

Your Ref.: A/YL-KTN/1177

Our Ref.: P23044/TL25424

8 December 2025

The Secretary
Town Planning Board
15/F., North Point Government Offices
333 Java Road, North Point, Hong Kong

By Post and E-mail
tpbpd@pland.gov.hk


Dear Sir,

Submission of Further Information (FI)

Temporary Animal Boarding Establishment with Ancillary Facilities and associated Filling of Land for a period of 5 years in "Agriculture" Zone, Lot Nos. 1493 (Part) and 1500 (Part) in D. D. 107 and Adjoining Government Land, Yuen Long, New Territories
(Application No. A/YL-KTN/1177)

We write to submit FI in response to comments from Drainage Services Department and an updated drainage record for the captioned application, which serves to supersede our previous FI submission under our reference P23044/TL25415 dated 4.12.2025.

Yours faithfully,
For and on behalf of
Goldrich Planners & Surveyors Ltd.



Francis LAU

Encl.

c.c.

DPO/FS&YLE, PlanD

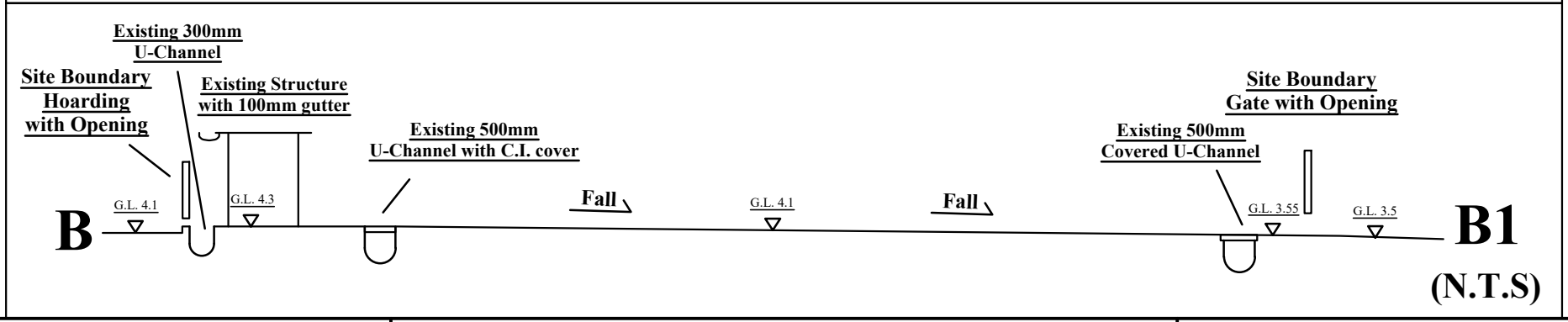
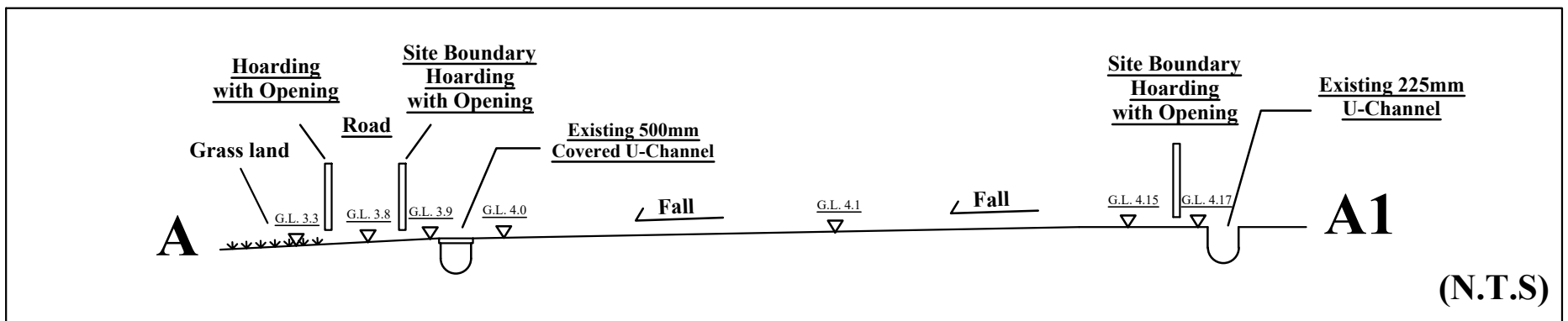
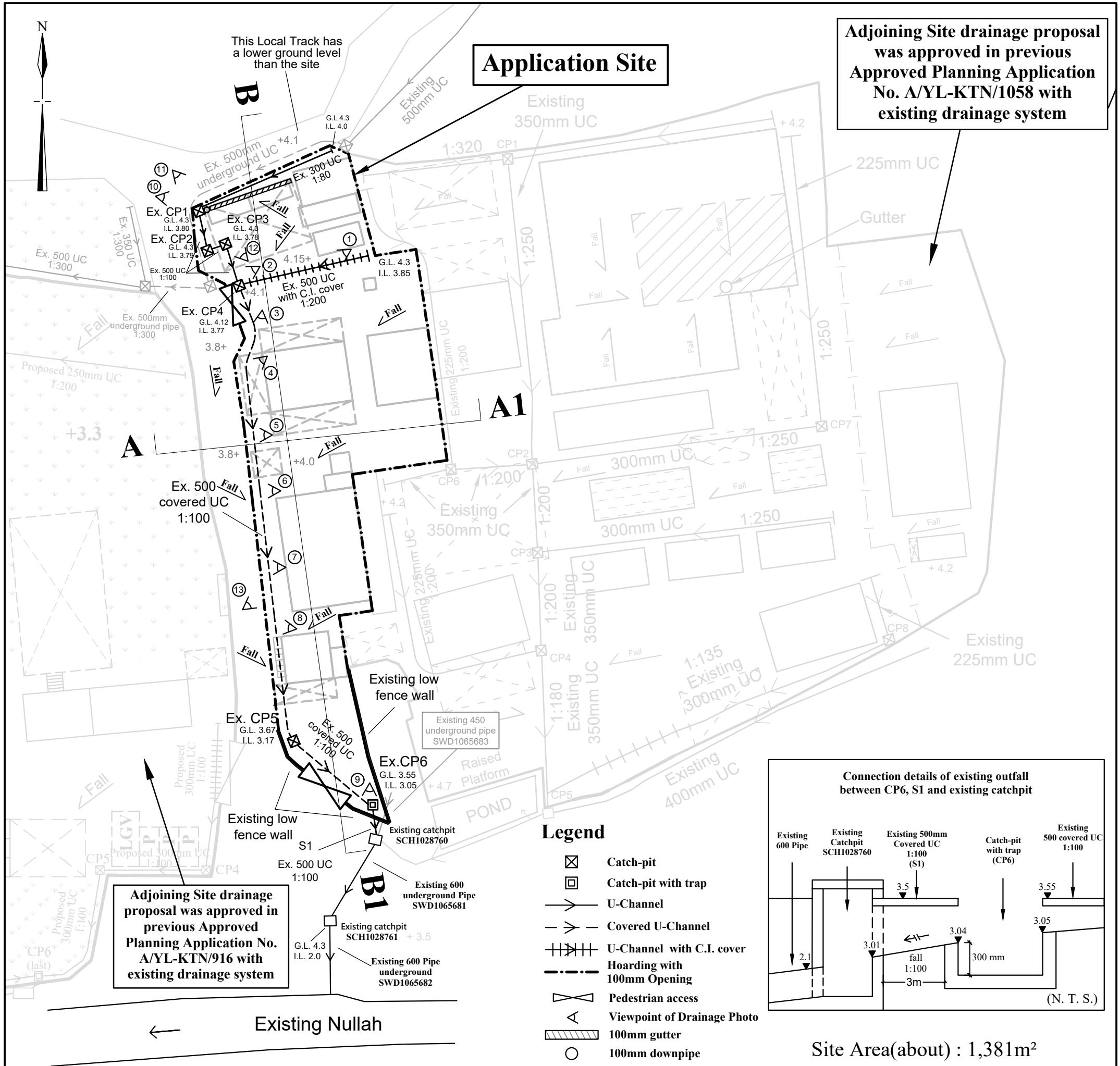
By E-mail only

