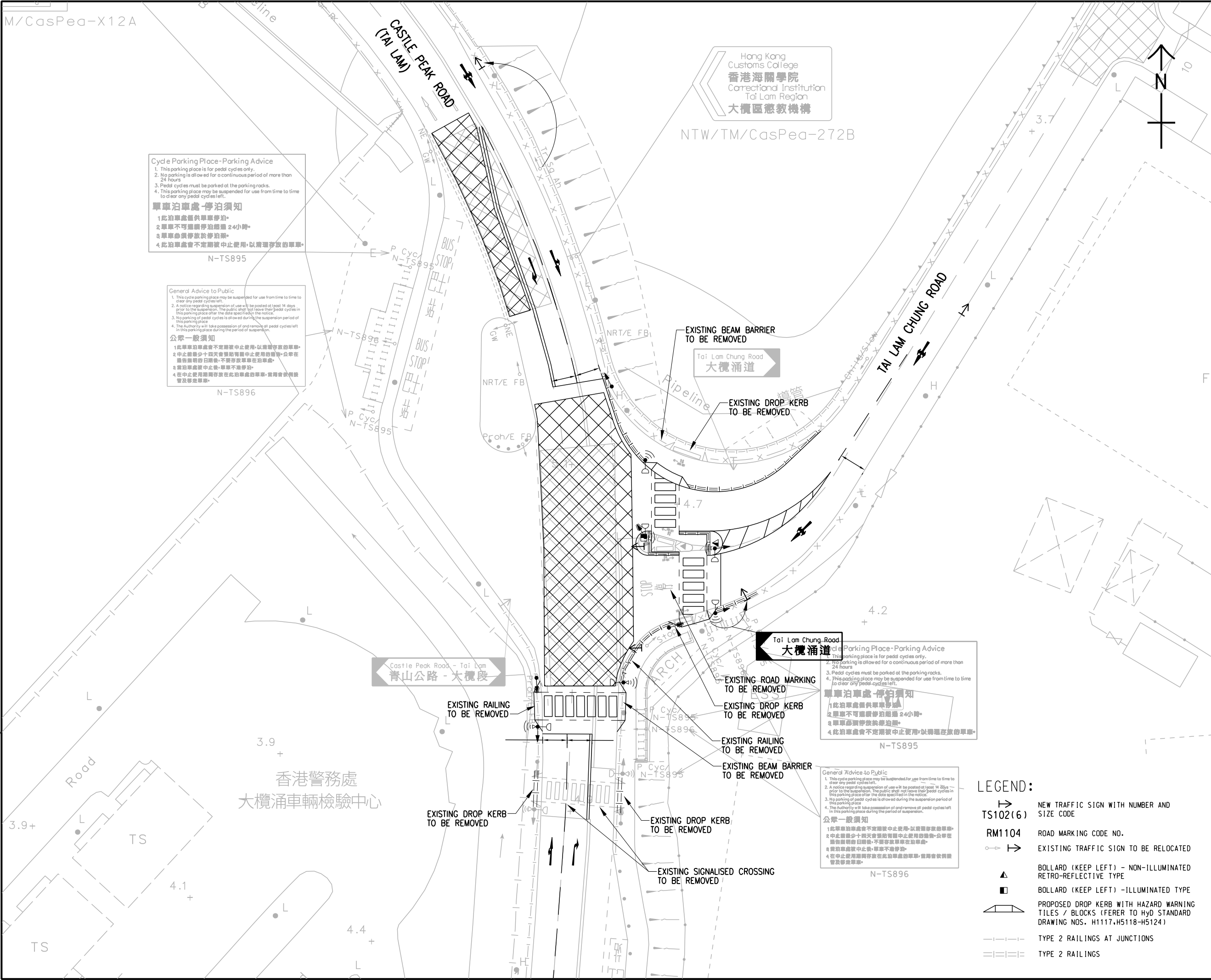
DEVELOPMENT TRAFFIC FLOW
(PROPOSED SCHEME)

SHEET NUMBER

FIGURE 5.4

ISO A1 594mm x 841mm
Approved:
Checked:
Designer:
Project Management Initials:

Plot File by: chancowa 8/4/2025
PATH: C:\Users\chancwa\OneDrive - AECOM\JOB BACKUP\00281828\TAIFIGURE 6.1.dgn



Cycle Parking Place-Parking Advice

1. This parking place is for pedal cycles only.
2. No parking is allowed for a continuous period of more than 24 hours.
3. Pedal cycles must be parked at the parking racks.
4. This parking place may be suspended for use from time to time to clear any pedal cycles left.

單車泊車處-停泊須知

1 此泊車處僅供單車停泊
2 單車不可連續停泊超過 24 小時
3 單車必須停放於停泊架
4 此泊車處會不定期被中止使用,以清理存放的單車

General Advice to Public

1. This cycle parking place may be suspended for use from time to time to clear any pedal cycles left.
2. A notice regarding suspension of use will be posted at least 14 days prior to the suspension. The public should leave their pedal cycles in this parking place after the date specified in the notice.
3. No parking of pedal cycles is allowed during the suspension period of this parking place.
4. The Authority will take possession of and remove all pedal cycles left in this parking place during the period of suspension.

公眾一般須知

1 此單車泊車處會不定期被中止使用,以清理存放的單車
2 中止前最少十四天會張貼有關中止使用的通告,公眾在通告張貼後十四天後,不要存放單車在泊車處
3 當泊車處中止後,單車不准停泊
4 在中止使用期間存放在此泊車處的單車,當局會收得該單車及移走單車

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3 當泊車處中止後,單車不准停泊
4 在中止使用期間存放在此泊車處的單車,當局會收得該單車及移走單車

- LEGEND:**
- ➡ TS102(6) NEW TRAFFIC SIGN WITH NUMBER AND SIZE CODE
 - RM1104 ROAD MARKING CODE NO.
 - ➡ EXISTING TRAFFIC SIGN TO BE RELOCATED
 - ▲ BOLLARD (KEEP LEFT) - NON-ILLUMINATED RETRO-REFLECTIVE TYPE
 - BOLLARD (KEEP LEFT) -ILLUMINATED TYPE
 - ▬ PROPOSED DROP KERB WITH HAZARD WARNING TILES / BLOCKS (FERER to HyD STANDARD DRAWING NOS. H1117,H5118-H5124)
 - |—|—|— TYPE 2 RAILINGS AT JUNCTIONS
 - ==||== TYPE 2 RAILINGS



PROJECT
項目

SECTION 16 PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO AN APPROVED COMPREHENSIVE RESIDENTIAL DEVELOPMENT SCHEME AND MINOR RELAXATION OF GROSS FLOOR AREA AND BUILDING HEIGHT RESTRICTIONS AT VARIOUS LOTS IN D.D. 385 AND ADJOINING GOVERNMENT LAND, TAI LAM CHUNG, TUEN MUN

CLIENT
業主

SUN HUNG KAI
REAL ESTATE AGENCY LTD.

CONSULTANT
工程顧問公司

AECOM Asia Company Ltd.
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SUB-CONSULTANTS
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ISSUE/REVISION				
修訂				
I/R	DATE	DESCRIPTION	CHK.	
修訂	日期	內容摘要	校核	校核

STATUS
階段

SCALE
比例

A3 1: 500

KEY PLAN
索引圖

PROJECT NO.
項目編號

60281828

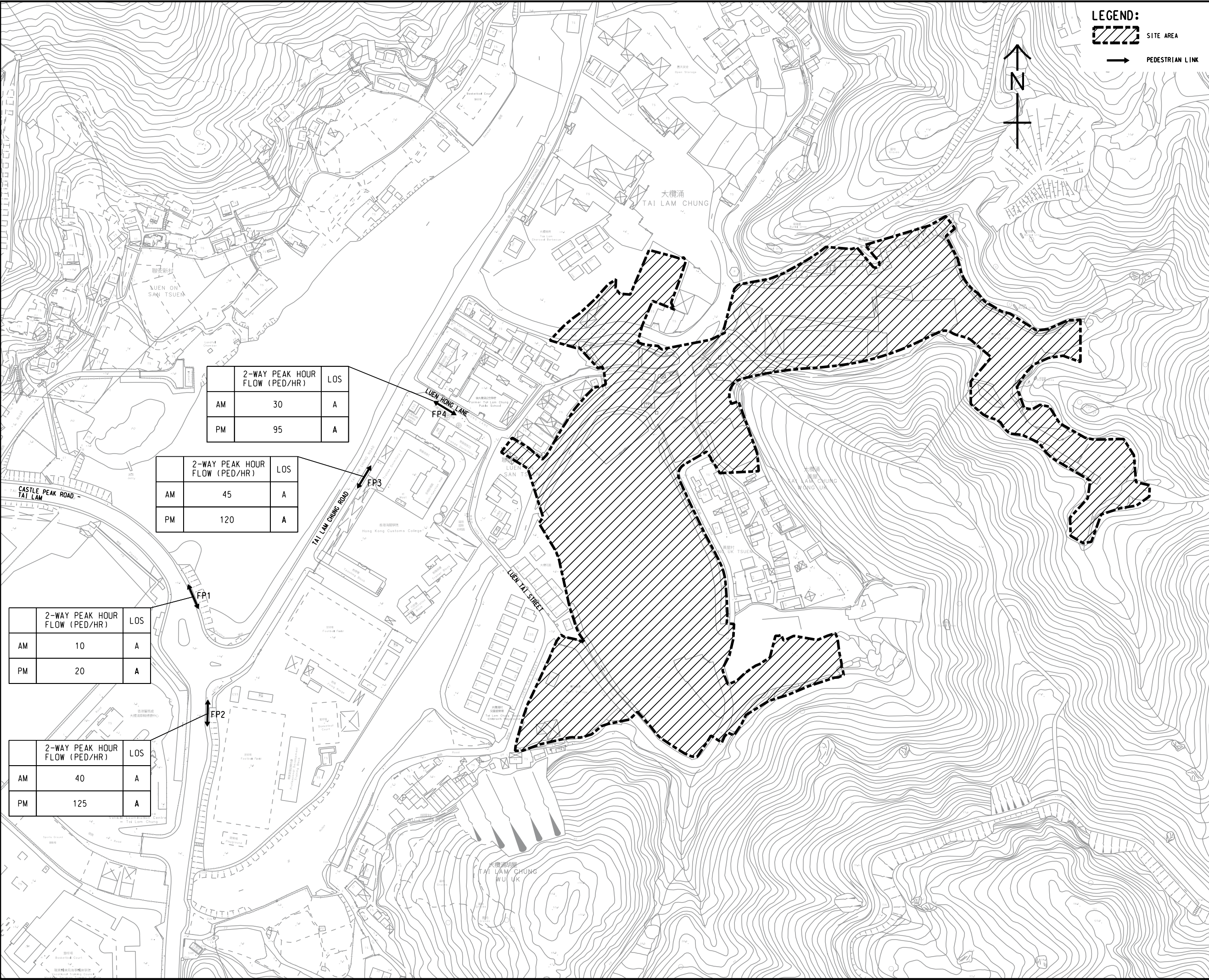
SHEET TITLE
圖紙名稱

PLANNED JUNCTION IMPROVEMENT SCHEME AT CASTLE PEAK ROAD - TAI LAM / TAI LAM CHUNG ROAD

SHEET NUMBER
圖紙編號

FIGURE 6.1





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項目
SECTION 16 PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO AN APPROVED COMPREHENSIVE RESIDENTIAL DEVELOPMENT SCHEME AND MINOR RELAXATION OF GROSS FLOOR AREA AND BUILDING HEIGHT RESTRICTIONS AT VARIOUS LOTS IN D.D. 385 AND ADJOINING GOVERNMENT LAND, TAI LAM CHUNG, TUN MUN

CLIENT
業主
SUN HUNG KAI
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ISSUE/REVISION
修訂

I/R 修訂	DATE 日期	DESCRIPTION 內容摘要	CHK. 校核
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STATUS
階段

SCALE
比例

A3 1 : 2500

KEY PLAN
索引圖

PROJECT NO.
項目編號

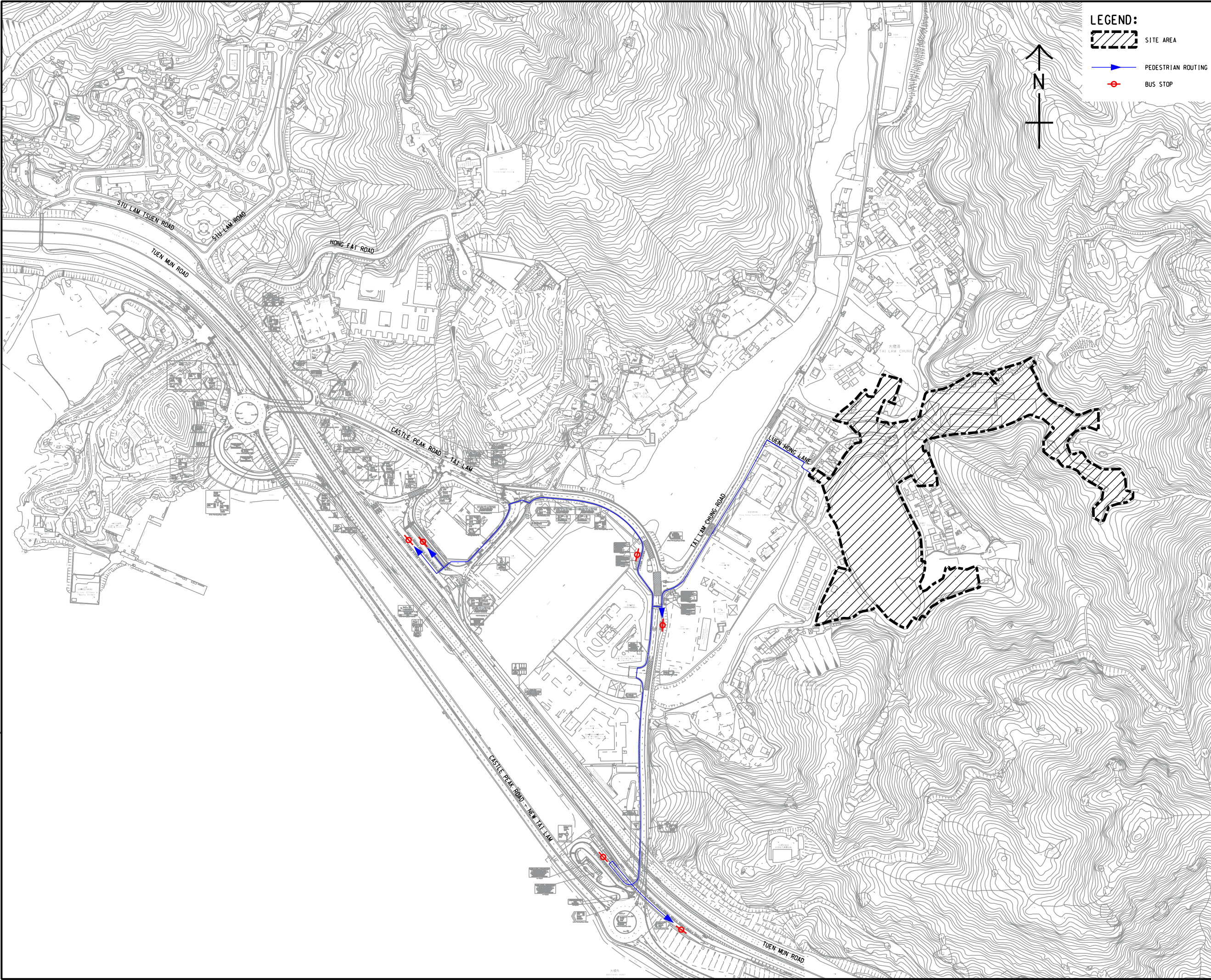
60281828

SHEET TITLE
圖紙名稱

YEAR 2024 OBSERVED
PEDESTRIAN FLOWS

SHEET NUMBER
圖紙編號

FIGURE 6.3



LEGEND:

SITE AREA

PEDESTRIAN ROUTING

BUS STOP

AECOM

PROJECT

SECTION 16 PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO AN APPROVED COMPREHENSIVE RESIDENTIAL DEVELOPMENT SCHEME AND MINOR RELAXATION OF GROSS FLOOR AREA AND BUILDING HEIGHT RESTRICTIONS AT VARIOUS LOTS IN D.D. 385 AND ADJOINING GOVERNMENT LAND, TAI LAM CHUNG, TUEN MUN

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修訂	日期	內容簡述	核校

STATUS

現狀

SCALE

比例

DIMENSION UNIT

尺寸單位

A3 1 : 5000

KEY PLAN

索引圖

PROJECT NO.

項目編號

60281828

SHEET TITLE

圖紙名稱

PEDESTRIAN ROUTING TO EXISTING BUS STOPS

SHEET NUMBER

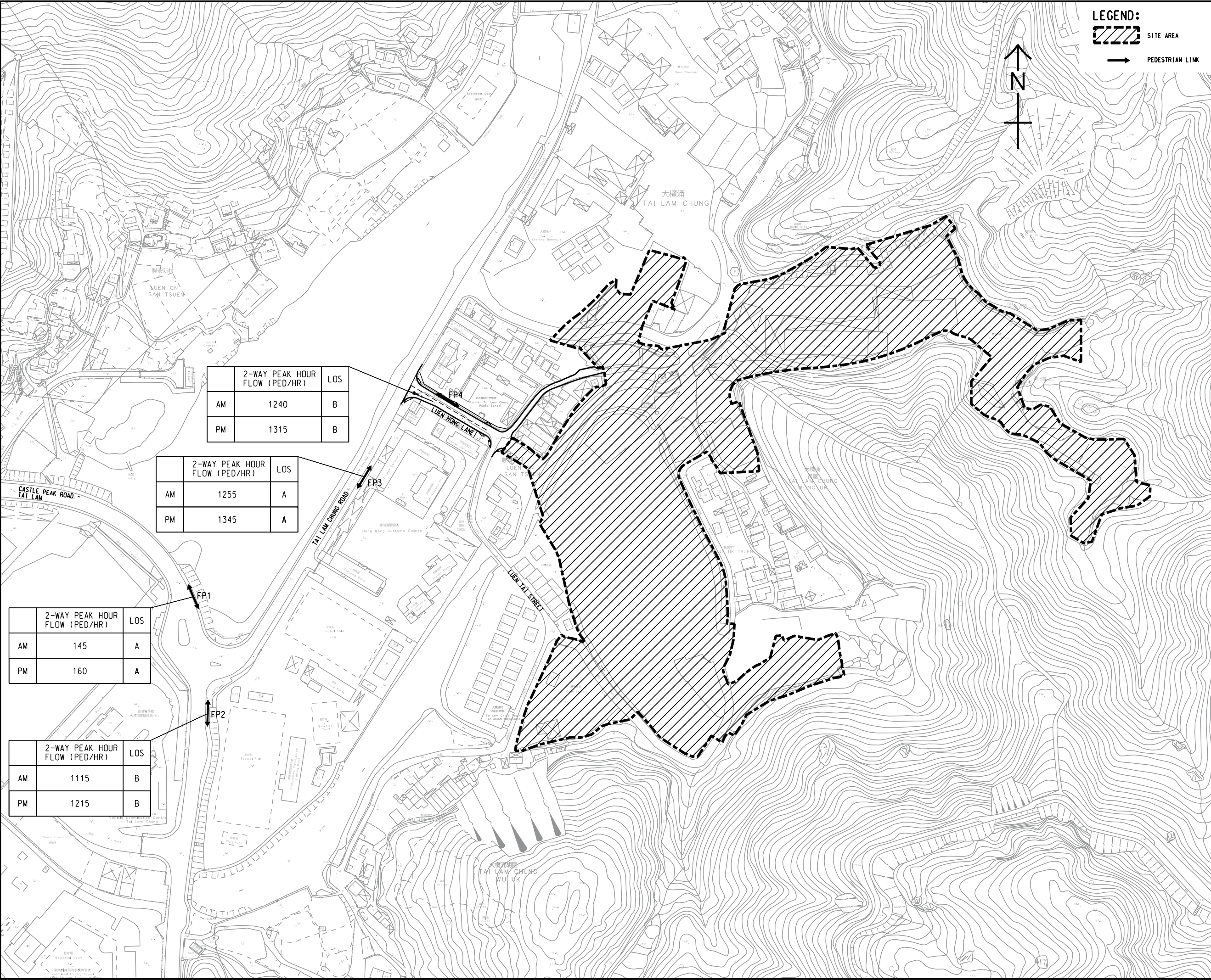
圖紙編號

FIGURE 6.4

CONTRACT NO.

合約編號

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項目
SECTION 16 PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO AN APPROVED COMPREHENSIVE RESIDENTIAL DEVELOPMENT SCHEME AND MINOR RELAXATION OF GROSS FLOOR AREA AND BUILDING HEIGHT RESTRICTIONS AT VARIOUS LOTS IN D.D. 385 AND ADJOINING GOVERNMENT LAND, TAI LAM CHUNG, TUN MUN

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修訂	日期	內容簡述	校核

STATUS
階段

SCALE
比例

A3 1 : 2500

KEY PLAN
索引圖

PROJECT NO.
項目編號

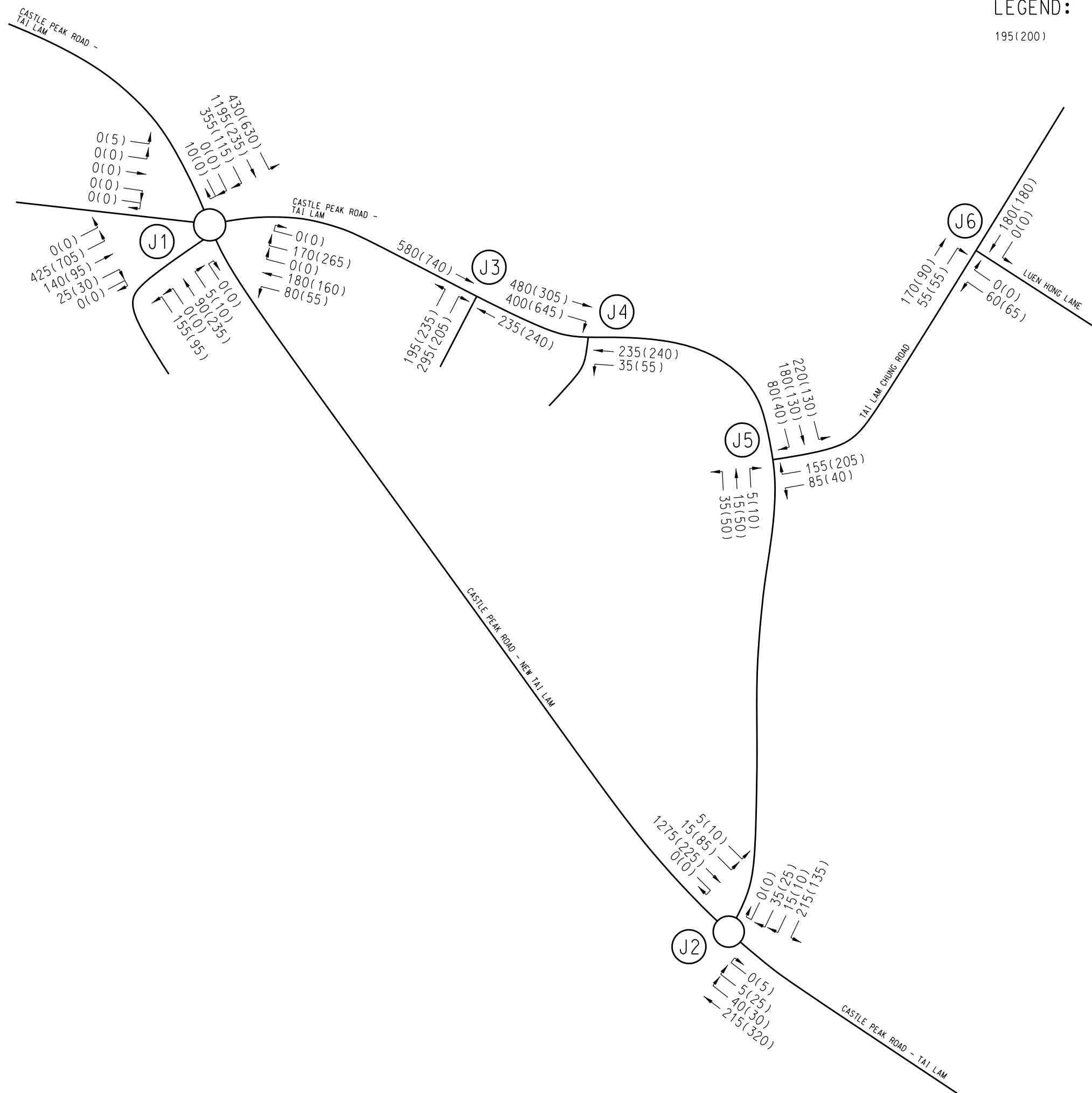
60281828

SHEET TITLE
圖紙名稱

YEAR 2033 DESIGN
PEDESTRIAN FLOWS

SHEET NUMBER
圖紙編號

FIGURE 6.5



LEGEND:

195(200)

AM(PM) PEAK HOUR
TRAFFIC FLOWS IN PCU/HR



PROJECT

SECTION 16 PLANNING APPLICATION FOR
PROPOSED AMENDMENTS TO AN
APPROVED COMPREHENSIVE RESIDENTIAL
DEVELOPMENT SCHEME AND MINOR
RELAXATION OF GROSS FLOOR AREA AND
BUILDING HEIGHT RESTRICTIONS AT
VARIOUS LOTS IN D.D. 385 AND ADJOINING
GOVERNMENT LAND, TAI LAM CHUNG,
TUEN MUN

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I/R	DATE	DESCRIPTION	CHK
修订	日期	内容摘要	复核

STATUS

SCALE

DIMENSION UNIT

A3 N.T.S.

KEY PLAN

PROJECT NO.

60281828

SHEET TITLE
[93] 40C 4 x 85

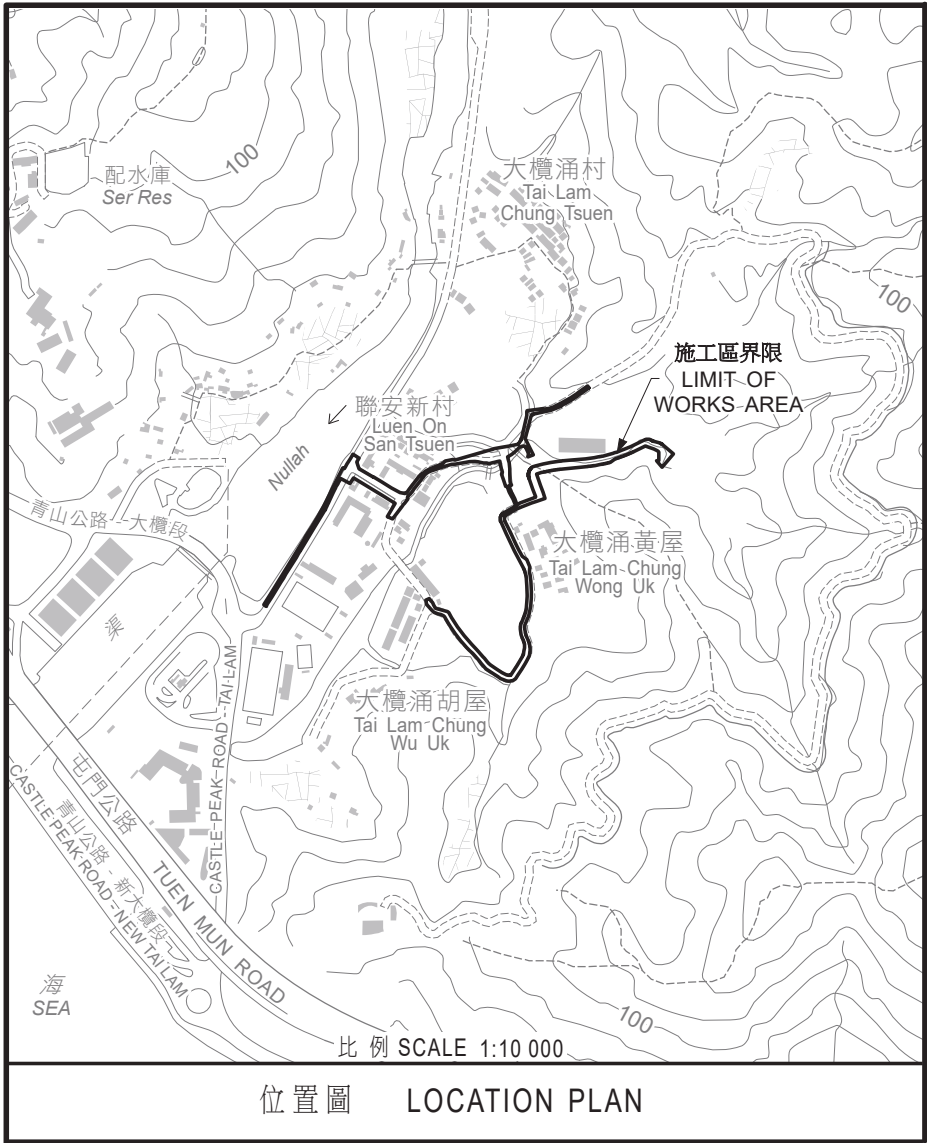
YEAR 2030 CONSTRUCTION
TRAFFIC FLOWS

SHEET NUMBER

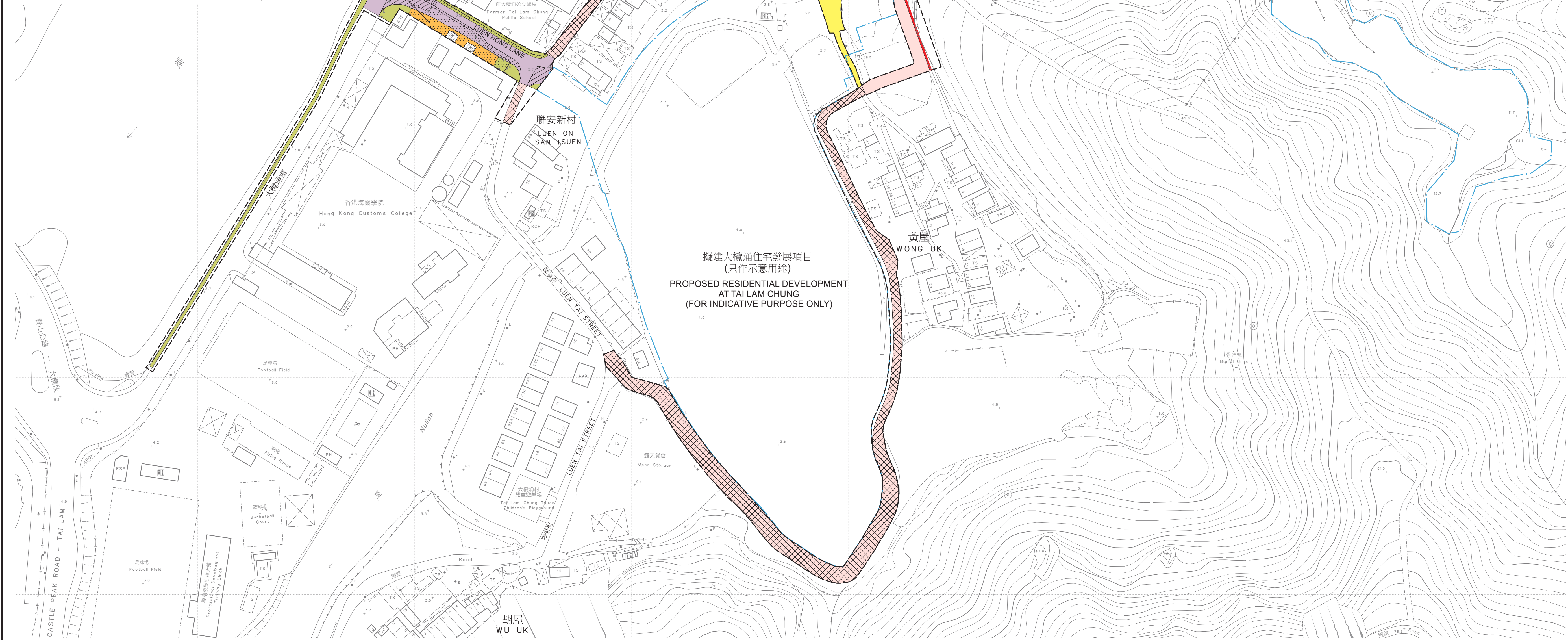
FIGURE 7.1

Annex A

Approved Gazette Plan for Luen Hong Lane



位置圖 LOCATION PLAN



比例尺 SCALE 1:1 000
metres 20 0 20 40 60 80 100 metres

- 註釋 NOTES:
- 除在其他方面指定外，所有量度以米為單位。
ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE SPECIFIED.
 - 所有水平均為約數，以米為單位，並基於香港主水平基準上。
ALL LEVELS ARE APPROXIMATE VALUES AND IN METRES ABOVE HONG KONG PRINCIPAL DATUM.
 - 如有需要，施工區界限內部分現有行車道、行人路及鄉村道路或會分段暫時封閉及實施臨時交通安排，以便進行工程。
SECTIONS OF THE EXISTING CARRIAGEWAYS, FOOTPATHS AND VILLAGE ACCESSES WITHIN THE LIMIT OF WORKS AREA MAY BE TEMPORARILY CLOSED IN PHASES AND TEMPORARY TRAFFIC ARRANGEMENT WILL BE IMPLEMENTED TO FACILITATE WORKS AS AND WHEN REQUIRED.
 - 如有需要，斜坡穩定工程或會在施工區界限之內進行。
SLOPE STABILIZATION WORKS MAY BE CARRIED OUT WITHIN THE LIMIT OF WORKS AREA AS AND WHEN REQUIRED.

- 圖例 LEGEND:
- 施工區界限
LIMIT OF WORKS AREA
 - 擬建鄉村道路
PROPOSED VILLAGE ACCESS
 - 現有行車道將永久封閉及改建為行人路
EXISTING CARRIAGEWAY TO BE PERMANENTLY CLOSED AND CONVERTED INTO FOOTPATH
 - 現有行人路將永久封閉及改建為行車道
EXISTING FOOTPATH TO BE PERMANENTLY CLOSED AND CONVERTED INTO CARRIAGEWAY
 - 現有大壩涌道兒童遊樂場將永久封閉及改建為行車道
EXISTING TAI LAM CHUNG ROAD CHILDREN'S PLAYGROUND TO BE PERMANENTLY CLOSED AND CONVERTED INTO CARRIAGEWAY
 - 現有大壩涌道兒童遊樂場將永久封閉及改建為行人路
EXISTING TAI LAM CHUNG ROAD CHILDREN'S PLAYGROUND TO BE PERMANENTLY CLOSED AND CONVERTED INTO FOOTPATH
 - 現有大壩涌道兒童遊樂場將永久封閉及改建為路旁帶
EXISTING TAI LAM CHUNG ROAD CHILDREN'S PLAYGROUND TO BE PERMANENTLY CLOSED AND CONVERTED INTO VERGE
 - 現有鄉村道路將於重慶工程完成後永久封閉
EXISTING VILLAGE ACCESS TO BE PERMANENTLY CLOSED UPON COMPLETION OF REPROVISIONING WORKS
 - 現有行車道將暫時封閉及重建
EXISTING CARRIAGEWAY TO BE TEMPORARILY CLOSED AND RECONSTRUCTED
 - 現有行人路將暫時封閉及重建
EXISTING FOOTPATH TO BE TEMPORARILY CLOSED AND RECONSTRUCTED
 - 現有鄉村道路將暫時封閉及重建
EXISTING VILLAGE ACCESS TO BE TEMPORARILY CLOSED AND RECONSTRUCTED
 - 擬建填土構築物
PROPOSED RETAINING STRUCTURE
 - 擬建大壩涌住宅發展項目的擬建地段界線(只作示意用途)
PROPOSED LOT BOUNDARY OF PROPOSED RESIDENTIAL DEVELOPMENT AT TAI LAM CHUNG (FOR INDICATIVE PURPOSE ONLY)

批註 Endorsed by

陳奕寶 CHAN Mable
運輸及房屋局常任秘書長(運輸)
Permanent Secretary for
Transport and Housing (Transport)

日期 Date

核准 Approved by

蔡繼珍 K C CHOI
總產業測量師/土地供應
地政總署
Chief Estate Surveyor / Land Supply
Lands Department

日期 Date

地政總署 土地供應組
Land Supply Section
Lands Department
圖則由屯門測量處繪製
Plan Prepared by District Survey Office, Tuen Mun
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圖則名稱
PLAN TITLE
工程名稱
PROJECT TITLE

根據《道路(工程、使用及補償)條例》(第370章)而在憲報公布之圖則
PLAN FOR GAZETTING UNDER ROADS (WORKS, USE AND COMPENSATION) ORDINANCE (CHAPTER 370)
屯門大壩涌的擬建道路工程
PROPOSED ROAD WORKS AT TAI LAM CHUNG, TUEN MUN

檔案編號 File No. L/M(1) to LD LS 269/CPD/LT/66 PL.III & LD DSO/TM/W 2001/44
測量圖編號 Survey Sheet No. 6-SW-24A & 24B
發展藍圖編號 Layout Plan No. --
參考圖編號 Reference Plan No. 60281828/GAZ/005 (LandsD)
圖則編號 PLAN No. TMM4185

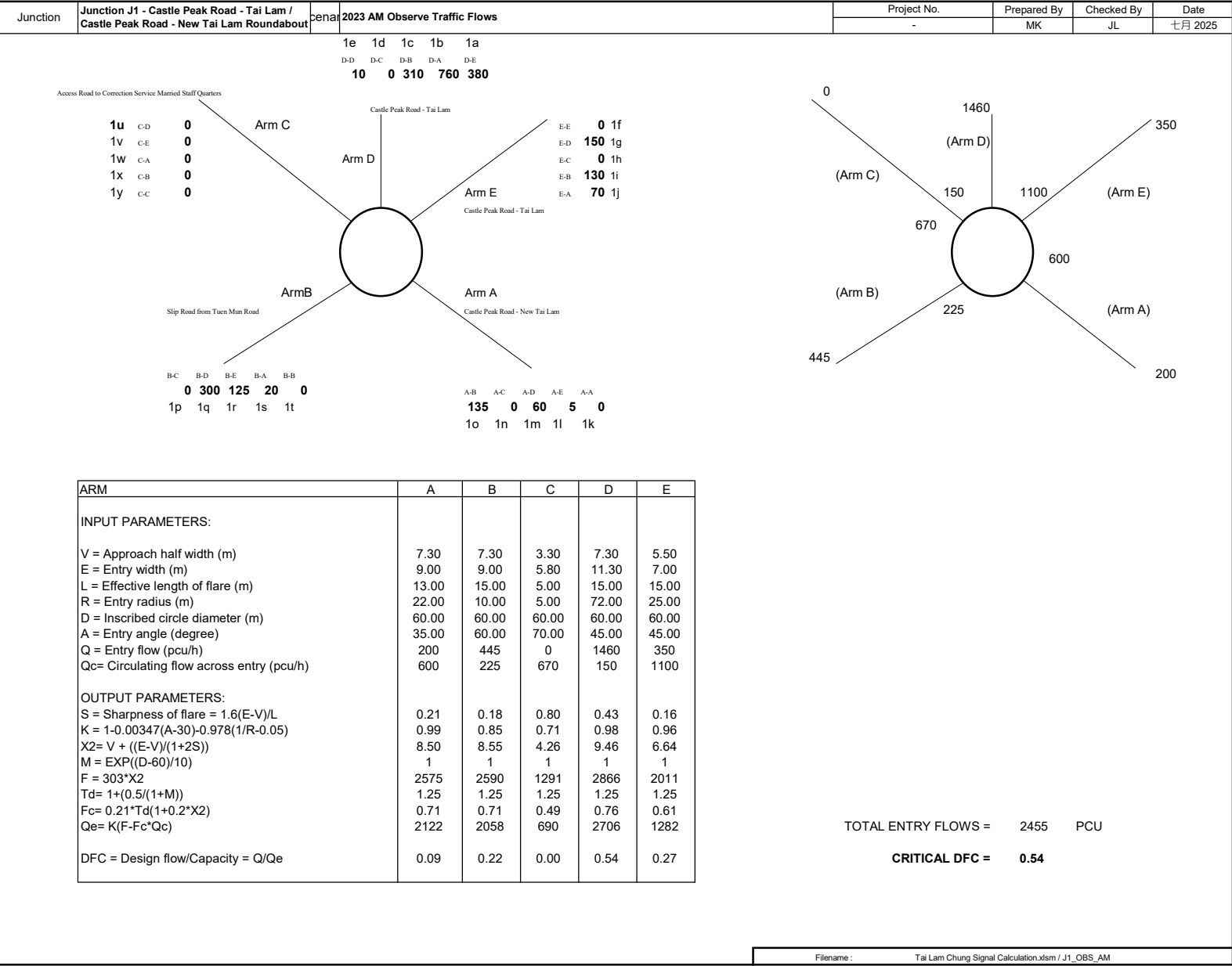
日期 Date 08/03/2021

Annex B

Junction Capacity Calculation Sheets

ROUNABOUT CAPACITY CALCUL

AECOM



ROUNABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	penal	2023 PM Observe Traffic Flows	Project No.	Prepared By	Checked By	Date
				-	MK	JL	七月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 5

1v C-E 0

1w C-A 0

1x C-B 0

1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

Arm E

Castle Peak Road - Tai Lam

Arm A

Castle Peak Road - New Tai Lam

ArmB

Slip Road from Tuen Mun Road

B-C B-D B-E B-A B-B

0 520 85 25 0

1p 1q 1r 1s 1t

A-B A-C A-D A-E A-A

85 0 180 10 0

1o 1n 1m 1l 1k

1e 1d 1c 1b 1a

D-D D-C D-B D-A D-E

0 0 100 90 555

5

(Arm C)

(Arm D)

745

120

215

400

(Arm E)

450

(Arm A)

275

425

(Arm B)

1055

630

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	275	630	5	745	400
Qc= Circulating flow across entry (pcu/h)	450	425	1055	120	215
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	2227	1938	556	2728	1800
DFC = Design flow/Capacity = Q/Qe	0.12	0.33	0.01	0.27	0.22

TOTAL ENTRY FLOWS =

2055

PCU

CRITICAL DFC =

0.33

Filename : Tai Lam Chung Signal Calculation.xlsm / J1_OBS_PM

ROUNDBOUT CAPACITY CALCULATIO

Junction	Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Scenario	Year 2023 Observed Traffic Flow - AM	Project No.	Prepared By	Checked By	Date
							28/七月/25

N

uen Mun Road Bus-Bus Interchange

Castle Peak Road - New Tai Lam (ARM C)

2a 830 →

2b 5 ←

2c 15 →

2d 0 ←

Castle Peak Road - Tai Lam (ARM B)

0 2i ←

5 2j →

35 2k ←

170 2l →

(ARM A)

250

1080

920

850 (ARM C)

210 (ARM B)

270

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	250	210	850	
Qc= Circulating flow across entry (pcu/h)	920	1080	270	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1150	1953	3206	
DFC = Design flow/Capacity = Q/Qe	0.22	0.11	0.27	

TOTAL ENTRY FLOWS 1310 PCU

CRITICAL DFC 0.27

ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2023 Observed Traffic Flow - PM				28/七月/25

Castle Peak Road - Tai Lam

(ARM A)

2e 0 2f 20 2f 20 2h 120

uen Mun Road Bus-Bus Interchange

Castle Peak Road - New Tai Lam (ARM C)

2a 80 2b 10 2c 75 2d 0

Castle Peak Road - Tai Lam (ARM B)

5 2i 20 2j 25 2k 255 2l

(ARM A)

160

245

235

165 (ARM C)

305 (ARM B)

345

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	160	305	165	
Qc= Circulating flow across entry (pcu/h)	235	245	345	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1523	2566	3143	
DFC = Design flow/Capacity = Q/Qe	0.11	0.12	0.05	

TOTAL ENTRY FLOWS 630 PCU

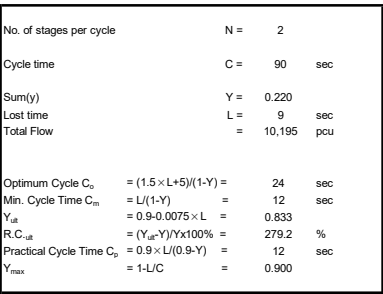
CRITICAL DFC 0.12

Filename : https://aecom-my.sharepoint.com/personal/gary_lei_aecom_com/Documents

Note:

AECOM

DATE: 十月 20



J3

Stage 1	Stage 2			

 $V/G = 5$

5

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 269\%$$
[illegible]

Designed By _____
 Checked By _____
 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

AECOM

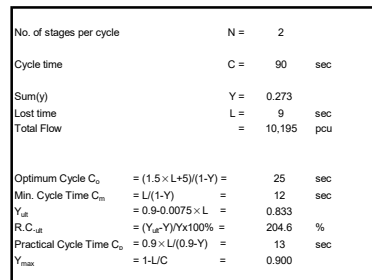
2023 PM Peak Observed Flows

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20



J3

Stage/Phase Diagrams



$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 196\%$$

$$I/G = 6$$
$$I/G = 5$$

5

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR-SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y	
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT			LEFT				RIGHT
↑	A	1	3.650	2				1		0		4100		650		650			4100	0.159	0.159	
↑	B	1	4.500	1				1		0		2065		185		185			2065	0.090		
↱	C	2	3.500	1	15			1		0		1965	205			205	100%		1786	0.115	0.115	
↴	D	2	4.500	1		25	0	1		0		2065			155	155		100%	1948	0.080		
Pedestrian Crossing																						
	Ep	1	min.	5	+	5	=	10	sec													
	Fp	1	min.	5	+	6	=	11	sec													
	Gp	2	min.	8	+	7	=	15	sec													

Designed By _____
 Checked By _____
 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

AECOM

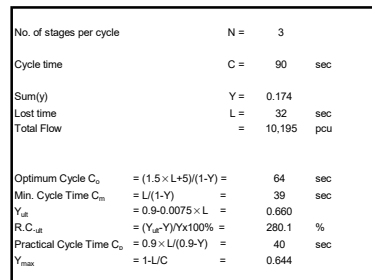
2023 AM Peak Observed Flows

DESIGN:

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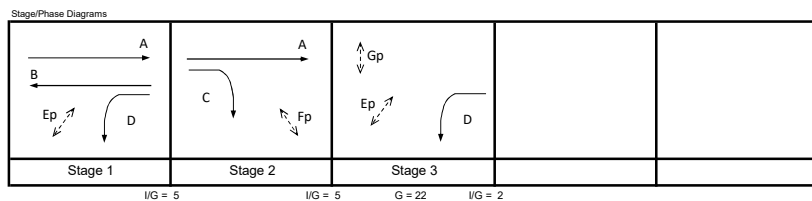
#VALUE!

DATE: 十月 20



Critical Case : B,C,Gp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 234\%$$

[illegible]

Job No. :
Job Title :
Junction Name : Castle Peak Road - Tai Lam / Slip Road to Tuen Mun Road
Junction No : J4
Design Year : 2023
AM/PM : AM Peak
State : Observed Flows

Designed By :
Checked By :
Reviewed By :
Design Date : 十月 20 6:11:01 下午

Reminder: Enter "P" next to the pedestrian phase under column B
Note:

JUNCTION CAPACITY CALCULATION														AECOM																																																																																																																																																																																																														
Junction J4 - Castle Peak Road - Tai Lam / Slip Road to Tuen Mun Road										2023 AM Peak Observed Flows		DESIGN:	CHECK:	#VALUE!	DATE: 十月 20																																																																																																																																																																																																													
<div>Traffic Flow Diagram (pcu/hr)</div> <div></div>														<div>No. of stages per cycle N = 3</div> <div>Cycle time C = 90 sec</div> <div>Sum(y) Y = 0.230</div> <div>Lost time L = 32 sec</div> <div>Total Flow = 10,195 pcu</div> <div>Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 69$ sec</div> <div>Min. Cycle Time $C_{min} = L / (1 - Y) = 42$ sec</div> <div>$Y_{sat} = 0.9 - 0.0075 \times L = 0.660$</div> <div>$R.C._{sat} = (Y_{sat} - Y) / Y \times 100\% = 186.8\%$</div> <div>Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 43$ sec</div> <div>$Y_{max} = 1 - L / C = 0.644$</div>				J4																																																																																																																																																																																																										
<div>Stage/Phase Diagrams</div> <div></div> <div>Stage 1 I/G = 5 Stage 2 I/G = 5 Stage 3 G = 22 I/G = 2</div>														<div>Critical Case : B,C,Gp</div> <div>$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 152\%$</div>																																																																																																																																																																																																														
<table><thead><tr><th rowspan="2">MOVEMENT</th><th rowspan="2">PHASE</th><th rowspan="2">STAGE</th><th rowspan="2">LANE WIDTH (m)</th><th rowspan="2">NO. OF LANES</th><th colspan="2">RADIUS (m)</th><th rowspan="2">OPPOSING TRAFFIC</th><th rowspan="2">NEAR SIDE LANE</th><th rowspan="2">UPHILL GRADIENT (%)</th><th rowspan="2">GRADIENT EFFECT (pcu/hr)</th><th rowspan="2">ADDITIONAL CAPACITY (pcu/hr)</th><th rowspan="2">STRAIGHT-AHEAD SAT. FLOW (pcu/hr)</th><th colspan="3">FLOW (pcu/hr)</th><th rowspan="2">TOTAL FLOW (pcu/hr)</th><th colspan="2">PROPORTION OF TURNING VEHICLES (%)</th><th rowspan="2">REVISED SAT. FLOW (pcu/hr)</th><th rowspan="2">FLOW FACTOR y</th><th rowspan="2">CRITICAL y</th></tr><tr><th>LEFT</th><th>RIGHT</th><th>LEFT</th><th>STRAIGHT AHEAD</th><th>RIGHT</th><th>LEFT</th><th>RIGHT</th></tr></thead><tbody><tr><td>↑</td><td>A</td><td>1,2</td><td>3.500</td><td>1</td><td></td><td></td><td></td><td>1</td><td></td><td>0</td><td></td><td>1965</td><td></td><td>240</td><td>240</td><td></td><td></td><td>1965</td><td>0.122</td><td></td></tr><tr><td>↑</td><td>B</td><td>1</td><td>3.500</td><td>1</td><td rowspan="2">15</td><td rowspan="2"></td><td rowspan="2"></td><td>0</td><td></td><td>0</td><td></td><td>2105</td><td rowspan="2">50</td><td>185</td><td>185</td><td rowspan="2">100%</td><td></td><td>2105</td><td>0.088</td><td rowspan="2">0.088</td></tr><tr><td>↓</td><td>D</td><td>1,3</td><td>3.000</td><td>1</td><td>1</td><td>0</td><td></td><td>1915</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1741</td><td>0.029</td></tr><tr><td>↗</td><td>C</td><td>2</td><td>3.500</td><td>2</td><td></td><td>25</td><td>0</td><td>0</td><td></td><td>0</td><td></td><td>4210</td><td></td><td></td><td>565</td><td>565</td><td>100%</td><td>3972</td><td>0.142</td><td>0.142</td></tr><tr><td colspan="18">Pedestrian Crossing</td><td>GM</td><td></td><td>FGM</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td>Ep</td><td>1,3</td><td>min.</td><td>7</td><td>+</td><td>14</td><td>=</td><td>21</td><td>sec</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td>Fp</td><td>2</td><td>min.</td><td>5</td><td>+</td><td>7</td><td>=</td><td>12</td><td>sec</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td>Gp</td><td>3</td><td>min.</td><td>11</td><td>+</td><td>11</td><td>=</td><td>22</td><td>sec</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr></tbody></table>																		MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y	LEFT	RIGHT	LEFT	STRAIGHT AHEAD	RIGHT	LEFT	RIGHT	↑	A	1,2	3.500	1				1		0		1965		240	240			1965	0.122		↑	B	1	3.500	1	15			0		0		2105	50	185	185	100%		2105	0.088	0.088	↓	D	1,3	3.000	1	1	0		1915								1741	0.029	↗	C	2	3.500	2		25	0	0		0		4210			565	565	100%	3972	0.142	0.142	Pedestrian Crossing																		GM		FGM											Ep	1,3	min.	7	+	14	=	21	sec													Fp	2	min.	5	+	7	=	12	sec													Gp	3	min.	11	+	11	=	22	sec											
MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)						REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y							CRITICAL y																																																																																																																																																																																													
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	Gp	3	min.	11	+	11	=	22	sec																																																																																																																																																																																																																			

Designed By _____
 Checked By _____
 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

AECOM

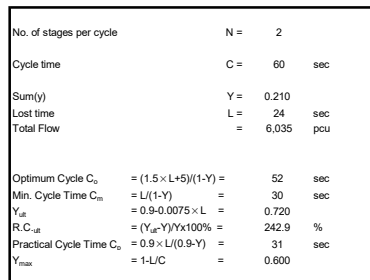
2023 AM Peak Observed Flows

DESIGN:

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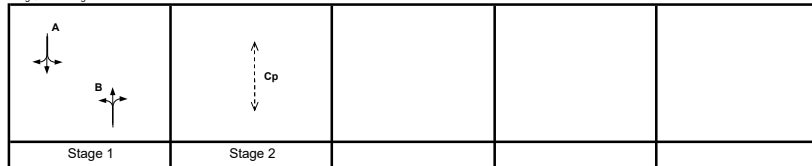
DATE: 十月 20



Critical Case : A,Cp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 157\%$$

Stage/Phase Diagrams

 $I/G = 4$

G = 18

$$I/G = 3$$

5

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR-SIDE LANES	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
<div> <div></div> <div></div> <div></div> </div>	A	1	3.500	1	20	20	0	1		0		1965	165	160	70	395	42%	18%	1881	0.210	0.210
	B	1	3.500	1	15			1		0		1965	30	0		30	100%		1786	0.017	
	B	1	3.500	1		15		0	0	0		2105		15	5	20		25%	2054	0.010	
Pedestrian Crossing				GM 9	+	FGM 9	=	18	sec												
	Cp	2	min.																		

Designed By _____
 Checked By _____
 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

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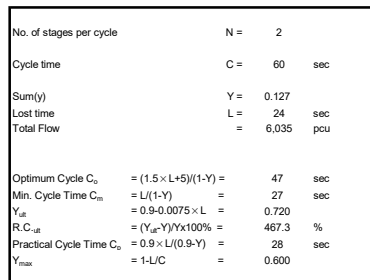
2023 PM Peak Observed Flows

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20



Stage/Phase Diagrams



$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 326\%$$

 $I/G = 4$

G = 18

$$I/G = 3$$

5

[illegible]

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2023 AM Observed Traffic Flows

Designed By :

Checked By :

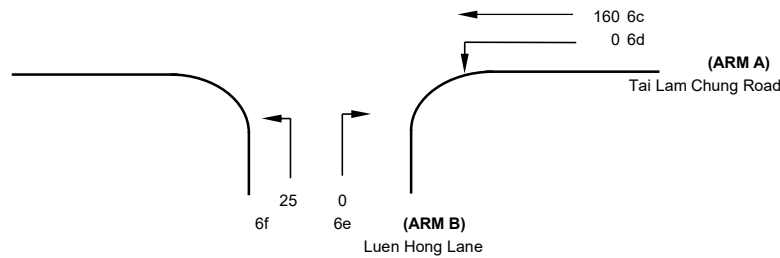
Job No. :

Date : 七月 25

Tai Lam Chung Road

(ARM C)

6a 150
6b 20



NOTES : (GEOMETRIC INPUT DATA)

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 160 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 150 (pcu/hr)
q c-b = 20 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 25 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 549
Q b-c = 759
Q c-b = 679
Q b-ac = 759

CRITICAL DFC = 0.03

COMPARISION OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.03
DFC c-b = 0.03
DFC b-ac = 0.03

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2023 AM Observed Traffic Flows

Designed By :

Checked By :

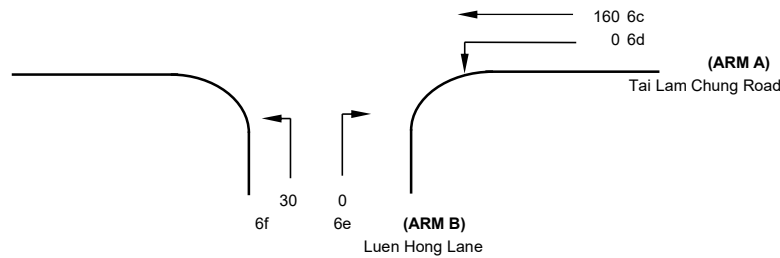
Job No. :

Date : 七月 25

Tai Lam Chung Road

(ARM C)

6a 80
6b 20



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 160 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 80 (pcu/hr)
q c-b = 20 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 30 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 561
Q b-c = 759
Q c-b = 679
Q b-ac = 759

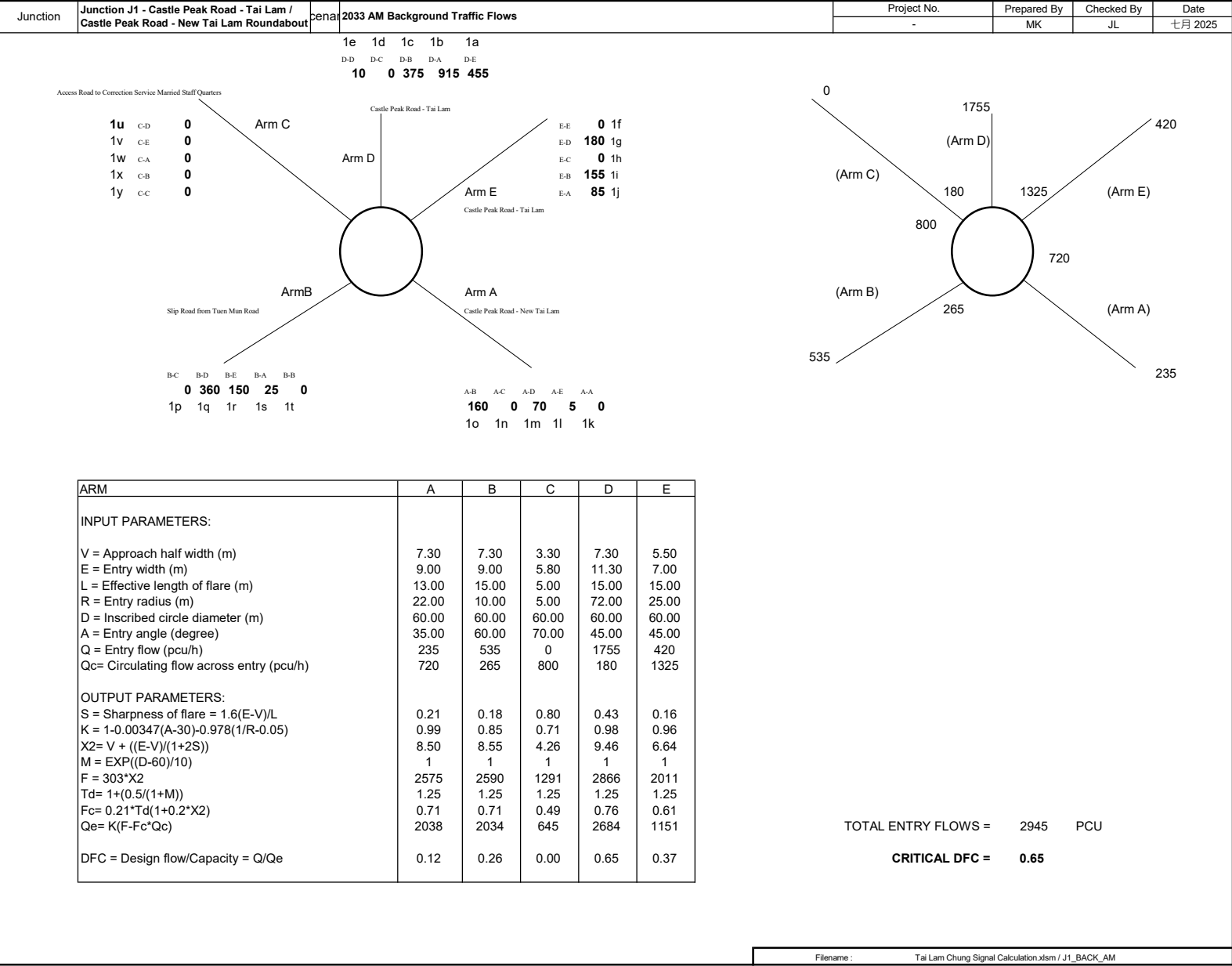
CRITICAL DFC = 0.04

COMPARISION OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.04
DFC c-b = 0.03
DFC b-ac = 0.04

ROUNDAABOUT CAPACITY CALCUL

AECOM

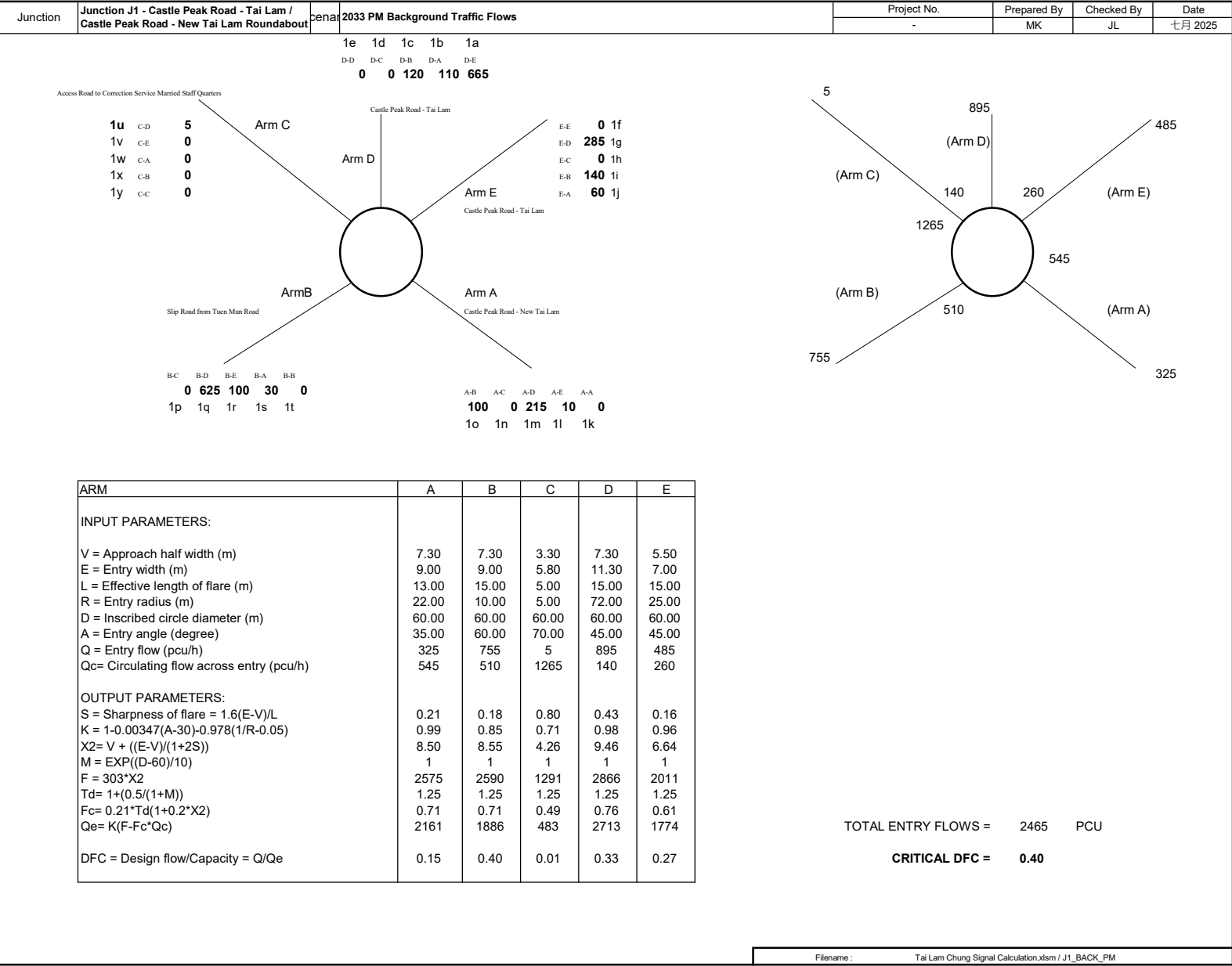


Filename :

Tai Lam Chung Signal Calculation.xlsm / J1_BACK_AM

ROUNABOUT CAPACITY CALCUL

AECOM



ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2033 Background Traffic Flow - AM				28/七月/25

N

uen Mun Road Bus-Bus Interchange

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	300	250	1025	
Qc= Circulating flow across entry (pcu/h)	1105	1300	320	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1049	1791	3164	
DFC = Design flow/Capacity = Q/Qe	0.29	0.14	0.32	

TOTAL ENTRY FLOWS 1575 PCU

CRITICAL DFC 0.32

ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2033 Background Traffic Flow - PM				28/七月/25

Diagram illustrating the roundabout layout and traffic flow data for the four arms (A, B, C, D).

Arm A (Castle Peak Road - Tai Lam):

- Approach 2e: 0
- Approach 2f: 25
- Approach 2f: 25
- Approach 2h: 145

Arm B (Castle Peak Road - Tai Lam):

- Approach 5 2i: 5
- Approach 25 2j: 25
- Approach 30 2k: 30
- Approach 305 2l: 305

Arm C (Castle Peak Road - New Tai Lam):

- Approach 2a: 95
- Approach 2b: 10
- Approach 2c: 90
- Approach 2d: 0

Arm D (Castle Peak Road - Tai Lam):

- Approach 195 (ARM C): 195
- Approach 280: 280
- Approach 365 (ARM B): 365
- Approach 415: 415

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	195	365	195	
Qc= Circulating flow across entry (pcu/h)	280	295	415	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1498	2529	3083	
DFC = Design flow/Capacity = Q/Qe	0.13	0.14	0.06	

TOTAL ENTRY FLOWS 755 PCU

CRITICAL DFC 0.14

AECOM

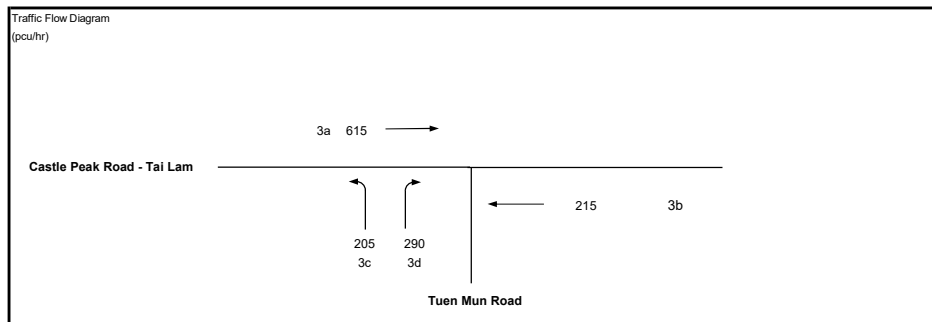
2033 AM Background Traffic Flows

DESIGN:	
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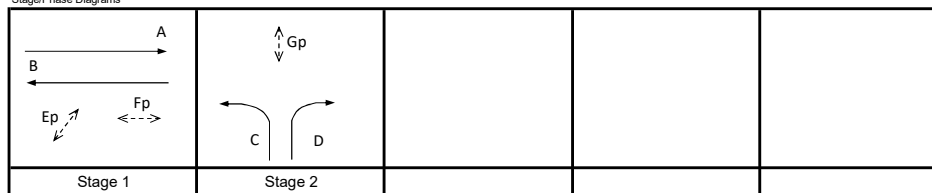
DATE: 十月 20



No. of stages per cycle	N =	2	
Cycle time	C =	90	sec
Sum(y)	Y =	0.265	
Lost time	L =	9	sec
Total Flow	=	10,195	pcu
Optimum Cycle C_o	$= (1.5 \times L + 5)(1 - Y) =$	25	sec
Min. Cycle Time C_m	$= L/(1 - Y) =$	12	sec
Y_{att}	$= 0.9 - 0.0075 \times L =$	0.833	
$R.C_{att}$	$= (Y_{att} \times Y)/Y \times 100\% =$	214.4	%
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	13	sec
Y_{max}	$= 1 - L/C =$	0.900	

J3

Stage/Phase Diagrams


$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 206\%$$

[illegible]

AECOM

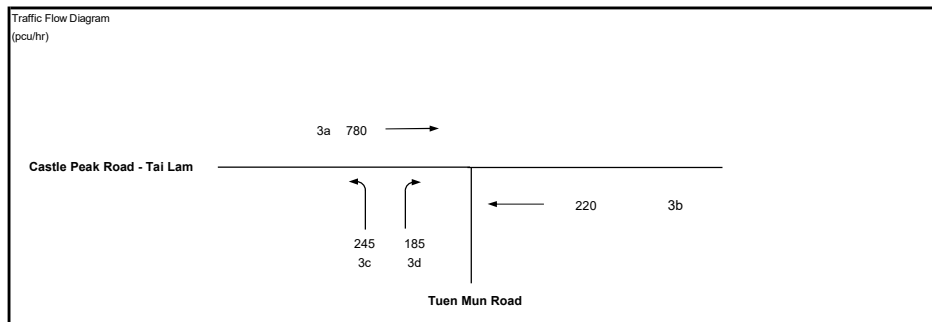
2033 PM Background Traffic Flows

DESIGN:	
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CHECK:

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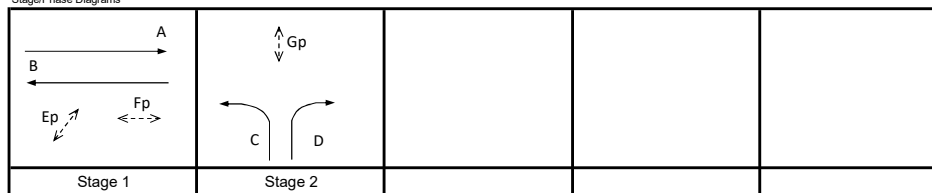
DATE: 十月 20



No. of stages per cycle	N =	2	
Cycle time	C =	90	sec
Sum(y)	Y =	0.327	
Lost time	L =	9	sec
Total Flow	=	10,195	pcu
Optimum Cycle C_o	$= (1.5 \times L + 5)(1-Y) =$	28	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	13	sec
Y_{att}	$= 0.9 - 0.0075 \times L =$	0.833	
R_{C-att}	$= (Y_{att} - Y)/Y \times 100\% =$	154.3	%
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	14	sec
Y_{max}	$= 1 - L/C =$	0.900	

J3

Stage/Phase Diagrams


$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 147\%$$

[illegible]

AECOM

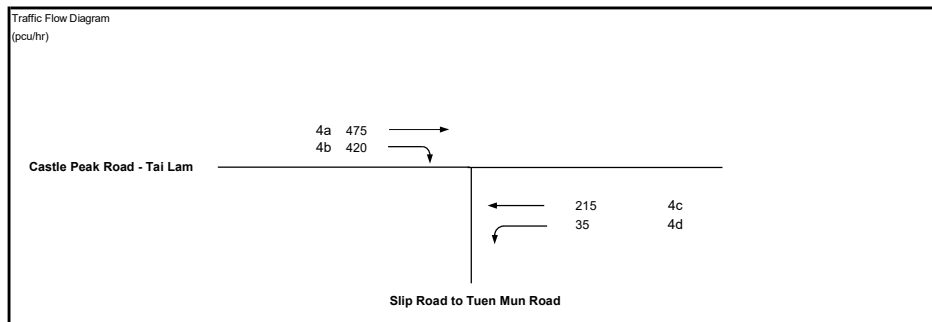
2033 AM Background Traffic Flows

DESIGN:	
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CHECK:

#VALUE!

DATE: 十月 20

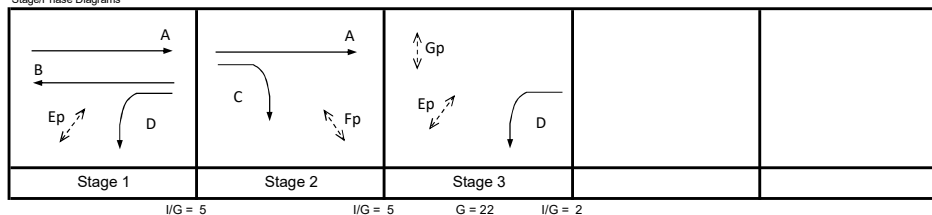


No. of stages per cycle	N =	3	
Cycle time	C =	90	sec
Sum(y)	Y =	0.208	
Lost time	L =	32	sec
Total Flow	=	10,195	pcu

Optimum Cycle C_o	$= (1.5 \times L + 5)/(1-Y) =$	67	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	40	sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.660	
$R.C._{ult}$	$= (Y_{ult} - Y)/Y \times 100\% =$	217.5	%
Practical Cycle Time C_p	$= 0.9 \times L/(0.9 - Y) =$	42	sec
Y_{max}	$= 1 - L/C =$	0.644	

J4

Stage/Phase Diagrams



Critical Case : B,C,Gp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 179\%$$

[illegible]

AECOM

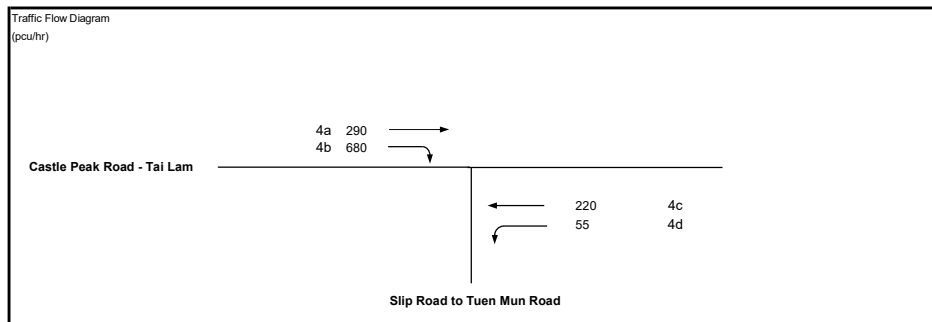
2033 PM Background Traffic Flows

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20

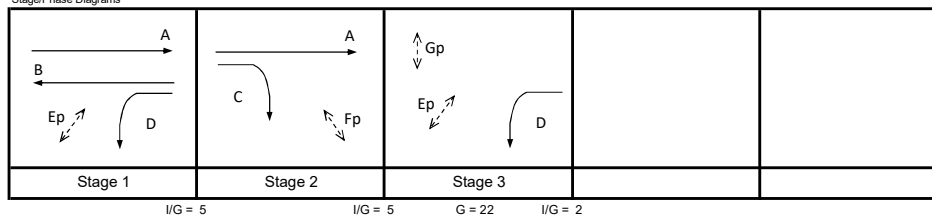


No. of stages per cycle	N =	3	
Cycle time	C =	90	sec
Sum(y)	Y =	0.276	
Lost time	L =	32	sec
Total Flow	=	10,195	pcu

Optimum Cycle C_o	$= (1.5 \times L + 5)/(1-Y) =$	73	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	44	sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.660	
$R.C_{ult}$	$= (Y_{ult} - Y)/Y \times 100\% =$	139.4	%
Practical Cycle Time C_p	$= 0.9 \times L/(0.9-Y) =$	46	sec
Y_{max}	$= 1 - L/C =$	0.644	

J4

Stage/Phase Diagrams



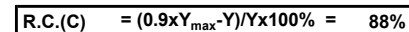
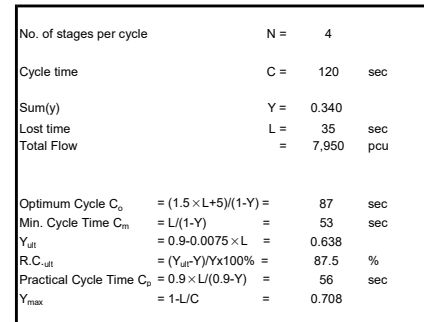
Critical Case : B,C,Gp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 110\%$$

[illegible]

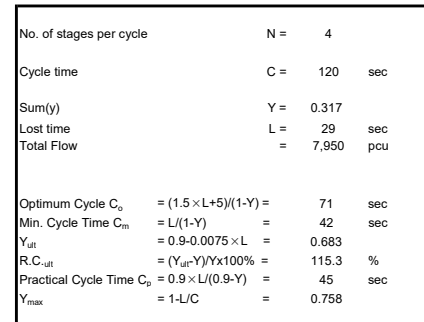
AECOM

DATE: 七月 23

[illegible]

AECOM

DATE: 七月 23



$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 115\%$$

[illegible]

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 AM Background Traffic Flows

Designed By :

Checked By :

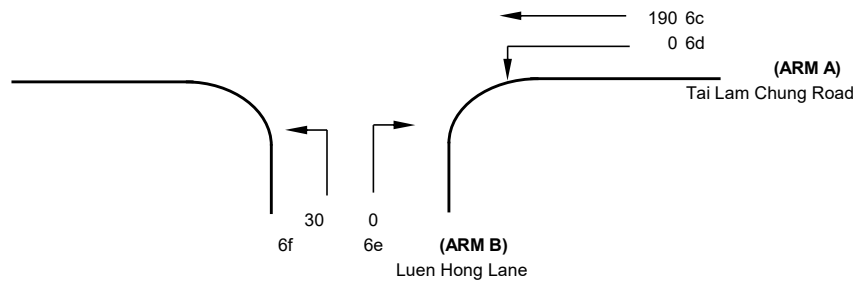
Job No. :

Date : 七月 25

Tai Lam Chung Road

(ARM C)

6a 180
6b 25



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 180 (pcu/hr)
q c-b = 25 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 30 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 534
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.04

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.04
DFC c-b = 0.04
DFC b-ac = 0.04

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 PM Background Traffic Flows

Designed By :

Checked By :

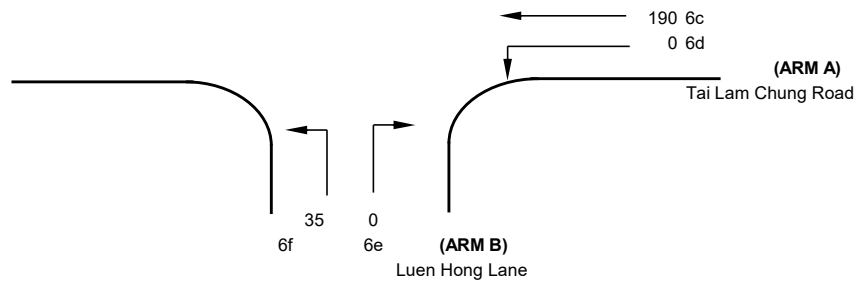
Job No. :

Date : 七月 25

Tai Lam Chung Road

(ARM C)

6a 95
6b 25



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 95 (pcu/hr)
q c-b = 25 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 35 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 548
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.05

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.05
DFC c-b = 0.04
DFC b-ac = 0.05

ROUNDAABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	2033 AM Reference Traffic Flows	Project No.	Prepared By	Checked By	Date
			-	MK	JL	八月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 0
1v C-E 0
1w C-A 0
1x C-B 0
1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

1e 1d 1c 1b 1a
D-D D-C D-B D-A D-E
10 0 375 1245 455

Arm E

Castle Peak Road - Tai Lam

E-E 0 1f
E-D 180 1g
E-C 0 1h
E-B 240 1i
E-A 85 1j

ArmB

Slip Road from Tuen Mun Road

B-C B-D B-E B-A B-B
0 445 195 25 0

1p 1q 1r 1s 1t

Arm A

Castle Peak Road - New Tai Lam

A-B A-C A-D A-E A-A
195 0 90 5 0
1o 1n 1m 1l 1k

0

(Arm C)

2085

(Arm D)

225

1655

(Arm E)

505

950

805

(Arm B)

285

(Arm A)

290

665

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	290	665	0	2085	505
Qc= Circulating flow across entry (pcu/h)	805	285	950	225	1655
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	1979	2022	593	2650	958
DFC = Design flow/Capacity = Q/Qe	0.15	0.33	0.00	0.79	0.53

TOTAL ENTRY FLOWS = 3545 PCU

CRITICAL DFC = 0.79

ROUNDAABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	2033 PM Reference Traffic Flows	Project No.	Prepared By	Checked By	Date
			-	MK	JL	八月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 5
1v C-E 0
1w C-A 0
1x C-B 0
1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

Arm E

Castle Peak Road - Tai Lam

Arm A

Castle Peak Road - New Tai Lam

Arm B

Slip Road from Tuen Mun Road

B-C B-D B-E B-A B-B
0 740 140 30 0

1p 1q 1r 1s 1t

A-B A-C A-D A-E A-A
115 0 245 10 0

1o 1n 1m 1l 1k

1e 1d 1c 1b 1a
D-D D-C D-B D-A D-E
0 0 120 245 665

E-E 0 1f
E-D 285 1g
E-C 0 1h
E-B 180 1i
E-A 60 1j

5

1030

(Arm D)

525

(Arm E)

395

585

(Arm A)

370

540

(Arm B)

1450

180

(Arm C)

910

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	370	910	5	1030	525
Qc= Circulating flow across entry (pcu/h)	585	540	1450	180	395
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/((1+M)))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	2133	1868	419	2684	1695
DFC = Design flow/Capacity = Q/Qe	0.17	0.49	0.01	0.38	0.31

TOTAL ENTRY FLOWS = 2840 PCU

CRITICAL DFC = 0.49

ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2033 Reference Traffic Flow - AM				11/八月/25

Diagram illustrating the roundabout layout and traffic flow data for the four arms (A, B, C, D).

Arm A (Castle Peak Road - Tai Lam):

- Approach 2e: 0
- Approach 2f: 70
- Approach 2f: 70
- Approach 2h: 250

Arm B (Castle Peak Road - Tai Lam):

- Approach 0 2i: 0
- Approach 5 2j: 5
- Approach 40 2k: 40
- Approach 225 2l: 225

Arm C (Castle Peak Road - New Tai Lam):

- Approach 2a: 1330
- Approach 2b: 5
- Approach 2c: 20
- Approach 2d: 0

Arm D (Castle Peak Road - Tai Lam):

- Approach (ARM A): 390
- Approach (ARM B): 270
- Approach (ARM C): 1355
- Approach (ARM D): 410

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	390	270	1355	
Qc= Circulating flow across entry (pcu/h)	1470	1720	410	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	850	1483	3087	
DFC = Design flow/Capacity = Q/Qe	0.46	0.18	0.44	

TOTAL ENTRY FLOWS 2015 PCU

CRITICAL DFC 0.46

ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2033 Reference Traffic Flow - PM				11/八月/25

Castle Peak Road - Tai Lam

(ARM A)

2e 0 2f 40 2f 40 2h 155

uen Mun Road Bus-Bus Interchange

Castle Peak Road - New Tai Lam (ARM C)

2a 230 2b 10 2c 90 2d 0

Castle Peak Road - Tai Lam (ARM B)

5 2i 25 2j 30 2k 335 2l

(ARM A)

235

470

430

330 (ARM C)

395 (ARM B)

475

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	235	395	330	
Qc= Circulating flow across entry (pcu/h)	430	470	475	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1416	2401	3032	
DFC = Design flow/Capacity = Q/Qe	0.17	0.16	0.11	

TOTAL ENTRY FLOWS 960 PCU

CRITICAL DFC 0.17

Filename : https://aecom-my.sharepoint.com/personal/gary_lei_aecom_com/Documents

AECOM

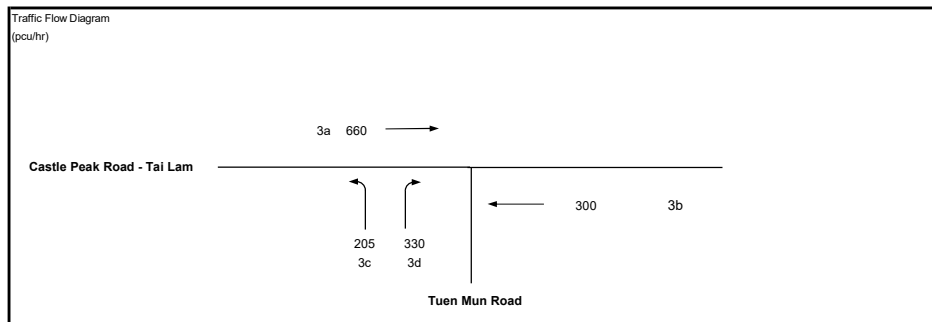
2033 AM Reference Traffic Flows

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20

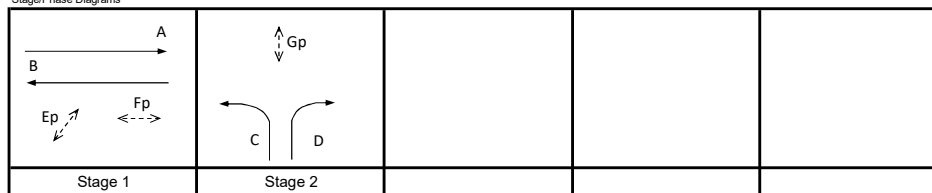


No. of stages per cycle	N =	2	
Cycle time	C =	90	sec
Sum(y)	Y =	0.276	
Lost time	L =	9	sec
Total Flow	=	10,195	pcu

Optimum Cycle C_o	$= (1.5 \times L + 5)/(1-Y) =$	26	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	12	sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.833	
$R.C_{ult}$	$= (Y_{ult} - Y)/Y \times 100\% =$	201.9	%
Practical Cycle Time C_p	$= 0.9 \times L/(0.9-Y) =$	13	sec
Y_{max}	$= 1 - L/C =$	0.900	

J3

Stage/Phase Diagrams


$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 194\%$$

[illegible]

Job No. : _____
 Job Title : _____
 Junction Name : Castle Peak Road - Tai Lam / Tuen Mun Road
 Junction No : J3
 Design Year : 2033
 AM/PM : PM
 State : Reference Traffic Flows

Designed By : _____
 Checked By : _____
 Reviewed By : _____
 Design Date : 十月 20 6:11:01 下午

Reminder: Enter "P" next to the pedestrian phase under column B
 Note: _____

JUNCTION CAPACITY CALCULATION										AECOM																																																																																																																																																																																	
Junction J3 - Castle Peak Road - Tai Lam / Tuen Mun Road					2033 PM Reference Traffic Flows					DESIGN:	CHECK:	#VALUEI	DATE: 十月 20																																																																																																																																																																														
<div style="border: 1px solid black; padding: 10px; min-height: 150px;"> <p>Traffic Flow Diagram (pcu/hr)</p> </div>										<div style="border: 1px solid black; padding: 10px;"> <p>No. of stages per cycle N = 2</p> <p>Cycle time C = 90 sec</p> <p>Sum(y) Y = 0.337</p> <p>Lost time L = 9 sec</p> <p>Total Flow = 10,195 pcu</p> <p>Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 28$ sec</p> <p>Min. Cycle Time $C_{min} = L / (1 - Y) = 14$ sec</p> <p>$Y_{sat} = 0.9 - 0.0075 \times L = 0.833$</p> <p>$R.C._{sat} = (Y_{sat} - Y) / Y \times 100\% = 146.9\%$</p> <p>Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 14$ sec</p> <p>$Y_{max} = 1 - L/C = 0.900$</p> </div>				J3																																																																																																																																																																													
<p>Stage/Phase Diagrams</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 25%; text-align: center; padding: 10px;"> </td> <td style="width: 25%; text-align: center; padding: 10px;"> </td> <td style="width: 25%;"></td> <td style="width: 25%;"></td> </tr> <tr> <td style="text-align: center;">Stage 1</td> <td style="text-align: center;">Stage 2</td> <td></td> <td></td> </tr> </table> <p style="text-align: center; margin-top: 5px;">I/G = 6 I/G = 5</p>																		Stage 1	Stage 2			<p>Critical Case : A,C</p> <div style="border: 1px solid black; padding: 5px; margin-top: 5px;"> <p>R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 140\%$</p> </div>																																																																																																																																																																					
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Job No. :
Job Title :
Junction Name : Castle Peak Road - Tai Lam / Slip Road to Tuen Mun Road
Junction No : J4
Design Year : 2033
AM/PM : AM
State : Reference Traffic Flows

Designed By :
Checked By :
Reviewed By :
Design Date : 十月 20 6:11:01 下午

Reminder: Enter "P" next to the pedestrian phase under column B
Note:

JUNCTION CAPACITY CALCULATION														AECOM																																																																																																																																																																																																					
Junction J4 - Castle Peak Road - Tai Lam / Slip Road to Tuen Mun Road										2033 AM Reference Traffic Flows		DESIGN:	CHECK:	#VALUE!	DATE: 十月 20																																																																																																																																																																																																				
<div>Traffic Flow Diagram (pcu/hr)</div> <div></div>														<div>No. of stages per cycle N = 3</div> <div>Cycle time C = 90 sec</div> <div>Sum(y) Y = 0.248</div> <div>Lost time L = 32 sec</div> <div>Total Flow = 10,195 pcu</div> <div>Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 71$ sec</div> <div>Min. Cycle Time $C_{min} = L / (1 - Y) = 43$ sec</div> <div>$Y_{sat} = 0.9 - 0.0075 \times L = 0.660$</div> <div>$R.C._{sat} = (Y_{sat} - Y) / Y \times 100\% = 165.8\%$</div> <div>Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 44$ sec</div> <div>$Y_{max} = 1 - L/C = 0.644$</div>				J4																																																																																																																																																																																																	
<div>Stage/Phase Diagrams</div> <div></div> <div>Stage 1 Stage 2 Stage 3</div> <div>I/G = 5 I/G = 5 G = 22 I/G = 2</div>														<div>Critical Case : B,C,Gp</div> <div>$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 134\%$</div>																																																																																																																																																																																																					
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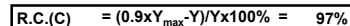
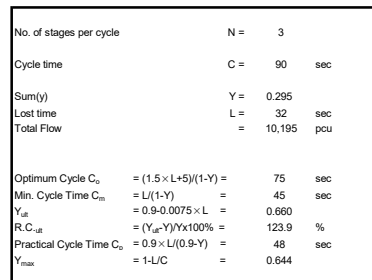
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 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

AECOM

DATE: 十月 20

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JUNCTION CAPACITY CALCULATION

Junction J5 - Castle Peak Road - Tai Lam / Tai Lam Chung Road

2033 AM Reference Traffic Flows with planned improvement scheme

DESIGN:

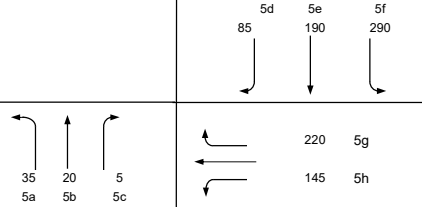
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DATE: 十月 20

Traffic Flow Diagram
(pcu/hr)

Castle Peak Road - Tai Lam



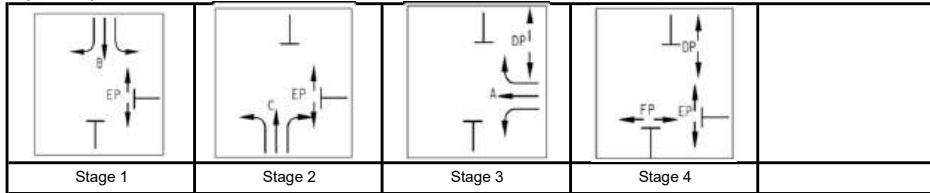
Slip Road to Tuen Mun Road

No. of stages per cycle N = 4
Cycle time C = 120 sec
Sum(y) Y = 0.463
Lost time L = 39 sec
Total Flow = 10,075 pcu

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 118$ sec
Min. Cycle Time $C_{min} = L / (1 - Y) = 73$ sec
 $Y_{crit} = 0.9 - 0.0075 \times L = 0.608$
 $R.C._{crit} = (Y_{crit} - Y) / Y \times 100\% = 31.1\%$
Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 80$ sec
 $Y_{max} = 1 - L / C = 0.675$

J5

Stage/Phase Diagrams



I/G = 5

G = 5

I/G = 6

I/G = 7

G = 16

I/G = 2

Critical Case : B,C,A,Fp

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 31\%$

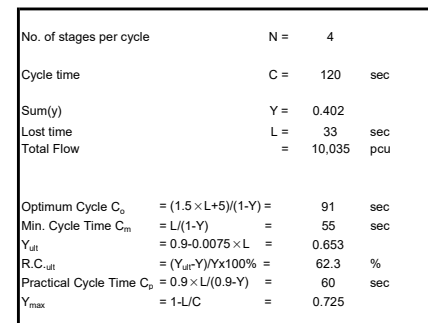
5

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
↓	B	1	3.500	1	12	0	0	1		0		1965	290	190		480	60%		1827	0.263	0.263
↑	B	1	3.500	1	0	25	0	0		0		2105			85	85		100%	1986	0.043	
↔	A	3	3.600	1	12	25	0	1		0		1975	145	0	220	365	40%	60%	1819	0.201	0.201
↔	C	2	3.300	1	15			1		0		1945	35	20		55	64%		1829	0.030	
↔	C	2	3.300	1		10	0	0		0		2085			5	5		100%	1813	0.003	
Pedestrian Crossing				GM		FGM															
	Dp	1,4	min.	6	+	12	=	18	sec												
	Ep	1,2,4	min.	6	+	11	=	17	sec												
	Fp	4	min.	5	+	11	=	16	sec												

#DIV/0!

AECOM

DATE: 十月 20



$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 62\%$$

[illegible]

#DIV/0!

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 AM Reference Traffic Flows

Designed By :

Checked By :

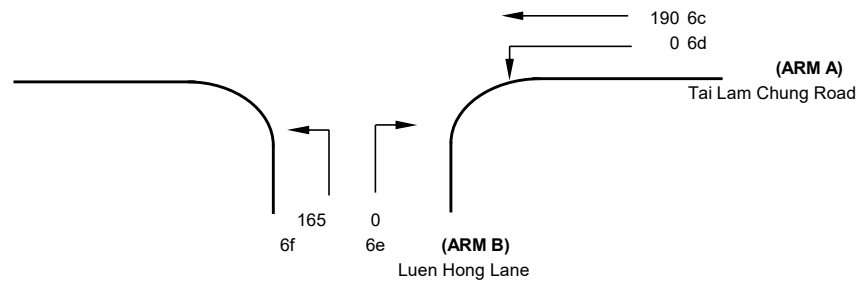
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 180
6b 110



NOTES : (GEOMETRIC INPUT DATA)

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 180 (pcu/hr)
q c-b = 110 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 165 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 501
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.22

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.22
DFC c-b = 0.16
DFC b-ac = 0.22

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 PM Reference Traffic Flows

Designed By :

Checked By :

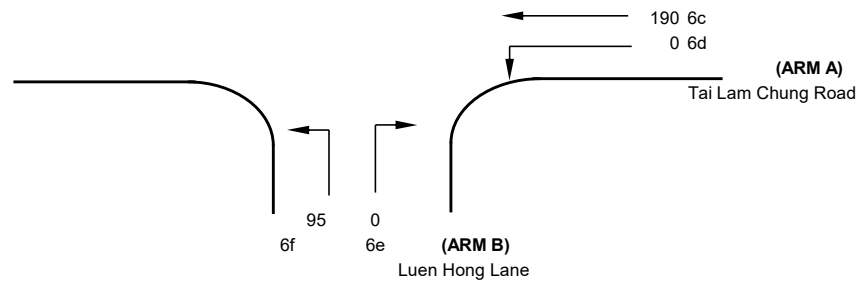
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 95
6b 110



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 95 (pcu/hr)
q c-b = 110 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 95 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 516
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.16

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.13
DFC c-b = 0.16
DFC b-ac = 0.13

ROUNDAABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	cenat	2033 AM Design Traffic Flows	Project No.	Prepared By	Checked By	Date
				-	MK	JL	八月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 0

1v C-E 0

1w C-A 0

1x C-B 0

1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

Arm E

Castle Peak Road - Tai Lam

Arm A

Castle Peak Road - New Tai Lam

ArmB

Slip Road from Tuen Mun Road

1p 1q 1r 1s 1t

0 445 225 25 0

1e 1d 1c 1b 1a

D-D D-C D-B D-A D-E

10 0 375 1245 475

1f 1g 1h 1i 1j

E-E E-D E-C E-B E-A

0 200 0 290 85

1o 1n 1m 1l 1k

A-B A-C A-D A-E A-A

215 0 90 5 0

0

(Arm C)

2105

(Arm D)

255

1655

(Arm E)

575

875

(Arm A)

310

305

(Arm B)

1000

695

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	310	695	0	2105	575
Qc= Circulating flow across entry (pcu/h)	875	305	1000	255	1655
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	1930	2010	575	2628	958
DFC = Design flow/Capacity = Q/Qe	0.16	0.35	0.00	0.80	0.60

TOTAL ENTRY FLOWS =

3685

PCU

CRITICAL DFC =

0.80

Filename : Tai Lam Chung Signal Calculation.xlsm / J1_DES_AM

Filename : Tai Lam Chung Signal Calculation.xlsm / J1_DES_AM

AECOM

Filename :	Tai Lam Chung Signal Calculation.xlsm / J1_DES_PM
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ROUNDAABOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2033 Design Traffic Flow - AM				11/八月/25

Castle Peak Road - Tai Lam (ARM A)

Castle Peak Road - Tai Lam (ARM B)

Castle Peak Road - New Tai Lam (ARM C)

Castle Peak Road - Tai Lam (ARM D)

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	445	295	1355	
Qc= Circulating flow across entry (pcu/h)	1515	1775	475	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	826	1443	3032	
DFC = Design flow/Capacity = Q/Qe	0.54	0.20	0.45	

TOTAL ENTRY FLOWS 2095 PCU

CRITICAL DFC 0.54

ROUNDAABOUT CAPACITY CALCULATIO

Junction	Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Scenario	Year 2033 Design Traffic Flow - PM				Project No.	Prepared By	Checked By	Date
										11/八月/25

N

uen Mun Road Bus-Bus Interchange

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	260	420	330	
Qc= Circulating flow across entry (pcu/h)	465	495	520	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1397	2382	2993	
DFC = Design flow/Capacity = Q/Qe	0.19	0.18	0.11	

TOTAL ENTRY FLOWS 1010 PCU

CRITICAL DFC 0.19

AECOM

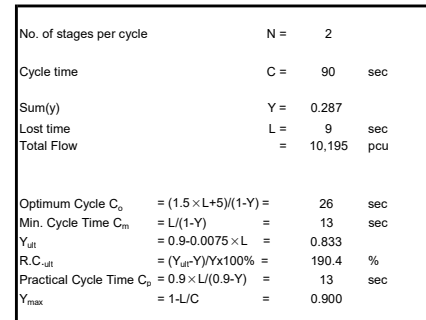
2033 AM Design Traffic Flows

DESIGN:

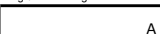

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DATE: 十月 20



J3

					
Stage 1	Stage 2				

$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 183\%$$

[illegible]

AECOM

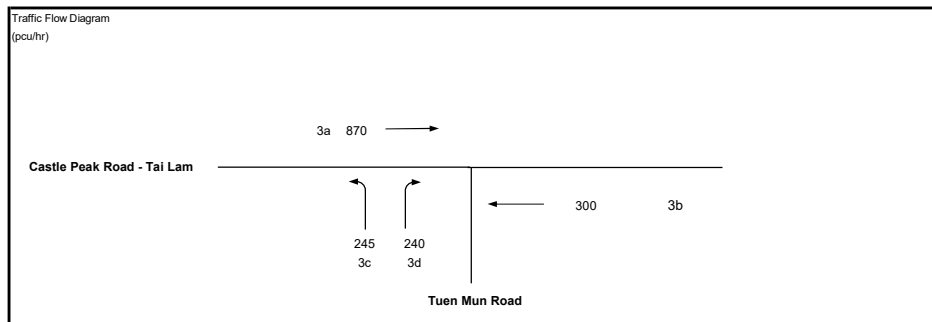
2033 PM Design Traffic Flows

DESIGN:

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DATE: 十月 20

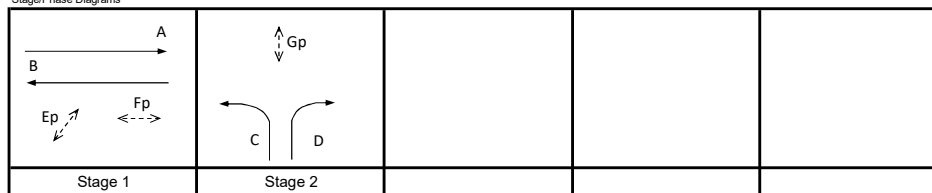


No. of stages per cycle	N =	2
Cycle time	C =	90 sec
Sum(y)	Y =	0.349
Lost time	L =	9 sec
Total Flow	=	10,195 pcu

Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	28	sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	14	sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.833	
R.C. _{ult}	$= (Y_{ult} - Y) / Y \times 100\% =$	138.3	%
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	15	sec
Y_{max}	$= 1 - L/C =$	0.900	

J3

Stage/Phase Diagrams


$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 132\%$$

[illegible]

AECOM

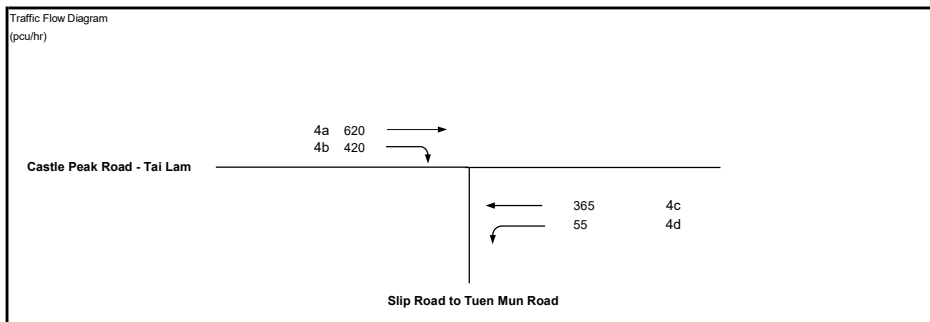
2033 AM Design Traffic Flows

DESIGN:

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#VALUE!

DATE: 十月 20

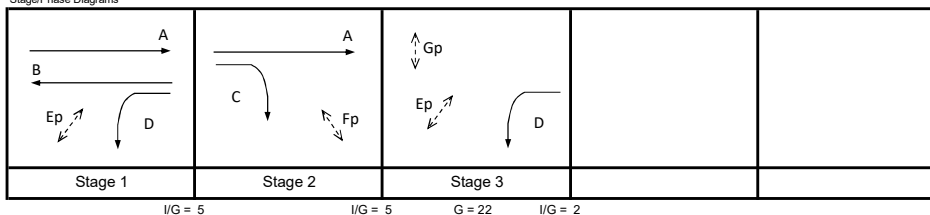


No. of stages per cycle	N =	3	
Cycle time	C =	90	sec
Sum(y)	Y =	0.279	
Lost time	L =	32	sec
Total Flow	=	10,195	pcu

Optimum Cycle C_o	$= (1.5 \times L + 5)/(1-Y) =$	74	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	44	sec
Y_{ut}	$= 0.9 - 0.0075 \times L =$	0.660	
R.C. _{ut}	$= (Y_{ut} - Y)/Y \times 100\% =$	136.4	%
Practical Cycle Time C_p	$= 0.9 \times L/(0.9-Y) =$	46	sec
Y_{max}	$= 1 - L/C =$	0.644	

J4

Stage/Phase Diagrams



Critical Case : B,C,Gp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 108\%$$

[illegible]

AECOM

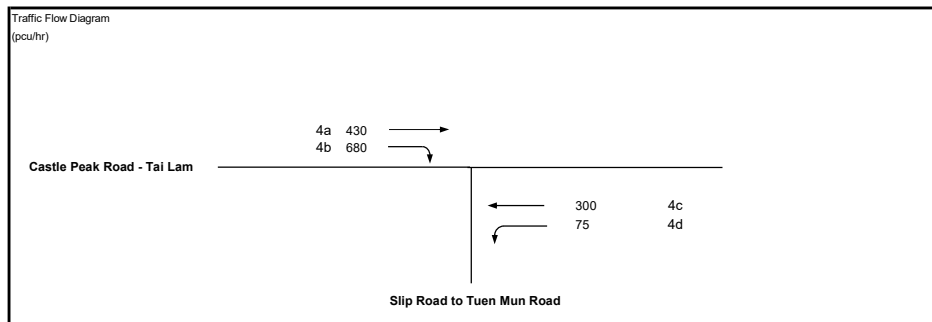
2033 PM Design Traffic Flows

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20

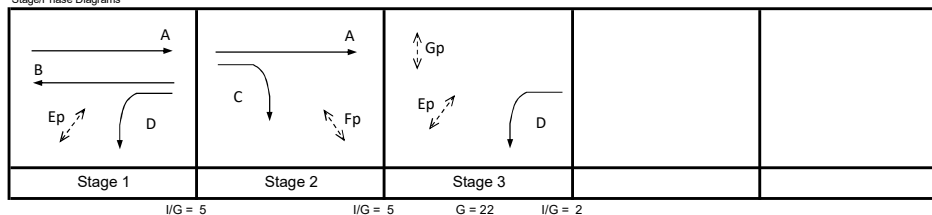


No. of stages per cycle	N =	3	
Cycle time	C =	90	sec
Sum(y)	Y =	0.314	
Lost time	L =	32	sec
Total Flow	=	10,195	pcu

Optimum Cycle C_o	$= (1.5 \times L + 5)/(1-Y) =$	77	sec
Min. Cycle Time C_m	$= L/(1-Y) =$	47	sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.660	
R.C. _{ult}	$= (Y_{ult} - Y)/Y \times 100\% =$	110.4	%
Practical Cycle Time C_p	$= 0.9 \times L/(0.9 - Y) =$	49	sec
Y_{max}	$= 1 - L/C =$	0.644	

J4

Stage/Phase Diagrams



Critical Case : B,C,Gp

$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 85\%$$

[illegible]

JUNCTION CAPACITY CALCULATION

AECOM

Junction J5 - Castle Peak Road (Tai Lam) / Tai Lam Chung Road

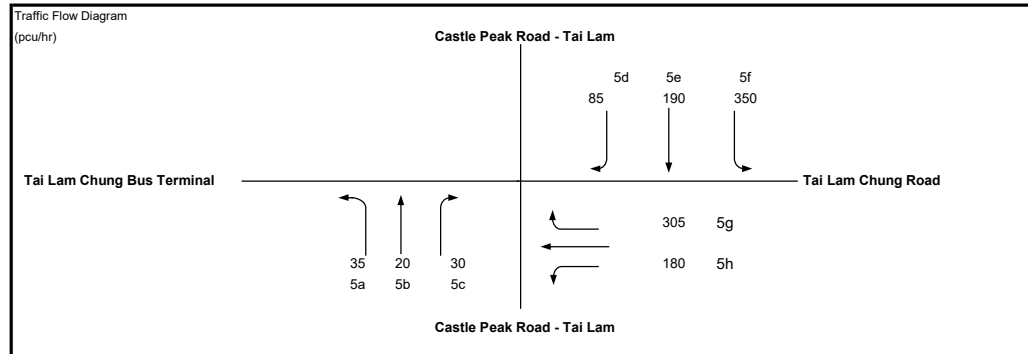
2032 AM Design Traffic Flows with proposed improvement scheme

DESIGN:

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DATE: 十月 20



No. of stages per cycle N = 4

Cycle time C = 90 sec

Sum(y) Y = 0.426

Lost time L = 37 sec

Total Flow = 7,739 pcu

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 105$ sec

Min. Cycle Time $C_m = L / (1 - Y) = 64$ sec

$Y_{ult} = 0.9 - 0.0075 \times L = 0.623$

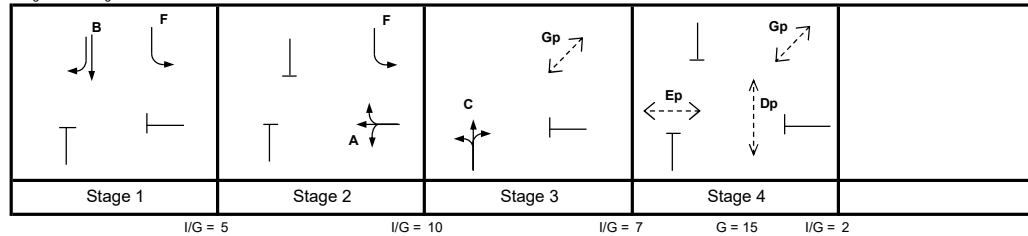
$R.C._{ult} = (Y_{ult} - Y) / Y \times 100\% = 46.2\%$

Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 70$ sec

$Y_{max} = 1 - L / C = 0.589$

J5

Stage/Phase Diagrams



Critical Case : B,A,C,Ep

$$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 24\%$$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y	Queue Length (Per Vehicle)
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT				
↓	B	1	3.500	1		25	0	0		0	-421	2105		190	85	275		31%	1653	0.166	0.166	5
↔	C	2	5.000	1	30	25	0	1		0		2115	35	20	30	85	41%	35%	2030	0.042		2
↔	A	3	3.600	1	30	25	0	1		0		1975	180	0	305	485	37%	63%	1870	0.259	0.259	9
↓	F	1,2	3.500	1	15			1		0		1965	350			350	100%		1786	0.196		6
Pedestrian Crossing																						
	Dp	4	min.	GM	+	7	=	12	sec													
	Ep	4	min.	5	+	10	=	15	sec													
	Gp	3,4	min.	5	+	5	=	10	sec													

JUNCTION CAPACITY CALCULATION

AECOM

Junction J5 - Castle Peak Road (Tai Lam) / Tai Lam Chung Road

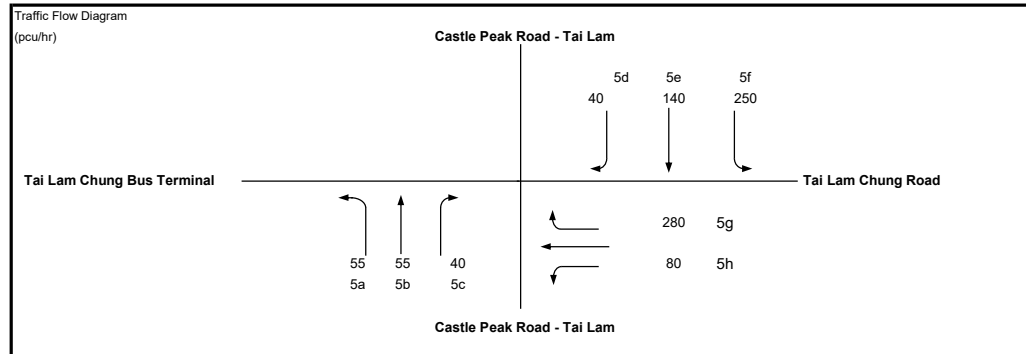
2032 AM Design Traffic Flows with planned improvement scheme

DESIGN:

CHECK:

#VALUE!

DATE: 十月 20



No. of stages per cycle N = 4

Cycle time C = 90 sec

Sum(y) Y = 0.374

Lost time L = 36 sec

Total Flow = 7,739 pcu

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 94$ sec

Min. Cycle Time $C_m = L / (1 - Y) = 58$ sec

$Y_{ult} = 0.9 - 0.0075 \times L = 0.630$

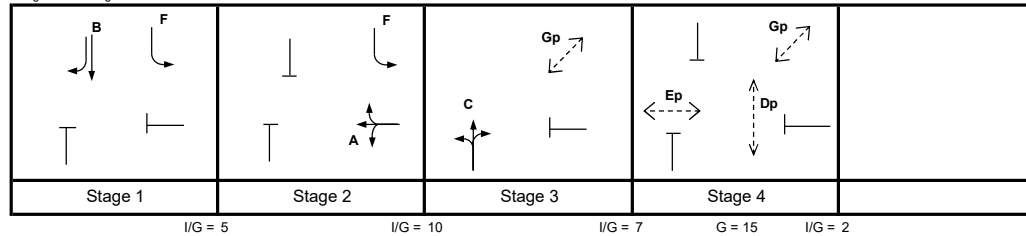
$R.C._{ult} = (Y_{ult} - Y) / Y \times 100\% = 68.2\%$

Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 62$ sec

$Y_{max} = 1 - L / C = 0.600$

J5

Stage/Phase Diagrams



Critical Case : B,A,C,Ep

$$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 44\%$$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y	Queue Length (Per Vehicle)
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT				
Left Turn	B	1	3.500	1		25	0	0		0	-421	2105		140	40	180		22%	1662	0.108	0.108	3
Through	C	2	5.000	1	30	25	0	1		0		2115	55	55	40	150	37%	27%	2045	0.073	0.073	3
Right Turn	A	3	3.600	1	30	25	0	1		0		1975	80	0	280	360	22%	78%	1867	0.193	0.193	6
Left Turn	F	1,2	3.500	1	15			1		0		1965	250			250	100%		1786	0.140		4
Pedestrian Crossing																						
	Dp	4	min.	GM	+	7	=	12	sec													
	Ep	4	min.	5	+	10	=	15	sec													
	Gp	3,4	min.	5	+	5	=	10	sec													

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 AM Design Traffic Flows

Designed By :

Checked By :

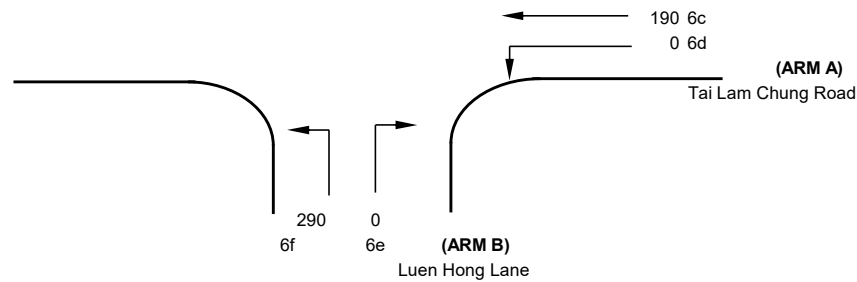
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 180
6b 200



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 180 (pcu/hr)
q c-b = 200 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 290 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 467
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.39

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.39
DFC c-b = 0.30
DFC b-ac = 0.39

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2033 PM Design Traffic Flows

Designed By :

Checked By :

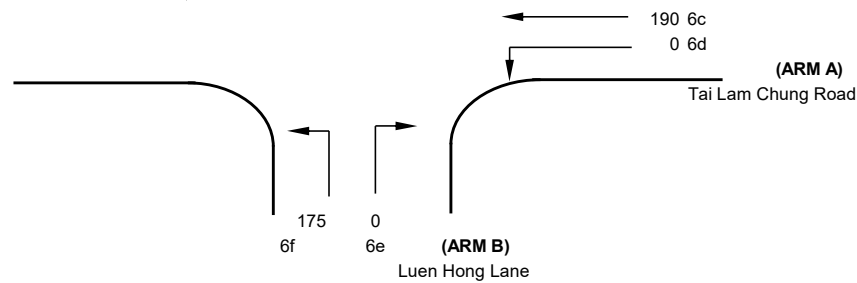
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 95
6b 195



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 190 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 95 (pcu/hr)
q c-b = 195 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 175 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 483
Q b-c = 750
Q c-b = 672
Q b-ac = 750

CRITICAL DFC = 0.29

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.23
DFC c-b = 0.29
DFC b-ac = 0.23

ROUNDAABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	2030 AM Construction Traffic Flows	Project No.	Prepared By	Checked By	Date
			-	MK	JL	八月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 0
1v C-E 0
1w C-A 0
1x C-B 0
1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

Arm E

Castle Peak Road - Tai Lam

Arm A

Castle Peak Road - New Tai Lam

Arm B

Slip Road from Tuen Mun Road

B-C 0 425 140 25 0
1p 1q 1r 1s 1t

B-D B-E B-A B-B

1e 1d 1c 1b 1a
D-D D-C D-B D-A D-E
10 0 355 1195 430

E-E 0 1f
E-D 170 1g
E-C 0 1h
E-B 180 1i
E-A 80 1j

A-B 155 0 90 5 0
1o 1n 1m 1l 1k

A-C A-D A-E A-A

0

1990

(Arm D)

430

(Arm E)

1585

715

(Arm A)

250

275

(Arm B)

865

170

(Arm C)

590

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	250	590	0	1990	430
Qc= Circulating flow across entry (pcu/h)	715	275	865	170	1585
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/((1+M)))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	2042	2028	622	2691	998
DFC = Design flow/Capacity = Q/Qe	0.12	0.29	0.00	0.74	0.43

TOTAL ENTRY FLOWS = 3180 PCU

CRITICAL DFC = 0.74

ROUNDAABOUT CAPACITY CALCUL

AECOM

Junction	Junction J1 - Castle Peak Road - Tai Lam / Castle Peak Road - New Tai Lam Roundabout	2033 PM Observe Traffic Flows	Project No.	Prepared By	Checked By	Date
			-	MK	JL	八月 2025

Access Road to Correction Service Married Staff Quarters

1u C-D 5
1v C-E 0
1w C-A 0
1x C-B 0
1y C-C 0

Arm C

Castle Peak Road - Tai Lam

Arm D

Arm E

Castle Peak Road - Tai Lam

Arm A

Castle Peak Road - New Tai Lam

Arm B

Slip Road from Tuen Mun Road

B-C 0 705 95 30 0
1p 1q 1r 1s 1t

B-D B-E B-A B-B
0 705 95 30 0

A-B 95 0 235 10 0
1o 1n 1m 1l 1k

1e 1d 1c 1b 1a
D-D D-C D-B D-A D-E
0 0 115 235 630

E-E 0 1f
E-D 265 1g
E-C 0 1h
E-B 160 1i
E-A 55 1j

5

980

(Arm D)

480

(Arm E)

380

540

(Arm A)

340

510

(Arm B)

1340

135

(Arm C)

830

ARM	A	B	C	D	E
INPUT PARAMETERS:					
V = Approach half width (m)	7.30	7.30	3.30	7.30	5.50
E = Entry width (m)	9.00	9.00	5.80	11.30	7.00
L = Effective length of flare (m)	13.00	15.00	5.00	15.00	15.00
R = Entry radius (m)	22.00	10.00	5.00	72.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00	60.00
A = Entry angle (degree)	35.00	60.00	70.00	45.00	45.00
Q = Entry flow (pcu/h)	340	830	5	980	480
Qc= Circulating flow across entry (pcu/h)	540	510	1340	135	380
OUTPUT PARAMETERS:					
S = Sharpness of flare = 1.6(E-V)/L	0.21	0.18	0.80	0.43	0.16
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.99	0.85	0.71	0.98	0.96
X2= V + ((E-V)/(1+2S))	8.50	8.55	4.26	9.46	6.64
M = EXP((D-60)/10)	1	1	1	1	1
F = 303*X2	2575	2590	1291	2866	2011
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.71	0.71	0.49	0.76	0.61
Qe= K(F-Fc*Qc)	2164	1886	457	2717	1703
DFC = Design flow/Capacity = Q/Qe	0.16	0.44	0.01	0.36	0.28

TOTAL ENTRY FLOWS = 2580 PCU

CRITICAL DFC = 0.44

Filename : Tai Lam Chung Signal Calculation.xlsm / J1_CTIA_PM

ROUNDBOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2030 Construction Traffic Flow - AM				11/八月/25

N

uen Mun Road Bus-Bus Interchange

Castle Peak Road - New Tai Lam (ARM C)

2a 1275 →

2b 5 →

2c 15 →

2d 0 →

Castle Peak Road - Tai Lam (ARM B)

0 2i

5 2j

40 2k

215 2l

(ARM A)

285

1560

1375

1295 (ARM C)

260 (ARM B)

330

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	285	260	1295	
Qc= Circulating flow across entry (pcu/h)	1375	1560	330	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	902	1600	3155	
DFC = Design flow/Capacity = Q/Qe	0.32	0.16	0.41	

TOTAL ENTRY FLOWS 1840 PCU

CRITICAL DFC 0.41

ROUNDAABOUT CAPACITY CALCULATIO

Junction	Scenario	Project No.	Prepared By	Checked By	Date
Junction J2 - Castle Peak Road - New Tai Lam / Castle Peak Road - Tai Lam Roundabout	Year 2030 Construction Traffic Flow - PM				11/八月/25

Castle Peak Road - Tai Lam (ARM A)

2e 0, 2f 25, 2f 25, 2h 135

Castle Peak Road - New Tai Lam (ARM C)

2a 225, 2b 10, 2c 85, 2d 0

Castle Peak Road - Tai Lam (ARM B)

5 2i, 25 2j, 30 2k, 320 2l

uen Mun Road Bus-Bus Interchange (ARM D)

320 (ARM C), 405, 415, 380 (ARM B), 430

ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	3.00	7.80	10.00	
E = Entry width (m)	10.20	11.00	13.30	
L = Effective length of flare (m)	12.50	8.00	10.00	
R = Entry radius (m)	80.00	80.00	50.00	
D = Inscribed circle diameter (m)	60.00	60.00	60.00	
A = Entry angle (degree)	45.00	45.00	45.00	
Q = Entry flow (pcu/h)	185	380	320	
Qc= Circulating flow across entry (pcu/h)	405	415	430	
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.92	0.64	0.53	
K = 1-0.00347(A-30)-0.978(1/R-0.05)	0.98	0.98	0.98	
X2= V + ((E-V)/(1+2S))	5.53	9.20	11.61	
M = EXP((D-60)/10)	1.00	1.00	1.00	
F = 303*X2	1676	2789	3516	
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	
Fc= 0.21*Td(1+0.2*X2)	0.55	0.75	0.87	
Qe= K(F-Fc*Qc)	1430	2441	3070	
DFC = Design flow/Capacity = Q/Qe	0.13	0.16	0.10	

TOTAL ENTRY FLOWS 885 PCU

CRITICAL DFC 0.16

Designed By _____
 Checked By _____
 Reviewed By _____

Design Date 十月 20 6:11:01 下午

Note:

AECOM

Junction J3 - Castle Peak Road - Tai Lam / Tuen Mun Road

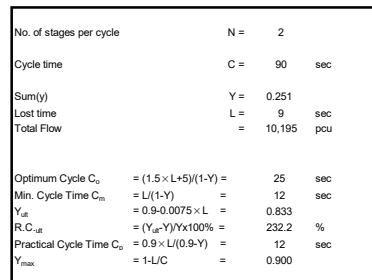
2030 AM Construction Traffic Flows

DESIGN:

CHECK:

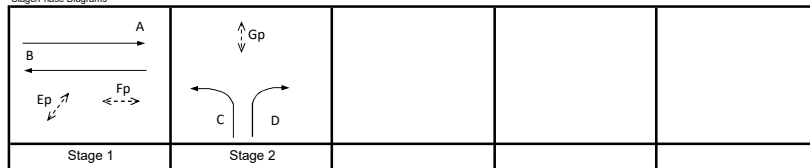
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DATE: 十月 20



J3

Stage/Phase Diagrams


$$I/G = 6$$
$$I/G = 5$$

5

Critical Case : A,C

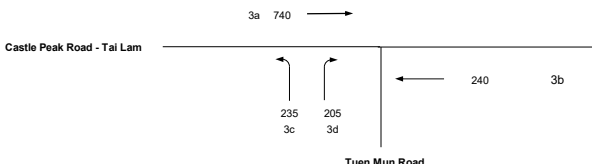
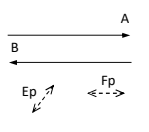
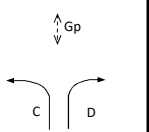
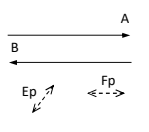
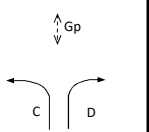
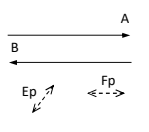
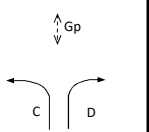
$$\text{R.C.(C)} = (0.9 \times Y_{\max} - Y) / Y \times 100\% = 223\%$$

[illegible]

Job No. :
 Job Title :
 Junction Name : Castle Peak Road - Tai Lam / Tuen Mun Road
 Junction No : J3
 Design Year : 2030
 AM/PM : PM
 State : Construction Traffic Flows

Designed By :
 Checked By :
 Reviewed By :
 Design Date : 十月 20 6:11:01 下午

Reminder: Enter "P" next to the pedestrian phase under column B
 Note:

JUNCTION CAPACITY CALCULATION															AECOM																																																																																																																																											
Junction J3 - Castle Peak Road - Tai Lam / Tuen Mun Road										2030 PM Construction Traffic Flows			DESIGN:	CHECK:	#VALUE!	DATE: 十月 20																																																																																																																																										
<div style="border: 1px solid black; padding: 5px; margin-bottom: 10px;"> <p style="font-size: x-small;">Traffic Flow Diagram (pcu/hr)</p>  </div> <div style="border: 1px solid black; padding: 5px;"> <p style="font-size: x-small;">Stage/Phase Diagrams</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 25%; text-align: center; vertical-align: middle;">  </td> <td style="width: 25%; text-align: center; vertical-align: middle;">  </td> <td style="width: 25%;"></td> <td style="width: 25%;"></td> </tr> <tr> <td style="text-align: center;">Stage 1</td> <td style="text-align: center;">Stage 2</td> <td></td> <td></td> </tr> </table> </div>																			Stage 1	Stage 2			<div style="border: 1px solid black; padding: 5px;"> <p style="font-size: x-small;">No. of stages per cycle N = 2</p> <p style="font-size: x-small;">Cycle time C = 90 sec</p> <p style="font-size: x-small;">Sum(y) Y = 0.312</p> <p style="font-size: x-small;">Lost time L = 9 sec</p> <p style="font-size: x-small;">Total Flow = 10,195 pcu</p> <p style="font-size: x-small;">Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 27$ sec</p> <p style="font-size: x-small;">Min. Cycle Time $C_{min} = L / (1 - Y) = 13$ sec</p> <p style="font-size: x-small;">$Y_{sat} = 0.9 - 0.0075 \times L = 0.833$</p> <p style="font-size: x-small;">$R.C._{sat} = (Y_{sat} - Y) / Y \times 100\% = 166.8\%$</p> <p style="font-size: x-small;">Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 14$ sec</p> <p style="font-size: x-small;">$Y_{max} = 1 - L/C = 0.900$</p> </div>		J3																																																																																																																																	
																																																																																																																																																										
Stage 1	Stage 2																																																																																																																																																									
<div style="display: flex; justify-content: space-between;"> <div> <p>Critical Case : A,C</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;"> <p>R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 160\%$</p> </div> </div> <div>5</div> </div>																																																																																																																																																										
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MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)						PROPORTION OF TURNING VEHICLES (%)								REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y																																																																																																																																																				
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Job No. :
Job Title :
Junction Name : Castle Peak Road (Tai Lam) / Tai Lam Chung Road
Junction No. : J5
Design Year : 2030
AM/PM : AM
State : Construction Traffic Flows

Designed By :
Checked By :
Reviewed By :
Design Date : 七月 23 4:00:15 下午

JUNCTION CAPACITY CALCULATION

Junction J5 - Castle Peak Road (Tai Lam) / Tai Lam Chung Road

2030 AM Construction Traffic Flows

DESIGN:

CHECK:

JOB NO:

DATE: 七月 23

Tai Lam Chung Road

Castle Peak Road (Tai Lam)

Tai Lam Chung Road

Castle Peak Road (Tai Lam)

5d
80

5e
180

5f
220

35
5a

15
5b

5
5c

155

0

85

5g

5h

No. of stages per cycle

N = 4

Cycle time

C = 120 sec

Sum(y)

Y = 0.351

Lost time

L = 35 sec

Total Flow

= 7,950 pcu

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 89$ sec

Min. Cycle Time $C_m = L / (1 - Y) = 54$ sec

$Y_{ult} = 0.9 - 0.0075 \times L = 0.638$

$R.C_{ult} = (Y_{ult} - Y) / Y \times 100\% = 81.4\%$

Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 57$ sec

$Y_{max} = 1 - L / C = 0.708$

Stage/Phase Diagrams

Stage 1

I/G = 5

Stage 2

I/G = 5

Stage 3

G = 5

Stage 4

I/G = 4

G = 5

I/G = 13

Critical Case : C,A,D,Hp

R.C.(C) = (0.9xY_{max}-Y)/Yx100% = 81%

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
↔	A	2	3.300	1	12	10	0	1		0		1945	220	180	0	400	55%	0%	1820	0.220	0.220
↔	C	1	3.600	1	12	25	0	1		0		1975	85	0	155	240	35%	65%	1824	0.132	0.132
↔	D	3	3.300	1	15			1		0		1945	35	15		50	70%		1818	0.028	
↔	E	3	3.300	1		10	0	0		0		2085			5	5		100%	1813	0.003	
Pedestrian Crossing																					
Fp		1,4	min.	GM		12															
Gp		2,3,4	min.			11															
Hp		4	min.			11															*

Tai Lam Chung Signal Calculation

Job No. :
 Job Title :
 Junction Name : Castle Peak Road (Tai Lam) / Tai Lam Chung Road
 Junction No : J5
 Design Year : 2030
 AM/PM : PM
 State : Construction Traffic Flows

Designed By :
 Checked By :
 Reviewed By :
 Design Date : 七月 23 3:38:31 下午

JUNCTION CAPACITY CALCULATION

AECOM

Junction J5 - Castle Peak Road (Tai Lam) / Tai Lam Chung Road

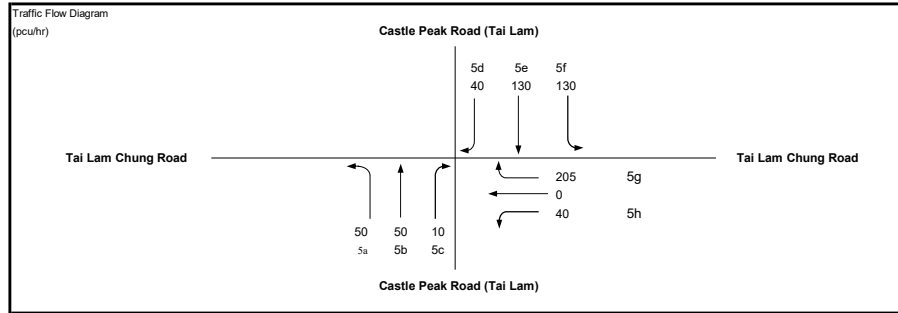
2030 PM Construction Traffic Flows

DESIGN:

CHECK:

JOB NO:

DATE: #####



No. of stages per cycle N = 4

Cycle time C = 120 sec

Sum(y) Y = 0.329

Lost time L = 29 sec

Total Flow = 7,950 pcu

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 72$ sec

Min. Cycle Time $C_m = L / (1 - Y) = 43$ sec

$Y_{sat} = 0.9 - 0.0075 \times L = 0.683$

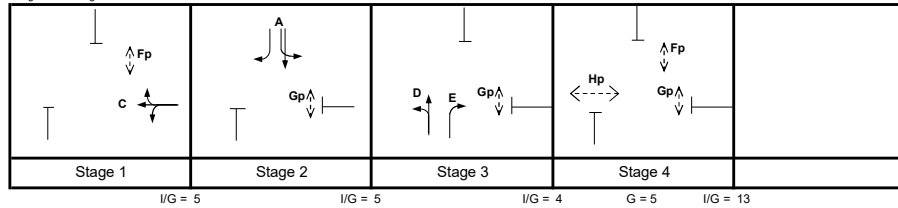
$R.C._{sat} = (Y_{sat} - Y) / Y \times 100\% = 107.6\%$

Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 46$ sec

$Y_{max} = 1 - L/C = 0.758$

J5

Stage/Phase Diagrams



Critical Case : C,A,D,Hp

$$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 108\%$$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
↔	A	2	3.300	1	12	10	0	1		0		1945	130	130	0	260	50%	0%	1831	0.142	0.142
↔	C	1	3.600	1	12	25	0	1		0		1975	40	0	205	245	16%	84%	1845	0.133	0.133
↑	D	3	3.300	1	15			1		0		1945	50	50		100	50%		1852	0.054	0.054
↑	E	3	3.300	1		10	0	0		0		2085			10	10		100%	1813	0.006	
Pedestrian Crossing				GM		FGM															*
	Fp	1,4	min.	6	+	12	=	18		sec											
	Gp	2,3,4	min.	6	+	11	=	17		sec											
	Hp	4	min.	5	+	11	=	16		sec											

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2030 AM Contruction Traffic Flows

Designed By :

Checked By :

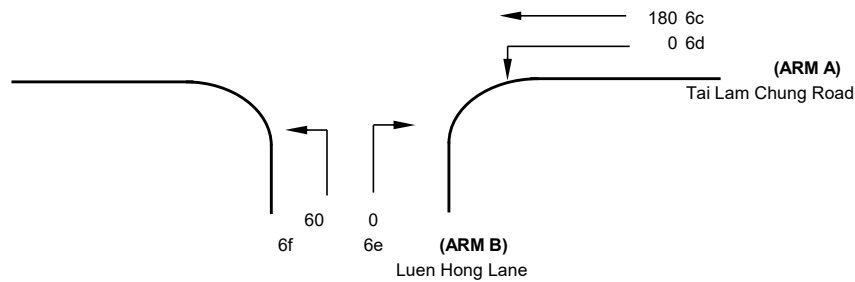
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 170
6b 55



NOTES : (GEOMETRIC INPUT DATA)

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibilitu to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 180 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 170 (pcu/hr)
q c-b = 55 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 60 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 527
Q b-c = 753
Q c-b = 674
Q b-ac = 753

CRITICAL DFC = 0.08

COMPARISION OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.08
DFC c-b = 0.08
DFC b-ac = 0.08

PRIORITY JUNCTION CAPACITY CALCULATION

AECOM

Junction J6 - Tai Lam Chung Road / Luen Hong Lane

2030 PM Construction Traffic Flows

Designed By :

Checked By :

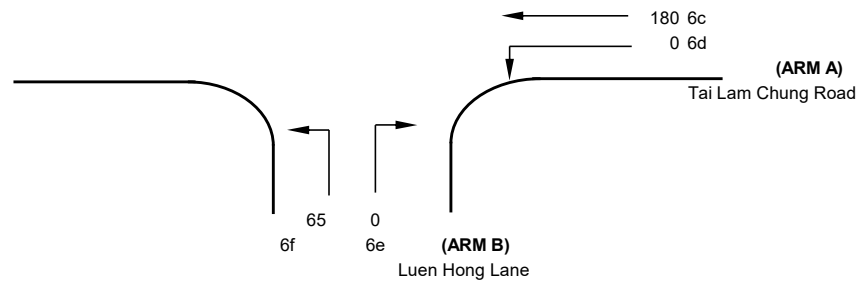
Job No. :

Date : 八月 25

Tai Lam Chung Road

(ARM C)

6a 90
6b 55



NOTES : (GEOMETRIC INPUT DATA)

J6

W = Major Road Width (6.4 - 20.0)
W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.7)
W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.7)
W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.7)
Vl b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

D = Stream-specific B-A
E = Stream-specific B-C
F = Stream-specific C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 7.55 (metres)
W cr = 0 (metres)
q a-b = 0 (pcu/hr)
q a-c = 180 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 4 (metres)
Vr c-b = 50 (metres)
q c-a = 90 (pcu/hr)
q c-b = 55 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 5.7 (metres)
W b-c = 5.7 (metres)
Vl b-a = 20 (metres)
Vr b-a = 16 (metres)
Vr b-c = 16 (metres)
q b-a = 0 (pcu/hr)
q b-c = 65 (pcu/hr)

GEOMETRIC FACTORS :

D = 0.996740
E = 1.081063
F = 0.967827
Y = 0.739525

THE CAPACITY OF MOVEMENT :

Q b-a = 540
Q b-c = 753
Q c-b = 674
Q b-ac = 753

CRITICAL DFC = 0.09

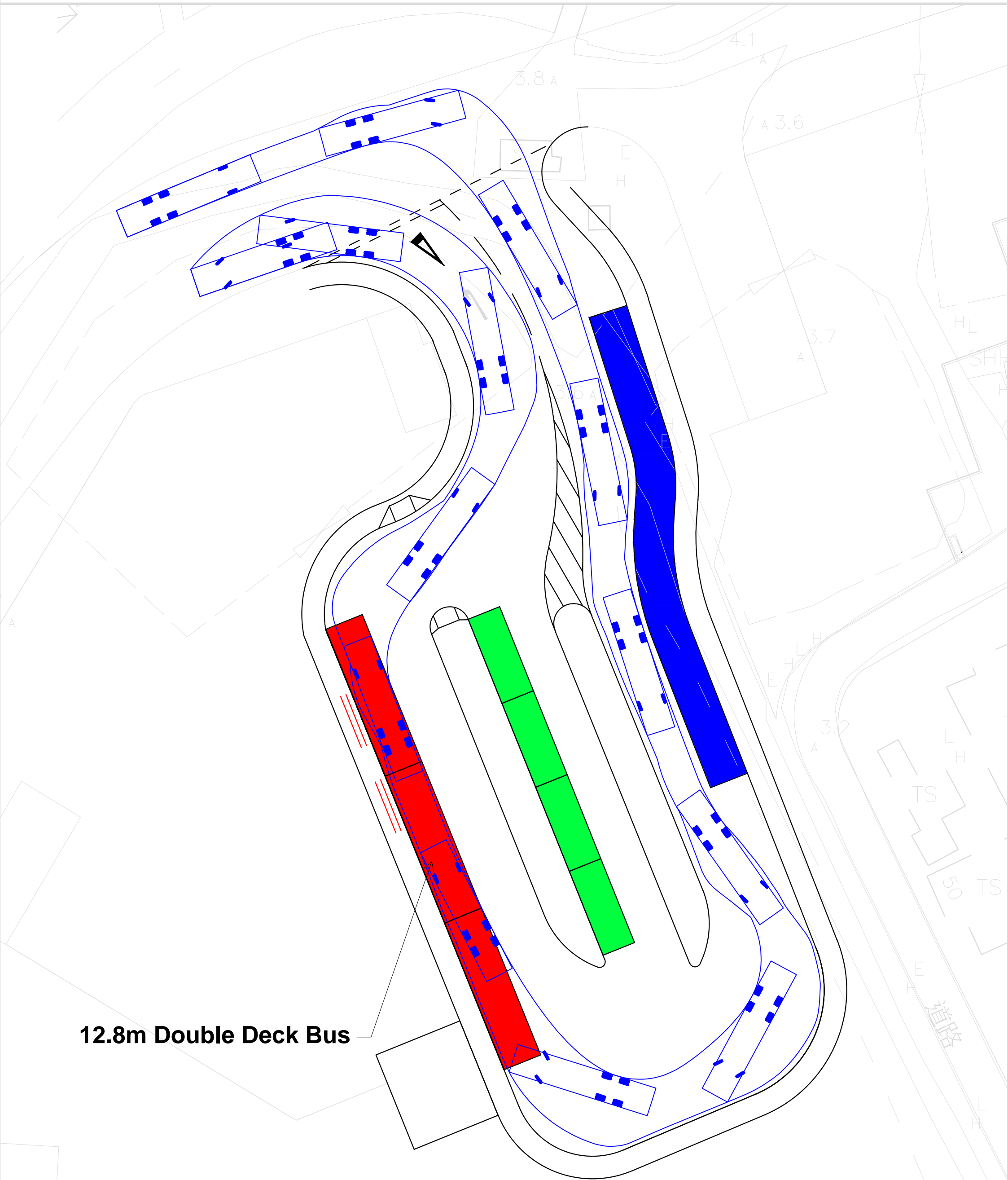
COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a = 0.00
DFC b-c = 0.09
DFC c-b = 0.08
DFC b-ac = 0.09

Annex C

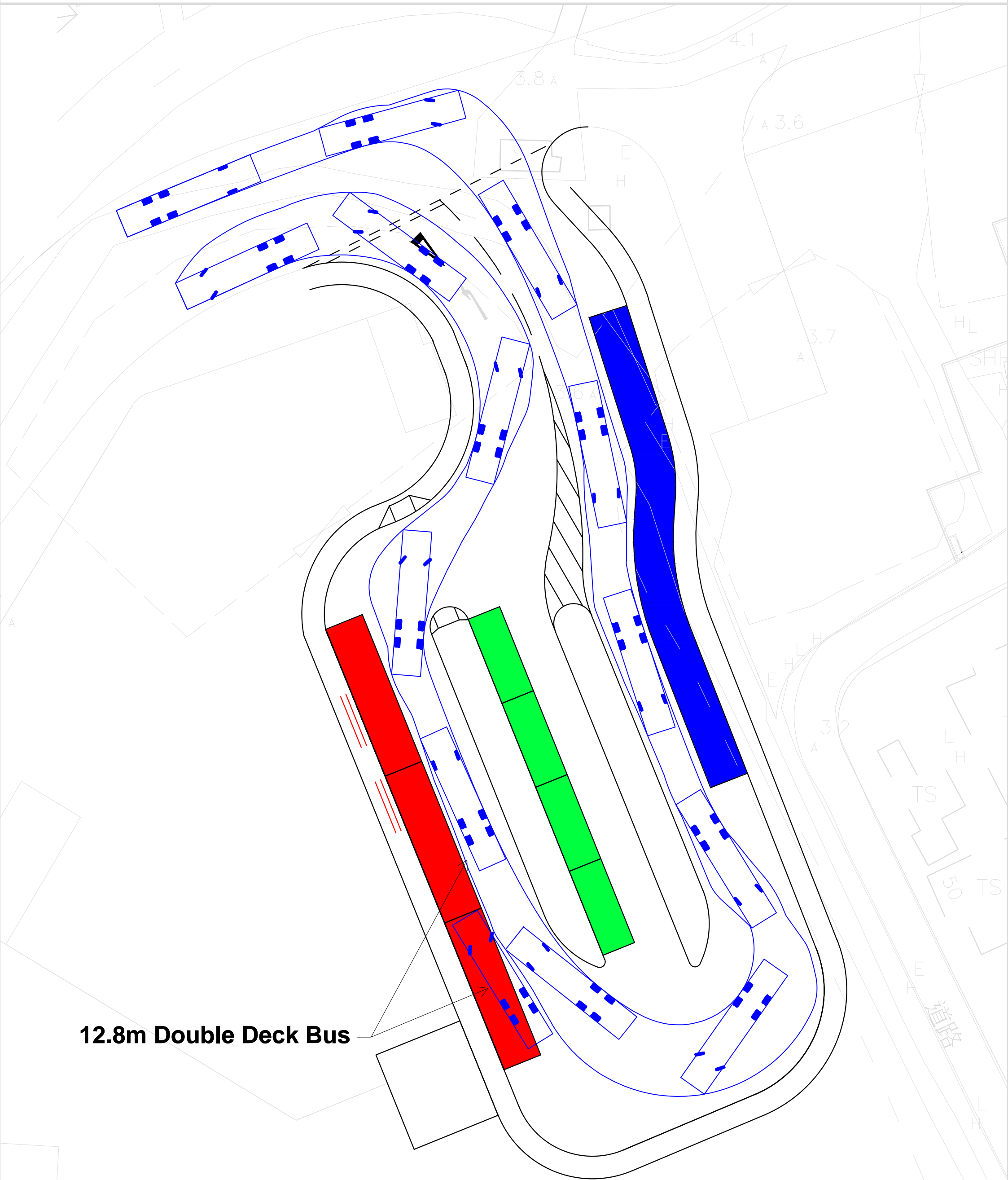
Swept Path Analysis





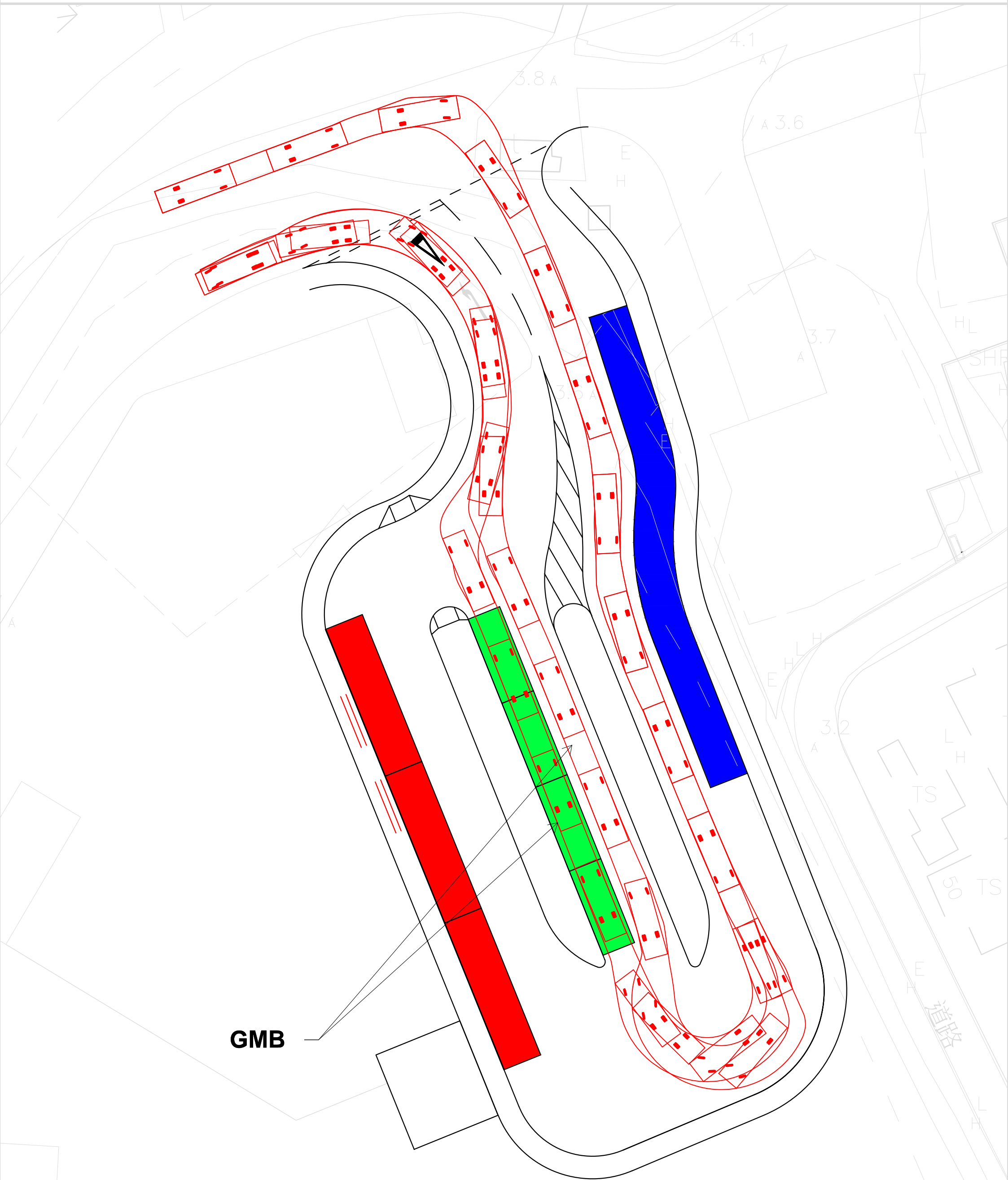
12.8m Double Deck Bus

ANNEX C - SWEPT PATH DIAGRAM AT TI
1:300 (A3)
Annex C - SK5

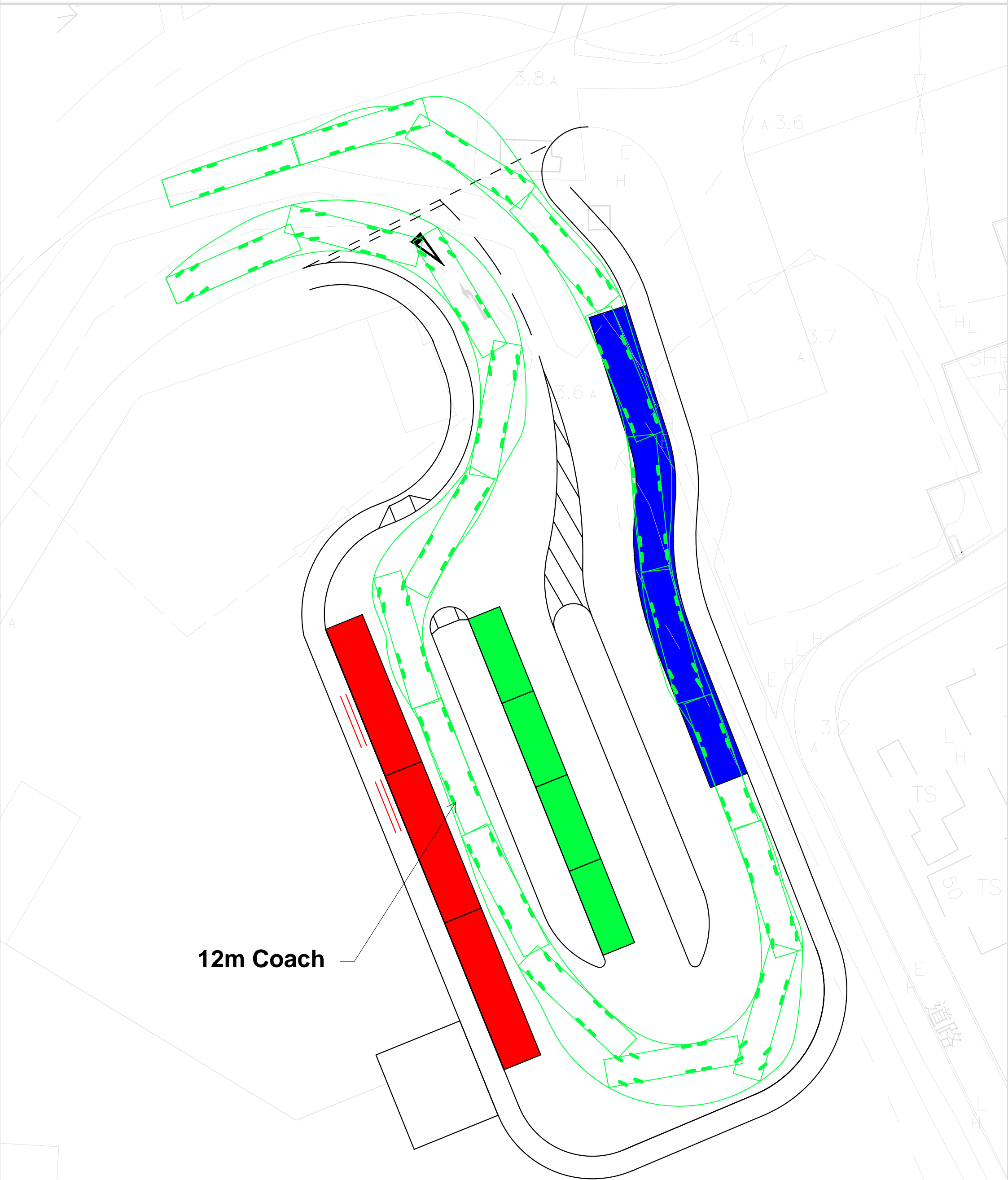


12.8m Double Deck Bus

ANNEX C - SWEPT PATH DIAGRAM AT TI
1:300 (A3)
Annex C - SK6



ANNEX C - SWEPT PATH DIAGRAM AT TI
1:300 (A3)
Annex C - SK7

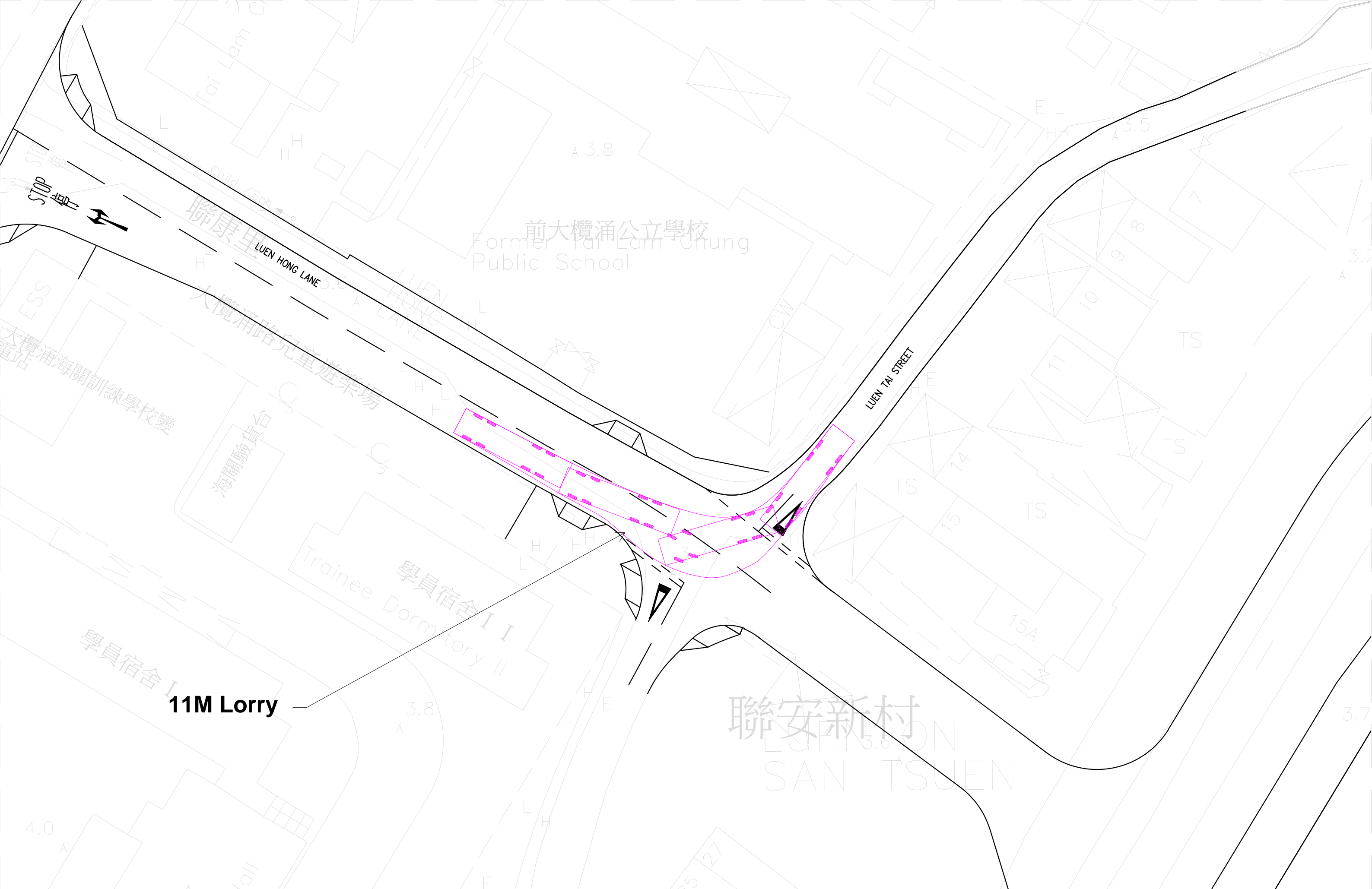


ANNEX C - SWEPT PATH DIAGRAM AT TI
1:300 (A3)
Annex C - SK8



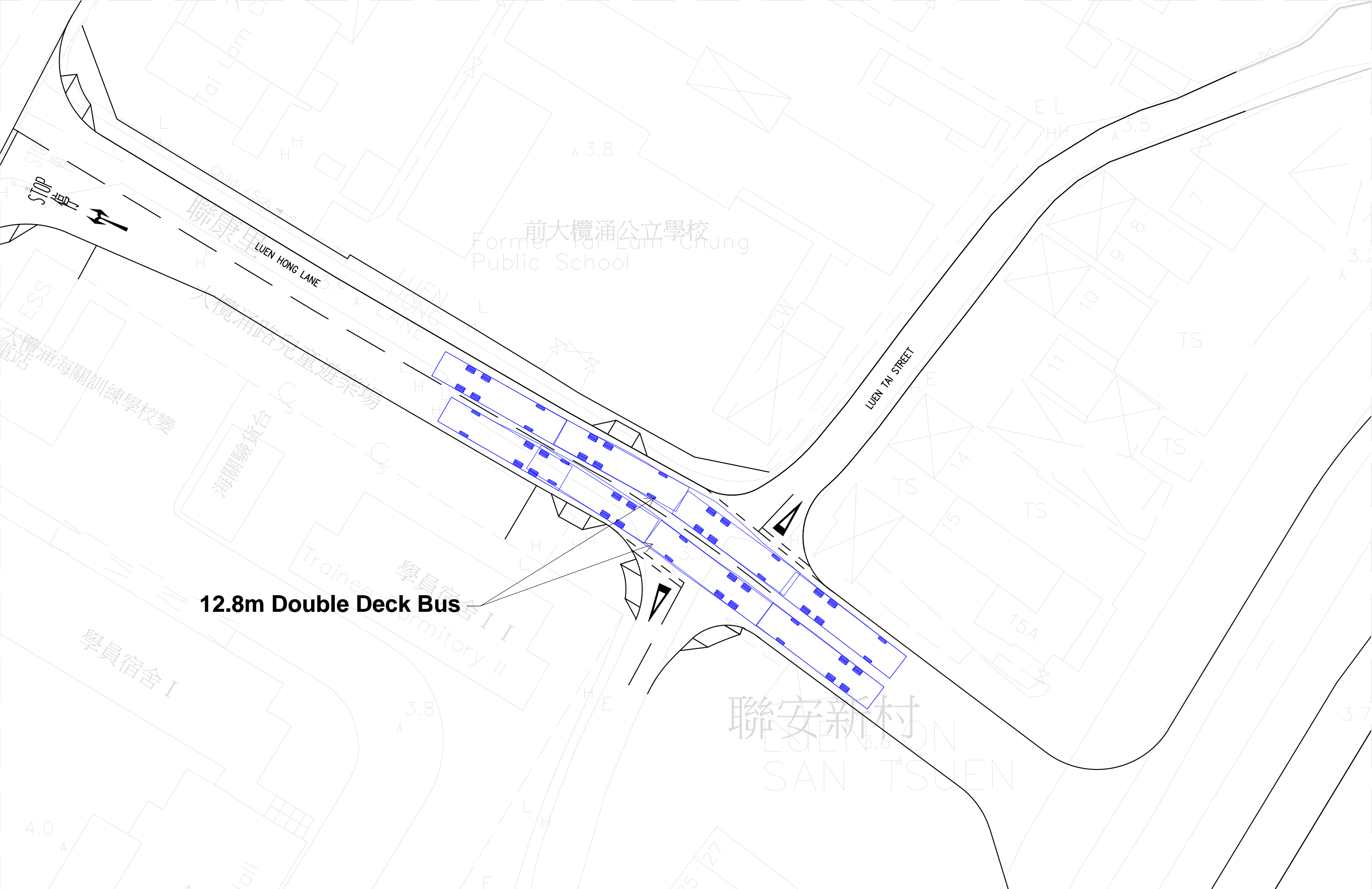
ANNEX C - SWEPT PATH DIAGRAM AT LUEN HONG LANE / LUEN TAI STREET JUNCTION
1:300 (A3)
Annex C - SK9



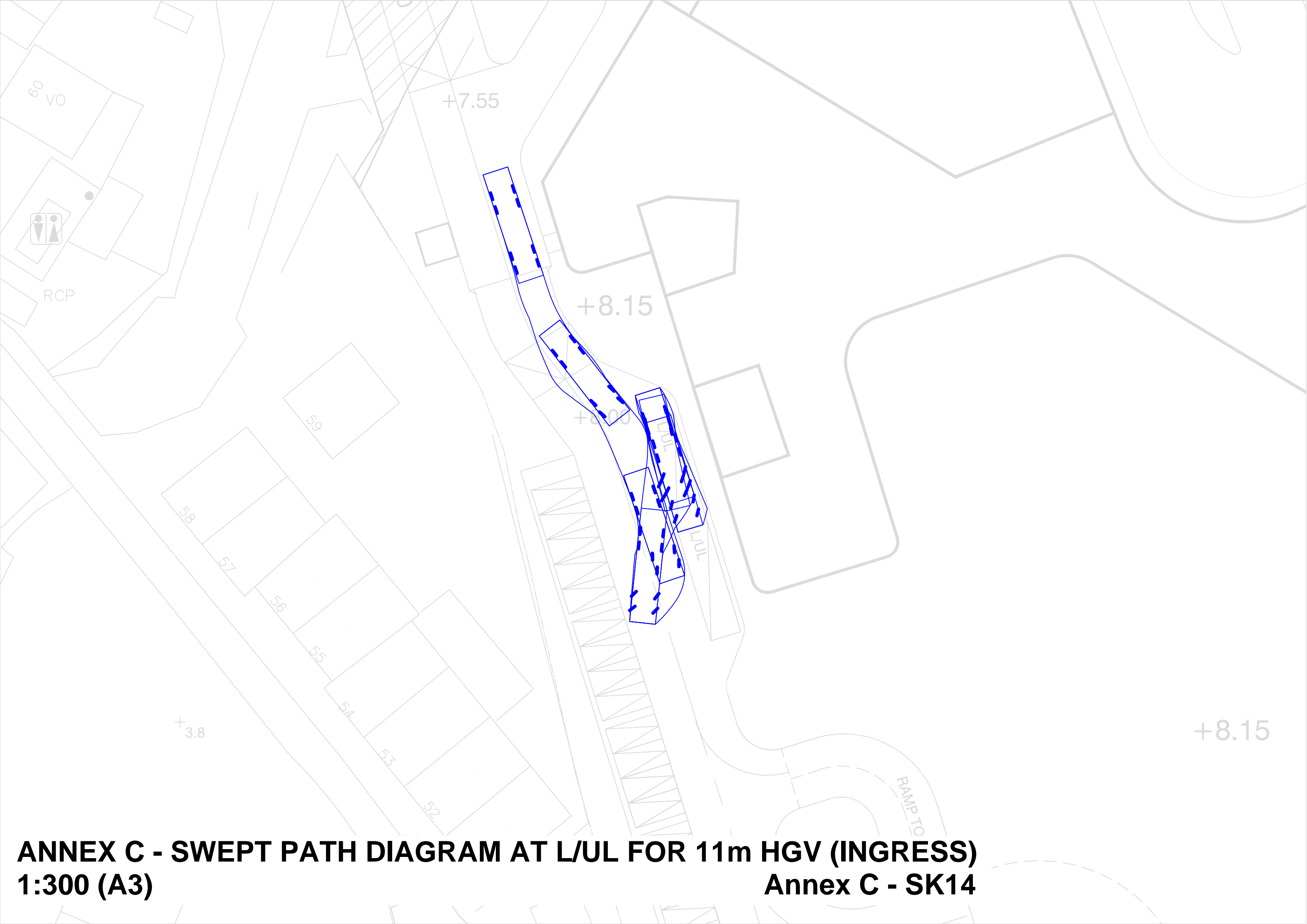


ANNEX C - SWEPT PATH DIAGRAM AT LUEN HONG LANE / LUEN TAI STREET JUNCTION
1:300 (A3)
Annex C - SK11

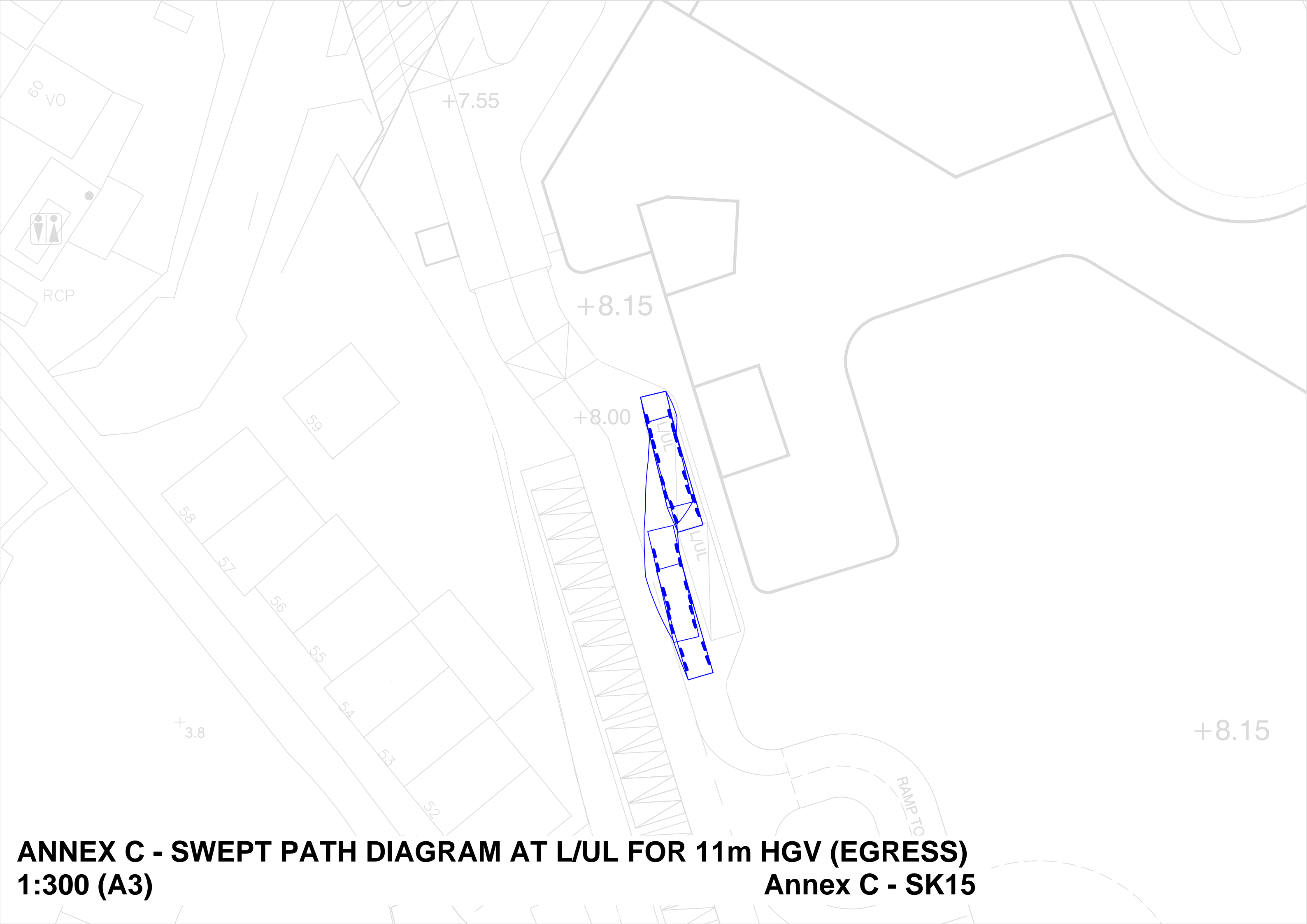




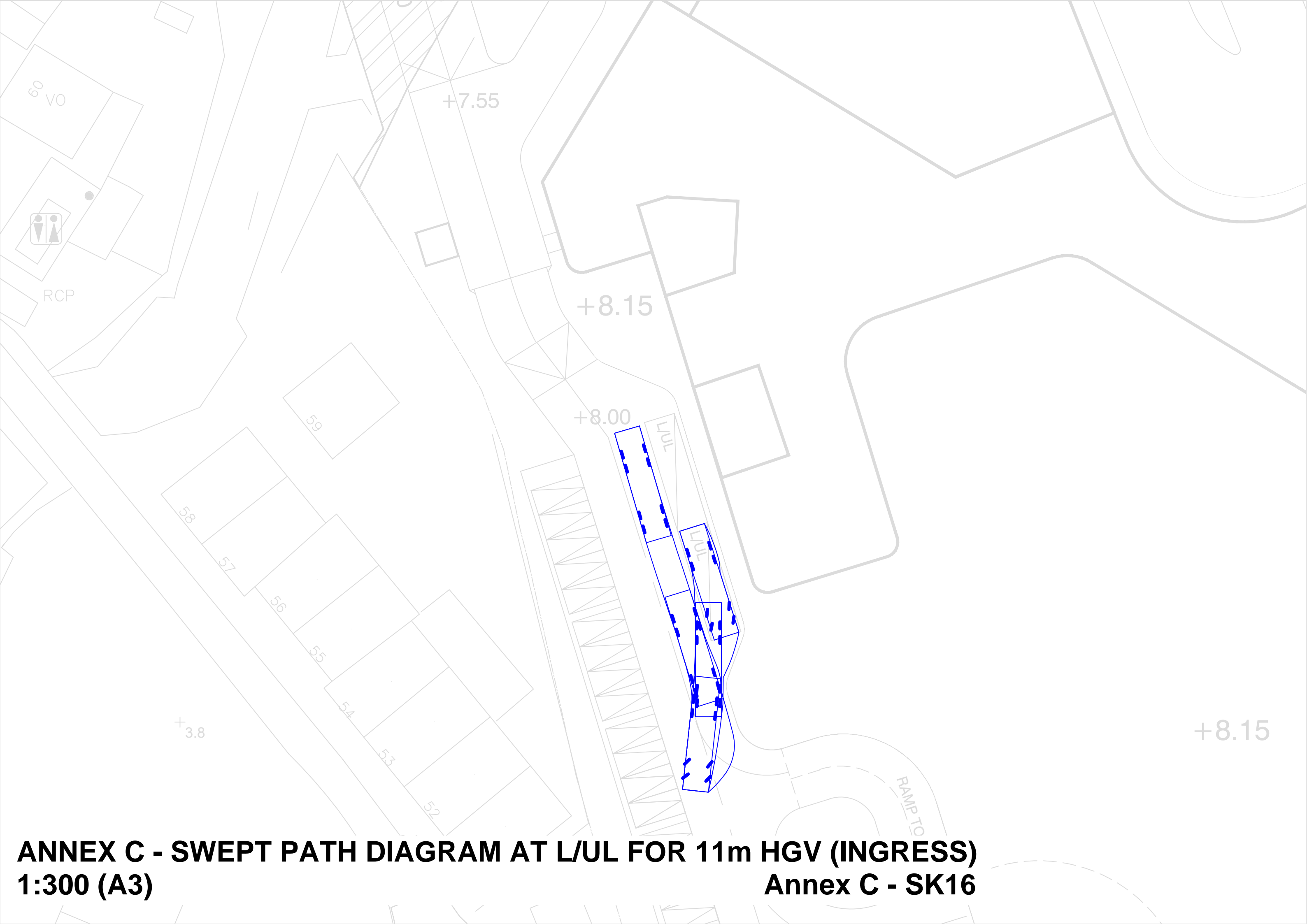
ANNEX C - SWEEP PATH DIAGRAM AT LUEN HONG LANE / LUEN TAI STREET JUNCTION
1:300 (A3)
Annex C - SK13



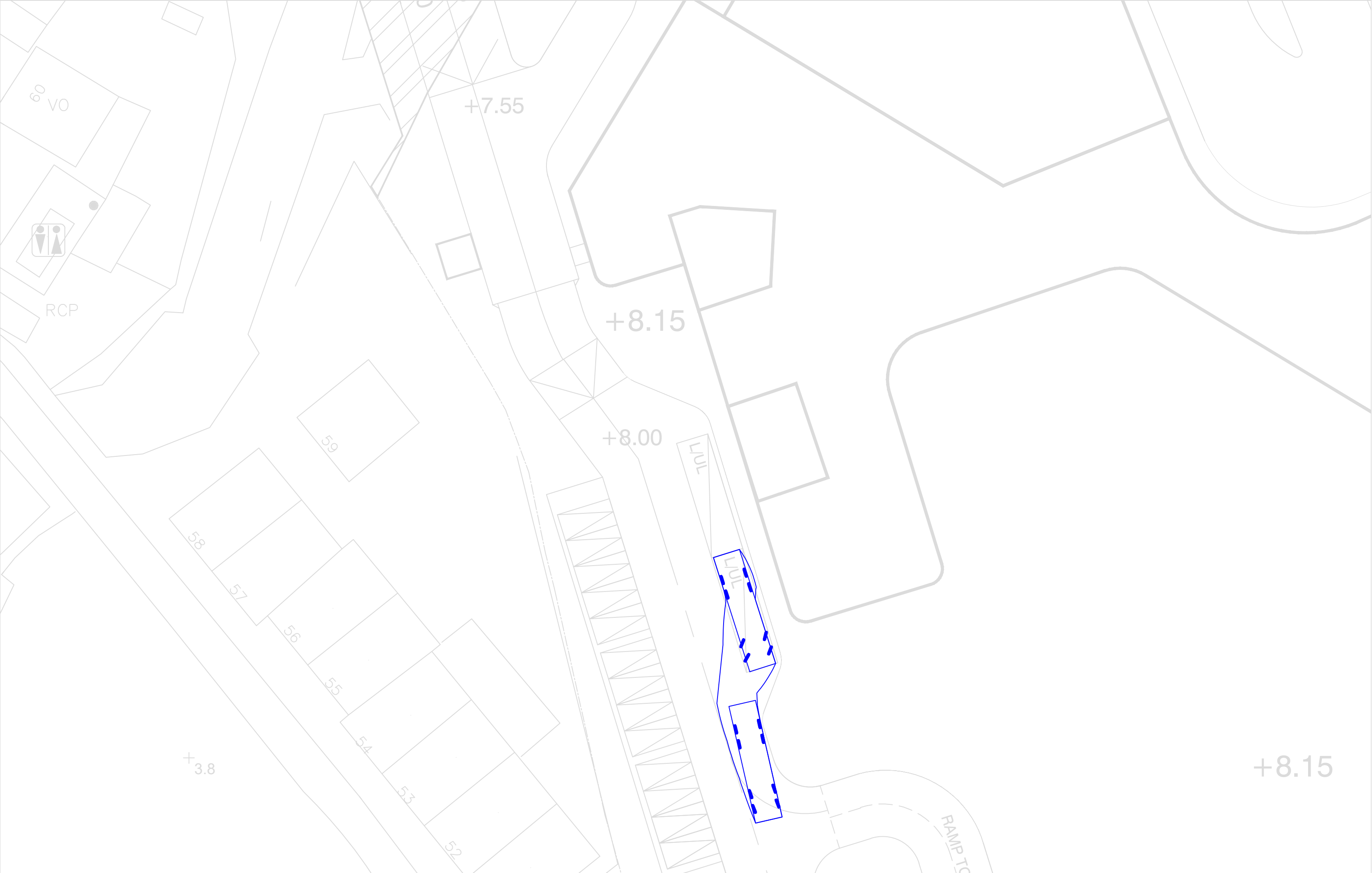
ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (INGRESS)
1:300 (A3)
Annex C - SK14



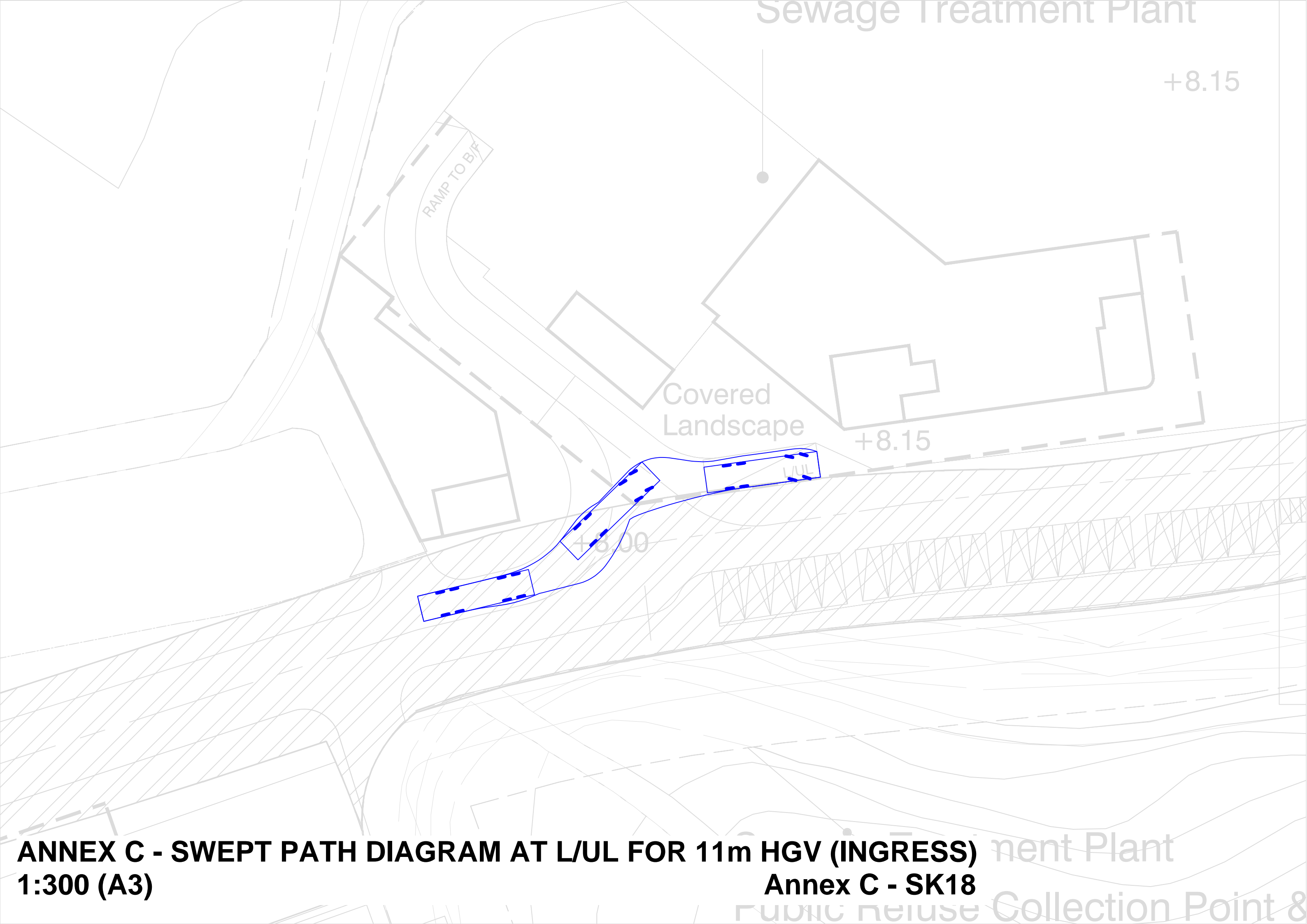
ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (EGRESS)
1:300 (A3)
Annex C - SK15



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (INGRESS)
1:300 (A3)
Annex C - SK16



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (EGRESS)
1:300 (A3)
Annex C - SK17



Sewage Treatment Plant

+8.15

RAMP TO BR

Covered
Landscape

+8.15

+8.00

L/UL

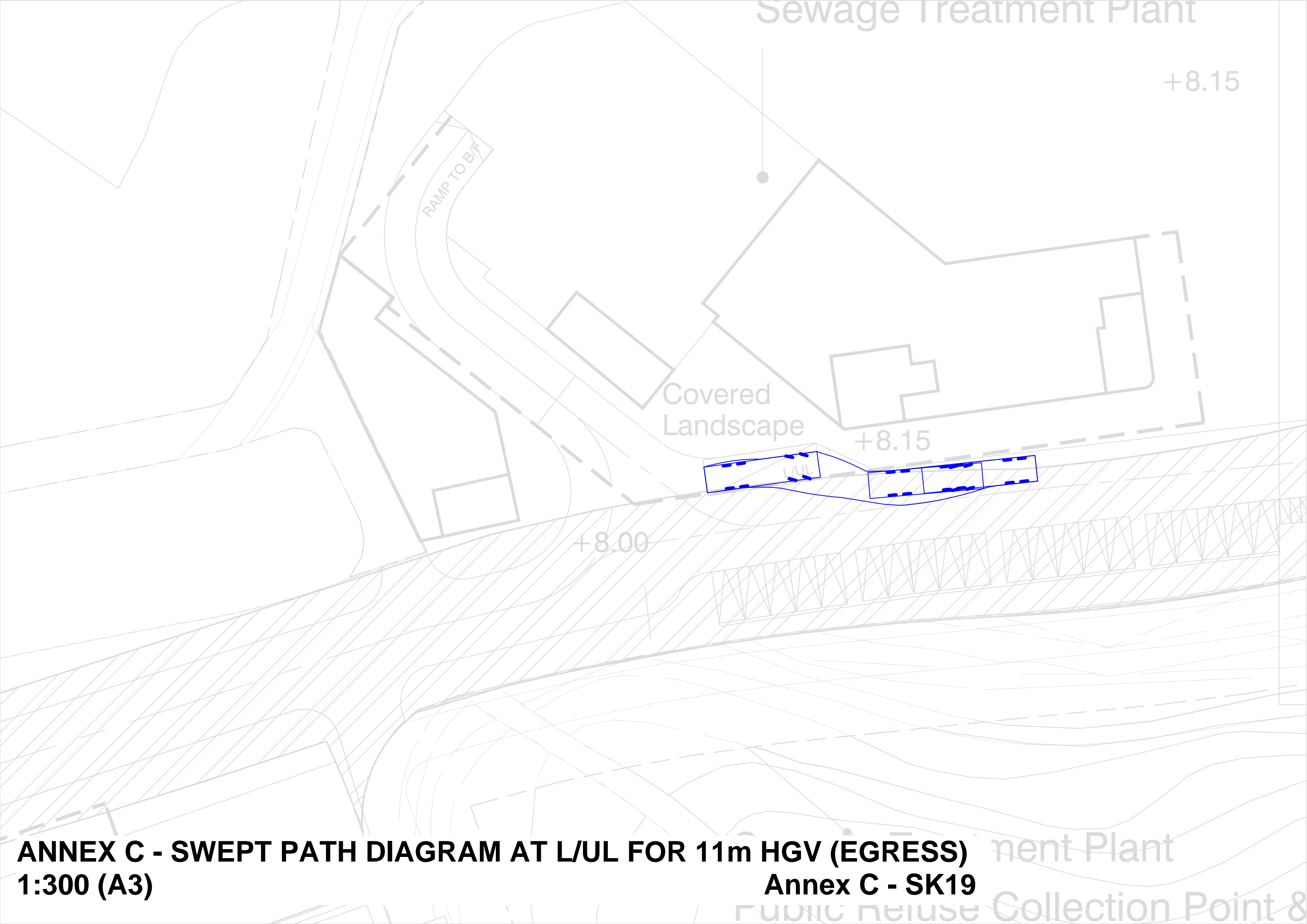
ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (INGRESS)

1:300 (A3)

Annex C - SK18

nent Plant

Collection Point &



Sewage Treatment Plant

+8.15

RAMP TO BIF

Covered
Landscape

+8.15

+8.00

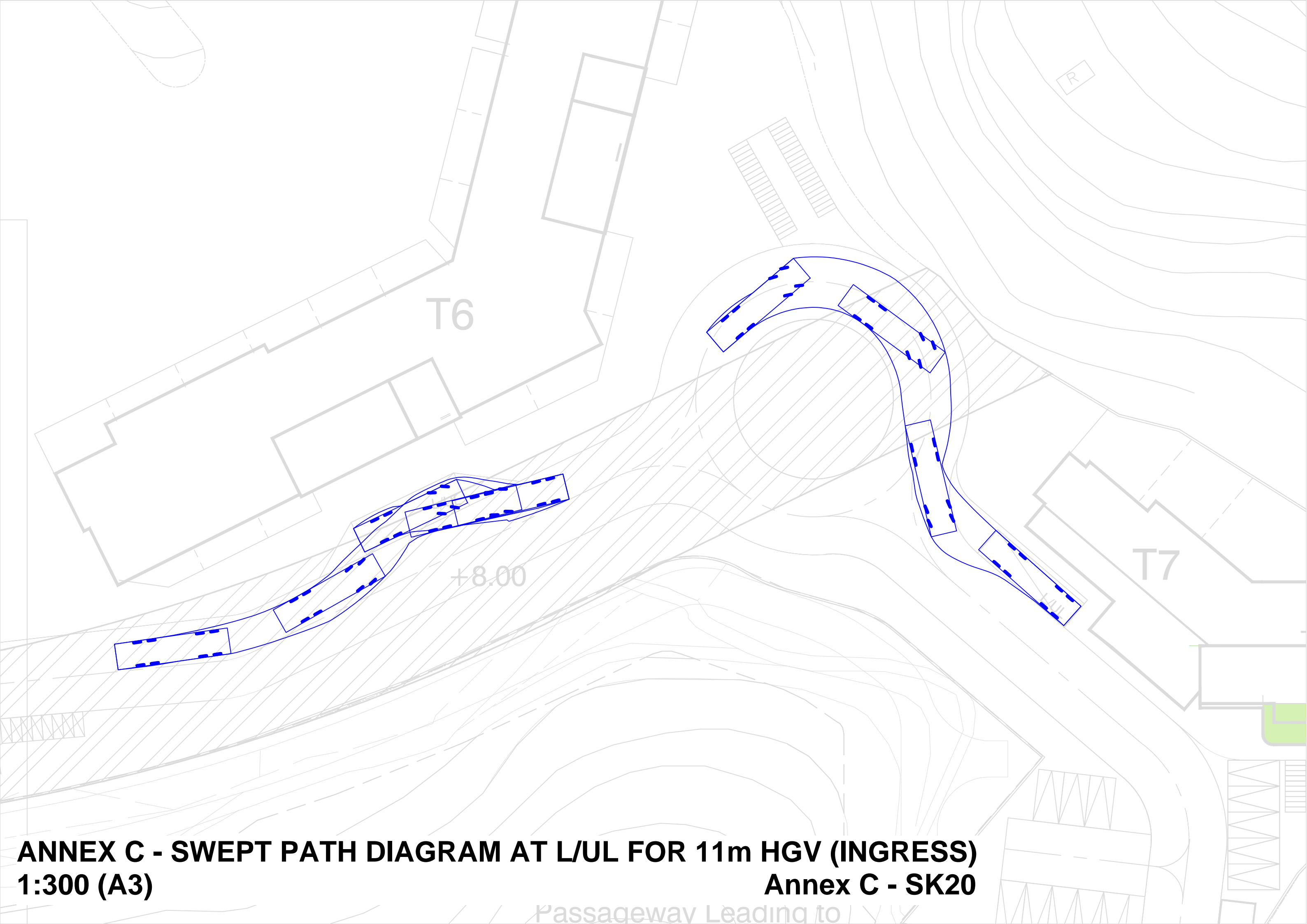
L/UL

ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (EGRESS)
1:300 (A3)

Annex C - SK19

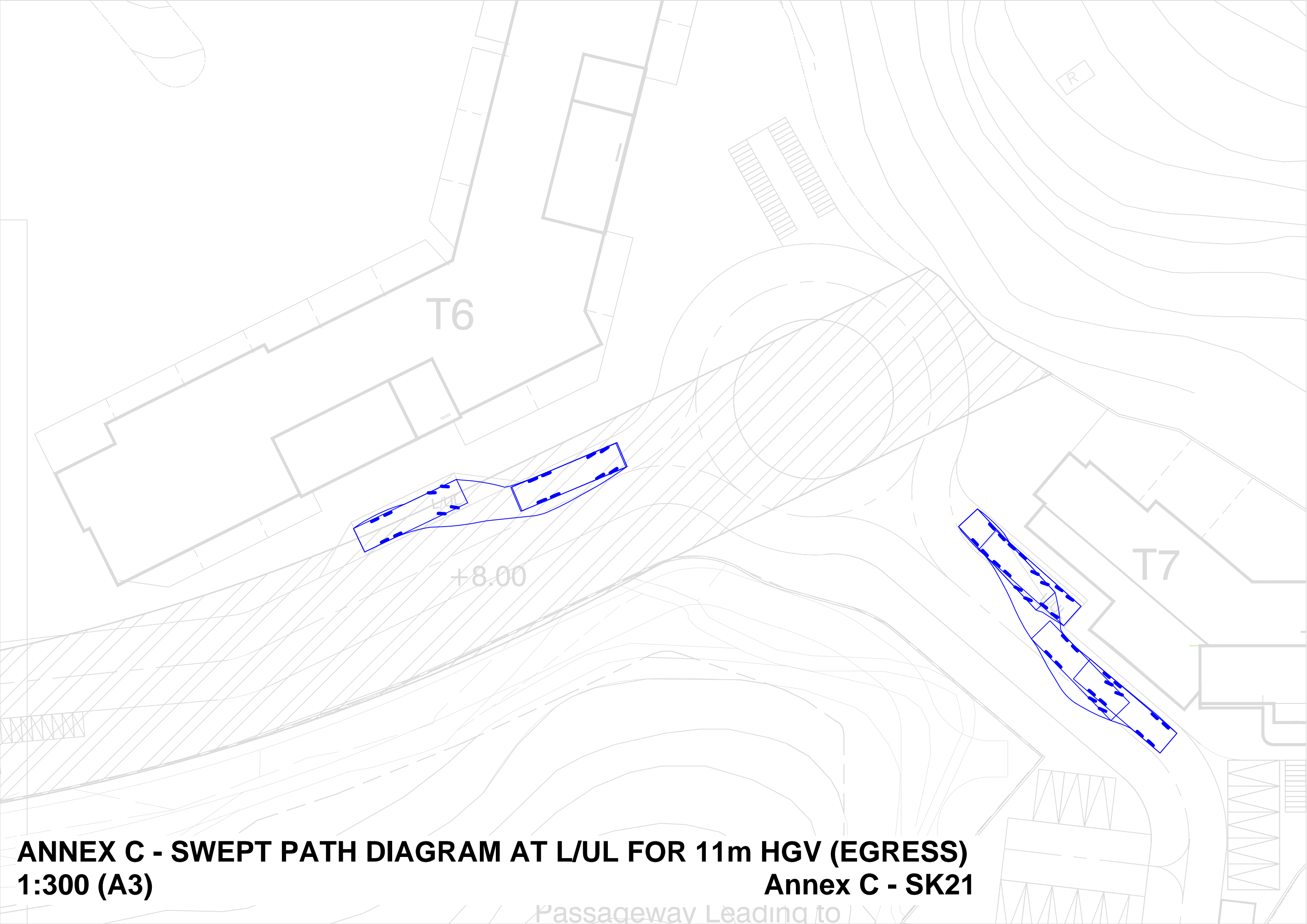
ment Plant

Collection Point &



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (INGRESS)
1:300 (A3) **Annex C - SK20**

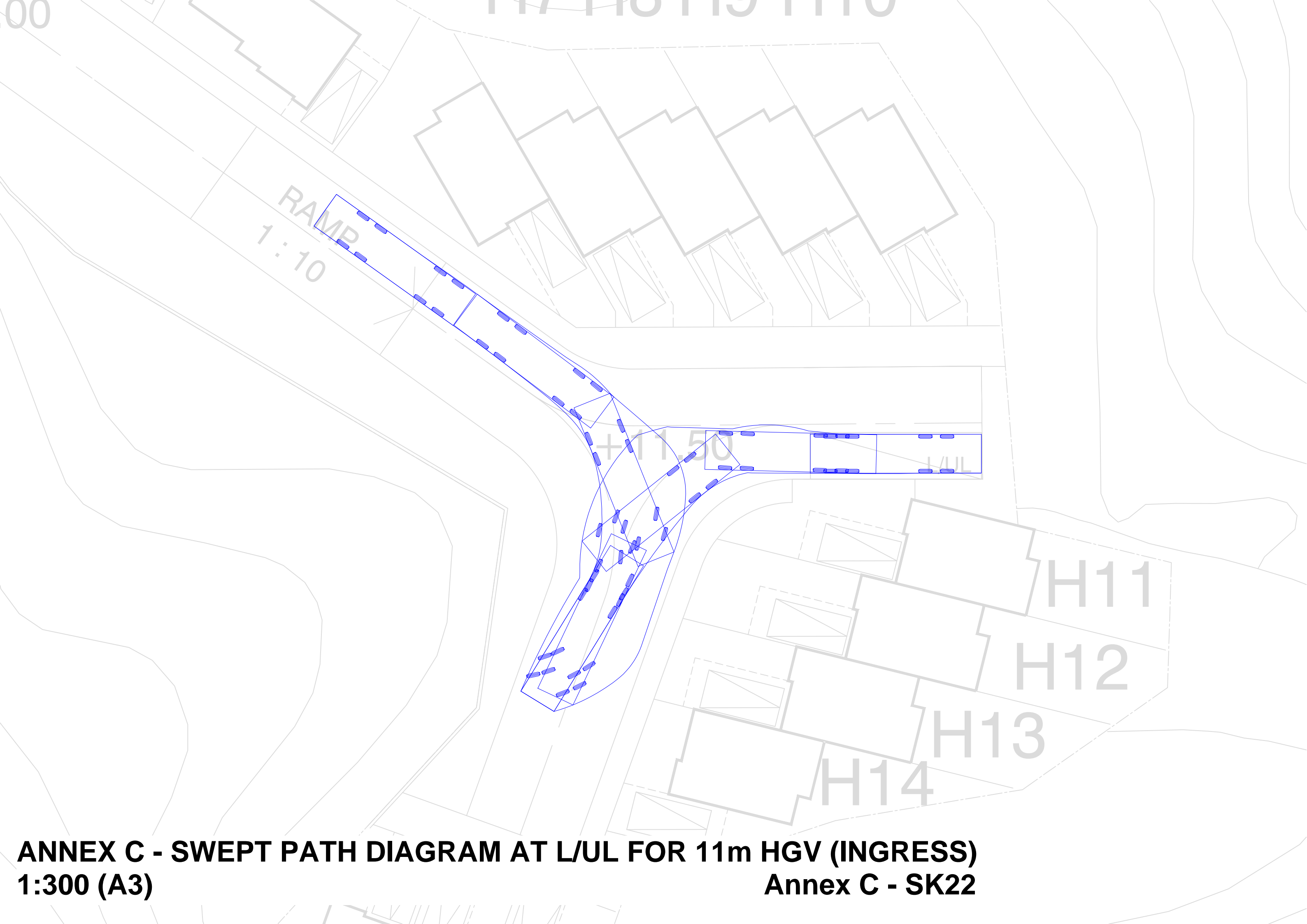
Passageway Leading to



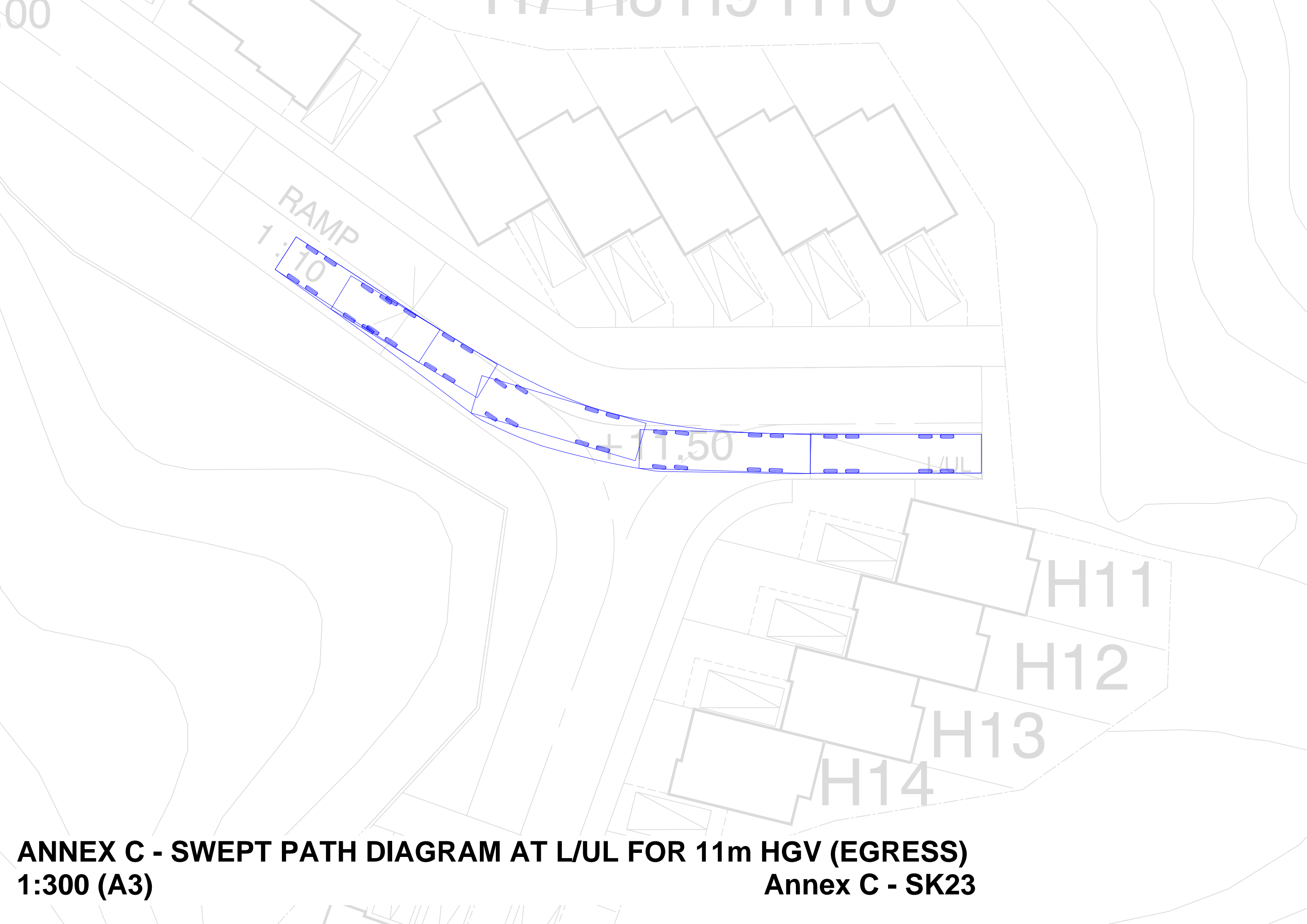
ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (EGRESS)
1:300 (A3)

Annex C - SK21

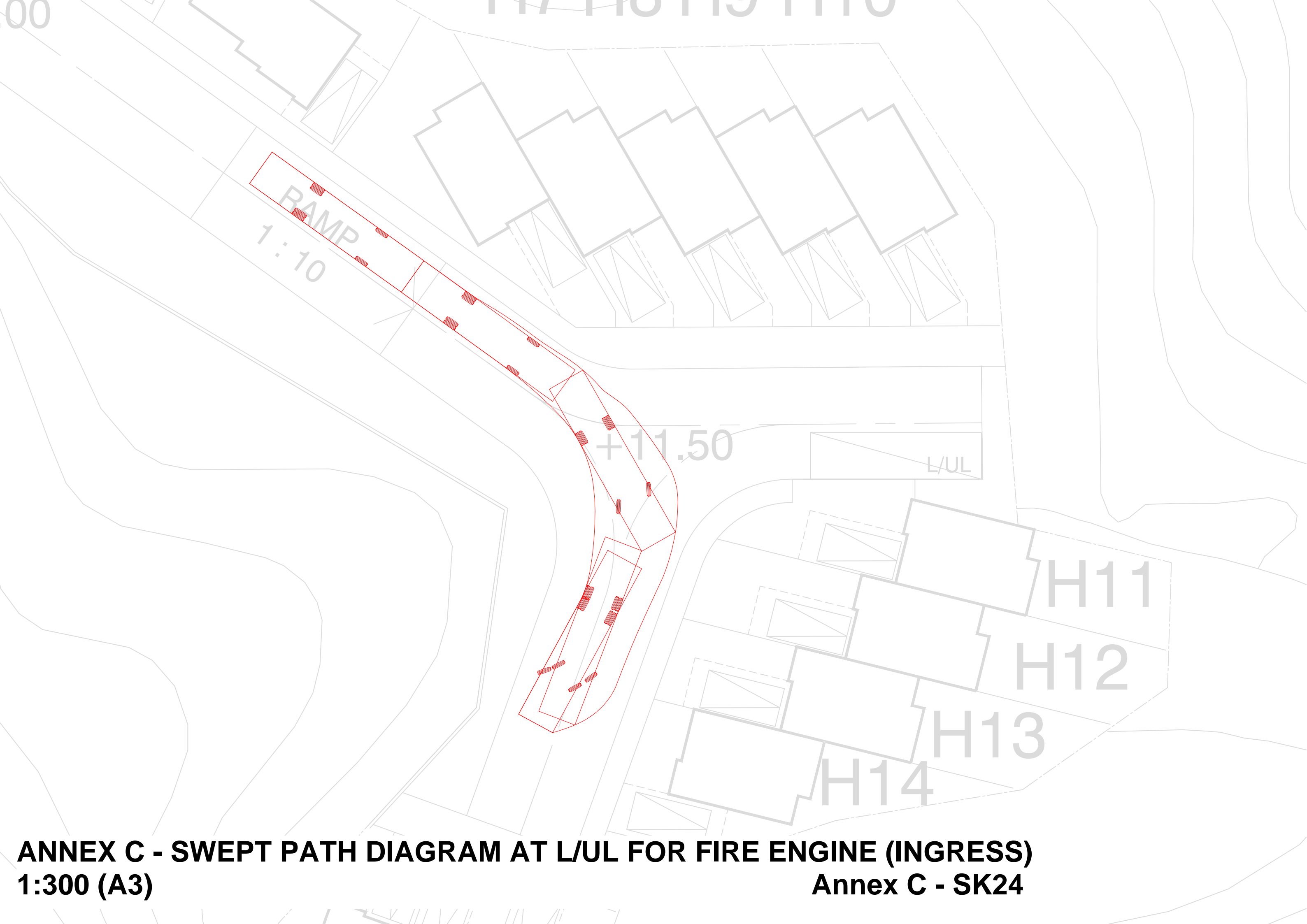
Passageway Leading to



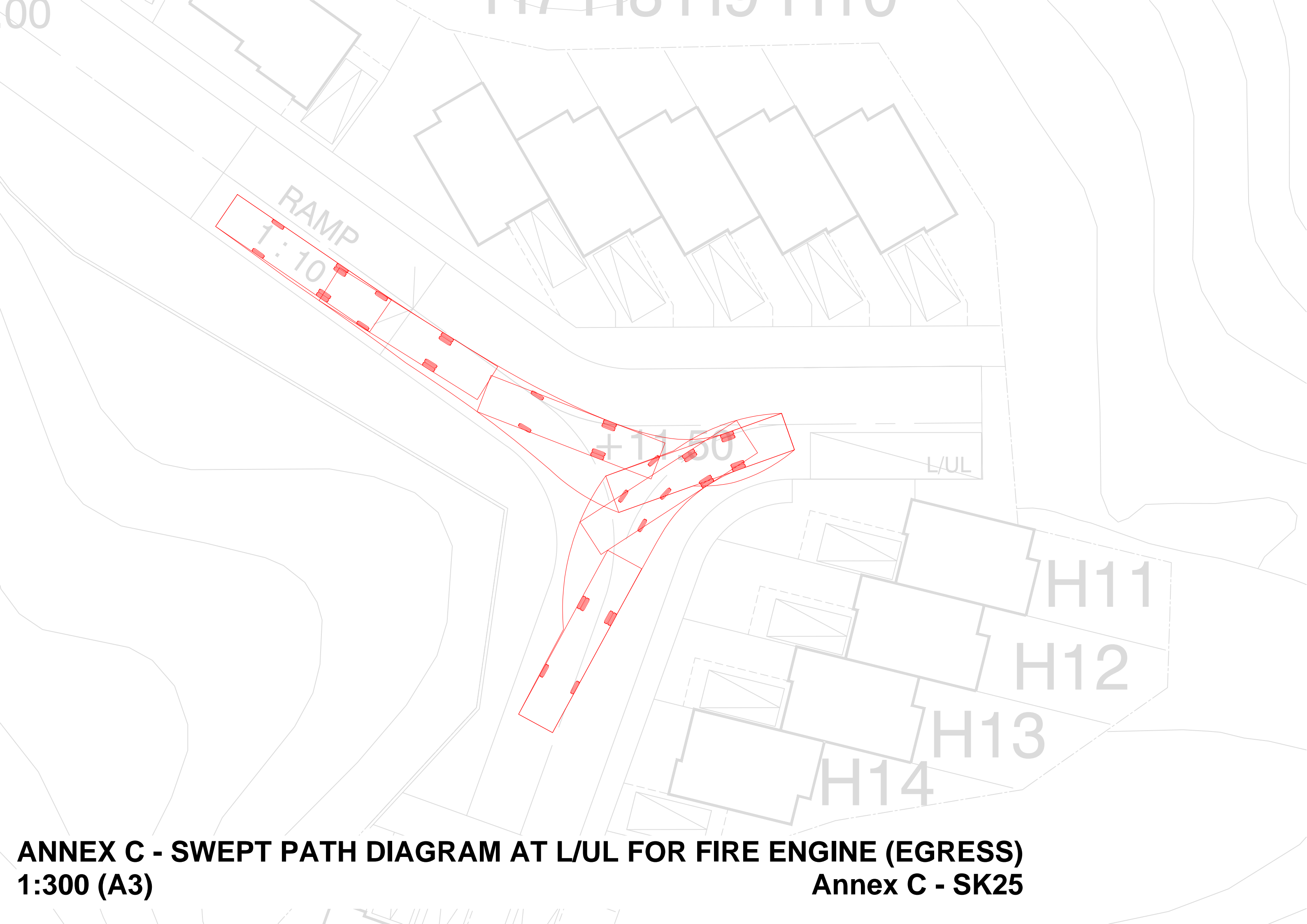
ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (INGRESS)
1:300 (A3)
Annex C - SK22



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR 11m HGV (EGRESS)
1:300 (A3) **Annex C - SK23**



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR FIRE ENGINE (INGRESS)
1:300 (A3)
Annex C - SK24



ANNEX C - SWEPT PATH DIAGRAM AT L/UL FOR FIRE ENGINE (EGRESS)
1:300 (A3) **Annex C - SK25**