GoldRich PLANNERS & SURVEYORS LTD.
金 潤 規 劃 測 量 師 行 有 限 公 司

[redacted]

Your Ref.: A/YL-KTS/1084 Our Ref.: P25012/TL25361 27 October 2025

The Secretary
Town Planning Board
15/F., North Point Government Offices
333 Java Road, North Point, Hong Kong

By E-mail
tpbpd@pland.gov.hk

Dear Sir,

Submission of Further Information (FI)

Proposed Temporary Warehouse (excluding Dangerous Goods Godown) With Ancillary Office and Associated Filling of Land for a Period of 3 Years in "Agriculture" Zone, Lots 1012 S.B, 1012 S.C, 1013, 1014 RP, 1015 S.A, 1015 S.B, 1015 RP, 1016 (Part), 1018, 1034 (Part) and 1035 in D.D.113, Kam Tin, Yuen Long, New Territories (Application No. A/YL-KTS/1084)

We write to submit FI in response to departmental comment(s) conveyed by the Planning Department for the captioned application.

Yours faithfully,
For and on behalf of
Goldrich Planners & Surveyors Ltd.

Francis Lau

Encl.

c.c.
DPO/FSYLE, PlanD (Attn.: Ms. Anna TONG)

Further Information for Planning Application No. A/YL-KTS/1084

Response-to-Comments

Comments from the Chief Engineer/Mainland North, Drainage Services Department

Contact person: Mr. Kenneth CHAN (Tel.: 2300 1257)

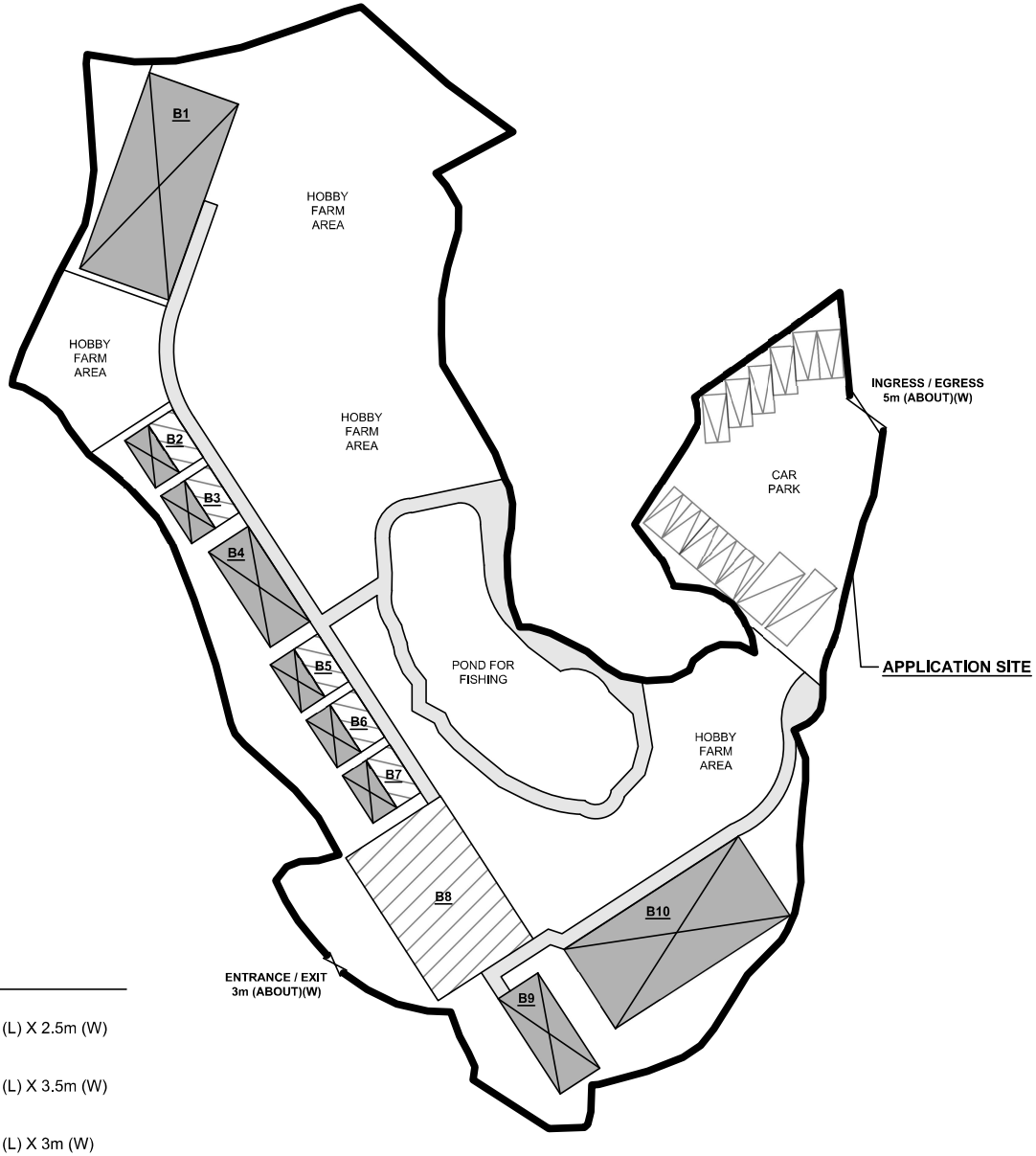
I.	Comments		Responses
1.	The proposed development site is encroached onto an existing water retention pond for collecting surface runoff from the vicinity during rainfall. The applicant is required to submit a Drainage Impact Assessment (DIA) instead of the drainage proposal to record the existing pond details and address the impacts caused by the proposed development. The drainage impacts should include, but not limited to, the existing natural streams, drainage systems, village drains and ditches in the adjacent areas etc. The DIA should also address how the impacts caused by the proposed development could properly be mitigated. Please refer to the DIA requirement details as stipulated in "DSD Advice Note No.1" for preparation of the assessment.		The Site is the subject of a previous application No. A/YL-KTS/1011 for 'Temporary Place of Recreation, Sports or Culture (Hobby Farm and Fishing Ground), Barbecue Site and Education Centre with Ancillary Eating Place' use, which was approved by the Town Planning Board on 28.2.2025. The pond within the Site is an artificial pond constructed by the applicant in 2024, which was intended for fishing for the hobby farm business (layout plan of the previous application No. A/YL-KTS/1011 is enclosed for reference). The pond is not intended for collecting surface runoff from the vicinity during rainfall. Aerial photos dated 27.2.2023 (before the pond is constructed) and 23.10.2024 (after the pond is constructed) are also enclosed for reference.
2.	Please refer to paragraph 6 of DSD Technical Note No.1 for sizing of the drainage channels.		The proposed sizing of the drainage channels is updated to 400mm UC which is able to collect all the overland flow and the runoff of the subject site. Please refer to the revised drainage proposal (Plan 6.1) and hydraulic calculations for details.
3.	Please take into account the rainfall increase due to climate change etc., as stated in the Stormwater Drainage Manual Corrigendum No. 1/2022 for the design calculation.		Noted. Please refer to the revised hydraulic calculations for details.

- END -

DEVELOPMENT PARAMETERS

APPLICATION SITE AREA	: 5,381 m^2	(ABOUT)
COVERED AREA	: 950 m^2	(ABOUT)
UNCOVERED AREA	: 4,431 m^2	(ABOUT)
PLOT RATIO	: 0.2	(ABOUT)
SITE COVERAGE	: 18 %	(ABOUT)
NO. OF STRUCTURE	: 10	
DOMESTIC GFA	: NOT APPLICABLE	
NON-DOMESTIC GFA	: 1,060 m^2	(ABOUT)
TOTAL GFA	: 1,060 m^2	(ABOUT)
BUILDING HEIGHT	: 3m - 6m	(ABOUT)
NO. OF STOREY	: 1 - 2	

STRUCTURE	USE	COVERED AREA	GFA	BUILDING HEIGHT
B1	AGRICULTURAL EDUCATION CENTRE	220m^2 (ABOUT)	220m^2 (ABOUT)	6m (ABOUT)(1-STOREY)
B2	AGR. ACTIVITIES ROOM WITH RAIN SHELTER	36m^2 (ABOUT)	36m^2 (ABOUT)	3m (ABOUT)(1-STOREY)
B3	AGR. ACTIVITIES ROOM WITH RAIN SHELTER	36m^2 (ABOUT)	36m^2 (ABOUT)	3m (ABOUT)(1-STOREY)
B4	REFRESHMENT KIOSK AND STORAGE OF TOOLS	55m^2 (ABOUT)	110m^2 (ABOUT)	6m (ABOUT)(2-STOREY)
B5	AGR. ACTIVITIES ROOM WITH RAIN SHELTER	36m^2 (ABOUT)	36m^2 (ABOUT)	3m (ABOUT)(1-STOREY)
B6	AGR. ACTIVITIES ROOM WITH RAIN SHELTER	36m^2 (ABOUT)	36m^2 (ABOUT)	3m (ABOUT)(1-STOREY)
B7	AGR. ACTIVITIES ROOM WITH RAIN SHELTER	36m^2 (ABOUT)	36m^2 (ABOUT)	3m (ABOUT)(1-STOREY)
B8	BARBEQUE AREA WITH RAIN SHELTER	220m^2 (ABOUT)	220m^2 (ABOUT)	6m (ABOUT)(1-STOREY)
B9	SITE OFFICE AND STORAGE OF TOOLS	55m^2 (ABOUT)	110m^2 (ABOUT)	6m (ABOUT)(2-STOREY)
B10	ANCILLARY EATING PLACE, WASHROOM AND CHANGING ROOM	220m^2 (ABOUT)	220m^2 (ABOUT)	6m (ABOUT)(1-STOREY)
	TOTAL	**950 m^2** (ABOUT)	**1,060 m^2** (ABOUT)	



PARKING AND LOADING/UNLOADING PROVISIONS

NO. OF PRIVATE CAR PARKING SPACE	: 11
DIMENSION OF PARKING SPACE	: 5m (L) X 2.5m (W)
NO. OF L/UL SPACE FOR LIGHT GOODS VEHICLE	: 1
DIMENSION OF L/UL SPACE	: 7m (L) X 3.5m (W)
NO. OF L/UL SPACE FOR LIGHT BUS	: 1
DIMENSION OF L/UL SPACE	: 8m (L) X 3m (W)

LEGEND

- APPLICATION SITE
- STRUCTURE (ENCLOSED)
- STRUCTURE (CANOPY)
- PARKING SPACE
- LOADING / UNLOADING SPACE
- INGRESS / EGRESS

NORTH
W E
S

PLANNING CONSULTANT
R-RICHES PROPERTY CONSULTANTS LIMITED

PROJECT
PROPOSED TEMPORARY PLACE OF RECREATION, SPORTS OR CULTURE (HOBBY FARM AND FISHING GROUND), BARBECUE SITE, EDUCATION CENTRE WITH ANCILLARY EATING PLACE FOR A PERIOD OF 3 YEARS AND LAND FILLING

SITE LOCATION
VARIOUS LOTS IN D.D. 113, KAM TIN, YUEN LONG, NEW TERRITORIES

SCALE
1 : 800 @ A4

DRAWN BY	DATE
MN	11.5.2022
REVISED BY	DATE
MN	21.7.2022
APPROVED BY	DATE

DWG. TITLE
LAYOUT PLAN

DWG NO.	VER.
Plan 1	001

Aerial photo dated 27.2.2023

Aerial photo dated 23.10.2024

N

Application Site

CP1 G.L. 19.4 I.L. 18.48
CP2 G.L. 19.4 I.L. 18.41
CP3 G.L. 19.3 I.L. 18.07
CP4 G.L. 19.2 I.L. 18.02
CP5 G.L. 19.2 I.L. 18.09
CP6 G.L. 19.4 I.L. 18.84
CP7 G.L. 19.4 I.L. 18.69
CP8 G.L. 19.4 I.L. 18.64
CP9 G.L. 19.3 I.L. 18.36
CP10 G.L. 19.3 I.L. 18.25
CP11 G.L. 19.2 I.L. 17.99

G.L. 19.4 I.L. 19.0
G.L. 19.4 I.L. 18.62
G.L. 19.2 I.L. 18.23
G.L. 19.2 I.L. 18.12

Proposed 400UC 1:200

Existing underground 600 pipe

Proposed 400 underground Pipe 1:200

A

A1

B

B1

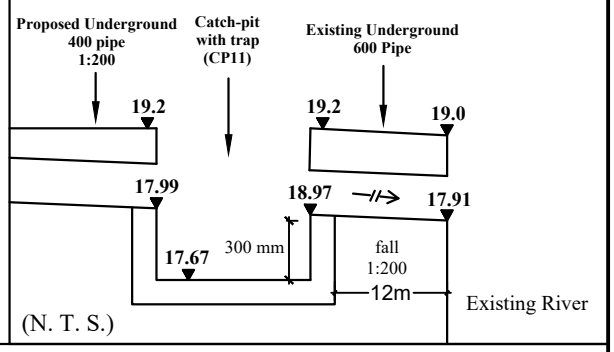
+19.5

Legend

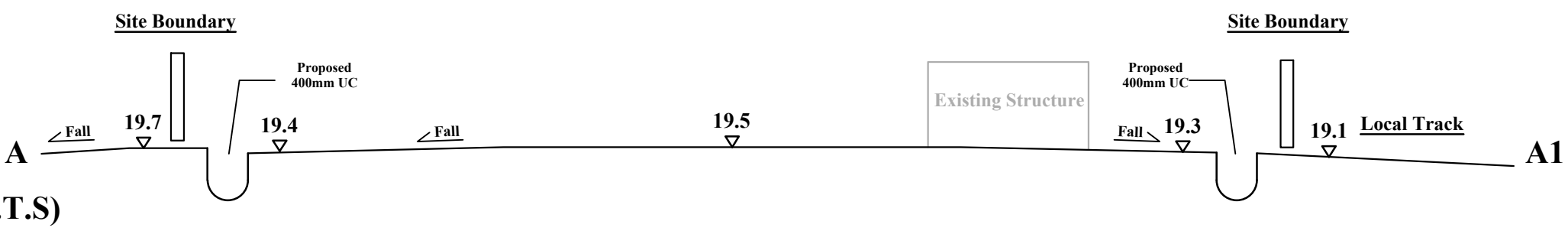
- Catch-pit
- Catch-pit with trap
- U-Channel
- Underground Pipe
- Vehicle Ingress / Egress

Site Area(about): 6,197m²

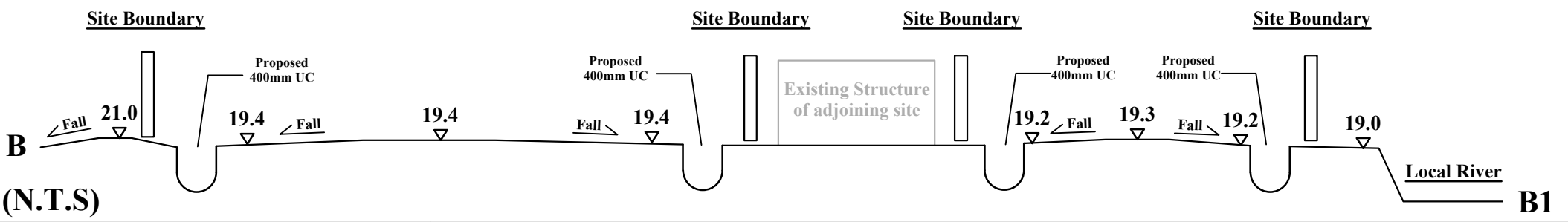
Connection details of proposed outfall between CP11 and existing river



(N. T. S.)



(N.T.S)



(N.T.S)

1:500 (A3)	**Drainage Proposal**	Goldrich Planners & Surveyors Ltd.
September 2025	Lots 1012 S.B, 1012 S.C, 1013, 1014RP, 1015 S.A, 1015 S.B, 1015 RP, 1016(part), 1018, 1034(part) and 1035 in D.D. 113 Yuen Long, N.T.	Plan 6.1 (P 25012)

1 For Catchment Area A

				Ref.
Area, A	=	2194	m^2	
Average slope, H	=	0.1	m per 100m	
Distance on the line of natural flow, L	=	22	m	
Time of concentraction, t_o	=	$0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (22) / (0.1^0.2*2194^0.1)		SDM 7.5.2 (d)
	=	2.3	min	

2 For Proposed UC in Catchment Area A

	From	To
Ground level (mPD)	19.40	19.20
Invert level (mPD)	18.62	18.02

Width of u-channel, w	=	400	mm	
Length of u-channel, L_c	=	119.4	m	
Depth of vertical part of u-channel, d	=	980	mm	
Gradient of u-channel, S_f	=	(18.62-18.02)/119.4 = 0.005		
Cross-Section Area, a	=	$0.5 \pi r^2 + w d$ = 0.5 x 3.14 x 200^2 + 400 x 980		
	=	0.455	m^2	
Wetted Perimeter, p	=	$\pi r + 2 d$ = 3.14 x 200 + 2 x 980		
	=	2.588	m	
Hydralic radius, R	=	a / p		SDM 8.2.1
	=	0.176	m	

3 Use Manning Equation for estimating velocity of stormwater

Take n	=	0.016	for concrete lined channels:-	SDM Table 13
Allowable velocity, v	=	$R^{1/6} x (RS_f)^{1/2}/n$ = (0.176)^1/6 x (0.176 x 0.005)^1/2 / 0.016		SDM Table 12
	=	1.39	m/s	
Time of flow, t_f	=	1.4	min	

4 Use "Rational Method" for calculation of design flow

Design intensity, i	=	$a / (t_o + t_f + b)^c$		SDM 4.3.2
	=	505.5 / (2.3+1.4+3.29)^0.355 for return period T = 50 years		Corrigendum 1/2024
	=	253		SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A	SDM 7.5.2 (b)
Flat Glassland(heavy soil)	0.25	0.0	0.0	
Concrete Paving	0.95	2194.0	2084.3	
		SUM =	2084.3	

Upstream flow, Q_u	=	0	m^3/s	
Design flow, Q_d	=	$1.16 \times 0.278i \Sigma C_i A_i + Q_u$ where A_i is in km^2		SDM 7.5.2 (a)
	=	1.16 x 0.278 x 253 x 2084.3 / 1000000 + 0		Corrigendum 1/2022
	=	0.170	m^3/s	
Allowable flow, Q_a	=	a x v		
	=	0.455 x 1.39		
	=	0.632	m^3/s	
	>	Q_d (O.K.)		

Reference was made to Stormwater Drainage Manual (SDM) by DSD

Scale: NA	**Hydraulic Calculation**	Goldrich Planners & Surveyors Ltd.
September 2025	Lots 1012 S.B, 1012 S.C, 1013, 1014 RP, 1015 S.A, 1015 S.B, 1015 RP, 1016 (Part), 1018, 1034 (Part) and 1035 in D.D.113, Kam Tin, Yuen Long, New Territories	Page 1 (P25012)

1 For Catchment Area B

Ref.

Area, A = 457 m^2
Average slope, H = 0.1 m per 100m
Distance on the line of natural flow, L = 12 m

Time of concentraction, t_o = $0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (12) / (0.1^0.2*457^0.1) — SDM 7.5.2 (d)
= 1.5 min

2 For Proposed UC in Catchment Area B

	From	To
Ground level (mPD)	19.20	19.20
Invert level (mPD)	18.23	18.02

Width of u-channel, w = 400 mm
Length of u-channel, L_c = 42.5 m
Depth of vertical part of u-channel, d = 980 mm
Gradient of u-channel, S_f = (18.23-18.02)/42.5 = 0.005

Cross-Section Area, a = $0.5 \pi r^2 + w d$ = 0.5 x 3.14 x 200^2 + 400 x 980
= 0.455 m^2
Wetted Perimeter, p = $\pi r + 2 d$ = 3.14 x 200 + 2 x 980
= 2.588 m
Hydralic radius, R = a / p — SDM 8.2.1
= 0.176 m

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:- — SDM Table 13
Allowable velocity, v = $R^{1/6} x (RS_f)^{1/2}/n$ = (0.176)^1/6 x (0.176 x 0.005)^1/2 / 0.016 — SDM Table 12
= 1.38 m/s
Time of flow, t_f = 0.5 min

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c$ — SDM 4.3.2
= 505.5 / (1.5+0.5+3.29)^0.355 for return period T = 50 years — Corrigendum 1/2024
= 280 — SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	457.0	434.2
		SUM =	434.2

— SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m^3/s

Design flow, Q_d = $0.278i \Sigma C_i A_i + Q_u$ where A_i is in km^2 — SDM 7.5.2 (a)
= 1.16 x 0.278 x 280 x 434.15 / 1000000 + 0 — Corrigendum 1/2022
= 0.039 m^3/s

Allowable flow, Q_a = a x v
= 0.455 x 1.38
= 0.627 m^3/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

1 For Connection between CP4 and CP11

Ref.

Area, A = **0** m^2
Average slope, H = **0.1** m per 100m
Distance on the line of natural flow, L = **0** m

Time of concentration, t_o = $0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (0) / (0.1^0.2*0^0.1) — SDM 7.5.2 (d)
= 0.0 min

2 For Proposed Pipe after CP4

Size(Diameter) w = **400** mm
Length of Pipe = **6** m
Design the pipe to 9/10 full bore capactity, then
Area of ventilated portion = 0.1 of pipe area
$½ r^2 \theta - ½ r^2 \sin(\theta) = 0.1 \pi r^2$
$\theta - \sin(\theta) = 0.2 \pi$
θ = 1.63 rad = 93.4^O (By trial and error)

Area A = $0.9 \pi r^2$
= 0.9 x 3.14 x 400^2 — SDM 8.2.1
= 452160 mm^2
= 0.452 m^2

Wetted Perimeter P = $2 \pi r - r \theta$ = 1861 mm
Hydralic radius R = A/P
242.9 mm

3 Use Manning Equation for estimating velocity of stormwater

Fall S = 1: **200**
Take n = 0.016 for concrete lined channels:- — SDM Table 13
Allowable velocity, v = $R^{1/6} x (RS_f)^{1/2}/n$ = (0.243)^1/6 * (0.243/200)^1/2 / 0.016 — SDM Table 12
= 1.72 m/s
Time of flow, t_f = 0.06 min

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c$ — SDM 4.3.2
= 505.5 / (0.0+0.06+3.29)^0.355 for return period T = 50 years — Corrigendum 1/2024
= 329 — SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A
Flat Glassland(heavy soil)	0.25	**0.0**	0.0
Concrete Paving	0.95	**0.0**	0.0
Macadam Roadways	0.425	**0.0**	0.0
Wooded Areas	0.105	**0.0**	0.0
		SUM =	0.0

SDM 7.5.2 (b)

Upstream flow, Q_u = **0.209** m^3/s

Design flow, Q_d = $0.278i \Sigma C_i A_i + Q_u$ where A_i is in km^2 — SDM 7.5.2 (a)
= 1.16 x 0.278 x 329 x 0 / 1000000 + 0.209 — Corrigendum 1/2022
= 0.209 m^3/s

Allowable flow, Q_a = a x v
= 0.45 x 1.72
= 0.778 m^3/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

1 For Catchment Area C

Ref.

Area, A = 3325 m^2
Average slope, H = 0.1 m per 100m
Distance on the line of natural flow, L = 25 m

Time of concentraction, t_o = $0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (25) / (0.1^0.2*3325^0.1) — SDM 7.5.2 (d)
= 2.5 min

2 For Proposed UC in Catchment Area C

	From	To
Ground level (mPD)	19.40	19.20
Invert level (mPD)	19.00	17.99

Width of u-channel, w = 400 mm
Length of u-channel, L_c = 203 m
Depth of vertical part of u-channel, d = 1010 mm
Gradient of u-channel, S_f = (19-17.99)/203 = 0.005

Cross-Section Area, a = $0.5 \pi r^2 + w d$ = 0.5 x 3.14 x 200^2 + 400 x 1010
= 0.467 m^2
Wetted Perimeter, p = $\pi r + 2 d$ = 3.14 x 200 + 2 x 1010
= 2.648 m
Hydralic radius, R = a / p — SDM 8.2.1
= 0.176 m

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:- — SDM Table 13
Allowable velocity, v = $R^{1/6} x (RS_f)^{1/2}/n$ = (0.176)^1/6 x (0.176 x 0.005)^1/2 / 0.016 — SDM Table 12
= 1.39 m/s
Time of flow, t_f = 2.4 min

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c$ — SDM 4.3.2
= 505.5 / (2.5+2.4+3.29)^0.355 for return period T = 50 years — Corrigendum 1/2024
= 239 — SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	3325.0	3158.8
		SUM =	3158.8

— SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m^3/s

Design flow, Q_d = $0.278i \Sigma C_i A_i + Q_u$ where A_i is in km^2 — SDM 7.5.2 (a)
= 1.16 x 0.278 x 239 x 3158.75 / 1000000 + 0 — Corrigendum 1/2022
= 0.243 m^3/s

Allowable flow, Q_a = a x v
= 0.467 x 1.39
= 0.647 m^3/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

1 For Catchment Area D

Ref.

Area, A = 221 m^2
Average slope, H = 0.1 m per 100m
Distance on the line of natural flow, L = 10 m

Time of concentraction, t_o = $0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (10) / (0.1^0.2*221^0.1) — SDM 7.5.2 (d)
= 1.3 min

2 For Proposed UC in Catchment Area D

	From	To
Ground level (mPD)	19.20	19.20
Invert level (mPD)	18.12	17.99

Width of u-channel, w = 400 mm
Length of u-channel, L_c = 25.2 m
Depth of vertical part of u-channel, d = 1010 mm
Gradient of u-channel, S_f = (18.12-17.99)/25.2 = 0.005

Cross-Section Area, a = $0.5 \pi r^2 + w d$ = 0.5 x 3.14 x 200^2 + 400 x 1010
= 0.467 m^2
Wetted Perimeter, p = $\pi r + 2 d$ = 3.14 x 200 + 2 x 1010
= 2.648 m
Hydralic radius, R = a / p — SDM 8.2.1
= 0.176 m

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:- — SDM Table 13
Allowable velocity, v = $R^{1/6} x (RS_f)^{1/2}/n$ = (0.176)^1/6 x (0.176 x 0.005)^1/2 / 0.016 — SDM Table 12
= 1.41 m/s
Time of flow, t_f = 0.3 min

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c$ — SDM 4.3.2
= 505.5 / (1.3+0.3+3.29)^0.355 for return period T = 50 years — Corrigendum 1/2024
= 287 — SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	221.0	210.0
		SUM =	210.0

SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m^3/s

Design flow, Q_d = $0.278i \Sigma C_i A_i + Q_u$ where A_i is in km^2 — SDM 7.5.2 (a)
= 1.16 x 0.278 x 287 x 209.95 / 1000000 + 0 — Corrigendum 1/2022
= 0.019 m^3/s

Allowable flow, Q_a = a x v
= 0.467 x 1.41
= 0.659 m^3/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

1 For Connection between CP11 to existing river

Ref.

Area, A = **0** m^2
Average slope, H = **0.1** m per 100m
Distance on the line of natural flow, L = **0** m

Time of concentraction, t_o = $0.14465L / (H^{0.2}A^{0.1})$ = 0.14465 (0) / (0.1^0.2*0^0.1) — SDM 7.5.2 (d)
= 0.0 min

2 For Pipe after Cp11

Size(Diameter) w = **600** mm
Length of Pipe = **12** m
Design the pipe to 9/10 full bore capactity, then
Area of ventilated portion = 0.1 of pipe area
$½ r^2 \theta - ½ r^2 \sin(\theta) = 0.1 \pi r^2$
$\theta - \sin(\theta) = 0.2 \pi$
θ = 1.63 rad = 93.4^O (By trial and error)

Area A = $0.9 \pi r^2$
= 0.9 x 3.14 x 600^2 — SDM 8.2.1
= 1017360 mm^2
= 1.017 m^2

Wetted Perimeter P = $2 \pi r - r \theta$ = 2792 mm
Hydralic radius R = A/P
364.4 mm

3 Use Manning Equation for estimating velocity of stormwater

Fall S = 1: **200**
Take n = 0.016 for concrete lined channels:- — SDM Table 13
Allowable velocity, v = $R^{1/6} x (RS_f)^{1/2}/n$ = (0.364)^1/6 * (0.364/200)^1/2 / 0.016 — SDM Table 12
= 2.25 m/s
Time of flow, t_f = 0.089 min

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_o + t_f + b)^c$ — SDM 4.3.2
= 505.5 / (0.0+0.09+3.29)^0.355 for return period T = 50 years — Corrigendum 1/2024
= 328 — SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m^2)	C x A
Flat Glassland(heavy soil)	0.25	**0.0**	0.0
Concrete Paving	0.95	**0.0**	0.0
Macadam Roadways	0.425	**0.0**	0.0
Wooded Areas	0.105	**0.0**	0.0
		SUM =	0.0

SDM 7.5.2 (b)

Upstream flow, Q_u = **0.471** m^3/s

Design flow, Q_d = $0.278i \Sigma C_f A_i + Q_u$ where A_i is in km^2 — SDM 7.5.2 (a)
= 1.16 x 0.278 x 328 x 0 / 1000000 + 0.471 — Corrigendum 1/2022
= 0.471 m^3/s

Allowable flow, Q_a = a x v
= 1.02 x 2.25
= 2.294 m^3/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD