
寄件者: king king <[REDACTED]>
寄件日期: 2026年03月23日星期一 10:48
收件者: tpbpd/PLAND
副本: Andrea Wing Yin YAN/PLAND; Ivan Sze Yuet FUNG/PLAND; David Chi Chiu
CHENG/PLAND; Yen PY LEUNG/PLAND
主旨: A/YL-KTN/1189
附件: KTN1189-ltr-03.pdf
類別: Internet Email

Dear Sir,

Please see attached letter for responding to the comments of Government departments. Thank you.

Best Regards,

Patrick Tsui

Mobile: [REDACTED]

Total: 21 pages

Date: 23 March 2026

TPB Ref.: A/YL-KTN/1189

By Email

Town Planning Board
15/F, North Point Government Offices
333, Java Road
North Point
Hong Kong
(Attn: The Secretary)

Dear Sir,

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) & Associated Filling of Land for a Period of 3 Years at Lots 953 S.B, 953 S.C, 956 (Part), 960RP (Part), 961 RP (Part), 1065 (Part), 1072, 1074, 1075, 1077 (Part), 1081 S.A RP, 1081 S.B RP, 1082 (Part), 1086 & 1088 in D.D. 107 & Adjoining Government Land, Kam Tin, Yuen Long, New Territories

Our response to the comments of CE/MN, DSD is found in the attachment.

Our response to the comments of Water Services Department is found below:

The applicant will bear the cost of any necessary diversion due to the proposed development.

Our response to the comments of Lands Department is found below:

The applicant will demolish all the temporary structures within the application site upon planning approval. Upon planning approval, the applicant will apply for Short Term Waiver (STW) before the erection of the proposed temporary structures as shown on the proposed layout plan. Upon planning approval, the applicant will also apply for Short Term Tenancy (STT) to the Lands Department for the occupation of adjoining Government land within the application site.

Our response to the comments of Environmental Protection Department

It is expected that brand new electronic goods such as mobile phones and tablets, brand new construction materials such as sanitary wares will be stored within the warehouses. No dusty materials will be stored within the warehouse.

Should you have any enquiries, please feel free to contact our Mr. Patrick Tsui at [REDACTED] at your convenience.

Yours faithfully,



The stamp is circular and purple, containing the text 'METRO PLANNING & DEVELOPMENT COMPANY LIMITED' around the perimeter and '都市規劃及發展顧問有限公司' in the center.

Patrick Tsui

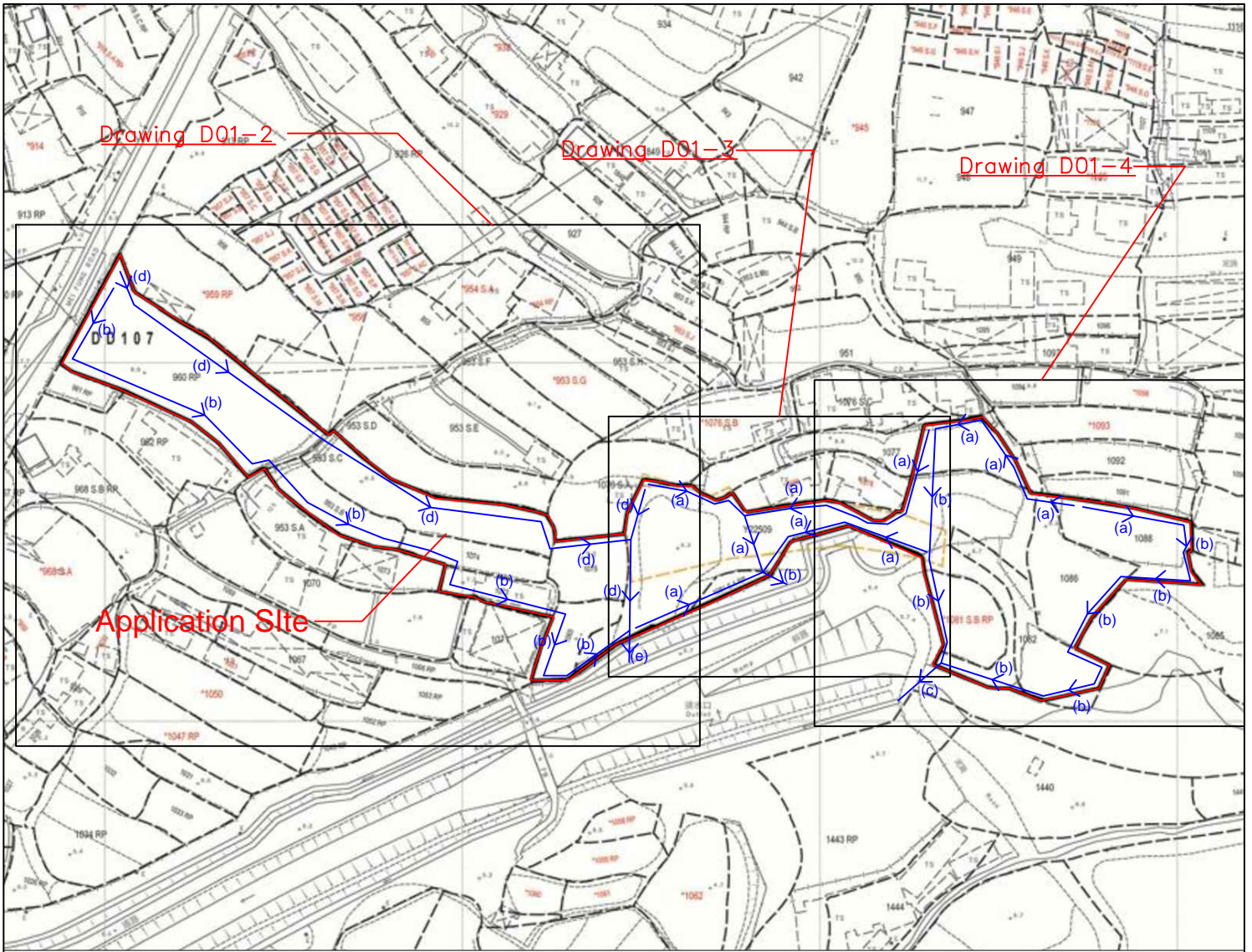
c.c. Fanling, Sheung Shui and Yuen Long East District Planning Office (Attn: Mr. Ivan FUNG) – By Email

KTN/1189 R-to-C

(A) Specific Comments

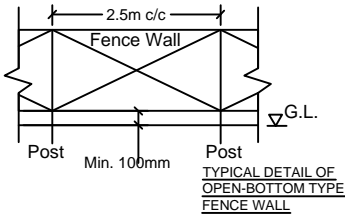
- (1) It is revised accordingly. There should be maximum 200mm concrete paving to be paved.
- (2) Peripheral surface channels are provided. No overland flow would be affected.
- (3) Cross sections are provided.
- (4) Hydraulic calculation is provided.
- (5) Outside catchment area is revised. Please note that there is existing solid fence wall in the north of the site.
- (6) Calculation checking the existing facilities to be discharged is provided.
- (7) The adopted runoff coefficient for outside catchment area is revised and justification is provided.
- (8) Deposition of sediment is considered in the design calculation (0.9 factor is applied).
- (9) The size and gradient of the proposed drainage facilities is indicated in Drawing (D01-2), and the typo is revised accordingly.
- (10) The size of the proposed surface channels at the downstream of the proposed catchpit 9a shall be 525UC, the connection details is revised accordingly
- (11) The ID nos and size of the existing public drainage facilities is indicated in Drawing (D01-4).
- (12) It should be CP4b, the note is revised accordingly.
- (13) The invert level is revised accordingly.
- (14) Catchpit is provided in every turning point.
- (15) Catchpit schedule is provided.
- (16) The proposed size of u-channels is indicated in Drawing (D01-1) and (D02)

All general comments are noted.



Note:

1. Catchpits (CP2) with desilting facility shall follow CEDD standard drawing No. C2406I.
2. Catchpit and UC follows Typical Details of Geotechnical Manual for Slope Fig.8.10 and Fig.8.11 respectively.
3. Fence Wall to be erected (if any) shall be Open-bottom type.
4. Maximum 200mm concrete paving to be paved.



LEGEND

- CP Proposed CatchPit
- (a) Proposed 375UC (1:200) with Cast Iron Cover
- (b) Proposed 525UC (1:200) with Cast Iron Cover
- (c) Proposed 525UC (1:150) with Cast Iron Cover
- (d) Proposed 600UC (1:150) with Cast Iron Cover
- (e) Proposed 750mm dia. concrete pipe (1:100)
- CP Existing CatchPit/Manhole
- Existing Drain (size as shown)

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CHING WAN ENGINEERING CONSULTANT COMPANY

Project:

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Title:

Drainage Proposal - LAYOUT

D01-1

Drawn by:

DM

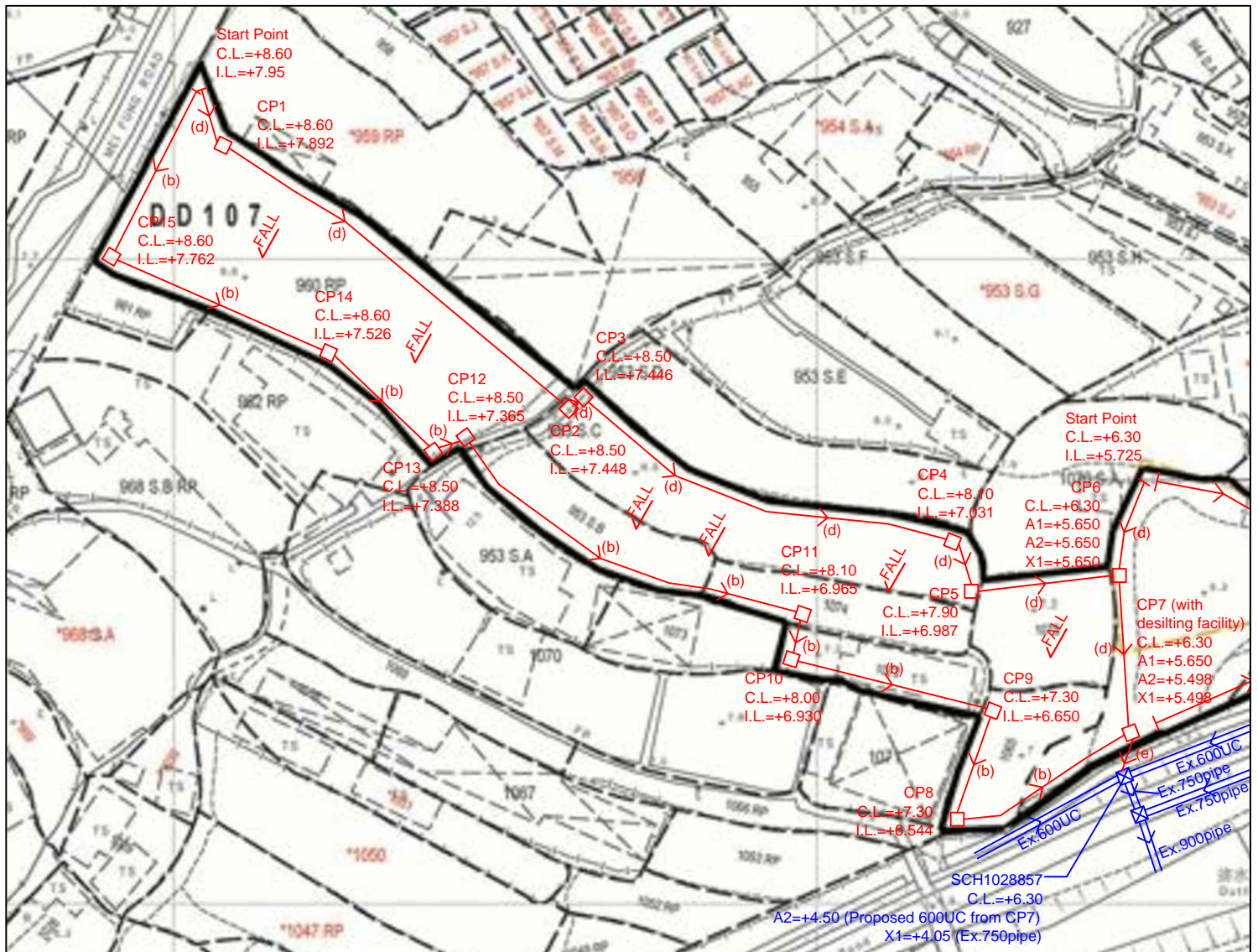
Date:

20-3-2026

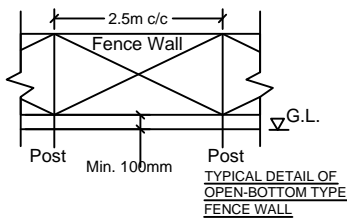
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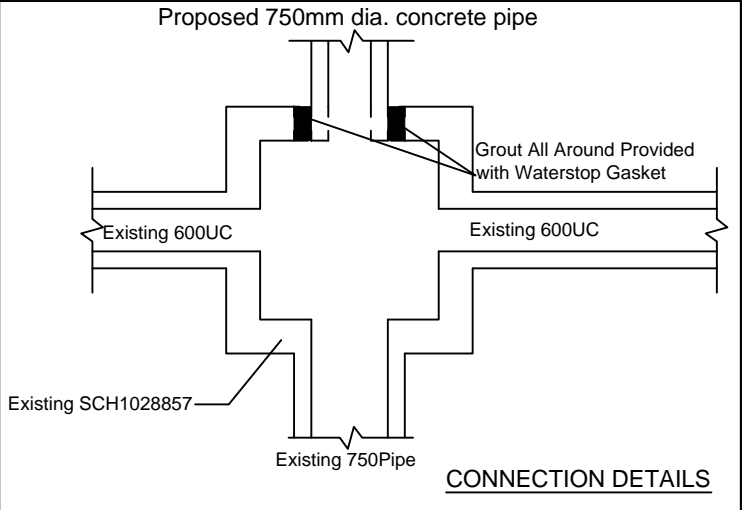
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- Note:**
- Catchpits (CP7) with desilting facility shall follow CEDD standard drawing No. C24061.
 - Catchpit and UC follows Typical Details of Geotechnical Manual for Slope Fig.8.10 and Fig.8.11 respectively.
 - Fence Wall to be erected (if any) shall be Open-bottom type.
 - Maximum 200mm concrete paving to be paved.



- LEGEND**
- CP Proposed CatchPit
 - (a) Proposed 375UC (1:200) with Cast Iron Cover
 - (b) Proposed 525UC (1:200) with Cast Iron Cover
 - (c) Proposed 525UC (1:150) with Cast Iron Cover
 - (d) Proposed 600UC (1:150) with Cast Iron Cover
 - (e) Proposed 750mm dia. concrete pipe (1:100)
 - CP Existing CatchPit/Manhole
 - Existing Drain (size as shown)



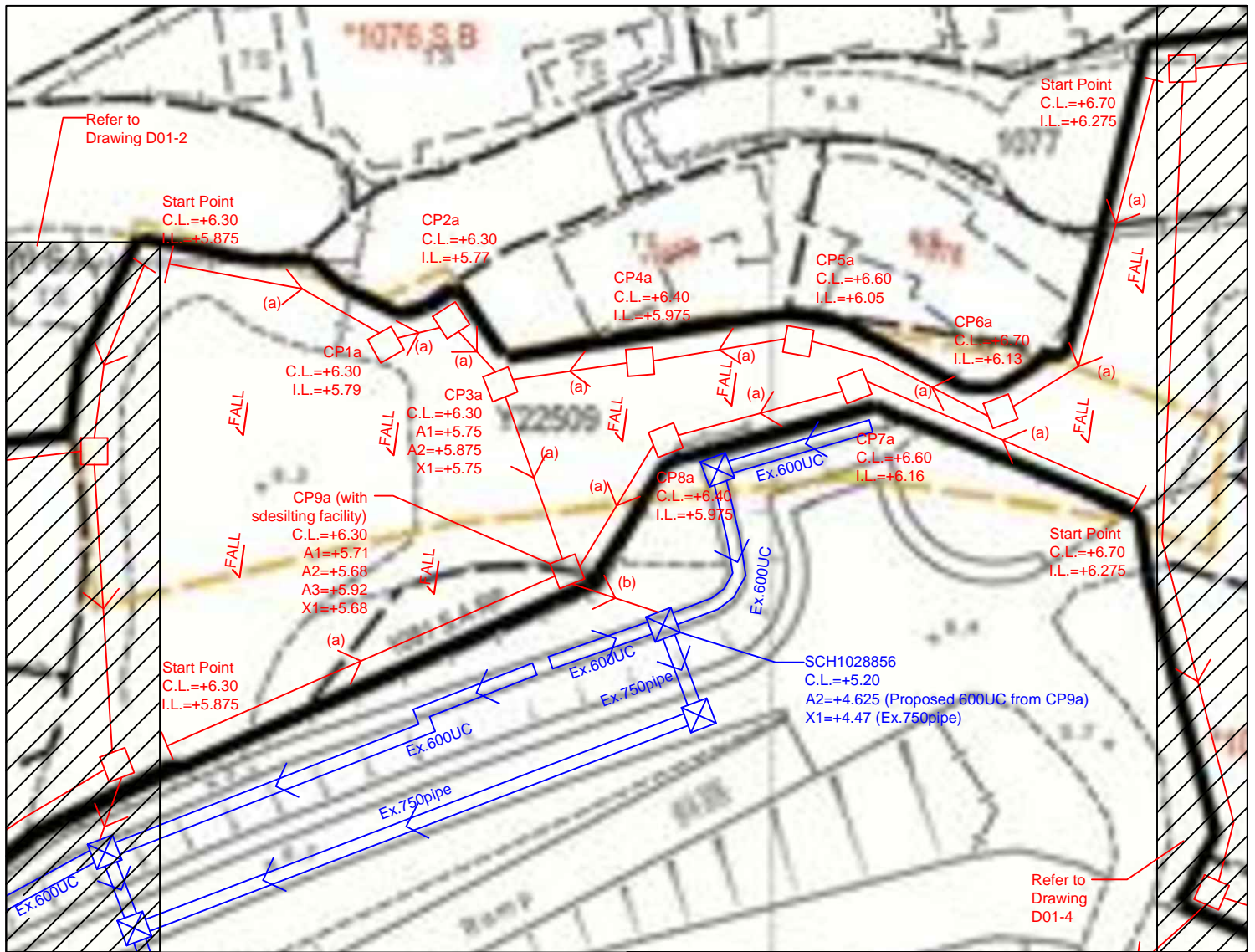
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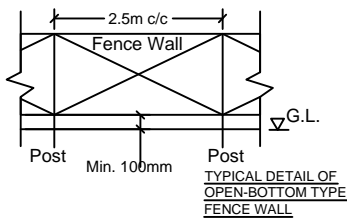
Title:
Drainage Proposal - LAYOUT

D01-2

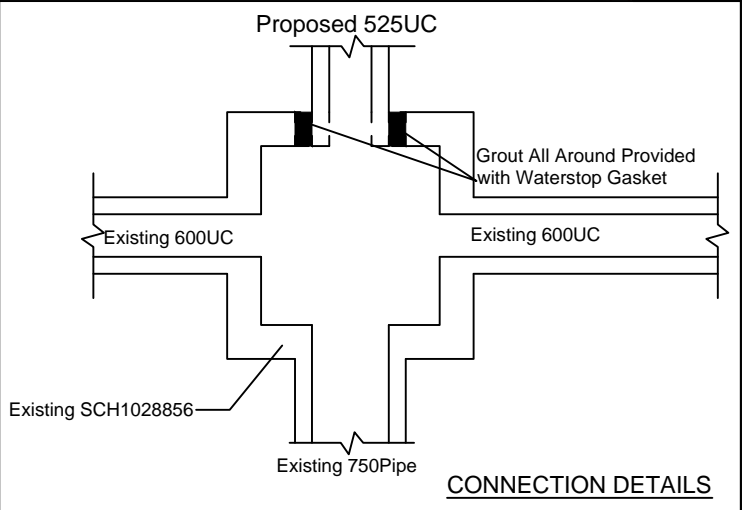
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Check by: DM	Scale: ----



- Note:**
- Catchpits (CP9a) with desilting facility shall follow CEDD standard drawing No. C2406I.
 - Catchpit and UC follows Typical Details of Geotechnical Manual for Slope Fig.8.10 and Fig.8.11 respectively.
 - Fence Wall to be erected (if any) shall be Open-bottom type.
 - Maximum 200mm concrete paving to be paved.



- LEGEND**
- CP Proposed CatchPit
 - (a) Proposed 375UC (1:200) with Cast Iron Cover
 - (b) Proposed 525UC (1:200) with Cast Iron Cover
 - (c) Proposed 525UC (1:150) with Cast Iron Cover
 - (d) Proposed 600UC (1:150) with Cast Iron Cover
 - (e) Proposed 750mm dia. concrete pipe (1:100)
 - ☒ CP Existing CatchPit/Manhole
 - ⇒ Existing Drain (size as shown)



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Title:

Drainage Proposal - LAYOUT

D01-3

Drawn by:

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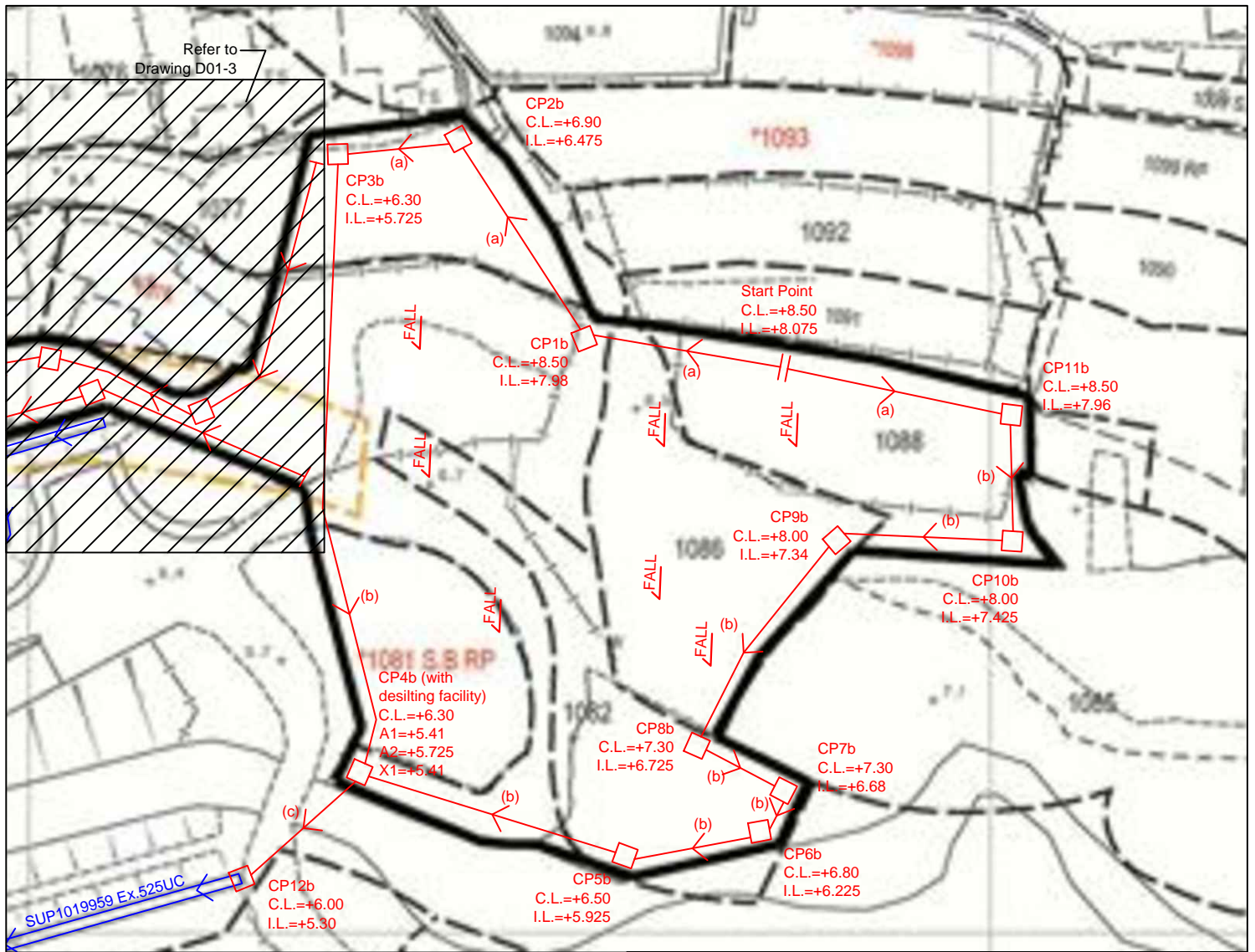
Date:

20-3-2026

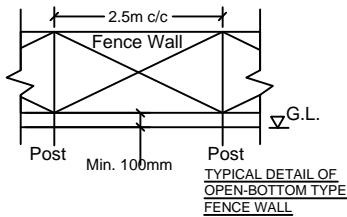
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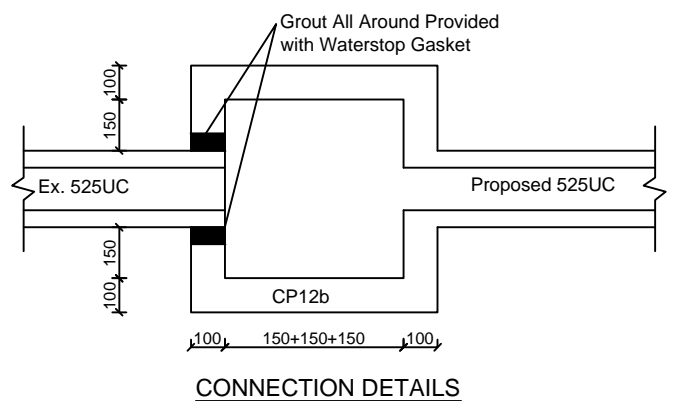
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- Note:**
- Catchpits (CP4b) with desilting facility shall follow CEDD standard drawing No. C2406I.
 - Catchpit and UC follows Typical Details of Geotechnical Manual for Slope Fig.8.10 and Fig.8.11 respectively.
 - Fence Wall to be erected (if any) shall be Open-bottom type.
 - Maximum 200mm concrete paving to be paved.



- LEGEND**
- CP Proposed CatchPit
 - (a) Proposed 375UC (1:200) with Cast Iron Cover
 - (b) Proposed 525UC (1:200) with Cast Iron Cover
 - (c) Proposed 525UC (1:150) with Cast Iron Cover
 - (d) Proposed 600UC (1:150) with Cast Iron Cover
 - (e) Proposed 750mm dia. concrete pipe (1:100)
 - CP Existing CatchPit/Manhole
 - Existing Drain (size as shown)



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Title:

Drainage Proposal - LAYOUT

D01-4

Drawn by:

DM

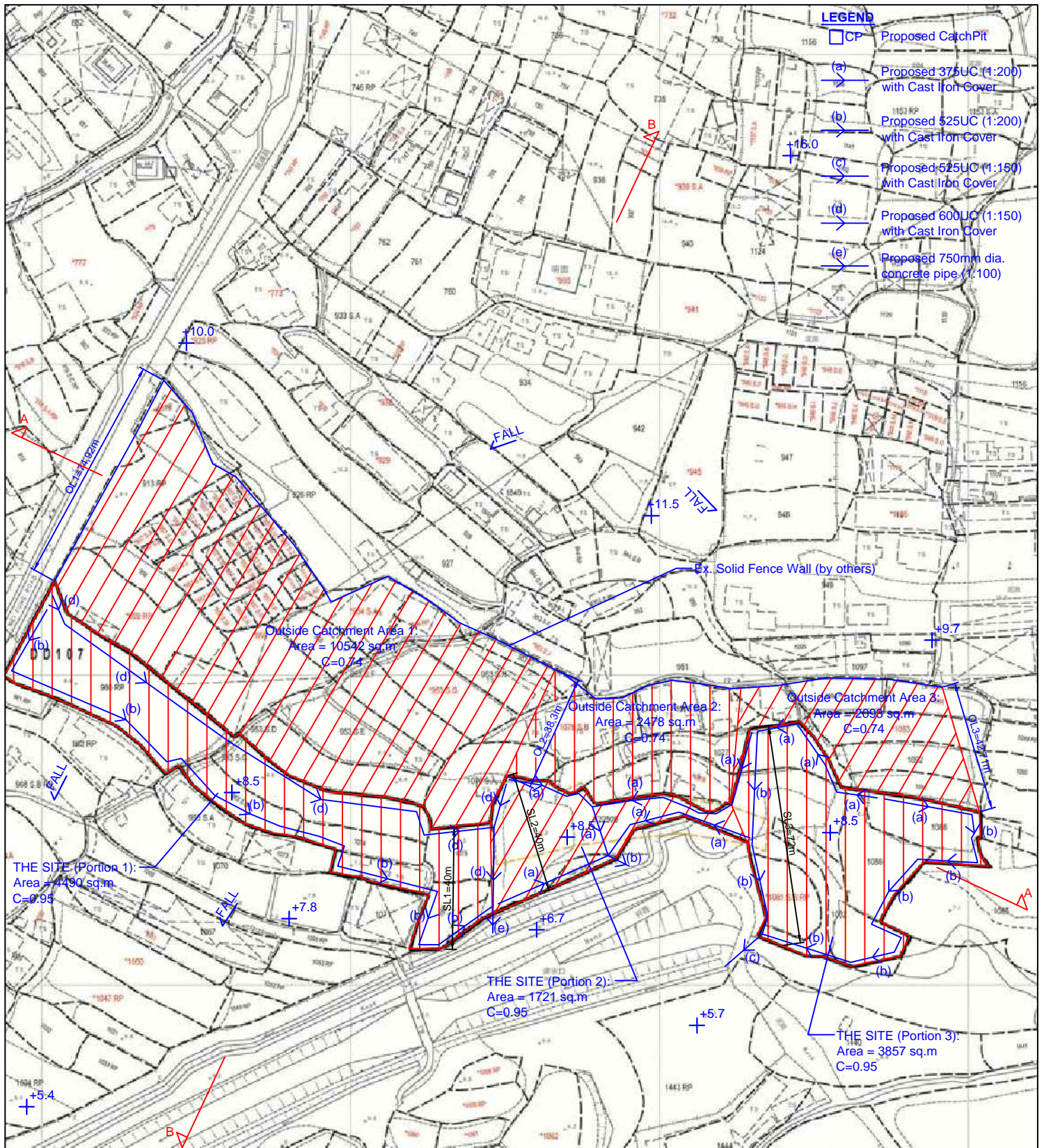
Date:

20-3-2026

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DM

Scale:



- LEGEND**
- CP Proposed CatchPit
 - (a) Proposed 375UC (1:200) with Cast Iron Cover
 - (b) Proposed 525UC (1:200) with Cast Iron Cover
 - (c) Proposed 525UC (1:150) with Cast Iron Cover
 - (d) Proposed 600UC (1:150) with Cast Iron Cover
 - (e) Proposed 750mm dia. concrete pipe (1:100)

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Title:
 Drainage Proposal -
 CATCHMENT AREA PLAN

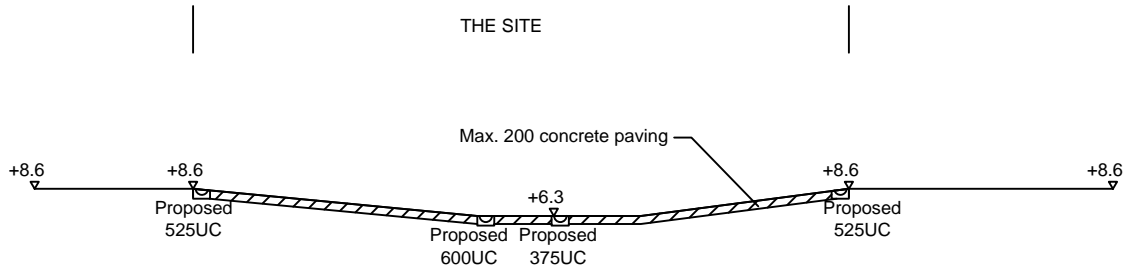
D02

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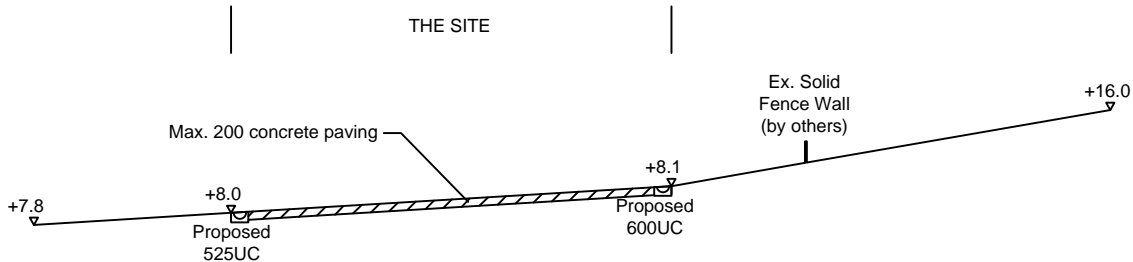
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 20-3-2026

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SECTION A-A



SECTION B-B

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Title:

Drainage Proposal -
SECTIONS

D03

Drawn by:

DM

Date:

20-3-2026

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Project:

Proposed Temporary Warehouse (Non-dangerous Goods) and Associated Filling of Land for a Period of 3 Years at Lot 953 S.B, 953 S.C, 956 (Part), 960RP (Part), 961RP (Part), 1065 (Part), 1072 1074, 1075, 1077 (Part), 1081 S.A. RP, 1081 S.B RP, 1082 (Part), 1086 & 1088 in D.D. 107 And Adjoining Government Land, Kam Tin, Yuen Long, New Territories

Catchpit Schedule		
Catchpit Number	C.L.	I.L.
CP1	+8.60	+7.892
CP2	+8.50	+7.448
CP3	+8.50	+7.446
CP4	+8.10	+7.031
CP5	+7.90	+6.987
CP6	+6.30	A1=+5.650, A2=+5.650, X1=+5.650
CP7	+6.30	A1=+5.650, A2=+5.498, X1=+5.498
CP8	+7.30	+6.544
CP9	+7.30	+6.650
CP10	+8.00	+6.930
CP11	+8.10	+6.650
CP12	+8.50	+7.365
CP13	+8.50	+7.388
CP14	+8.60	+7.526
CP15	+8.60	+7.762
CP1a	+6.30	+5.79
CP2a	+6.30	+5.77
CP3a	+6.30	A1=+5.750, A2=+5.875, X1=+5.75
CP4a	+6.40	+5.975
CP5a	+6.60	+6.05
CP6a	+6.70	+6.13
CP7a	+6.60	+6.16
CP8a	+6.40	+5.975
CP9a	+6.30	A1=+5.71, A2=+5.68, A3=+5.92, X1=+5.68
CP1b	+8.50	+7.98
CP2b	+6.90	+6.475
CP3b	+6.30	+5.725
CP4b	+6.30	A1=+5.41, A2=+5.725, X1=5.41
CP5b	+6.50	+5.925
CP6b	+6.80	+6.225
CP7b	+7.30	+6.68
CP8b	+7.30	+6.725
CP9b	+8.00	+7.34
CP10b	+8.00	+7.425
CP11b	+8.50	+7.96
CP12b	+6.00	+5.30

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Title:

Drainage Proposal -
Catchpit Schedule

D04

Drawn by:

DM

Date:

20-3-2026

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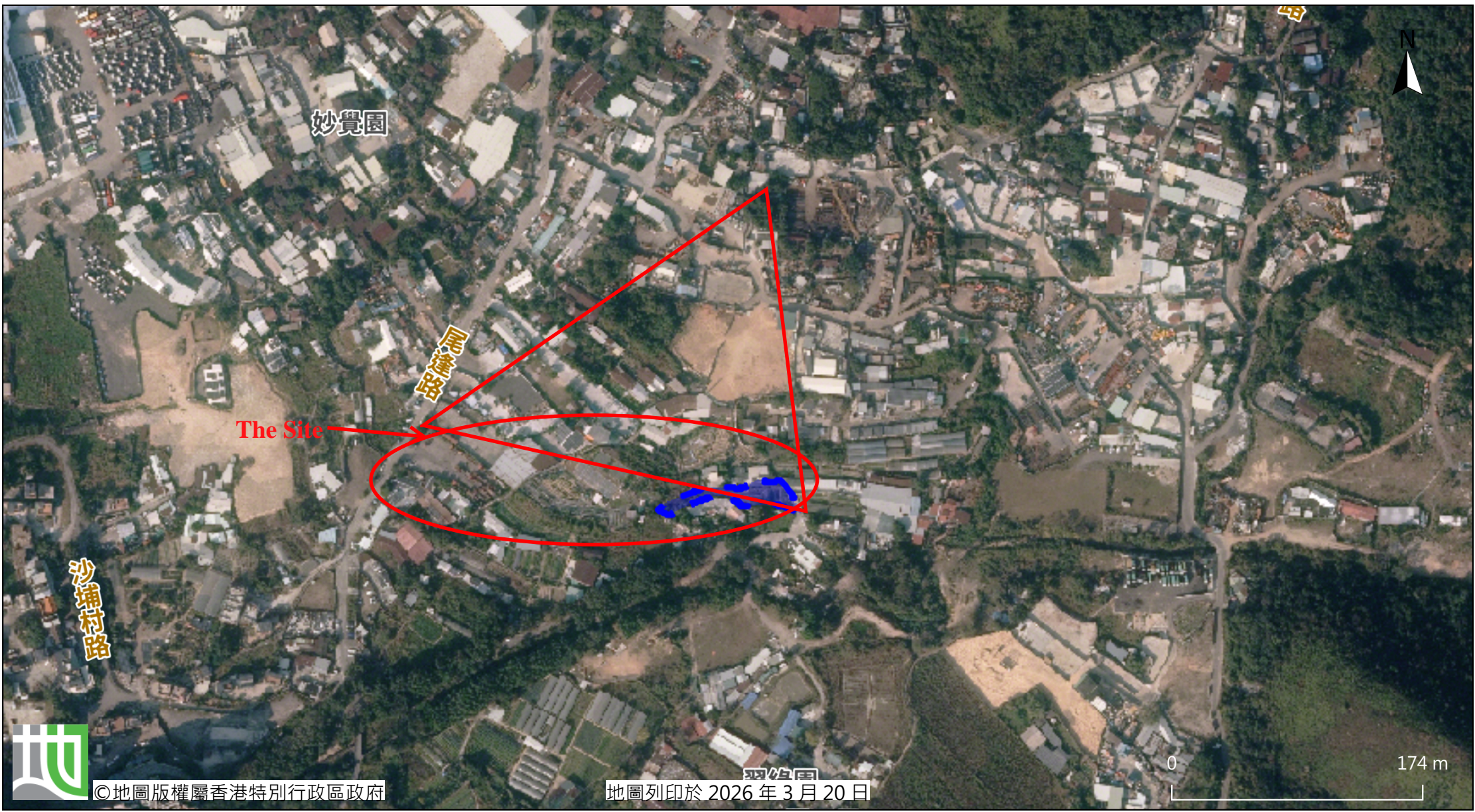
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前往地圖: <https://www.map.gov.hk/gm/geo:22.4547,114.0600?z=4514>

Justification of C Value of Outside Catchment Area (30% soil ground ==> $C=0.7*0.95+0.3*0.25=0.74$)



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地圖列印於 2026 年 3 月 20 日

Outside Catchment Area 1 , Area	= 10542	m ²	(C= 0.74)	OL1= 74.92 m, H1= 2.002 m
Outside Catchment Area 2 , Area	= 2478	m ²	(C= 0.74)	OL2= 38.3 m H2= 7.833 m
Outside Catchment Area 3 , Area	= 2093	m ²	(C= 0.74)	OL3= 42.7 m, H3= 2.810 m
The Site, (Portion 1), Area	= 4490	m ²	(C= 0.95)	SL1= 40 m
The Site, (Portion 2), Area	= 1721	m ²	(C= 0.95)	SL2= 40 m
The Site, (Portion 3), Area	= 3857	m ²	(C= 0.95)	SL3= 72 m

Calculation of Design Runoff of the Proposed Development.

For the design of drains at northern side of The Site (Portion 1), Outside Catchment Area 1

$$\Sigma Q = \Sigma 0.278 C i A$$

A = 10542 m²
= 10542 km²
= 0.010542 km²

t = 0.14465 OL1/ H1^{0.2} A^{0.1}
= 0.14465*74.92/2.002^{0.2}*12878^{0.1}
= 3.735 min

i = 1.11*a/(t+b)^c (50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11% increase due to climate change)
= 1.11*505.5/(3.735+3.29)^{0.355}
= 280.9 mm/hr

Therefore, Q = 0.278*0.5*280.9*0.010542/0.9 (0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)
= 0.6768 m³/sec
= **40606** lit/min

Provide 600UC (1:150) is OK

For the design of drains at southern side of The Site (Portion 1), The Site (Portion 1)

$$\Sigma Q = \Sigma 0.278 C i A$$

A = 4490 m²
= 4490 km²
= 0.00449 km²

t = 0.14465 SL1/ H^{0.2} A^{0.1}
= 0.14465*40/1^{0.2}*4490^{0.1}
= 2.495 min

i = 1.11*a/(t+b)^c (50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11% increase due to climate change)
= 1.11*505.5/(2.495+3.29)^{0.355}
= 300.9 mm/hr

Therefore, Q = 0.278*0.95*300.9*0.00449/0.9 (0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)
= 0.3964 m³/sec
= **23787** lit/min

Provide 525UC (1:100) is OK

For the design of Outfall from The Site (Portion 1), The Site (Portion 1) + Outside Catchment Area 1

Q = 40606 + 23787
= **64393** lit/min

Provide 750 concrete pipe (1:100)

Calculation of Design Runoff of the Proposed Development,

For the design of drains at northern side of The Site (Portion 2), Outside Catchment Area 2

$$\Sigma Q = \Sigma 0.278 C i A$$

$$\begin{aligned} A &= 2478 && \text{m}^2 \\ &= 2478 \\ &= 0.002478 && \text{km}^2 \end{aligned}$$

$$\begin{aligned} t &= 0.14465 OL_2 / H_2^{0.2} A^{0.1} \\ &= 0.14465 * 38.3 / 7.833^{0.2} * 2478^{0.1} \\ &= 1.680 && \text{min} \end{aligned}$$

$$\begin{aligned} i &= 1.11 * a / (t+b)^c && \text{(50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11% increase due to climate change)} \\ &= 1.11 * 505.5 / (1.680 + 3.29)^{0.355} \\ &= 317.6 && \text{mm/hr} \end{aligned}$$

$$\begin{aligned} \text{Therefore, } Q &= 0.278 * 0.5 * 317.6 * 0.002478 / 0.9 && \text{(0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)} \\ &= 0.1799 && \text{m}^3/\text{sec} \\ &= \mathbf{10793} && \text{lit/min} \end{aligned}$$

Provide 375UC (1:200) is OK

For the design of drains at southern side of The Site (Portion 2), The Site (Portion 2)

$$\Sigma Q = \Sigma 0.278 C i A$$

$$\begin{aligned} A &= 1721 && \text{m}^2 \\ &= 1721 \\ &= 0.001721 && \text{km}^2 \end{aligned}$$

$$\begin{aligned} t &= 0.14465 SL_2 / H^{0.2} A^{0.1} \\ &= 0.14465 * 40 / 1^{0.2} * 1721^{0.1} \\ &= 2.747 && \text{min} \end{aligned}$$

$$\begin{aligned} i &= 1.11 * a / (t+b)^c && \text{(50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11% increase due to climate change)} \\ &= 1.11 * 505.5 / (2.747 + 3.29)^{0.355} \\ &= 296.4 && \text{mm/hr} \end{aligned}$$

$$\begin{aligned} \text{Therefore, } Q &= 0.278 * 0.95 * 296.4 * 0.001721 / 0.9 && \text{(0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)} \\ &= 0.1497 && \text{m}^3/\text{sec} \\ &= \mathbf{8981} && \text{lit/min} \end{aligned}$$

Provide 375UC (1:200) is OK

For the design of Outfall from The Site (Portion 2), The Site (Portion 2) + Outside Catchment Area 2

$$\begin{aligned} Q &= \mathbf{10793} && + && \mathbf{8981} \\ &= \mathbf{19773} && && \text{lit/min} \end{aligned}$$

Provide 525UC (1:200) is OK

Calculation of Design Runoff of the Proposed Development,

For the design of drains at northern side of The Site (Portion 3), Outside Catchment Area 3

$$\Sigma Q = \Sigma 0.278 C i A$$

$$\begin{aligned} A &= 2093 && \text{m}^2 \\ &= 2093 && \\ &= 0.002093 && \text{km}^2 \end{aligned}$$

$$\begin{aligned} t &= 0.14465 \text{ OL}_3 / \text{H}_3^{0.2} A^{0.1} \\ &= 0.14465 * 42.7 / 2.810^{0.2} * 4180^{0.1} \\ &= 2.338 && \text{min} \end{aligned}$$

$$\begin{aligned} i &= 1.11 * a / (t+b)^c && \text{(50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11\% increase due to climate change)} \\ &= 1.11 * 505.5 / (2.338 + 3.29)^{0.355} \\ &= 303.8 && \text{mm/hr} \end{aligned}$$

$$\begin{aligned} \text{Therefore, } Q &= 0.278 * 0.5 * 303.8 * 0.002093 / 0.9 && \text{(0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)} \\ &= 0.1454 && \text{m}^3/\text{sec} \\ &= \mathbf{8722} && \text{lit/min} \end{aligned}$$

Provide 375UIC (1:200) is OK

For the design of drains at southern side of The Site (Portion 3), The Site (Portion 3)

$$\Sigma Q = \Sigma 0.278 C i A$$

$$\begin{aligned} A &= 3857 && \text{m}^2 \\ &= 3857 && \\ &= 0.003857 && \text{km}^2 \end{aligned}$$

$$\begin{aligned} t &= 0.14465 \text{ SL}_3 / \text{H}^{0.2} A^{0.1} \\ &= 0.14465 * 72 / 1^{0.2} * 3857^{0.1} \\ &= 4.561 && \text{min} \end{aligned}$$

$$\begin{aligned} i &= 1.11 * a / (t+b)^c && \text{(50 yrs return period, Table 3a, Corrigendum 2024, SDM) and (11\% increase due to climate change)} \\ &= 1.11 * 505.5 / (4.561 + 3.29)^{0.355} \\ &= 270.0 && \text{mm/hr} \end{aligned}$$

$$\begin{aligned} \text{Therefore, } Q &= 0.278 * 0.95 * 270.2 * 0.003857 / 0.9 && \text{(0.9 factor is adopted for sedimentation according to SDM 2018, section 9.3)} \\ &= 0.3056 && \text{m}^3/\text{sec} \\ &= \mathbf{18335} && \text{lit/min} \end{aligned}$$

Provide 525UIC (1:200) is OK

For the design of Outfall from The Site (Portion 3), The Site (Portion 3) + Outside Catchment Area 3

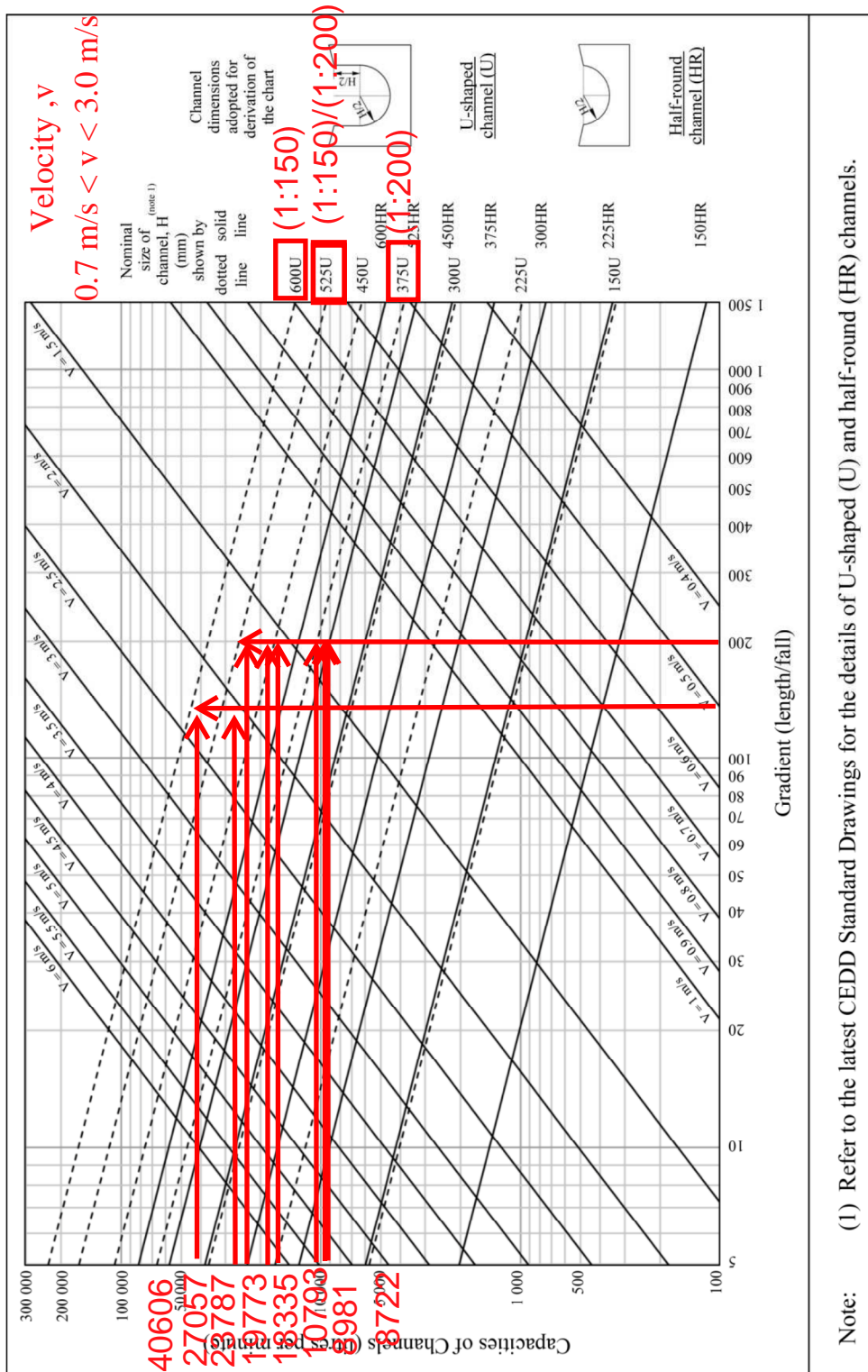
$$\begin{aligned} Q &= 8722 && + && 18335 \\ &= \mathbf{27057} && && \text{lit/min} \end{aligned}$$

Provide 525UIC (1:150) is OK

GEO Technical Guidance Note No. 43 (TGN 43)
Guidelines on Hydraulic Design of U-shaped and Half-round Channels on Slopes

Issue No.: 1 Revision: - Date: 05.06.2014 Page: 3 of 3

Figure 1 - Chart for the rapid design of U-shaped and half-round channels up to 600 mm



Check 750mm dia. Pipes by Colebrook-White Equation

$$V = -\sqrt{(8gDs)} \log\left(\frac{ks}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}}\right)$$

where :

V	=		mean velocity (m/s)
g	=	9.81	m/s ² gravitational acceleration (m/s ²)
D	=	0.75	m internal pipe diameter (m)
ks	=	0.00003	m hydraulic pipeline roughness (m) (Table14, from DSD SDM 2018, upvc pipe)
v	=	1.14E-06	m ² /s kinematic viscosity of fluid (m ² /s)
s	=	0.01	hydraulic gradient (1: 100)

Therefore, design V of pipe capacity = 3.59 m/s

Q= 0.8VA		(0.8 factor for sedimentation)
= 1.270	m ³ /s	
= 76186	lit/min	
> 64393	lit/min	Ok

Check Ex. 900mm dia. Pipes by Colebrook-White Equation

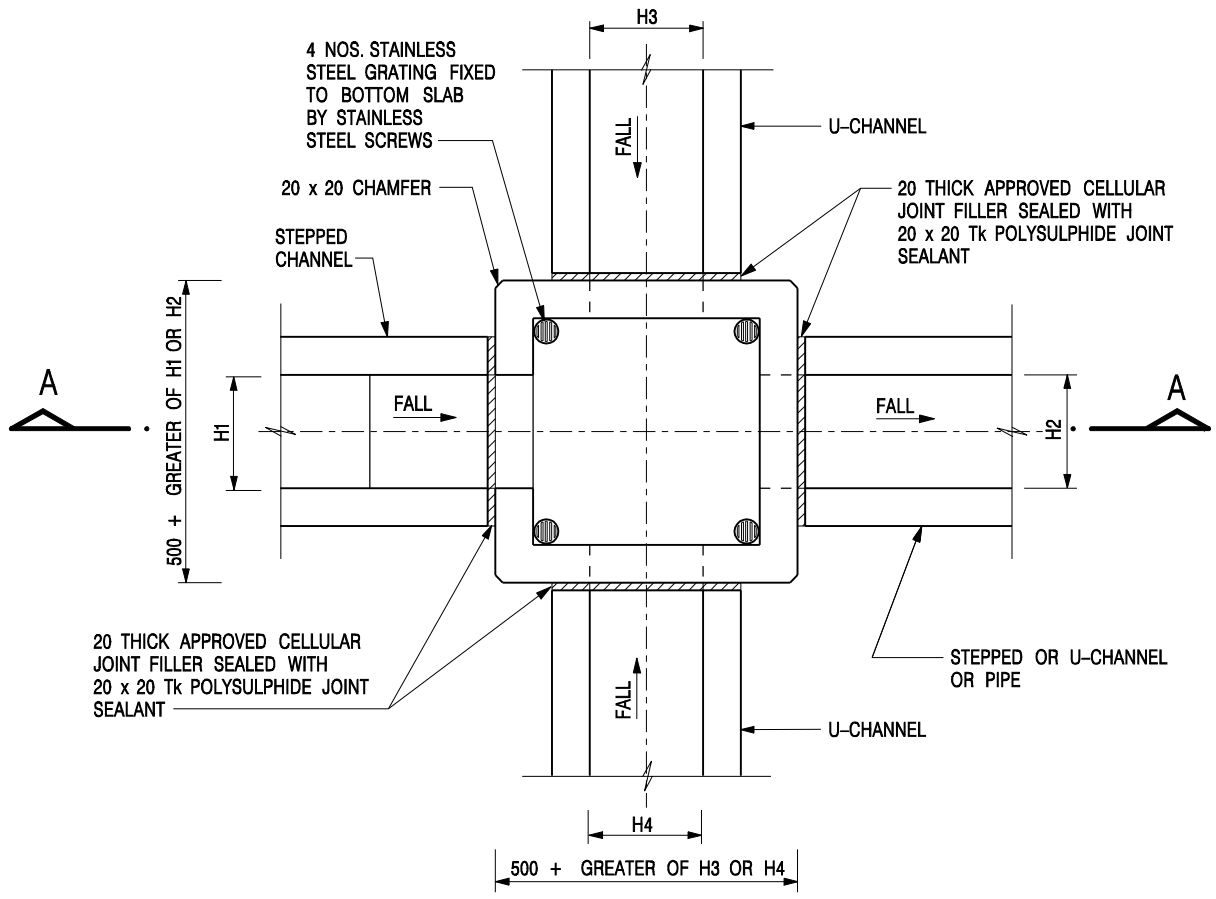
$$V = -\sqrt{(8gDs)} \log\left(\frac{ks}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}}\right)$$

where :

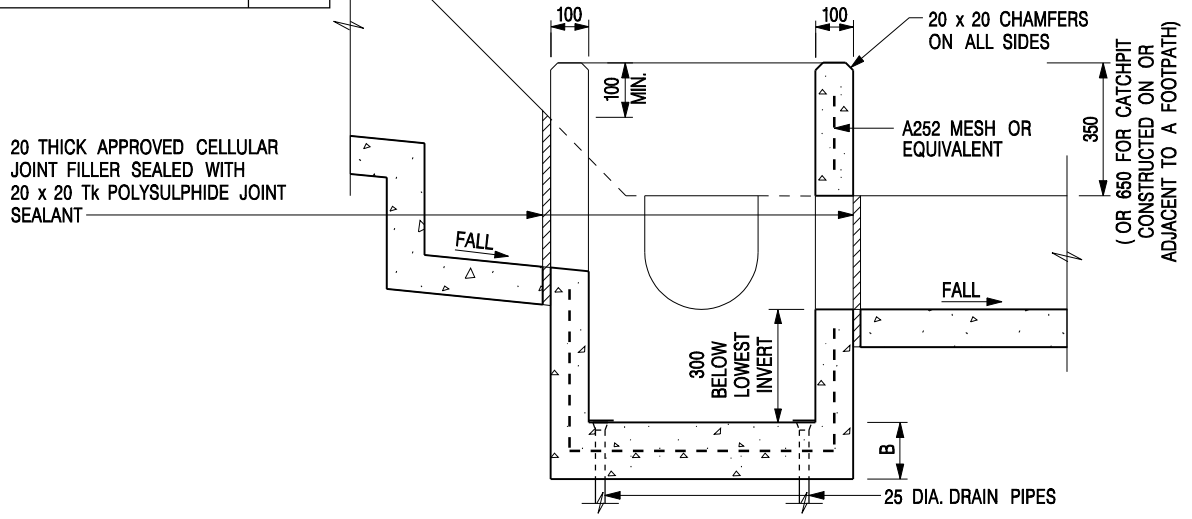
V	=		mean velocity (m/s)
g	=	9.81	m/s ² gravitational acceleration (m/s ²)
D	=	0.9	m internal pipe diameter (m)
ks	=	0.00006	m hydraulic pipeline roughness (m) (Table14, from DSD SDM 2018, concrete pipe)
v	=	1.14E-06	m ² /s kinematic viscosity of fluid (m ² /s)
s	=	0.01	hydraulic gradient (1: 100)

Therefore, design V of pipe capacity = 3.86 m/s

Q= 0.8VA	(0.8 factor for sedimentation)
= 1.964	m ³ /s
= 117848	lit/min
> 111223	lit/min Ok




NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
300 - 600	150
675 - 900	175



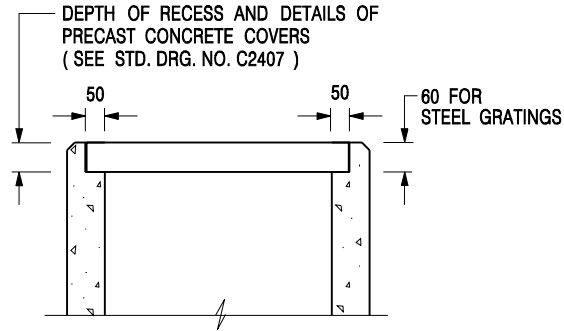
- NOTES:**
1. ALL DIMENSIONS ARE IN MILLIMETRES.
 2. REFER TO SHEET 2 FOR OTHER NOTES.

-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

CATCHPIT WITH TRAP
(SHEET 1 OF 2)


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1 : 20	DRAWING NO.
DATE JAN 1991	C2406 /1




**ALTERNATIVE TOP SECTION
FOR PRECAST CONCRETE COVERS / GRATINGS**

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

A	MINOR AMENDMENT.	Original Signed	04.2016
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

**CATCHPIT WITH TRAP
(SHEET 2 OF 2)**

 CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT	
SCALE 1 : 20	DRAWING NO.
DATE JAN 1991	C2406 /2A

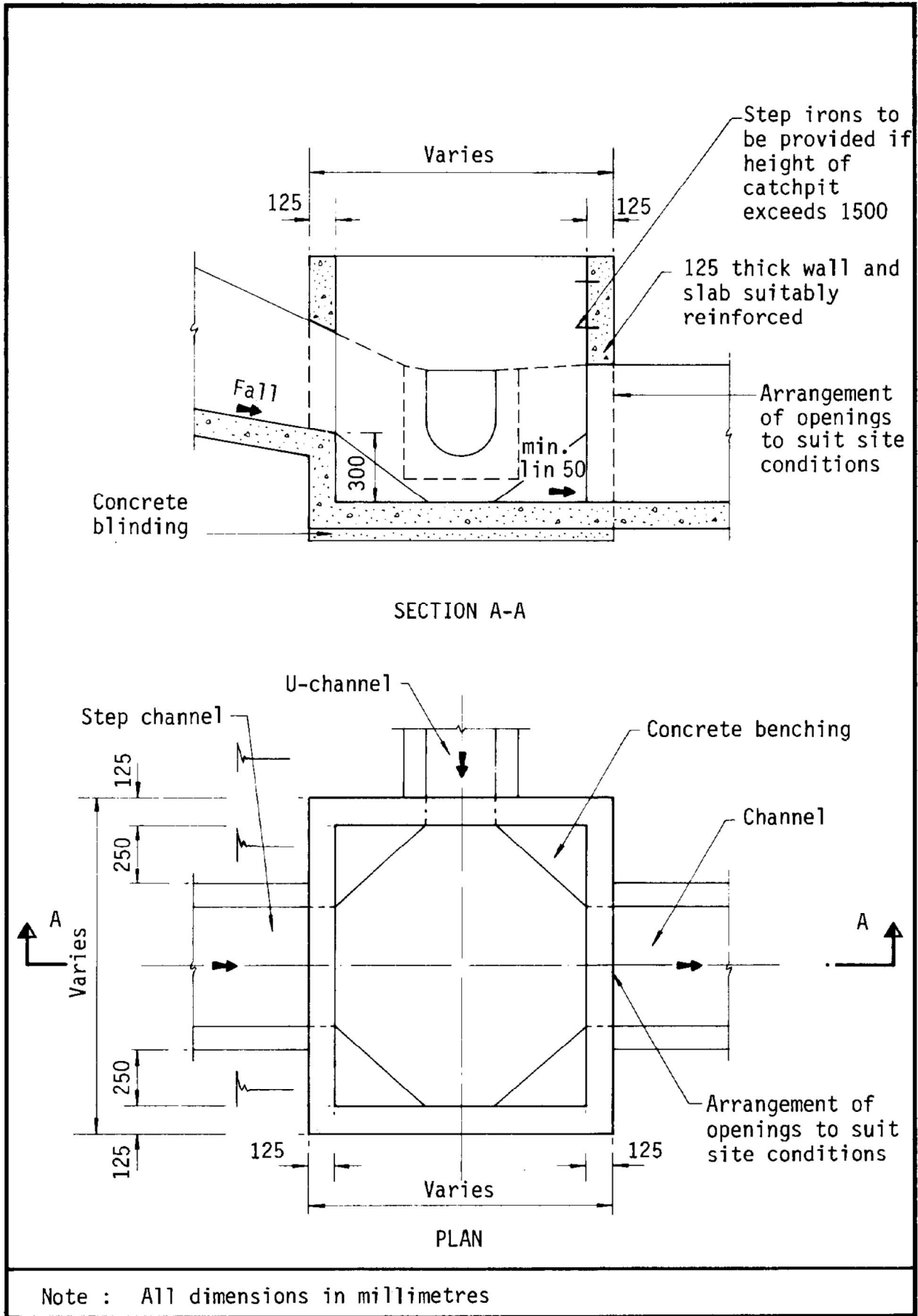
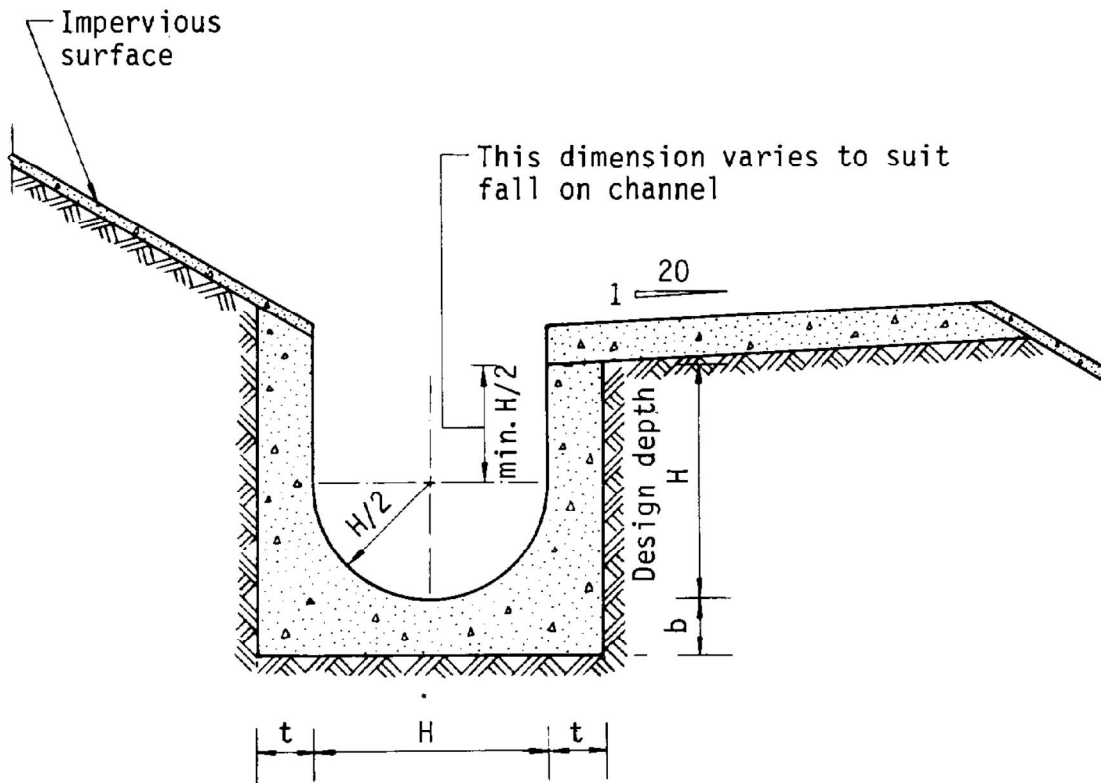


Figure 8.10 - Typical Details of Catchpits



Dimensions of U - channel

Nominal size of channel H (mm)	Thickness t (mm)	Thickness b (mm)
225 to 600	150	150
675 to 1200	175	225

Figure 8.11 - Typical U-channel Details