# Appendix II

Traffic Impact Assessment



Traffic Impact Assessment Report – Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land









Document No. W1037/TIA/001/DBR

**Issue 1** 

**July 2025** 



W1037/TIA/001/DBR Issue 1 July 2025

Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land for a Period of 3 Years, Various Lots in D.D. 128, Pak Nai, Yuen Long, New Territories

Traffic Impact Assessment Report – Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land

Approved for Issue by:

Position:

Project Manager

Date:

July 2025

Sum Wui Investment Limited 205A, Sik Kong Tsuen, Ha Tsuen, Yeun Long, N.T.

Mannings (Asia) Consultants Ltd 5/F, Winning Commercial Building 46-48 Hillwood Road, Tsim Sha Tsui Kowloon, Hong Kong

Traffic Impact Assessment Report – Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land

Issue	Prepared by	Reviewed by	Date
1	НС	KW	July 2025

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Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land for a Period of 3 Years, Various Lots in D.D. 128, Pak Nai, Yuen Long, New Territories

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# 1.0 INTRODUCTION

#### 1.1 Project Background

- 1.1.1. Mannings (Asia) Consultants Ltd (MANN) was commissioned by Sum Wui Investment Limited to undertake the Traffic Impact Assessment (TIA) study for the Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land (The Site) located on Various Lots in D.D. 128, Pak Nai, Yuen Long, New Territories.
- 1.1.2. The Site falls within an area zoned "Agriculture" ("AGR") on the Approved Ha Tsuen Fringe Outline Zoning Plan (OZP) No.: S/YL-HTF/12. The Site occupies an area of 9,938 m² (about). A 2-storey structure is proposed at the Site for site office and guardroom uses with total gross floor area (GFA) of 60 m² (about). The remaining area is reserved for area for open storage operations, vehicle parking and loading/unloading (L/UL) spaces and circulation area.
- 1.1.3. The Site is accessible via Deep Bay Road, Kai Pak Ling Road, and a temporary road constructed by another CEDD contract, which connects to the at-grade road network of Kong Shum Western Highway. The operation hours of the proposed development are Monday to Saturday from 09:00 to 19:00. There is no operation on Sunday and public holidays.
- 1.1.4. The Considering the potential for increased traffic from the Site, this TIA study will be conducted to evaluate the effects on the surrounding road network.

## 2.0 OBJECTIVES

- 2.0.1. The objectives of this TIA study cover:
  - To evaluate the feasibility of the Site from traffic engineering perspectives; and
  - To assess the traffic impact of the Site to the adjacent road network and road junction during operation of the Site.



#### 3.0 EXISTING TRAFFIC CONDITION

#### 3.1 Existing Traffic Pattern

- 3.1.1. Under the operation stage, the Site is accessible via Deep Bay Road, Kai Pak Ling Road, and a temporary road constructed by another CEDD contract, which connects to the at-grade road network of Kong Shum Western Highway. This is the proposed delivery route to the Site and mainly divided into three road section. The specifics of the delivery route and the details of three road sections are presented in Drawings No. Figure 1 of Delivery Route Plan in Appendix A.
- 3.1.2. Regarding Road Section 1, Deep Bay Road between the Site and Kai Pak Ling Road, the road width, as measured by the basemap of Lands Department, is approximately 3.0 meters. Observations and on-site measurements indicate that vehicles utilize the verge area, resulting in a total width exceeding 3.5 meters for vehicle use. However, due to the lack of intervisible passing bays, it is considered a substandard single-track access road.
- 3.1.3. Regarding Road Section 2, Kai Pak Ling Road, which lies between Deep Bay Road and a temporary road constructed under a separate CEDD contract, this section of Kai Pak Ling Road is a standard single-track access road. It features an approximate road width of 3.5 meters and includes passing bays that are intervisible, ensuring adequate provision for vehicles.
- 3.1.4. Regarding Road Section 3, the temporary road built by another CEDD contract, situated between Kai Pak Ling Road and the at-grade road network of Kong Shum Western Highway, this section of temporary road partially utilizes the permanent road configuration for public use during its construction phase. The road width of this temporary road is approximately 7 meters which is a single carriageway. Under the CEED contract, the permanent road directly connects with the existing roundabout of the at-grade road network of Kong Shum Western Highway.



#### 3.2 Observed Traffic Flow

3.2.1. Manual classified traffic count survey in the study area were carried out on 11 June 2025 (Wednesday) from 07:00 to 20:00 in order to collect the most updated traffic flow volume of the affected road section and access the feasibility of the works as shown in **Table 1** and the survey locations are indicated in Drawing No. **Figure 2** in **Appendix A.** 

Table 1 - Affected Road Junctions and Roundabout

J1	The priority junction of Deep Bay Road with Kai Pak Ling Road
12	The roundabout of Deep Bay Road with Lau Fau Shan Road / Shan
JZ	Tung Street

3.2.2. According to the survey results, the peak hour of the affected junctions is different during the survey period. The peak hour flows are summarized in **Table 2**.

Table 2 - Peak Hour Flow of the Affected Road Junctions / Roundabout

	Affected Road Section / Junction	AM	PM
	The priority junction of Deep Bay Road with Kai Pak Ling Road	PEAK	PEAK
J1	The priority junction of Doop Poy Bood with Kei Dok Ling Bood	07:45-	16:15-
JI	The phority junction of Deep Bay Road with Rai Pak Ling Road	08:45	17:15
12	The roundabout of Deep Bay Road with Lau Fau Shan Road / Shan	07:30-	17:15-
JZ	Tung Street	08:30	18:15

3.2.3. The peak hour flow at each affected junction varies from 07:30 to 08:45 (AM PEAK) and 16:15 to 18:15 (PM PEAK). In order to present the peak hour flow at each junction for the most critical scenario, we have used the flow data at the peak hours of each junction and assemble them together in one traffic flownet as shown in Figure 3 in Appendix A.



#### 4.0 TRAFFIC FORECAST

- 4.1. According to the preliminary plan, the Site is expected to be completed by 2025 and operate for a period of three years. However, since the planning application involves a 3-year development period, the study conservatively adopts 2028 as the design year. Accordingly, traffic flows during the operational phase should be projected based on conditions in 2028.
- 4.2. Traffic forecasts are estimated based on the results of the observed traffic survey and the 2019-Based Territorial Population and Employment Data Matrices (2019 TPEDM) published by Planning Department and the Annual Average Daily Traffic (AADT) data of the latest five years. The three sets of data aim to facilitate the assessment of the strategic development opportunities in the territory.
- 4.3. Territorial Population and Employment Data Matrices (TPEDM)
- 4.3.1. Table 3 presented the population and employment data in Northwest New Territories for 2019 and 2026 from 2019-based Territorial Population and Employment Data Matrices (TPEDM) provided by Planning Department.

Table 3 - Territorial Population and Employment Data Matrix (TPEDM)

Cotogory	T	Annual			
Category	2019	2019 2023 <sup>(1)</sup> 202		Growth	
Population	222,800	232,200	239,250	1.02%	
Employment	58,400	68,943	76,850	4.00%	
Total	281,200	301,143	316,100	1.69%	

Source: 2019-based TPEDM published by Planned Department

Note (1): 2023 population and employment places are calculated by interpolation

- 4.4. Annual Average Daily Traffic (AADT)
- 4.4.1. Reference is made from the Annual Traffic Census (ATC) Reports for the ATC stations within the Study Area, Table 4 describes the location of the nearby ATC station and provides the corresponding traffic data.

Table 4 - Annual Traffic Census (ATC) Data

Location	Stn No.	from	to	AADT (veh / day)					Annual Growth	
				2018	2019	2020	2021	2022	2023	
Ping Ha Rd & Fau Shan Rd	5858	Tin Ha Rd	Deep Bay Rd	12,680	12,590	12,070	10,310	8,390	8,590	-7.49%



# 4.5. Method of Forecasting

4.5.1. The traffic growth rates over successive years are presented in Table 3 and Table 4, respectively. The purpose of forecasting traffic flow for the year 2028 is to support traffic impact assessments during both the construction and operational phases as well as to anticipate future conditions. An annual growth rate of 1.69% is identified in Table 3, whereas a negative annual growth rate of -7.49% is shown in Table 4. Therefore, to adopt a conservative approach, the higher annual growth rate of 1.69% has been used for forecasting traffic flow in 2028.

#### 4.6. Future Vehicular Flows

- 4.6.1. As the planning application indicates that the temporary open storage development will run for a period of 3 years, and the expected end year for the project site is 2028. This design year was adopted to reflect the operational period of the open storage, which aligns with the 3-year project duration described throughout the report. The traffic flow in year 2025 was obtained from the manual traffic count surveys undertaken 11 June 2025 (Wednesday). These survey flows were subsequently used as the base year traffic flows for the required traffic forecast.
- 4.6.2. As The forecasted traffic flows for year 2028 are based on the estimation equation as shown in Table 5. The resultant factor is shown in Table 6 for Traffic Growth Factor. This growth factor is applied to the relevant road sections in respect to the proximity of the locations.

Table 5 - Traffic Flows Estimation Equation (Peak 15 mins)

Scenario	Equation		
2028 Traffic Flows	$2025 \text{ Flows} \times (1+1.69\%)^3$		

Table 6 - Traffic Growth Factors (Peak 15 mins)

Scenario	2025 Growth Factor		
2028 Traffic Flows	2025 Flows × 1.032		

2028 Reference Flows = 2025 Flows x annual growth factors

2028 Design Flows = 2028 Reference Flows + Additional Traffic by Development

4.6.3. The 2028 Reference Traffic flownet and 2028 Design Traffic flownet are shown in Figure 4 and Figure 6 in Appendix A. And, the additional traffic flow by the development is shown in Figure 5 in Appendix A.



#### 5.0 VEHICULAR TRAFFIC IMPACT ASSESSMENT

- 5.1. Estimation of Development Flows
- 5.1.1. To estimate the vehicular trips generated from the Site, trip rate derived from the TIA Final Report prepared by CKM Asia Limited under planning permission No. A/YL-HTF/1133 for the use of "Proposed Temporary Open Storage of New Vehicles (Private Cars), Construction Materials, Machineries, Equipment and Storage of Tools and Parts with Ancillary Site Office for a Period of 3 Years and Filling of Land at Various Lots in D.D. 128 and adjoining Government Land, Ha Tsuen, Yuen Long, New Territories" (hereinafter called "Previous CKM Study") is adopted in this Study.
- 5.1.2. Adopted trip rate and projected development traffic for the Site are presented in Table 7-1 and Table 7-2 respectively.

Table 7-1 Adopted Daily Trip Rate from TIA Report under Previous CKM Study

Development Type	Daily Trips Rate		
Open storage	0.00036 veh/m <sup>2</sup>		

- 5.1.3. Refer to the TIA Final Report under Previous CKM Study, 25% of traffic are generated during the AM and PM Peak periods. The calculated AM and PM peak hour traffic generation by the Site are presented in Table 7-2.
- 5.1.4. Table 7-2 Calculated Peak Hour Traffic Flows for the Site

Development Type			Vehicular Trips				
	Parameter for the Site	Item	Weekday AM		Weekday PM		
			In	Out	In	Out	
Open storage	Site Area =	Trip Generation (veh/hr)	1	1	1	1	
	9,938 m <sup>2</sup>	Trip Generation (pcu/hr) <sup>(1)</sup>	3	3	3	3	

Note: (1) For conservative approach, it is assumed that all vehicles are heavy vehicles with pcu factor 2.5.

5.1.5. The calculated peak hour development traffic flow for the Site is expected to be 3 pcu's (equivalent to 1 veh.) per direction for both AM and PM peak hours.



- 5.2. Future Link Capacity Assessment
- 5.2.1. In order to determine the utilization level of the affected, the Vehicle Capacity (VC) has been adopted. To estimate the traffic flow generated from the Site, it is assumed that 3 pcu's (equivalent to 1 veh.) per direction for both AM and PM peak hours
- 5.2.2. The link capacity assessments for year 2028 Reference and Design Scenario carried out and the results are presented in Table 8.

Table 8 - Summary of Future Link Capacity Assessment

Road Section		•	Design Capacity	2028 Reference				2028 Design			
	Location	Dir.		AM		PM		AM		PM	
				Flows (veh/hr)	P/Df <sup>(1)</sup>						
R1	Deep Bay Road	2-way	100	67	0.67	60	0.60	69	0.69	62	0.62
R2	Kai Pak Ling Road	2-way	100	40	0.40	28	0.28	42	0.42	30	0.30
R3	Temporary road	2-way	800	68	0.09	44	0.06	70	0.09	46	0.06

Notes: (1) P/Df = Peak Hourly Flows/ Design Flow Ratios for road links

- 5.2.3. The results in Table 8 indicate that all the concerned road links in the Study Area operate satisfactorily during the peak hours under the 2028 Reference Scenario (Without the Site) and Design Scenario (with the Site).
- 5.3. Future Junction Capacity Assessment
- 5.3.1. The junction capacity assessments for year 2028 Reference and Design Scenario carried out and the results are presented in Table 9. The detailed calculation sheets are shown in Appendix B.

Table 9 - Summary of Future Junction Capacity Assessment

Junction	Location	Туре	Capacity	2028 Re	ference	2028 Design		
	Location		Index	AM	PM	AM	PM	
J1	Deep Bay Rd/ Kai Pak Ling Rd	Priority	DFC	0.02	0.02	0.02	0.02	

5.3.2. Referring to the results in Table 9, the affected junction would be operating within capacity during peak hours for both the 2028 Reference Scenario (Without the Site) and Design Scenario (with the Site).



5.3.1. Although the proposed delivery route is not planned to pass through Junction J2, a conservative approach has been adopted to account for possible deviations in vehicle movements. It is assumed that approximately 10% of delivery vehicles may inadvertently enter Junction J2. Therefore, J2 has also been included in the capacity assessment to ensure the robustness and completeness of the evaluation. Detailed junction capacity assessments are provided in Appendix B.

Table 10 – Junction Capacity Assessment for Affected Roundabout

Junction	Location	Туре	Capacity Index	2028 Reference		2028 Design	
				AM	PM	AM	PM
J2	Deep Bay Rd/ Lau Fau Shan Rd	Roundabout	DFC	0.44	0.35	0.44	0.35

5.3.3. Referring to the results in Table 10, the affected junction would be operating within capacity during peak hours for both the 2028 Reference Scenario (Without the Site) and Design Scenario (with the Site).



#### 6.0 DEEP BAY ROAD UPGRADE WORKS

- 6.1. Based on Section 3.1, the proposed delivery route's Road Section 2 (Kai Pak Ling Road) and Road Section 3 (the temporary road constructed under a separate CEDD contract) are expected to meet standard road provisions for public use. Conversely, Road Section 1 (Deep Bay Road), connecting the Site to Kai Pak Ling Road, is identified as a substandard single-track access road, primarily due to the absence of intervisible passing bays.
- 6.2. Traffic assessment conducted during the operational phase of the Site indicates that the generated traffic will not significantly impact the roads along the delivery route. To further ensure smooth traffic flow, particularly on Road Section 1 of Deep Bay Road, the Site owner intends to implement mitigation measures. These measures include upgrading Road Section 1 of Deep Bay Road to a standard single-track access road with the provision of adequate, intervisible passing bays to facilitate two-way traffic flow. These enhancements will not only improve traffic flow but also optimize logistics within the surrounding local areas.
- 6.3. Appropriate warning signs and lighting would be provided on the approaches to and along the works areas in accordance with the standards and requirements as stipulated in the latest version of the "Code of Practice for the Lighting, Signing and Guarding of Road Works" and the "Transport Planning and Design Manual".



- 6.4. The design parameters for the design of single track access road refers to TPDM Volume 2, Chapter 3.11 Single Track Access Road and the details are summarized below:
  - As the roads serve as an Emergency Access for fire engines a minimum carriageway width of 3.5m should be provided.
  - At passing bays, lay-bys and elsewhere where a two lane section of road is required a nominal carriageway width of 6.0m should be provided
  - The main criterion for passing places is that they should be intervisible. Where forward visibility is unrestricted passing places should be provided at intervals of approximately 60m (measured from the end of one to the start of the next) consistent with adjacent topography and land tenure.
  - Each passing place should preferably be at least 12m long to accommodate two light vehicles, plus nominal tapers of 1:3
  - Where a road is initially two lane for a short section prior to becoming a single track road, traffic sign 604 (TC 304) "Single track road with passing places" should be erected.
  - The speed limit will normally be 50 km/h.
  - Passing bays should normally be signed by means of traffic sign 620 (TC 313).
- 6.5. Based on the design requirement, the proposed passing bay locations at concerned Road Section 1 of Deep Bay Road is shown in Figure 7 Passing Bays Plan in Appendix A.



# 7.0 SUMMARY AND CONCLUSION

- 7.1. This report has been undertaken for the "Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land" (The Site) located on Various Lots in D.D. 128, Pak Nai, Yuen Long, New Territories. The study evaluates the existing traffic conditions, forecasts future traffic demands, and assesses the traffic impact of the development over a 3-year operational period up to the year 2028.
- 7.2. Under the operation stage, the Site is accessible via Deep Bay Road, Kai Pak Ling Road, and a temporary road constructed by another CEDD contract, which connects to the atgrade road network of Kong Shum Western Highway. This is the proposed delivery route to the Site and mainly divided into three road section.
- 7.3. In order to appraise the existing traffic condition, manual traffic count surveys were conducted on 11 June 2025 (Wednesday) from 07:00 to 20:00. These observed traffic flow data were subsequently used for undertaking the assessment of the proposed TTA schemes in 2025.
- 7.4. Forecasts were prepared with reference to the 2019-Based Territorial Population and Employment Data Matrices (TPEDM) and the Annual Average Daily Traffic (AADT) data, resulting in the adoption of a conservative annual traffic growth rate of 1.69%.
- 7.5. Refer to the road link capacity assessments, all the concerned road links in the Study Area operate satisfactorily during the peak hours under the 2028 Reference Scenario (Without the Site) and Design Scenario (with the Site)
- 7.6. For junction capacity assessments, all the affected junction would be operating within capacity during peak hours for both the 2028 Reference Scenario (Without the Site) and Design Scenario (with the Site)
- 7.7. The proposed delivery route's Road Section 2 (Kai Pak Ling Road) and Road Section 3 (the temporary road constructed under a separate CEDD contract) are expected to meet standard road provisions for public use. Conversely, Road Section 1 (Deep Bay Road), connecting the Site to Kai Pak Ling Road, is identified as a substandard single-track access road, primarily due to the absence of intervisible passing bays.
- 7.8. Traffic assessment conducted during the operational phase of the Site indicates that the generated traffic will not significantly impact the roads along the delivery route. To further ensure smooth traffic flow, particularly on Road Section 1 of Deep Bay Road, the Site owner intends to implement mitigation measures. These measures include upgrading Road Section 1 of Deep Bay Road to a standard single-track access road with the provision of adequate, intervisible passing bays to facilitate two-way traffic flow. These enhancements will not only improve traffic flow but also optimize logistics within the surrounding local areas.



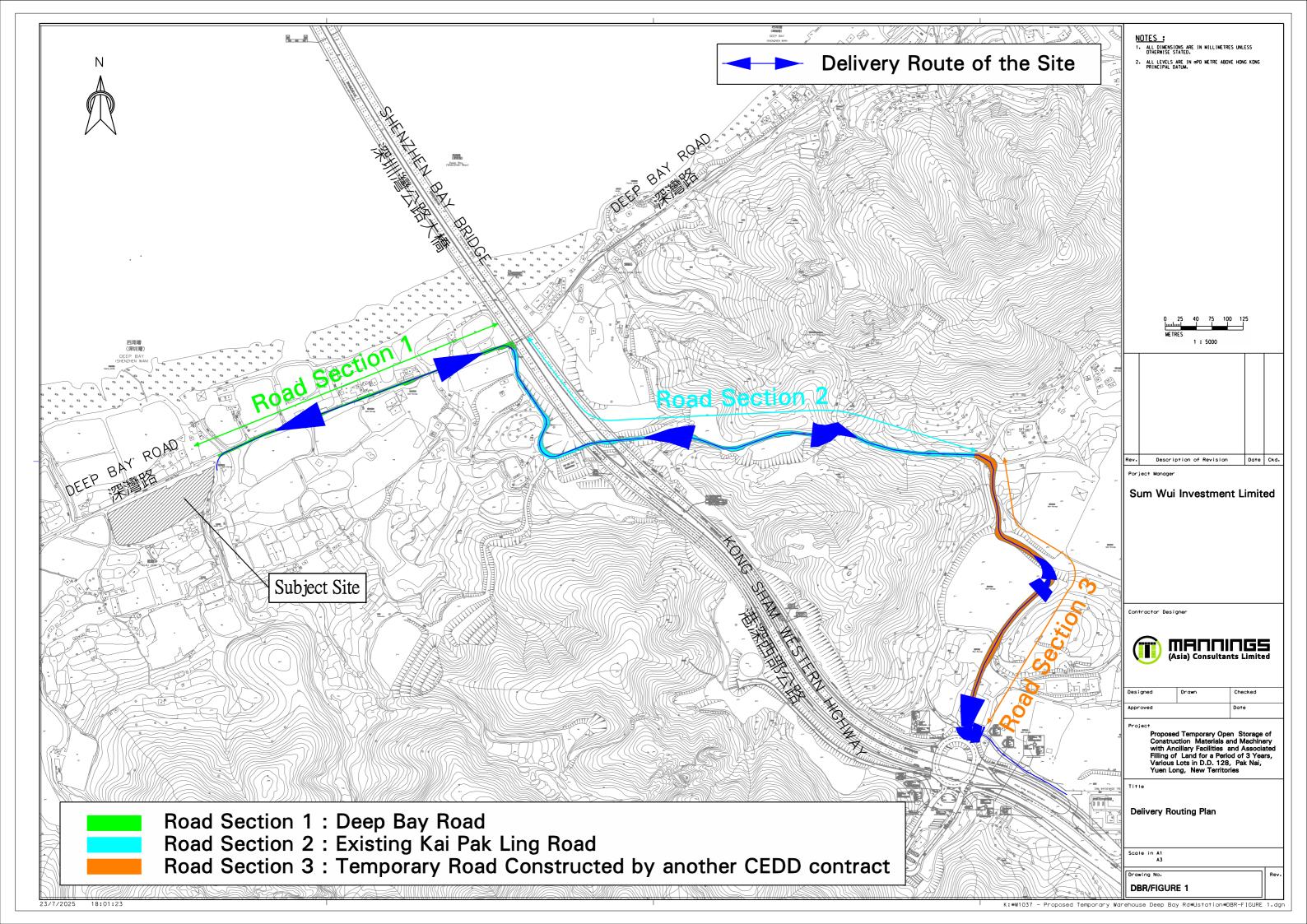
7.9. In conclusion, the projected traffic volume from the Site is anticipated to have a negligible impact on the adjacent road networks. Furthermore, the proposed mitigation measures include upgrading Road Section 1 of Deep Bay Road to a standard single-track access road, with the provision of adequate, intervisible passing bays to facilitate two-way traffic flow to ensure efficient two-way traffic flow, thereby benefiting the local community. Therefore, it is acceptable from traffic point of view.

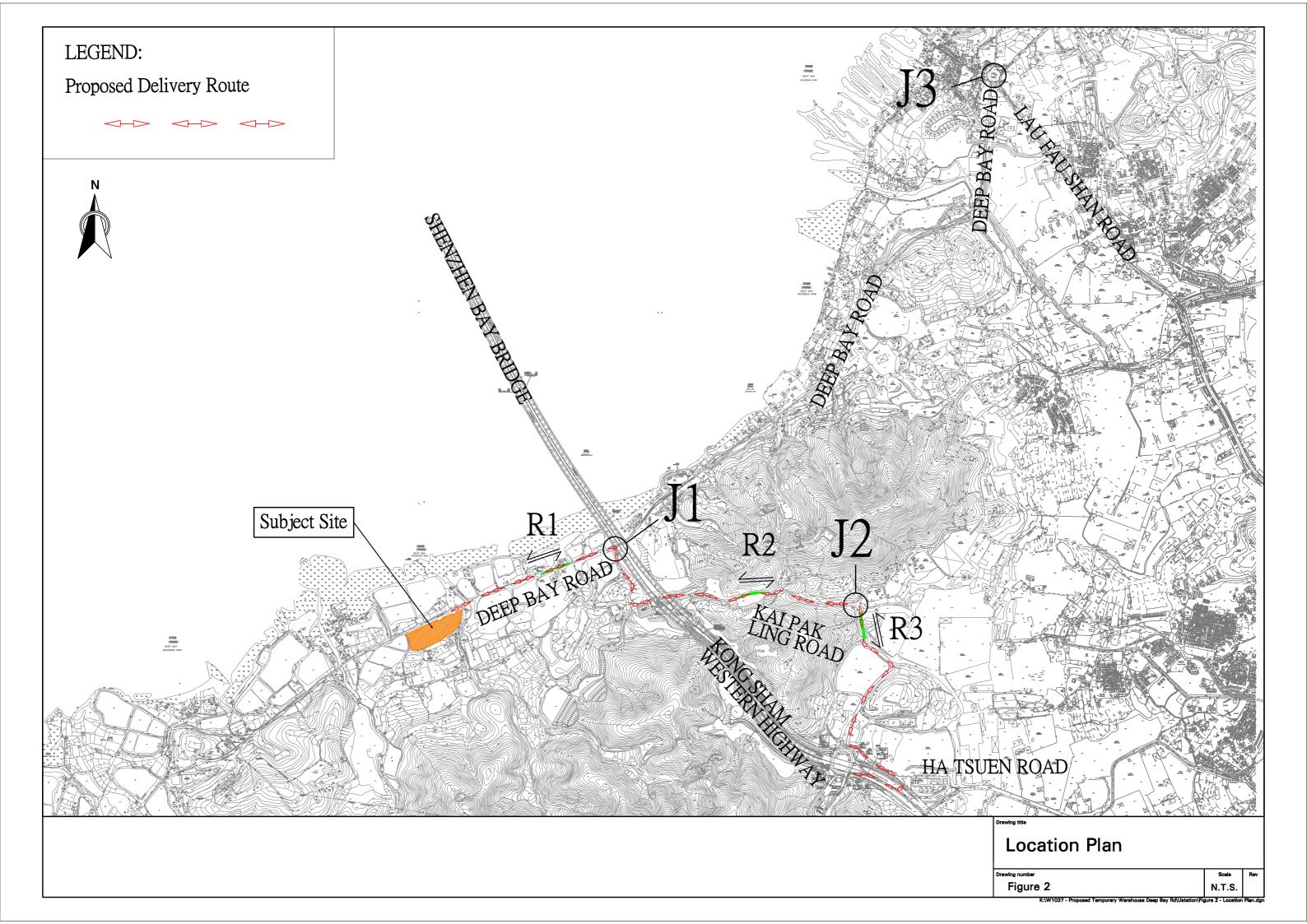


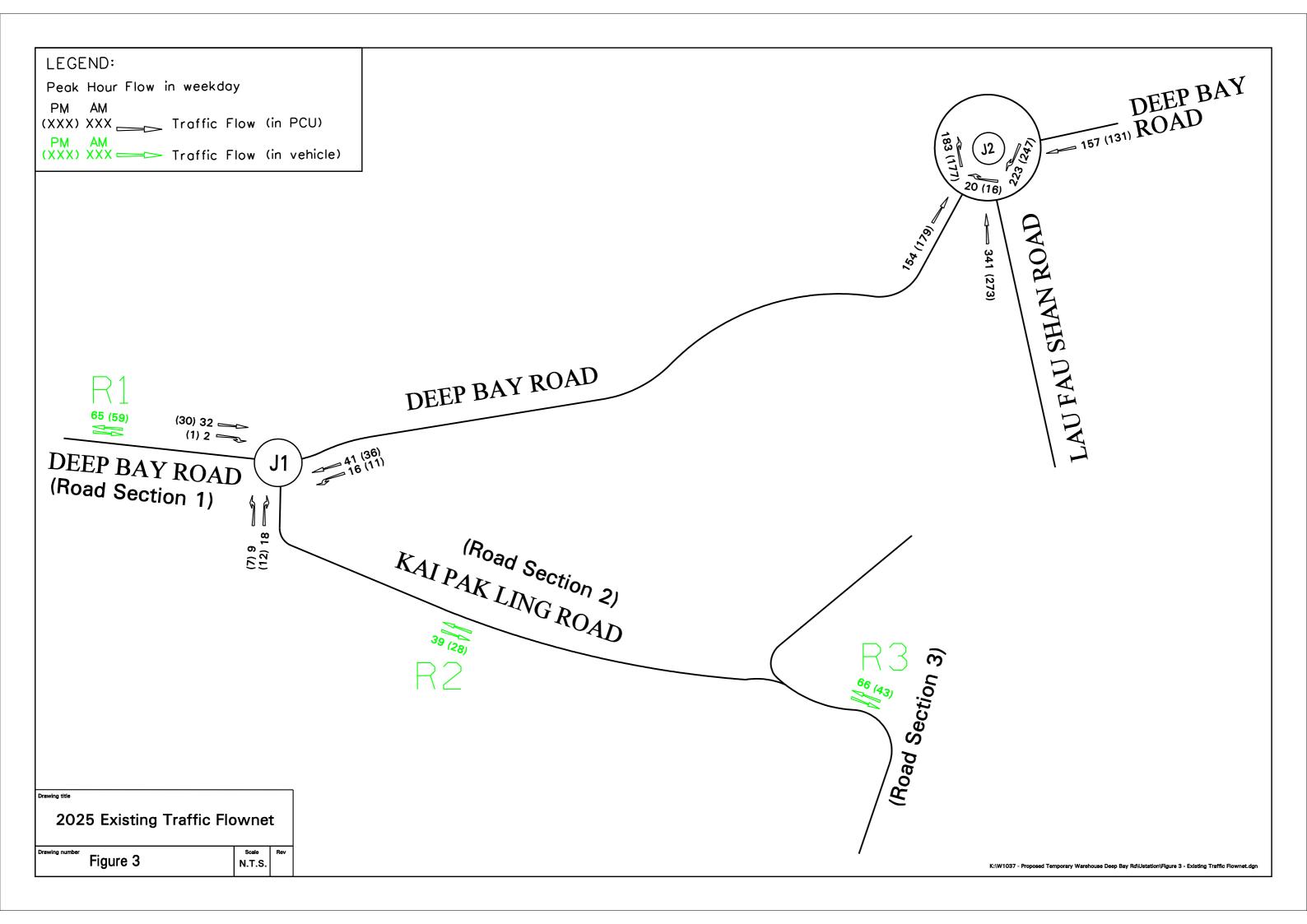
# **APPENDIX A**

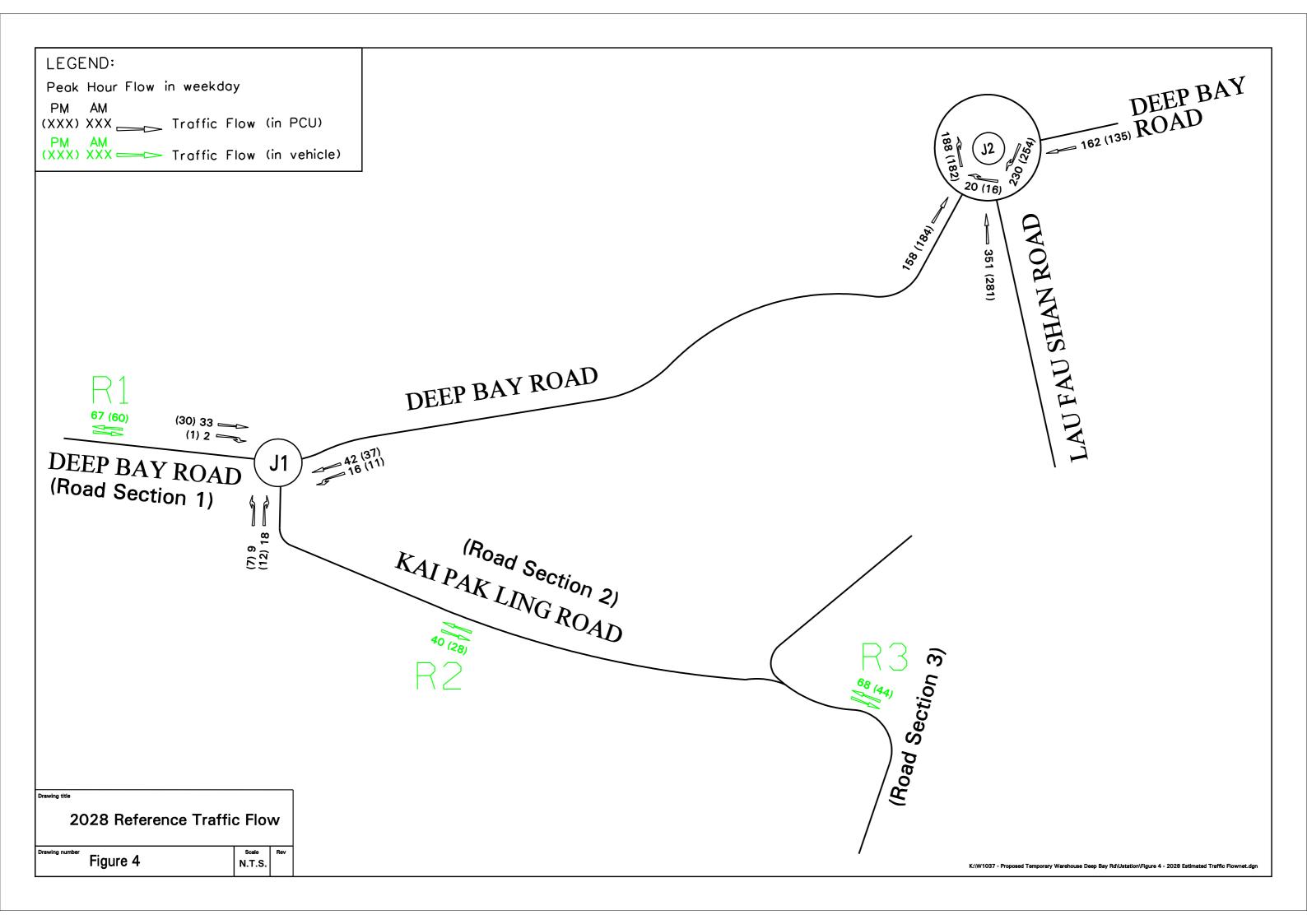
**Drawings** 

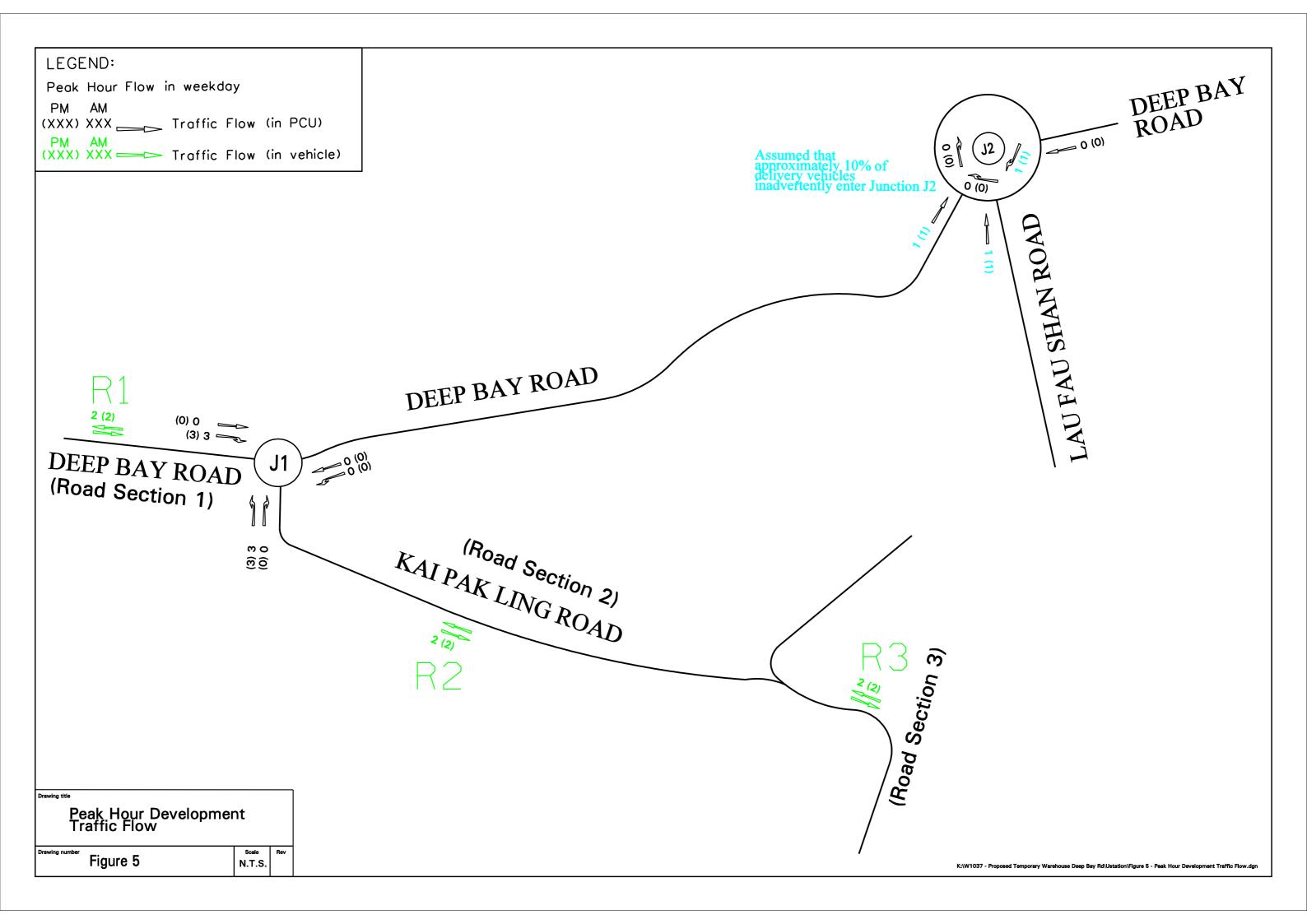
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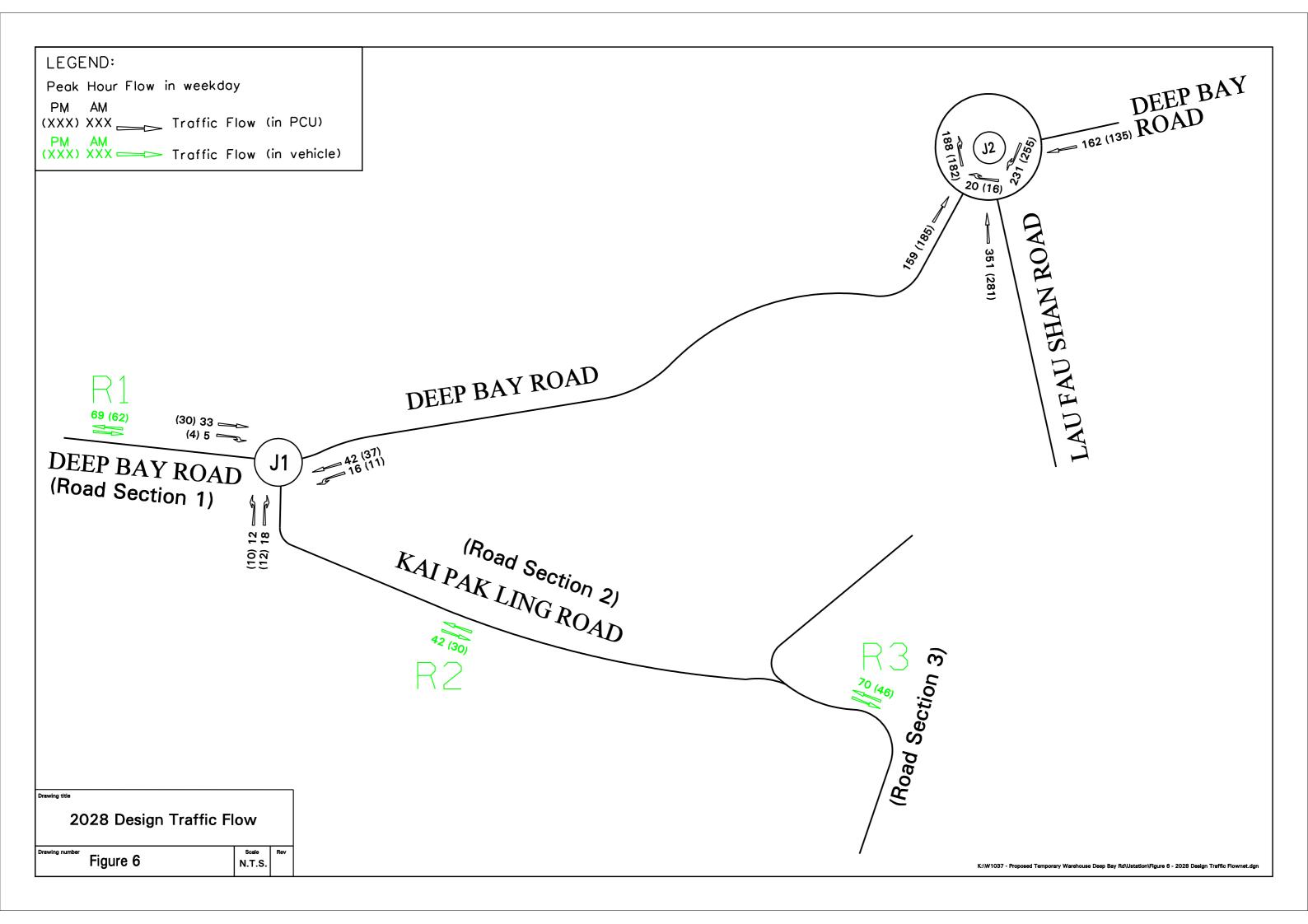


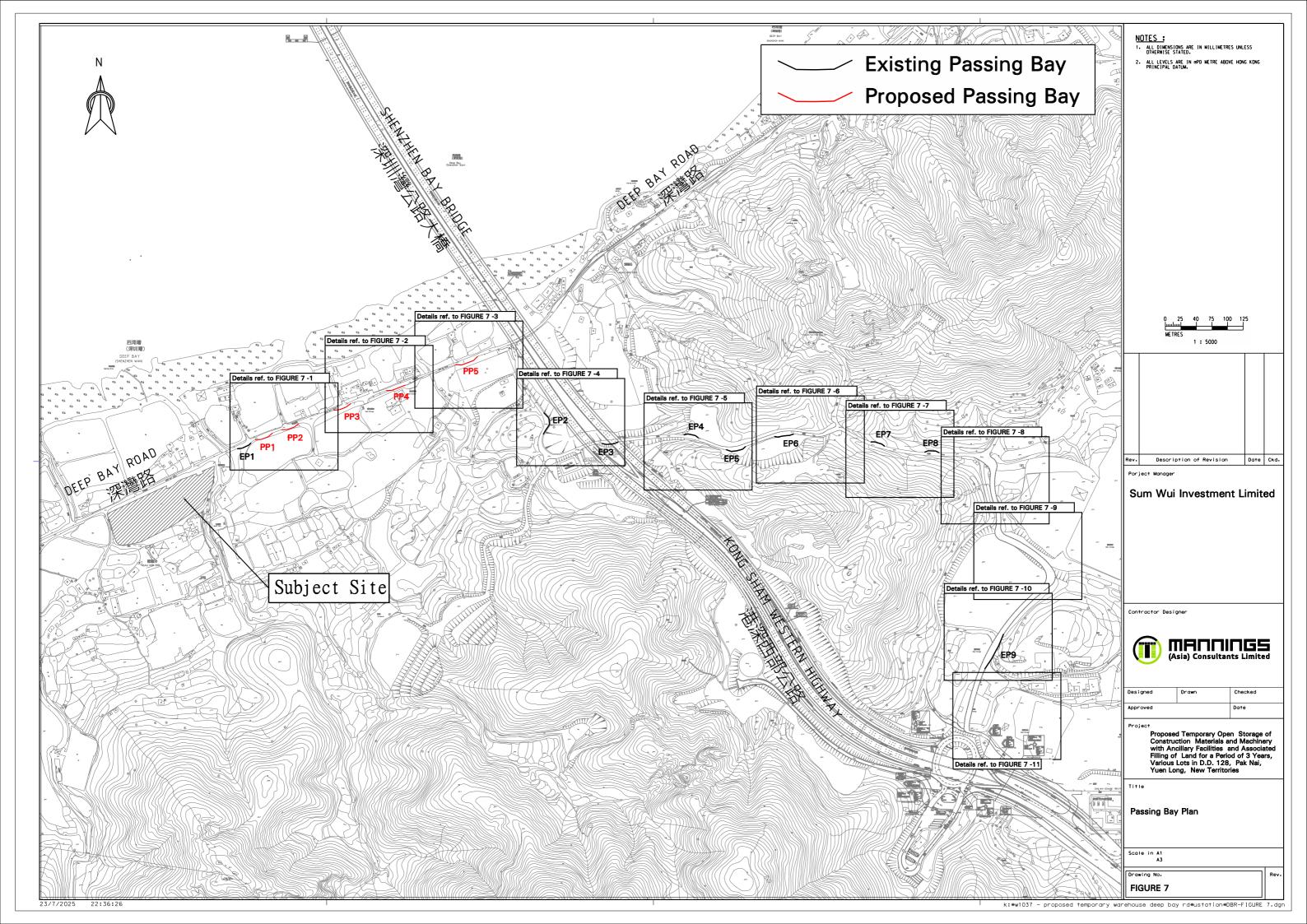


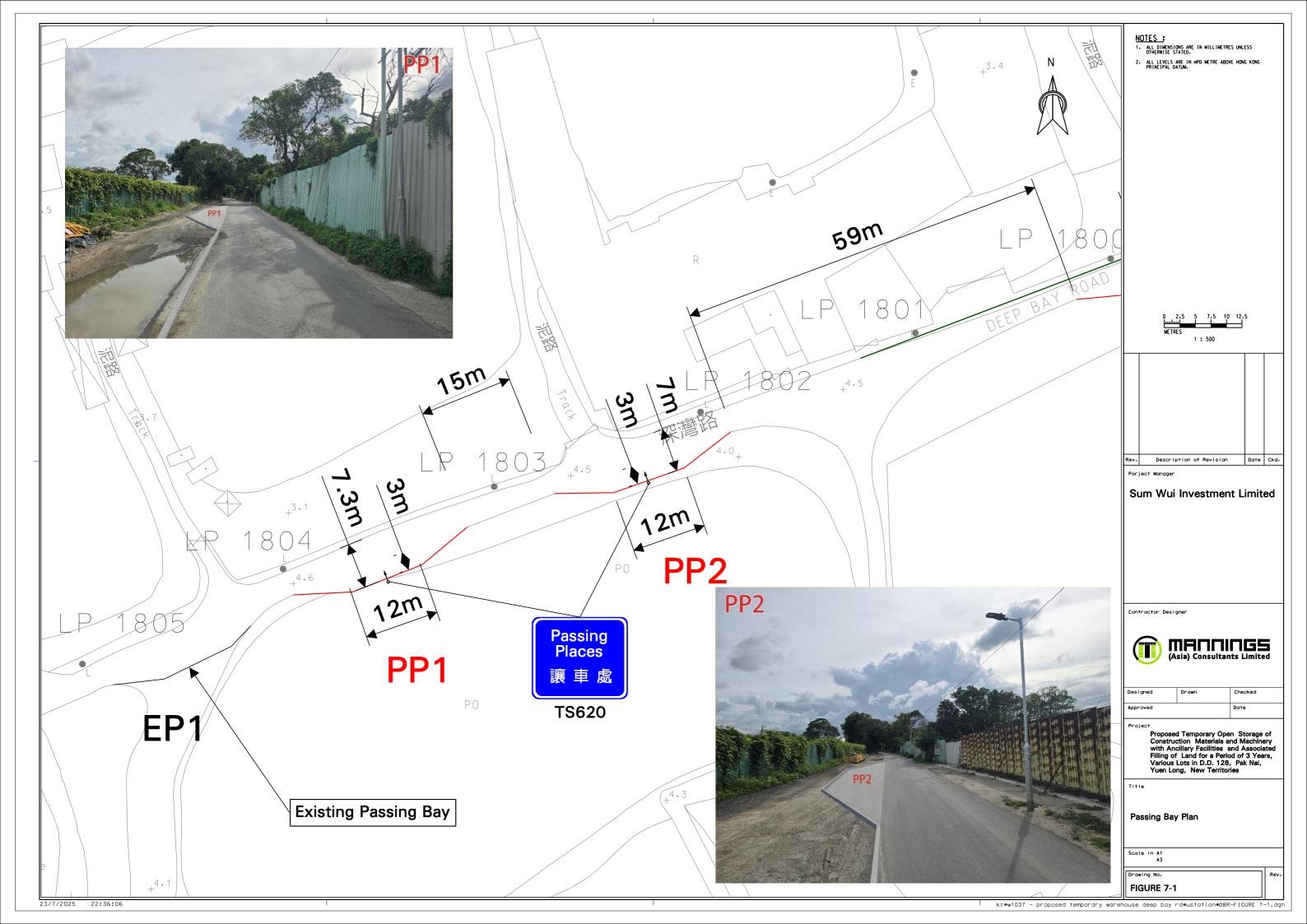




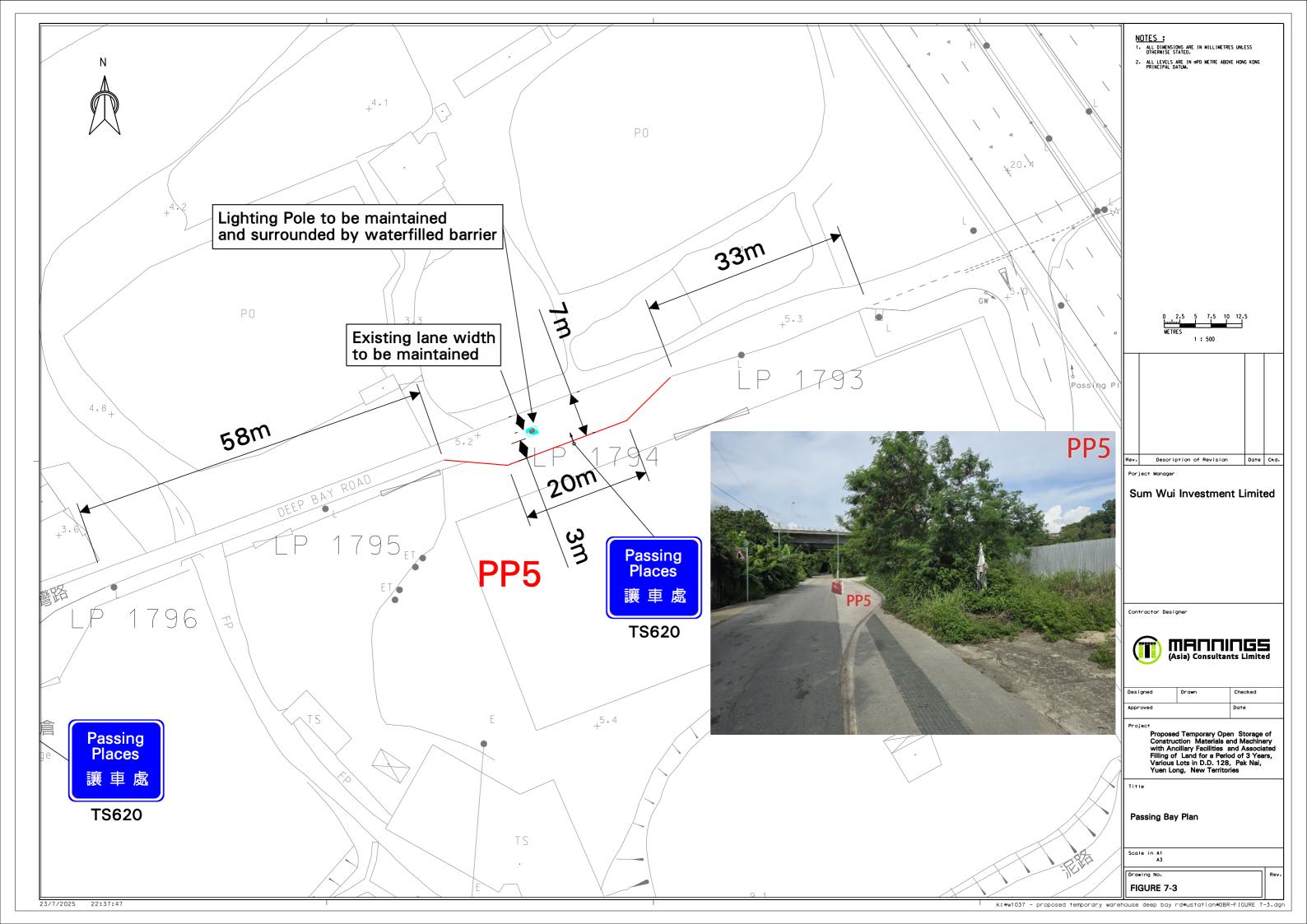


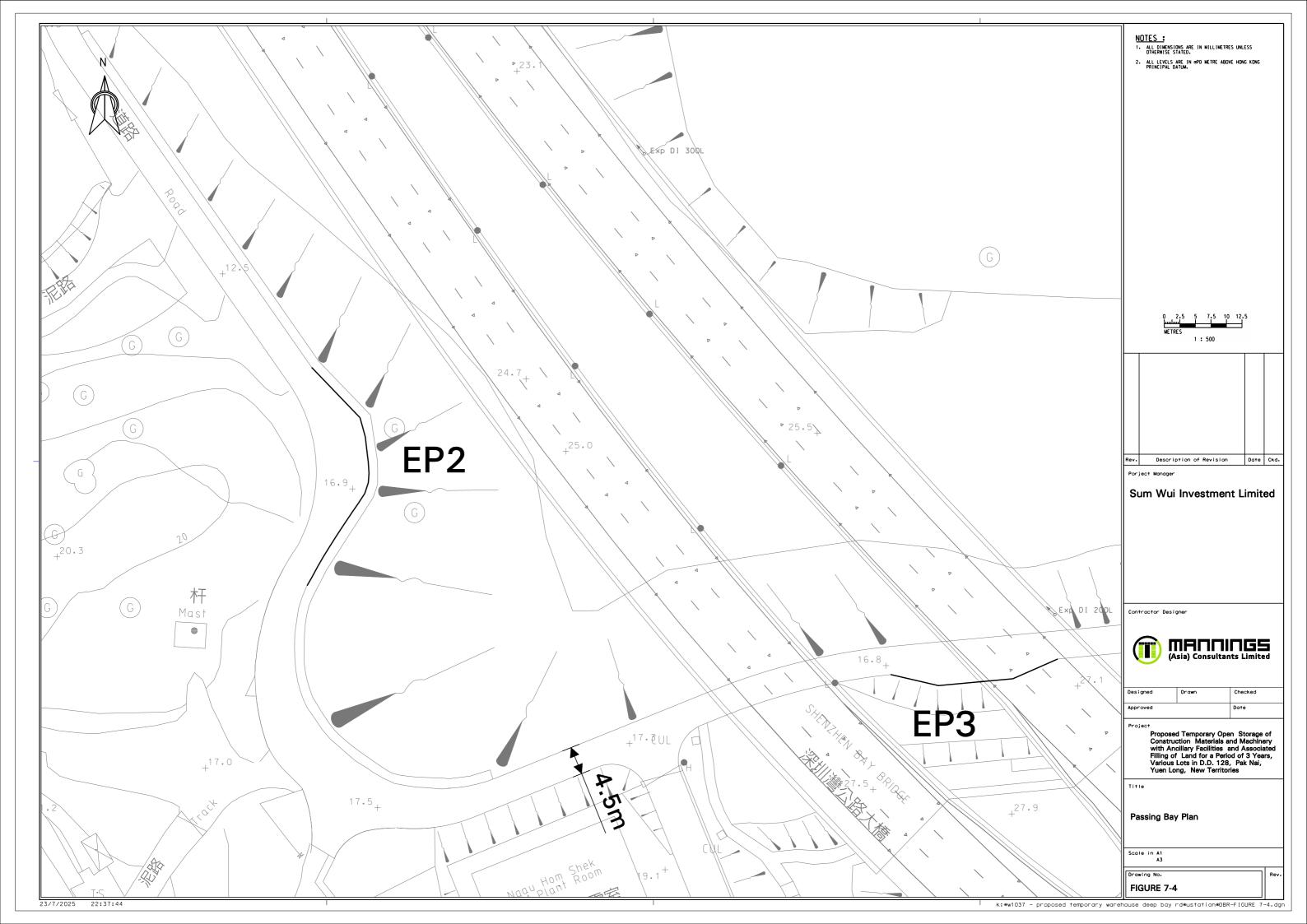


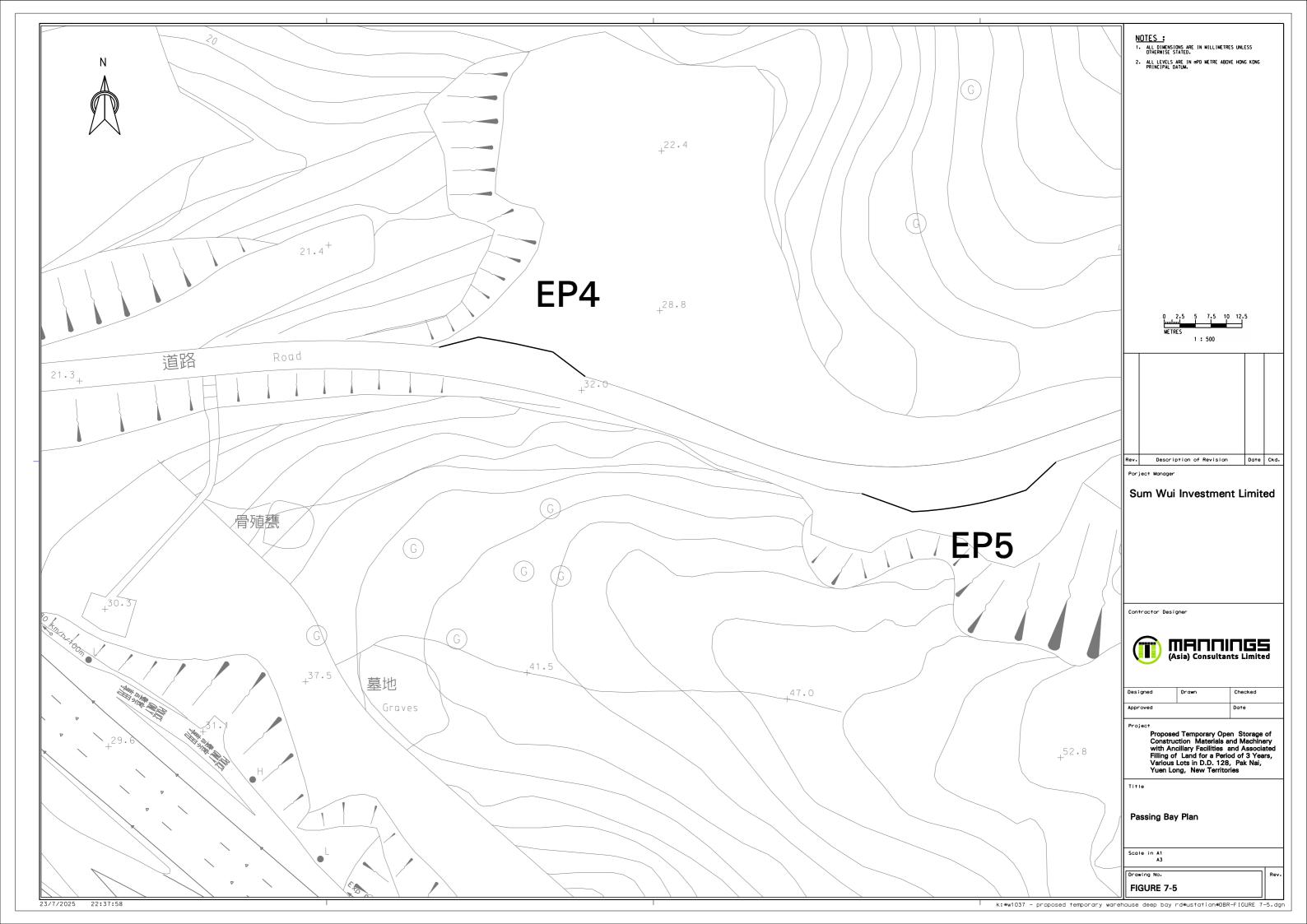


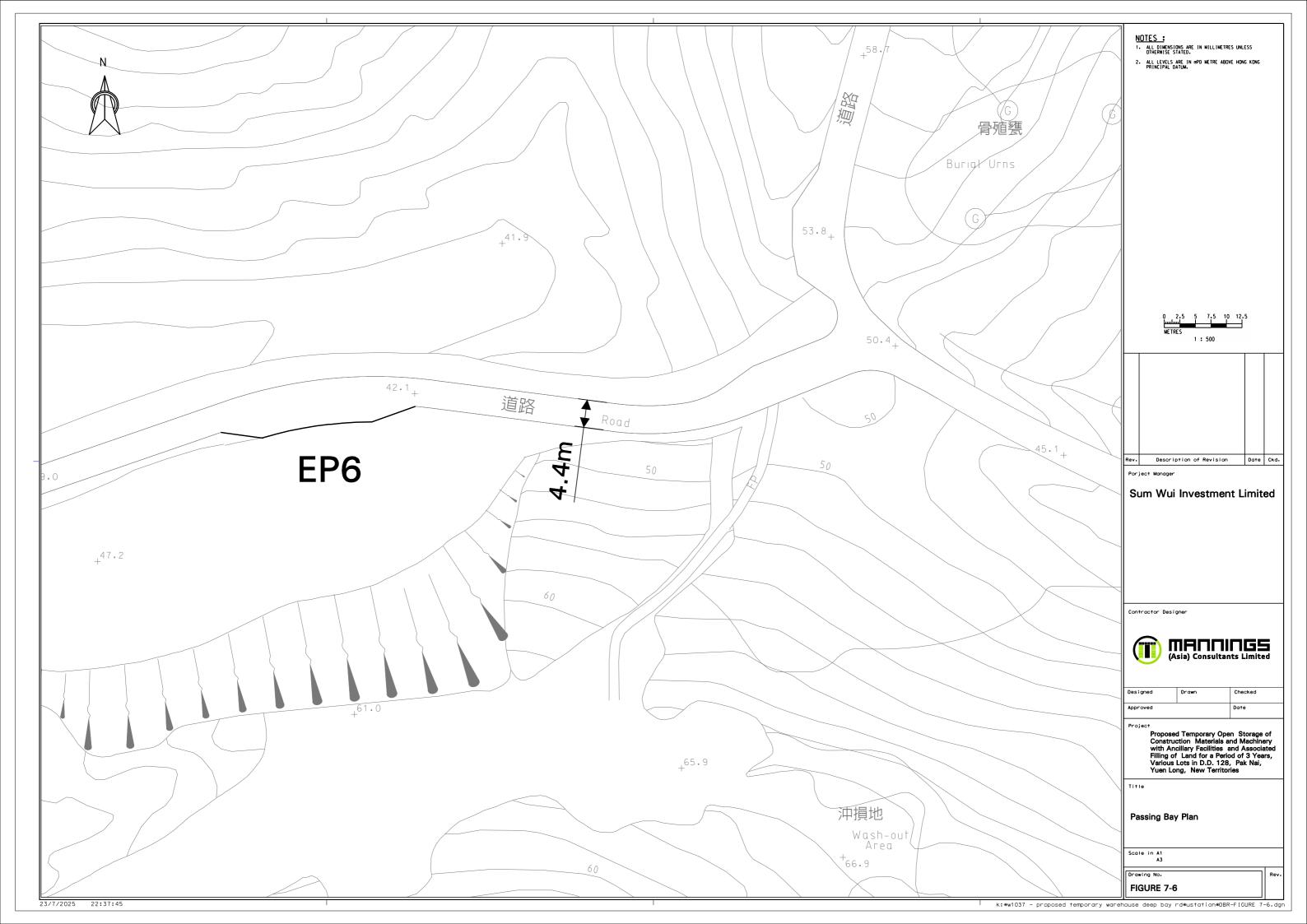


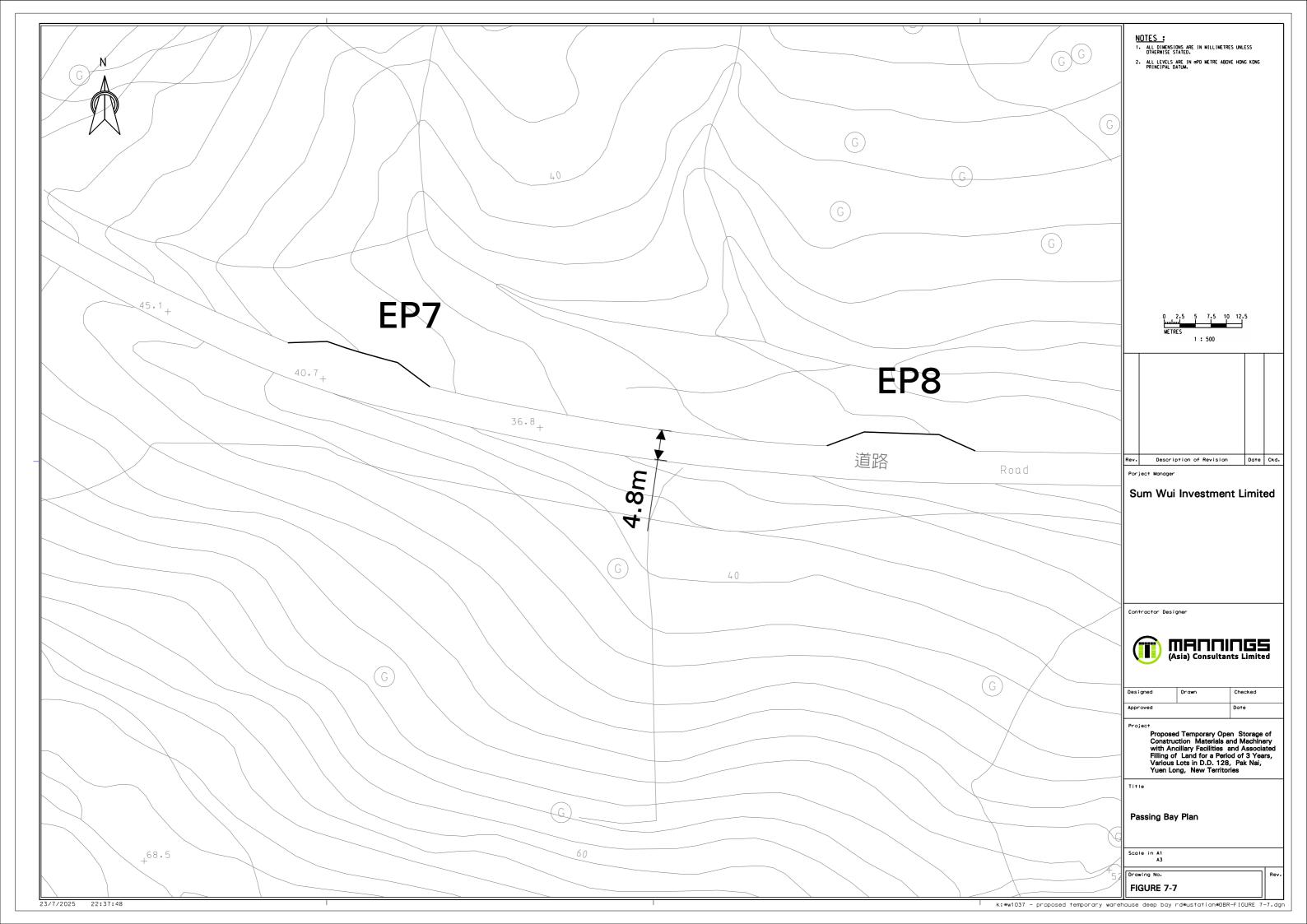


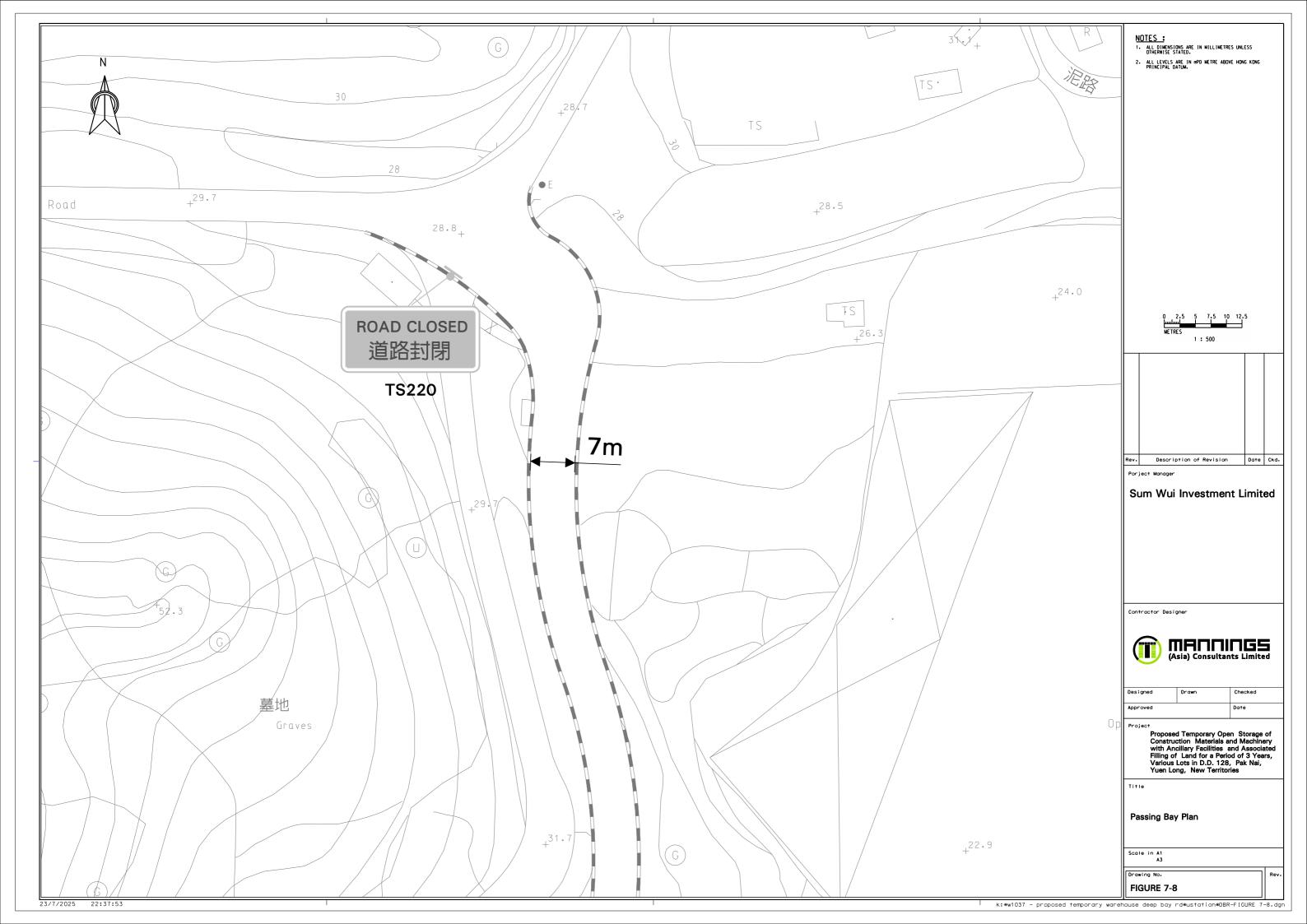


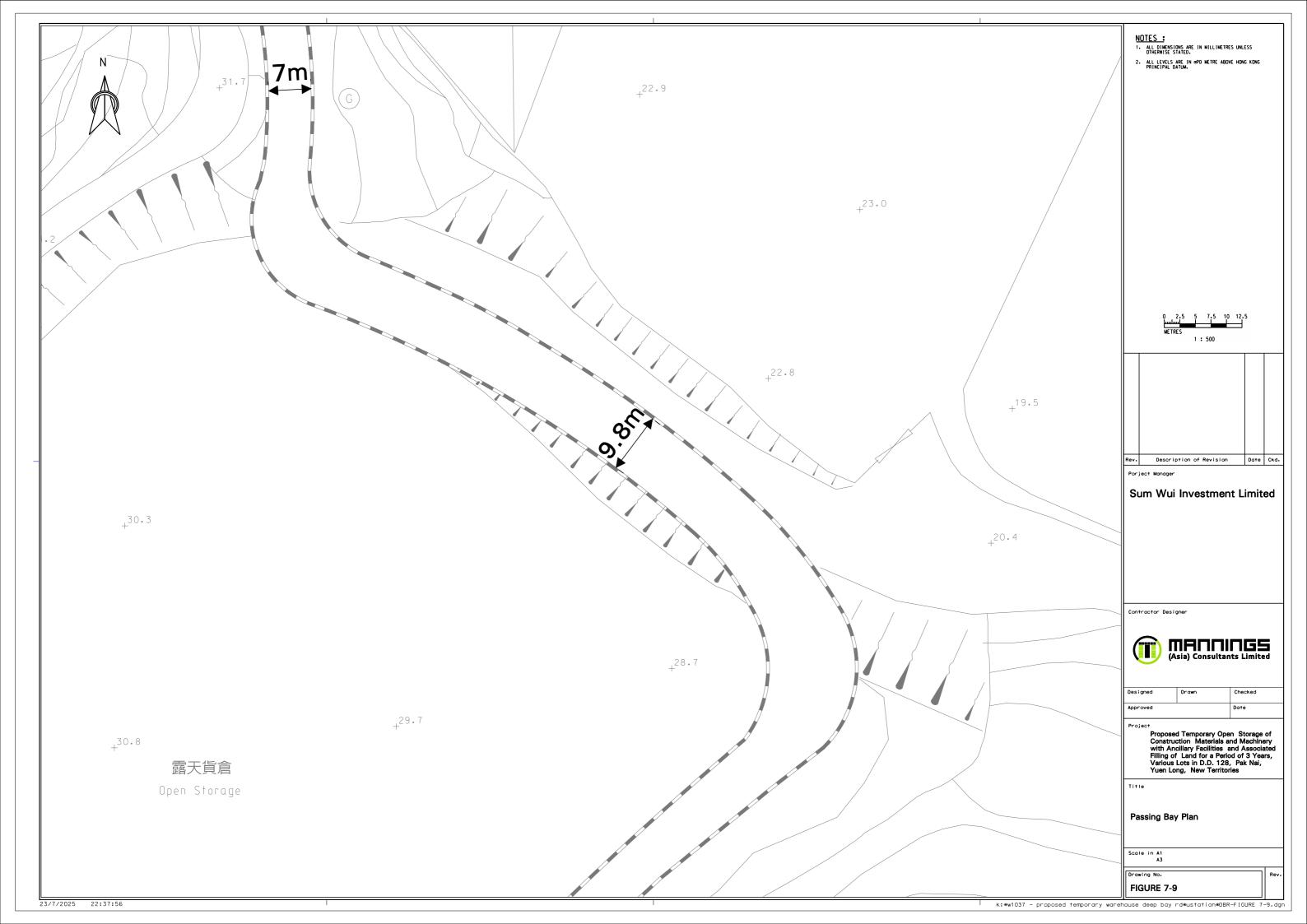


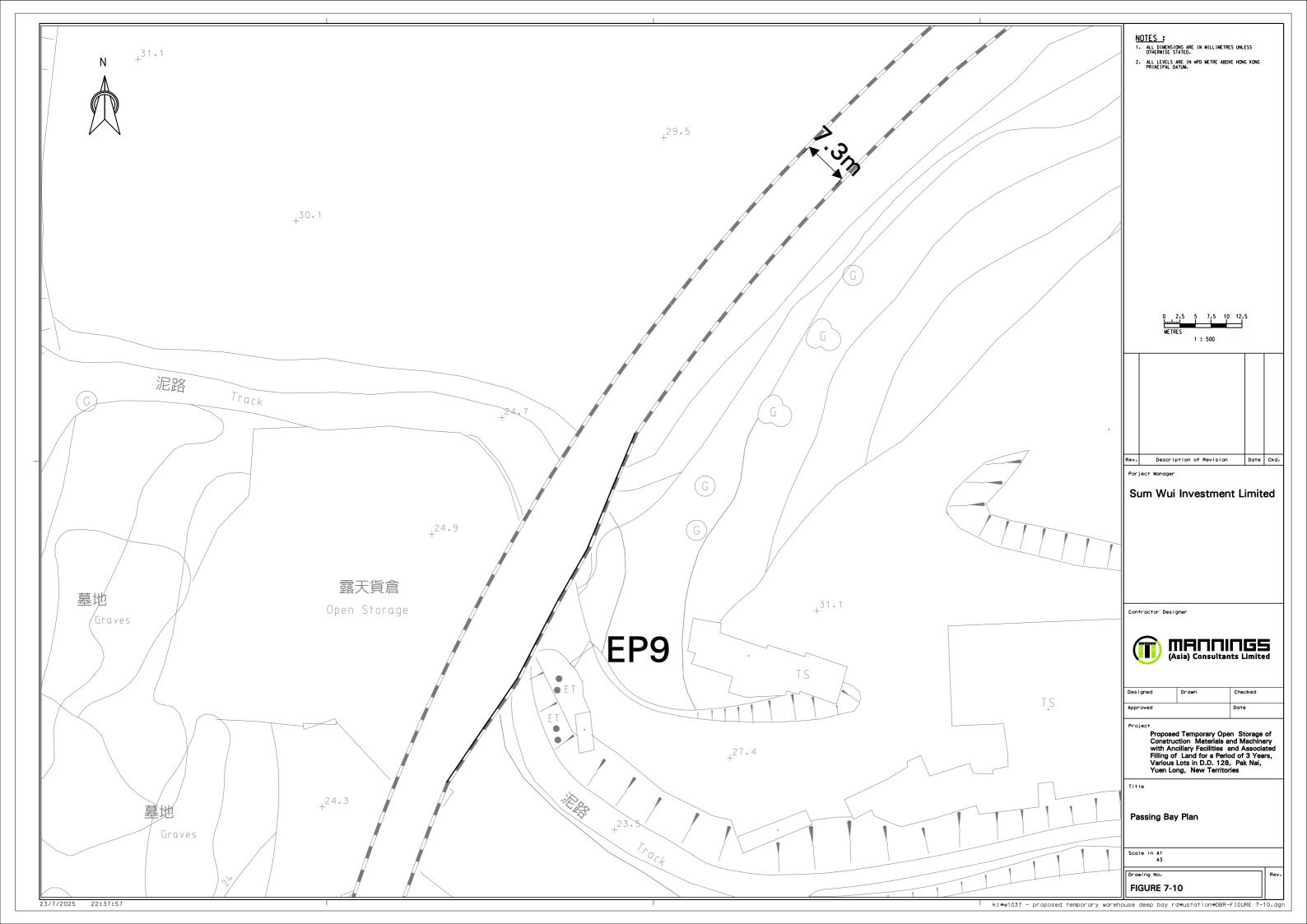


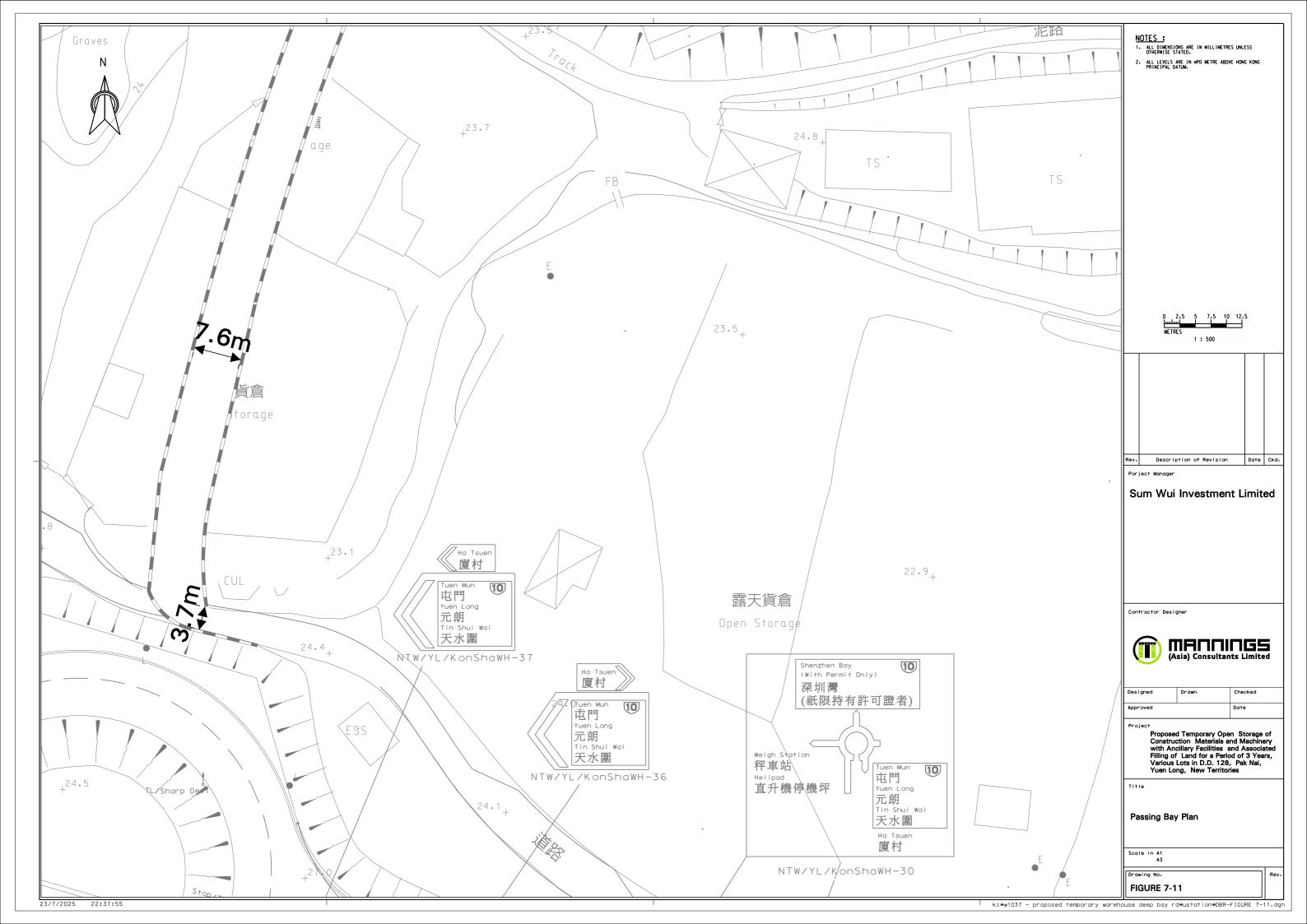














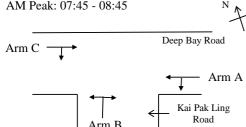
Proposed Temporary Open Storage of Construction Materials and Machinery with Ancillary Facilities and Associated Filling of Land for a Period of 3 Years, Various Lots in D.D. 128, Pak Nai, Yuen Long, New Territories

APPENDIX B

**Traffic Analysis** 

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Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	1 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	Existing Traffic Condition From 07:00-20:00 Weekday (AM Peak)	Drg. Ref.			
	05.45.00.45				



W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

VI b-a— Visibility to the left for veh. waiting in stream b-a

# **GEOMETRIC DETAILS:**

W	=	4 m						
Wcr	=	0 m						
q a-b	=	16 pcu/hr						
q a-c	=	41 pcu/hr						
q c-a	=	32 pcu/hr	Wc-a	=	4 m	Vr b-a	=	70 m
q c-b	=	2 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	18 pcu/hr	Wb-a	=	4 m	Vr c-b	=	70 m
q b-c	=	9 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

#### GEOMETRIC PARAMETERS:

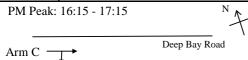
D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

#### **CAPACITY OF MOVEMENT:**

Q b-a = 568 Q b-c = 720Q c-b = 717

#### RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	2 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	Existing Traffic Condition From 07:00-20:00 Weekday (PM Peak)	Drg. Ref.			
	N				



Arm A

Kai Pak Ling

W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

VI b-a— Visibility to the left for veh. waiting in stream b-a

# GEOMETRIC DETAILS:

W	=	4 m						
Wcr	=	0 m						
q a-b	=	11 pcu/hr						
q a-c	=	36 pcu/hr						
q c-a	=	30 pcu/hr	Wc-ad	=	4 m	Vr b-a	=	70 m
q c-b	=	1 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	12 pcu/hr	Wb-ad	=	4 m	Vr c-b	=	70 m
q b-c	=	7 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

## **GEOMETRIC PARAMETERS:**

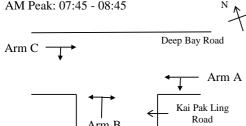
D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

## **CAPACITY OF MOVEMENT:**

Q b-ad = 571 Q b-c = 722 Q c-b = 720

## RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	1 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	2028 Reference Traffic Condition	Drg. Ref.			
	05 45 00 45				



W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

VI b-a— Visibility to the left for veh. waiting in stream b-a

# **GEOMETRIC DETAILS:**

W	=	4 m						
Wcr	=	0 m						
q a-b	=	16 pcu/hr						
q a-c	=	42 pcu/hr						
q c-a	=	33 pcu/hr	Wc-a	=	4 m	Vr b-a	=	70 m
q c-b	=	2 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	18 pcu/hr	Wb-a	=	4 m	Vr c-b	=	70 m
q b-c	=	9 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

#### GEOMETRIC PARAMETERS:

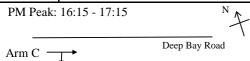
D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

#### **CAPACITY OF MOVEMENT:**

Q b-a = 568 Q b-c = 720Q c-b = 717

#### RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	2 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	2028 Reference Traffic Condition	Drg. Ref.			
DM D 1	16.15 17.15 N				



Arm A

Kai Pak Ling Road W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

Vl b-a— Visibility to the left for veh. waiting in stream b-a

# GEOMETRIC DETAILS:

W	=	4 m						
Wcr	=	0 m						
q a-b	=	11 pcu/hr						
q a-c	=	37 pcu/hr						
q c-a	=	30 pcu/hr	Wc-ad	=	4 m	Vr b-a	=	70 m
q c-b	=	1 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	12 pcu/hr	Wb-ad	=	4 m	Vr c-b	=	70 m
q b-c	=	7 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

## **GEOMETRIC PARAMETERS:**

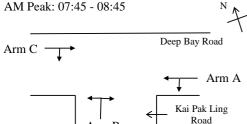
D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

## **CAPACITY OF MOVEMENT:**

Q b-ad = 571 Q b-c = 722 Q c-b = 720

# RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	1 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	2028 Design Traffic Condition	Drg. Ref.			
	05.45.00.45				



W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

VI b-a— Visibility to the left for veh. waiting in stream b-a

# **GEOMETRIC DETAILS:**

W	=	4 m						
Wcr	=	0 m						
q a-b	=	16 pcu/hr						
q a-c	=	42 pcu/hr						
q c-a	=	33 pcu/hr	Wc-a	=	4 m	Vr b-a	=	70 m
q c-b	=	5 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	18 pcu/hr	Wb-a	=	4 m	Vr c-b	=	70 m
q b-c	=	12 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

#### GEOMETRIC PARAMETERS:

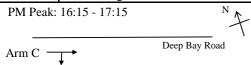
D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

#### **CAPACITY OF MOVEMENT:**

Q b-a = 566 Q b-c = 720 Q c-b = 717

#### RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

Job No.	W1037	File Name	W1037_DFC_DBR_KPLR	Page	2 of 2
Client	Sum Wui Investment Limited	Calculated	НС	Date	25/6/2025
Subject		Checked	KW		
	Junnciton Capacity Analysis of the junction of Deep Bay Road with Kai Pak Ling Road				
	2028 Design Traffic Condition	Drg. Ref.			
DM D 1	16.15 17.15 N				



Arm A

Kai Pak Ling Road W — Major road width

Wcr — Central reserve width

Wc-a — Lane width available to veh. waiting in stream c-a

Wc-b — Lane width available to veh. waiting in stream c-b

Vr c-a— Visibility to the right for veh. waiting in stream c-a

Vl b-a— Visibility to the left for veh. waiting in stream b-a

## **GEOMETRIC DETAILS:**

W	=	4 m						
Wcr	=	0 m						
q a-b	=	11 pcu/hr						
q a-c	=	37 pcu/hr						
q c-a	=	30 pcu/hr	Wc-ad	=	4 m	Vr b-a	=	70 m
q c-b	=	4 pcu/hr	Wc-b	=	4 m	Vr b-c	=	70 m
q b-a	=	12 pcu/hr	Wb-ad	=	4 m	Vr c-b	=	70 m
q b-c	=	10 pcu/hr	Wb-c	=	4 m	Vl b-a	=	70 m

## **GEOMETRIC PARAMETERS:**

D = 0.9391 pcu/hr E = 0.9864 pcu/hr F = 0.9864 pcu/hr Y = 0.8620 pcu/hr

## **CAPACITY OF MOVEMENT:**

Q b-ad = 569 Q b-c = 722 Q c-b = 720

## RATIO OF DESIGN FLOW TO CAPACITY FOR EACH APPROACH:

. W1037								File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
Sum Wui	Investme	ent Limited						Calculated	HC	Date	25/6/2025
		for the junction of Deep Bay Road with Lau Fau Shan	Road / Shan Tung Street - J2					Checked	KW	Date	20.0.2020
		ondition From 07:00-20:00 Weekday (AM Peak)					•				
AM Peak:											
	Deep	Bay Road Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road	223	157 20 Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road	ı	
Design Pa	arameters:	Proposed Roundabout Layor:	ut				Traffic Fl	ow Within the Roundabou	Ŀ		
				Arm A	Arm B	Arm C					
e	=	entry width (m)	=	4.1	4.2	3.9					
v	=	approach half width (m)	=	2.5	2.6	2.5					
L	=	effective length of flare (m)	=	12.8	4.8	6.9					
S	=	sharpness of flare	=	0.20	0.53	0.32					
ф	=	entry angle (°)	=	51	53	41					
D	=	inscribed circle diameter (m)	=	20	20	20					
r	=	entry radius (m)	=	73	5.5	7.9					
Calculatio	on:										
				Arm A	Arm B	Arm C					
$q_c$	=		=	223	20	183					
K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
$\mathbf{x}_2$	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
F	=	303x <sub>2</sub>	=	1103.79	1022.38	1014.70					
$t_D$	=	- :	=	1.49	1.49	1.49					
$f_c$	=	- D( - 2)	=	0.54	0.52	0.52					
$Q_{E}$	=	(	=	946	801	815					
DFC	=	traffic flow into the roundabout/Q <sub>E</sub>	=	0.17	0.43	0.19					

No.	W1037		_		•				File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
ent	Sum Wui Inv	estment	t Limited						Calculated	HC	Date	25/6/2025
bject	Signal calcul	ation for	r the junction of Deep Bay Road with Lau Fau Shan Road	/ Shan Tung Street - J2					Checked	KW	Date	
	Existing Trai	fic Con	dition From 07:00-20:00 Weekday (PM Peak)									
	PM Peak: 17:	15 - 18:1:	5									
		Deep B.	Say Road Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road —	247	16	Arm A Deep Bay Road  Arm B Lau Fau Shan Road		
	Design Parar	notors:	Proposed Roundabout Layout					,	low Within the Roundabout			
	Design Parai	neters.			Arm A	Arm B	Arm C					
	e	=	entry width (m)	=	4.1	4.2	3.9					
	v	=	approach half width (m)	=	2.5	2.6	2.5					
	L	=	effective length of flare (m)	=	12.8	4.8	6.9					
	s	=	sharpness of flare	=	0.20	0.53	0.32					
	ф	=	entry angle (°)	=	51	53	41					
	Ď	=	inscribed circle diameter (m)	=	20	20	20					
	r	=	entry radius (m)	=	73	5.5	7.9					
	Calculation:				Arm A	Arm B	Arm C					
	$q_c$	=	circulating flow across entry	=	247	16	177					
	K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
	$\mathbf{x}_2$	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
	M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
	F	=	$303x_2$	=	1103.79	1022.38	1014.70					
	$t_{\rm D}$	=	1+0.5/(1+M)	=	1.49	1.49	1.49					
	$f_c$	=	$0.21t_D(1+0.2x_2)$	=	0.54	0.52	0.52					
	$Q_{E}$	=	$K(F-f_cq_c)$	=	934	802	818					
	DFC	=	traffic flow into the roundabout/Q <sub>E</sub>	=	0.14	0.34	0.22					

No.	W1037								File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
ient	Sum Wui Inv								Calculated	HC	Date	25/6/2025
bject			r the junction of Deep Bay Road with Lau Fau Shan Road	I / Shan Tung Street - J2					Checked	KW	Date	
	2028 Referen	nce Traf	fic Condition									
	AM Peak: 07:	30 - 08:3	30									
		Deep В	Say Road Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road	230	162 20 188 Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road		
	Design Parar	matauri	Proposed Roundabout Layout					•	ow Within the Roundabout	t.		
	Design Parai	neters:			Arm A	Arm B	Arm C					
	e	=	entry width (m)	=	4.1	4.2	3.9					
	v	_	approach half width (m)	=	2.5	2.6	2.5					
	L	_	effective length of flare (m)	=	12.8	4.8	6.9					
	s	=	sharpness of flare	=	0.20	0.53	0.32					
	ф	=	entry angle (°)	=	51	53	41					
	Ď	=	inscribed circle diameter (m)	=	20	20	20					
	r	=	entry radius (m)	=	73	5.5	7.9					
			, , ,									
	Calculation:											
					Arm A	Arm B	Arm C					
	$q_c$	=	circulating flow across entry	=	230	20	188					
	K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
	$\mathbf{x}_2$	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
	M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
	F	=	303x <sub>2</sub>	=	1103.79	1022.38	1014.70					
	$t_D$	=	1+0.5/(1+M)	=	1.49	1.49	1.49					
	$f_c$	=	$0.21t_D(1+0.2x_2)$	=	0.54	0.52	0.52					
	$Q_E$	=	$K(F-f_cq_c)$	=	943	801	813					
	DFC	=	traffic flow into the roundabout/QE	=	0.17	0.44	0.19					

. W1037								File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
Sum Wui	Investme	nt Limited						Calculated	HC	Date	25/6/2025
		or the junction of Deep Bay Road with Lau Fau Shan	Road / Shan Tung Street - J2					Checked	KW	Date	
		affic Condition									
PM Peak: 1							Į.		1		
	Deep :	Bay Road Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road	N 254	135 182 Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road	ı	
Design Pa	rameters:	Proposed Roundabout Layou					•	ow Within the Roundabou	į.		
				Arm A	Arm B	Arm C					
e	=	entry width (m)	=	4.1	4.2	3.9					
v	=	approach half width (m)	=	2.5	2.6	2.5					
L	=	effective length of flare (m)	=	12.8	4.8	6.9					
s	=	sharpness of flare	=	0.20	0.53	0.32					
ф	=	entry angle (°)	=	51	53	41					
D	=	inscribed circle diameter (m)	=	20	20	20					
r	=	entry radius (m)	=	73	5.5	7.9					
Calculatio	n:										
		in latin day and a		Arm A	Arm B	Arm C					
q <sub>c</sub>	=	circulating flow across entry	=	254	16	182					
K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
x <sub>2</sub>	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
F	=	303x <sub>2</sub>	=	1103.79	1022.38	1014.70					
t <sub>D</sub>	=	1+0.5/(1+M)	=	1.49	1.49	1.49					
f <sub>c</sub>	=	$0.21t_D(1+0.2x_2)$	=	0.54	0.52	0.52					
$Q_E$	=	$K(F-f_cq_c)$	=	930	802	816					
DFC	=	traffic flow into the roundabout/QE	=	0.15	0.35	0.23					

No. W1037								File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
	ui Investmen	nt Limited						Calculated	HC	Date	25/6/2025
		or the junction of Deep Bay Road with Lau Fau Shan Road	Shan Tung Street - 12					Checked	KW	Date	23, 6, 202.
	esign Traffic		Shan Tang Succe-32					Circkeu	17.11	Date	
	c: 07:30 - 08:3								<u> </u>		
	Deep E	Bay Road  Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road -	231	188	Arm A Deep Bay Road  Arm B Lau Fau Shan Road	ı	
Design I	Parameters:	Proposed Roundabout Layout		Arm A	Arm B	Arm C	,	Flow Within the Roundabout			
e	=	entry width (m)	=	4.1	4.2	3.9					
v	=	approach half width (m)	=	2.5	2.6	2.5					
L	=	effective length of flare (m)	=	12.8	4.8	6.9					
s	=	sharpness of flare	=	0.20	0.53	0.32					
ф	=	entry angle (°)	=	51	53	41					
D	=	inscribed circle diameter (m)	=	20	20	20					
r	=	entry radius (m)	=	73	5.5	7.9					
Calculat	tion:			Arm A	Arm B	Arm C					
$q_c$	=	circulating flow across entry	=	231	20	188					
K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
$\mathbf{x}_2$	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
F	=	$303x_2$	=	1103.79	1022.38	1014.70					
$t_{\rm D}$	=	1+0.5/(1+M)	=	1.49	1.49	1.49					
$f_c$	=	$0.21t_D(1+0.2x_2)$	=	0.54	0.52	0.52					
$Q_E$	=	$K(F-f_cq_c)$	=	942	801	813					
- CE		·									

0.17 0.44 0.20

DFC = traffic flow into the roundabout/ $Q_E$ 

o. W1037								File Name	W1037_DFC_DBR_LFSR_SHTS	Page	1 of 1
	Investme	ent Limited						Calculated	HC	Date	25/6/2025
		for the junction of Deep Bay Road with Lau Fau Shar	Road / Shan Tung Street - J2					Checked	KW	Date	25, 5, 2025
		ic Condition	Troud / Shair Tang Sacce 12				•	Спесией	12.11	Dute	
PM Peak:							ı				
	Deep	o Bay Road  Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road			Deep Bay Road	255 185	135 1182 Arm C	Arm A Deep Bay Road  Arm B Lau Fau Shan Road	ı	
Design Pa	arameters	Proposed Roundabout Layo	<u>ut</u>				•	ow Within the Roundabou	į.		
				Arm A	Arm B	Arm C					
e	=	entry width (m)	=	4.1	4.2	3.9					
v	=	approach half width (m)	=	2.5	2.6	2.5					
L	=	effective length of flare (m)	=	12.8	4.8	6.9					
S	=	sharpness of flare	=	0.20	0.53	0.32					
ф	=	entry angle (°)	=	51	53	41					
D	=	inscribed circle diameter (m)	=	20	20	20					
r	=	entry radius (m)	=	73	5.5	7.9					
Calculation	on:										
				Arm A	Arm B	Arm C					
$q_c$	=		=	255	16	182					
K	=	1-0.00347(f-30)-0.978(1/r-0.05)	=	0.96	0.79	0.89					
x <sub>2</sub>	=	v+((e-v)/(1+2s))	=	3.64	3.37	3.35					
M	=	exp((D-60)/10)	=	0.02	0.02	0.02					
F	=	303x <sub>2</sub>	=	1103.79	1022.38	1014.70					
$t_D$	=		=	1.49	1.49	1.49					
$f_c$	=	20	=	0.54	0.52	0.52					
$Q_E$	=	(	=	930	802	816					
DFC	=	traffic flow into the roundabout/Q <sub>E</sub>	=.	0.15	0.35	0.23					