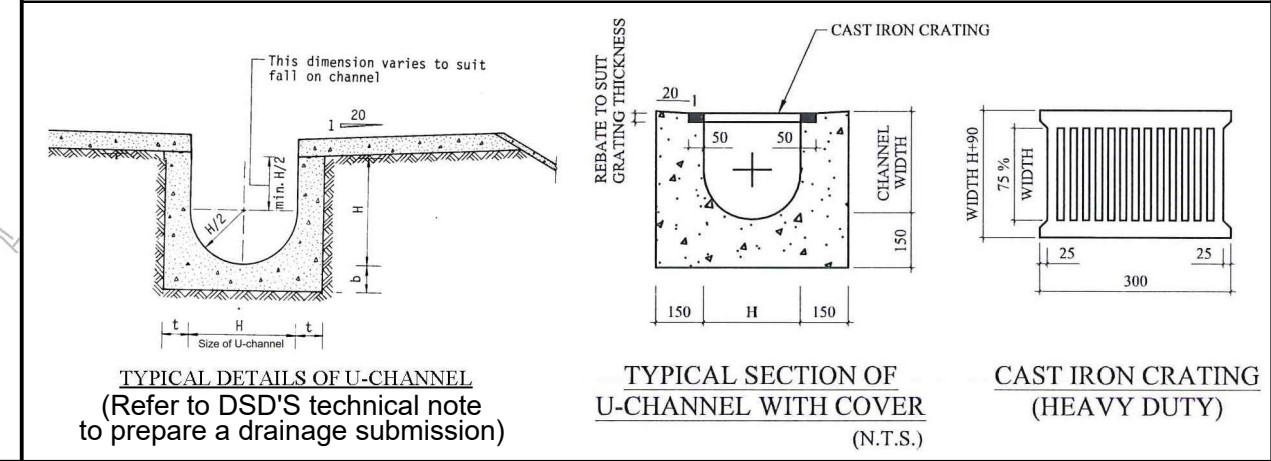
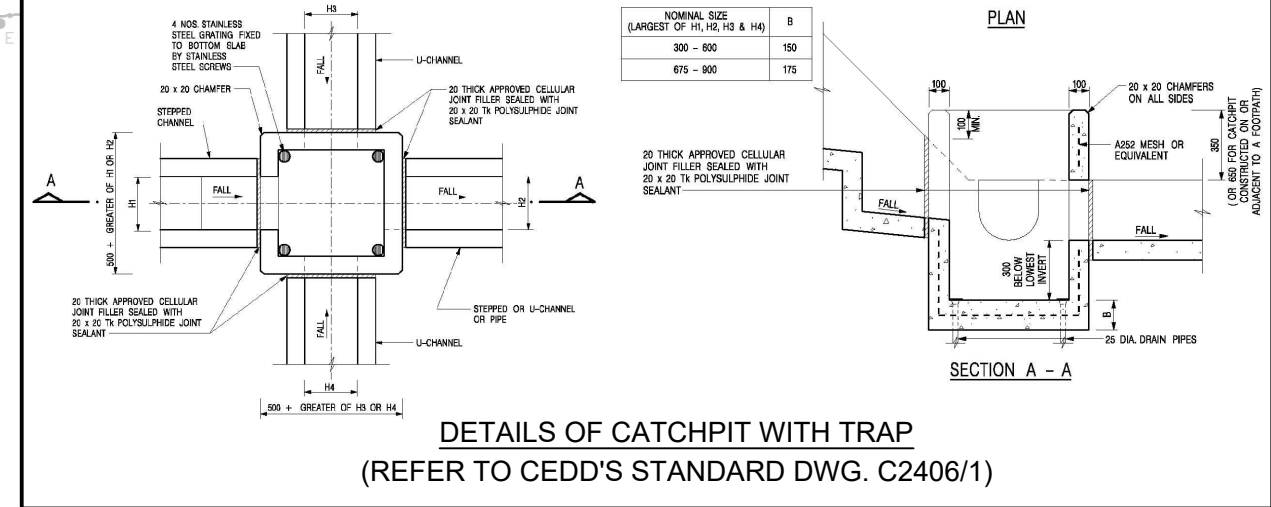
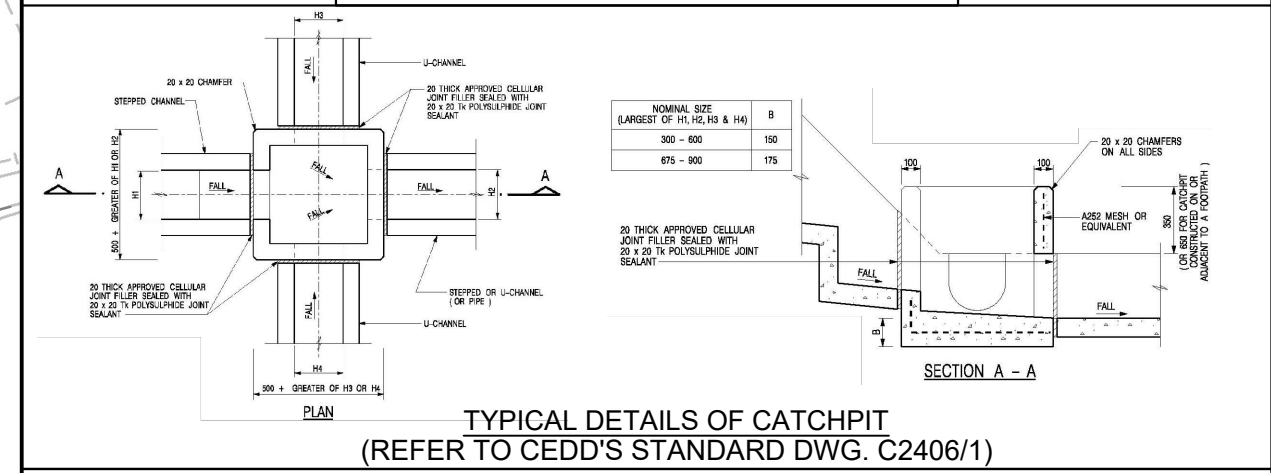
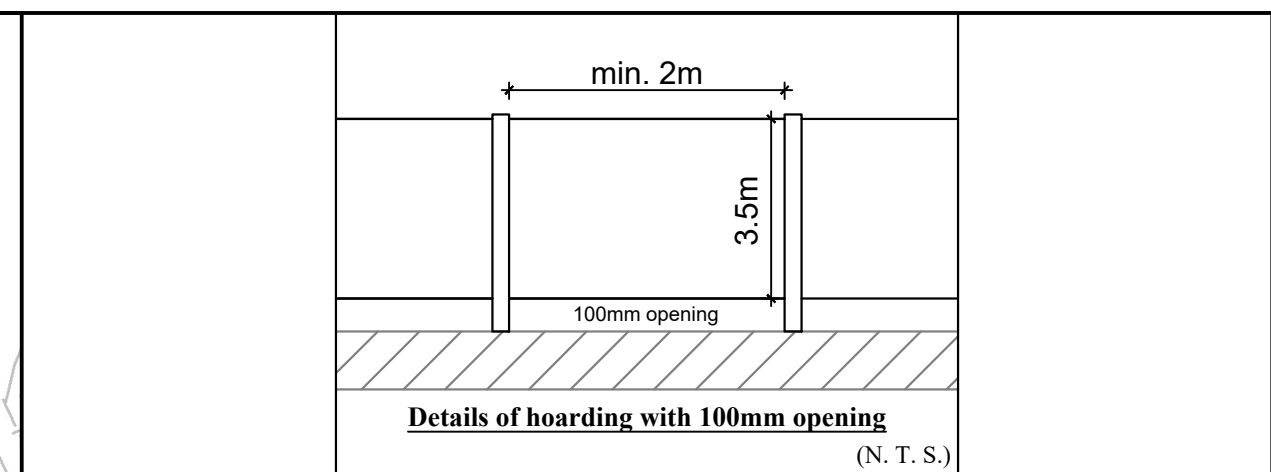
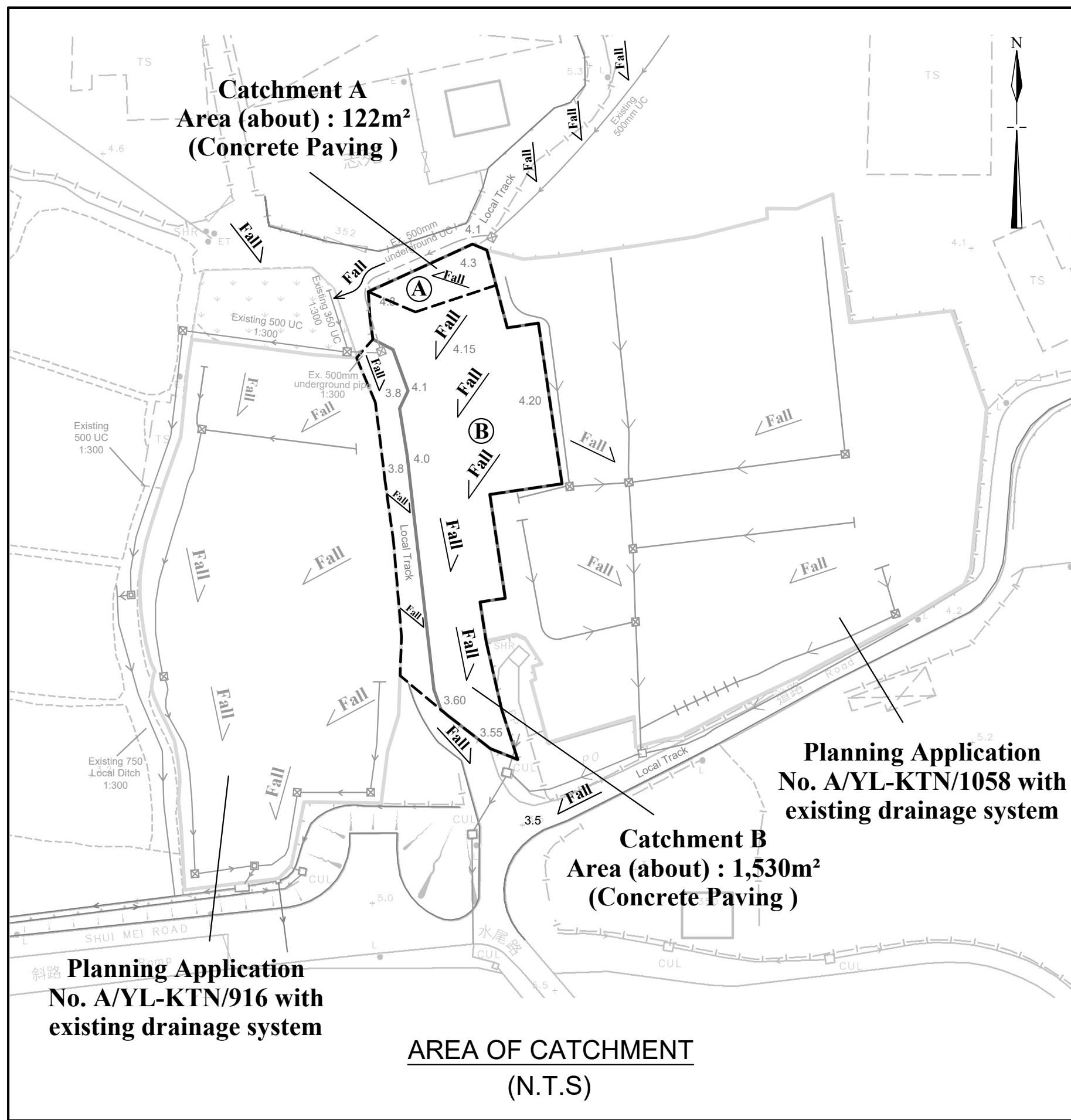
Further Information for Planning Application No. A/YL-KTN/1177**Response-to-Comments****Comments from Drainage Services Department**

(Contact Person: Ms Jessica KWAN; Tel.: 2300 1444)

I.	Comments	Responses
1.	Sections - Please indicate the hoarding opening in drawings.	Hoarding with opening is provided at the western side of the site. Please refer to updated Drainage Proposal (Plan 6.1b) for details.
2.	Section B - Please review the adjacent site ground levels. According to our record, the adjacent road level is higher than the application site, external catchment area should be considered. Please update the section and calculations if necessary.	Portion of the adjacent road at the west is at a higher level than the site. External catchment area is included. Please refer to updated Drainage Proposal (Plan 6.1b and Plan 6.2b) and Hydraulic Calculations for details.

- END -





N.T.S

December 2025

Drainage Proposal

Lots 1493 (part), 1500 (part) in DD. 107
and adjoining government land
Kam Tin North, New Territories

Goldrich Planners & Surveyors Ltd.

Plan 6.2b
(P 23044)

Viewpoint 1



Viewpoint 2



Viewpoint 3



Viewpoint 4



Viewpoint 5



Viewpoint 6



Viewpoint 7



Viewpoint 8



Viewpoint 9



Viewpoint 10



The ground level of the Site has been higher than the local track for many years. The surface runoff from the local track will not flow to the Site.

Viewpoint 11

Existing gutter which collects runoff from the roof to avoid runoff falls to the local track at the north.



Viewpoint 12



Viewpoint 13



1 For Catchment Area of Subject Site

Area, A = 122 m²
 Average slope, H = 0.1 m per 100m
 Distance on the line of natural flow, L = 3.5 m

$$\text{Time of concentration, } t_o = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (3.5) / (0.1^{0.2} \times 122^{0.1}) = 0.5 \text{ min}$$

Ref.

SDM 7.5.2 (d)

2 For Existing 500mm U-Channel to SCH1028760

	From	To
Ground level (mPD)	4.30	4.30
Invert level (mPD)	4.00	3.80

Width of u-channel, w = 300 mm
 Length of u-channel, L_c = 16.5 m
 Depth of vertical part of u-channel, d = 350 mm
 Gradient of u-channel, S_f = (4-3.8)/16.5 = 0.0121

Cross-Section Area, a = $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 150^2 + 300 \times 350 \times 0.9 = 0.126 \text{ m}^2$
 Wetted Perimeter, p = $\pi r + 2 d = 3.14 \times 150 + 2 \times 350 = 1.171 \text{ m}$
 Hydraulic radius, R = $a / p = 0.108 \text{ m}$

SDM 9.3 (b)
 Sedimentation Reduction

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-
 Allowable velocity, v = $R^{1/6} \times (RS_f)^{1/2} / n = (0.108)^{1/6} \times (0.108 \times 0.0121)^{1/2} / 0.016 = 1.56 \text{ m/s}$
 Time of flow, t_f = 0.2 min

SDM Table 13
 SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c = 505.5 / (0.5 + 0.2 + 3.29)^{0.35} = 310$ for return period T = 50 years

SDM 4.3.2
 Corrigendum 1/2024
 SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m ²)	C x A
Flat Glassland (heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	122.0	115.9
SUM =			115.9

SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m³/s

Design flow, Q_d = $0.278i \sum C_j A_j \times 1.16 + Q_u$ where A_j is in km²
 = $0.278 \times 310 \times 115.9 / 1000000 \times 1.16 + 0 = 0.012 \text{ m}^3/\text{s}$

SDM 7.5.2 (a)
 Corrigendum 1/2022

Allowable flow, Q_a = a x v = $0.126 \times 1.56 = 0.177 \text{ m}^3/\text{s}$

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

1 For Catchment Area of Subject Site

Area, A = 1530 m²
 Average slope, H = 0.1 m per 100m
 Distance on the line of natural flow, L = 25 m

$$\text{Time of concentration, } t_o = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (25) / (0.1^{0.2} \times 1530^{0.1}) = 2.8 \text{ min}$$

Ref.

SDM 7.5.2 (d)

2 For Existing 500mm U-Channel to SCH1028760

	From	To
Ground level (mPD)	4.30	3.55
Invert level (mPD)	3.80	3.05

Width of u-channel, w = 500 mm
 Length of u-channel, L_c = 86 m
 Depth of vertical part of u-channel, d = 250 mm
 Gradient of u-channel, S_f = (3.8-3.05)/86 = 0.0087

Cross-Section Area, a = $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 250^2 + 500 \times 250 \times 0.9 = 0.201 \text{ m}^2$
 Wetted Perimeter, p = $\pi r + 2 d = 3.14 \times 250 + 2 \times 250 = 1.285 \text{ m}$
 Hydraulic radius, R = $a / p = 0.156 \text{ m}$

SDM 9.3 (b)
 Sedimentation Reduction

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-
 Allowable velocity, v = $R^{1/6} \times (RS_f)^{1/2} / n = (0.156)^{1/6} \times (0.156 \times 0.009)^{1/2} / 0.016 = 1.69 \text{ m/s}$
 Time of flow, t_f = 0.8 min

SDM Table 13
SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c = 505.5 / (2.8 + 0.8 + 3.29)^{0.35} = 255$ for return period T = 50 years

SDM 4.3.2
 Corrigendum 1/2024
 SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m ²)	C x A
Flat Glassland (heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	1530.0	1453.5
SUM =			1453.5

SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m³/s

Design flow, Q_d = $0.278i \sum C_j A_j \times 1.16 + Q_u$ where A_j is in km²
 = $0.278 \times 255 \times 1453.5 / 1000000 \times 1.16 + 0 = 0.119 \text{ m}^3/\text{s}$

SDM 7.5.2 (a)
 Corrigendum 1/2022

Allowable flow, Q_a = a x v = 0.201 x 1.69 = 0.306 m³/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD