

Our Ref. : DD 109 Lot 348 & VL Your Ref. : TPB/A/YL-KTS/1088



The Secretary,
Town Planning Board,
15/F, North Point Government Offices,
333 Java Road,
North Point, Hong Kong

By E-mail

14 November 2025

Dear Sir,

3rd Further Information

Proposed Temporary Shop and Services, Eating Place and Public Vehicle Park
(Excluding Container Vehicle) for a Period of 5 Years in "Residential (Group C)" Zone,
Lots 347 S.B (Part), 347 S.D (Part) and 348 (Part) in D.D. 109, Kam Tin, Yuen Long, New Territories

(S.16 Planning Application No. A/YL-KTS/1088)

We write to submit Further Information in response to departmental comments on the captioned application (**Appendix I**).

| Should you | require | more | information | regarding | the | application, | please | contact | our | Mr. |
|--------------------|-----------|--------|-------------|-----------|------|--------------|----------|----------|------|------|
| Danny NG at | | | | | or t | he undersign | ned at y | our conv | enie | nce. |
| Thank you for your | kind atte | ntion. | | | | | | | | |

Yours faithfully,

For and on behalf of R-riches Planning Limited

Christian CHIM
Town Planner

cc DPO/FSYLE, PlanD

(Attn.: Mr. Ajyum CHAN

(Attn.: Ms. Anna TONG

(Attn.: Mr. Thomas LAU

email: adchan@pland.gov.hk

email: akytong@pland.gov.hk

)

)

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Response-to-Comment (RtC)

Proposed Temporary Shop and Services, Eating Place and Public Vehicle Park (Excluding Container Vehicle) for a Period of 5 Years in "Residential (Group C)" Zone, Lots 347 S.B (Part), 347 S.D (Part) and 348 (Part) in D.D. 109, Kam Tin, Yuen Long, New Territories

(S.16 Planning Application No. A/YL-KTS/1088)

(i) A RtC table:

| | Departmental Comments | Applicant's Responses | | | | | |
|------|---|---|--|--|--|--|--|
| 1. (| Comments of the Chief Engineer/Mainland North, | Drainage Services Department (CE/MN, DSD) | | | | | |
| (a) | The size and gradient of the proposed/existing drainage facilities to be discharged from the site and/or the overland flow intercepted from the adjacent lands should be indicated in the drainage plan. The applicant should check and ensure the hydraulic capacity of the existing drainage facilities would not be adversely affected by the captioned development. | Noted. Drainage plan is updated and shown in Figure 3 of Annex I. The hydraulic capacity is shown in Appendix A for your perusal (Annex I). | | | | | |
| (b) | Please provide catchpit at the turning points of the channel. | Noted. | | | | | |
| (c) | Where walls or hoarding are erected or laid along the site boundary, adequate opening should be provided to intercept the existing overland flow passing through the site. | Noted. 100mm opening from ground level along wall/ hoarding or equivalent to be provided where it is erected. | | | | | |
| (d) | Standard details should be provided to indicate the sectional details of the proposed u-channel and the catchpit/sand trap. | Noted. Please find the standard details of channel in <i>Appendix C</i> for your perusal (Annex I). | | | | | |
| (e) | The development should neither obstruct overland flow nor adversely affect existing natural streams, village drains, ditches and the adjacent areas, etc. | Noted. | | | | | |
| (f) | The existing drainage channel of the proposed discharge point is not maintained by this Department. The applicant shall resolve any conflict/disagreement with relevant lot owner(s) and seek LandsD's permission for laying new | Noted. | | | | | |



S.16 Planning Application No. A/YL-KTS/1088

| | drains/channels and/or modifying/upgrading existing ones in other private lots or on Government land outside the application site. | |
|-----|--|--|
| (g) | Connection details at discharge point (including cross section) with C.L., I.L and catchpit/channel B.L. should be shown in the drawing. | Noted. Please refer to <i>Detail A</i> and <i>Photo A</i> in updated <i>Figure 3</i> for your perusal (Annex I). |
| (h) | Please include the existing planter details in section A-A. | Noted. Please refer to updated Figure 5 (Annex I). Please kindly note that the existing planter level is about the same level of existing paved area. Overland flow would not be obstructed by existing planter. |
| (i) | Colour photos to indicate the current conditions of the existing drainage-facilities should be included in the submission. The photos taken locations and angles should be shown on the layout plan. | Noted. Please refer to updated <i>Appendix D</i> (Annex I). |

(ii) Noting that the surface runoff is proposed to be discharged to Kam Tin River via a section of the existing drainage channel within a fenced-off area under MTRC management, the applicant has submitted an enquiry to seek comments from MTRC on 12.11.2025, which is still pending reply as of today.



Drainage Proposal

Oct 2025



Drainage Proposal

2

2

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Drainage Proposal

1 Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Lots 347 S.B (Part), 347 S.D (Part) and 348 (Part) in D.D. 109, Kam Tin, Yuen Long, New Territories (the Site) for 'Proposed Temporary Shop and Services, Eating Place and Public Vehicle Park (Excluding Container Vehicle) for a Period of 5 Years'.
- 1.1.2 This report aims to support the development in drainage aspect.

1.2 Application Site

- 1.2.1 The application site is situated beside Kat Hing Garden, south of Kam Sheung Road. It has an area of approx. 1,210 m². The site location is shown in **Figure 1**.
- 1.2.2 The existing site is mostly paved. The existing site levels are approx. + 5.50mPD. No major site formation works for the application site is anticipated.
- 1.2.3 There is an existing minimum 375mm channel by the west boundary of the application site. There is also an existing 525mm channel which would discharge to existing watercourse and eventually discharge to Kam Tin River. **Figure 2** indicate the existing drainage system of the area.

Drainage Proposal

2 Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 1,210 m². The existing planter at the west portion of the site would be maintained. The catchment plan is shown in **Figure 4**.

| Proposed Development Area (Approx.) | | | | | | | |
|--------------------------------------|-------|--|--|--|--|--|--|
| Total Site Area (m²) | 1,210 | | | | | | |
| Planter Area before Development (m²) | 223 | | | | | | |
| Planter Area after Development (m²) | 140 | | | | | | |
| Paved Area after Development (m²) | 1,070 | | | | | | |

Table 1 - Site Development Area

3 Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 2** below.

| Description | Design Return Periods |
|---|-----------------------|
| Intensively Used Agricultural Land | 2 – 5 Years |
| Village Drainage Including Internal Drainage System under a polder Scheme | 10 Years |
| Main Rural Catchment Drainage Channels | 50 Years |
| Urban Drainage Trunk System | 200 Years |
| Urban Drainage Branch System | 50 Years |

Table 2- Design Return Periods under SDM

3.1.2 The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 50 years return period is adopted for the drainage design.

Drainage Proposal

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Zone. Therefore, for 50 years return period, the following values are adopted.

(Corrigendum No.1/2024)

The development is proposed for temporary use for a period of 5 years. 11.1% rainfall increase due to climate change is considered.

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278 \text{CiA}$

where
$$Q_p$$
 = peak runoff in m³/s
 C = runoff coefficient (dimensionless)
 i = rainfall intensity in mm/hr
 A = catchment area in km²

- 3. The run-off coefficient (C) of surface runoff are taken as follows:
 - Paved Area: C = 0.95
 Unpaved Area: C = 0.35

Oct-25 3

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the pipe flow (m/s)

S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:
$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V = velocity of the pipe flow (m/s)

 S_f = hydraulic gradient k_f = roughness value (m)

v = kinematics viscosity of fluid

D = pipe diameter (m) R = hydraulic radius (m)

Drainage Proposal

4 Proposed Drainage System

4.1. Proposed Channels

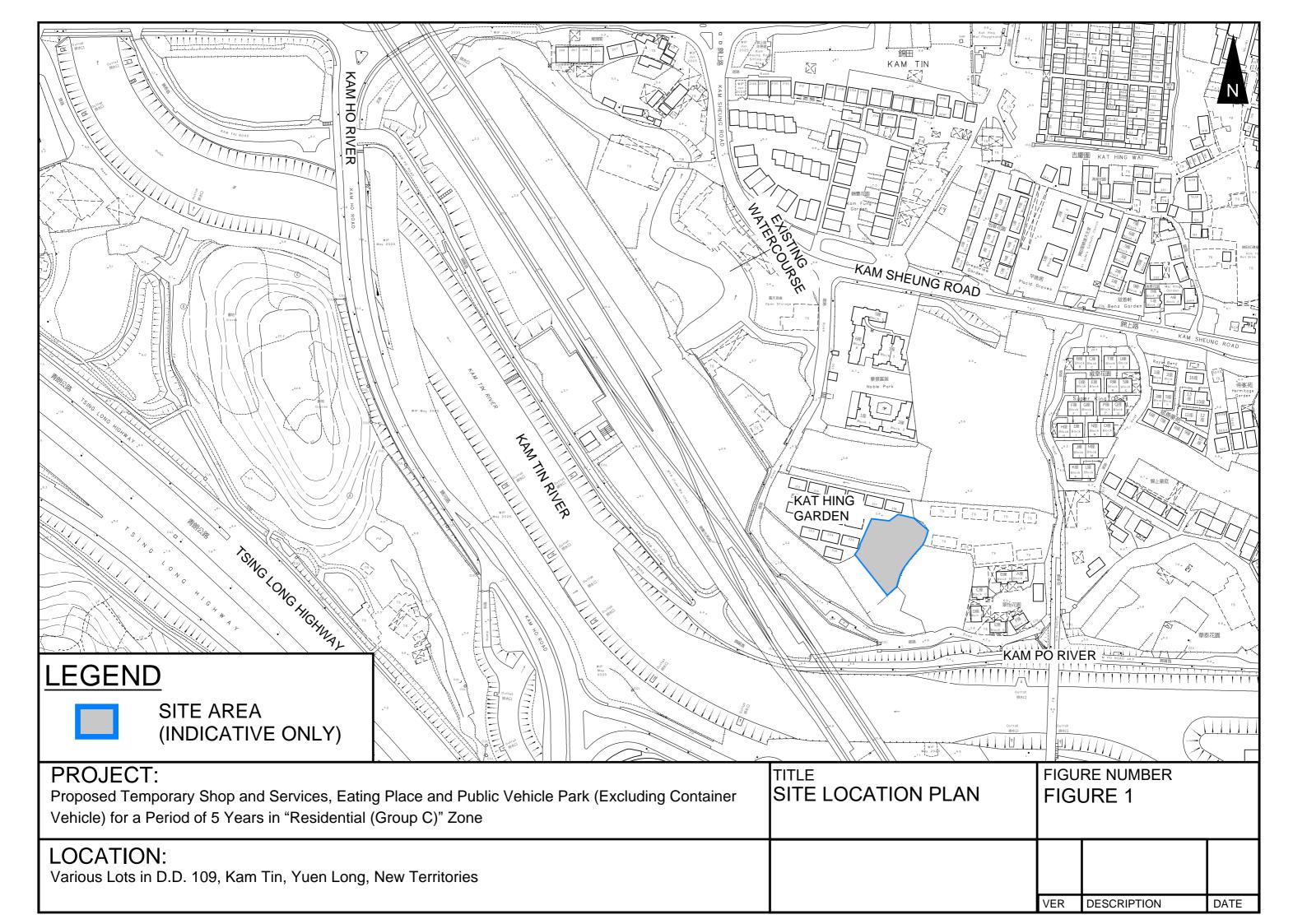
- 4.1.1 Proposed Channels are designed for collection of runoff for internal catchment. They are proposed to discharge to existing channel and watercourse which will eventually discharge to Kam Tin River. As the change in equivalent area is only 49.8 m², the increase of flow due to change of equivalent to existing water course is only 0.1%. The existing channels and watercourse checking are shown in **Appendix A**. They have enough capacity.
- 4.1.2 The design calculations of proposed UChannel are shown in **Appendix A**.
- 4.1.3 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4**.
- 4.1.5 A section of existing min. 375 mm channel downstream to CP1.03 to be reconstructed, if it cannot match with invert of CP1.03. Detail see **Figure 3**.
- 4.1.6 Reference Drawings are shown in **Appendix C** for reference.

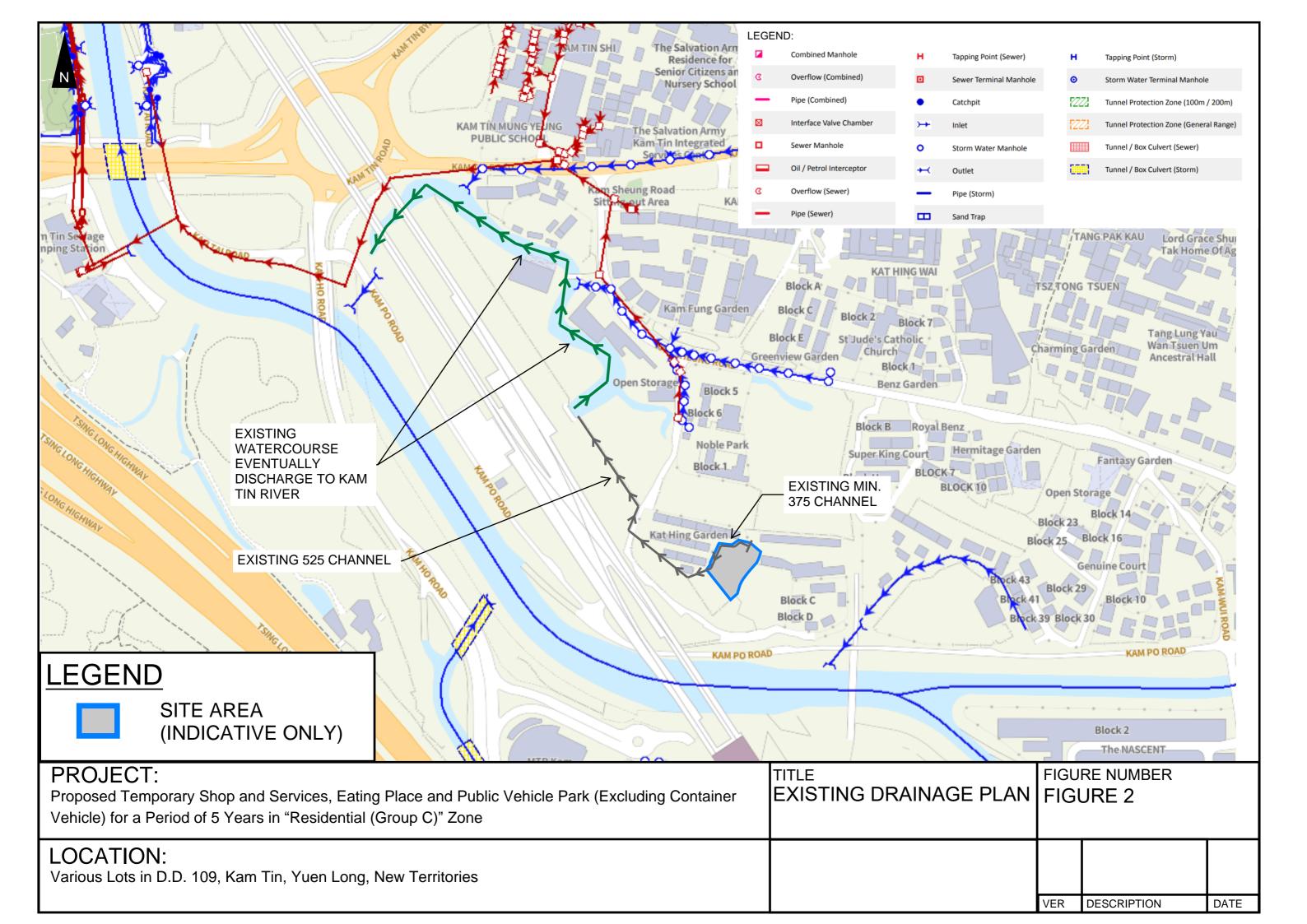
5 Conclusion

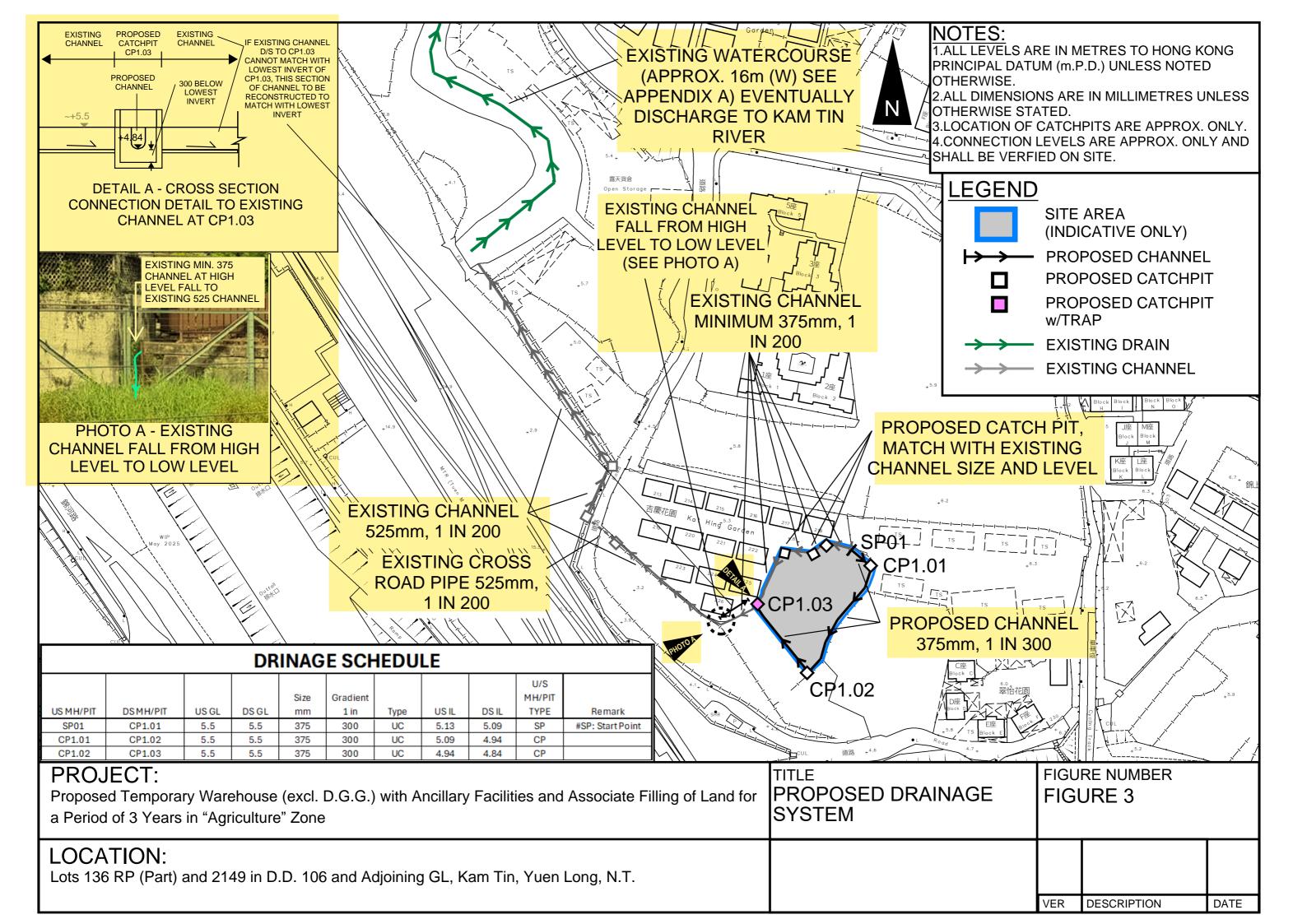
5.1.1 Drainage review has been conducted for the Proposed Development. The surface runoff will be collected by the proposed drains and discharged to existing drainage system. With implementation of the above drainage system, no unacceptable drainage impact is anticipated.

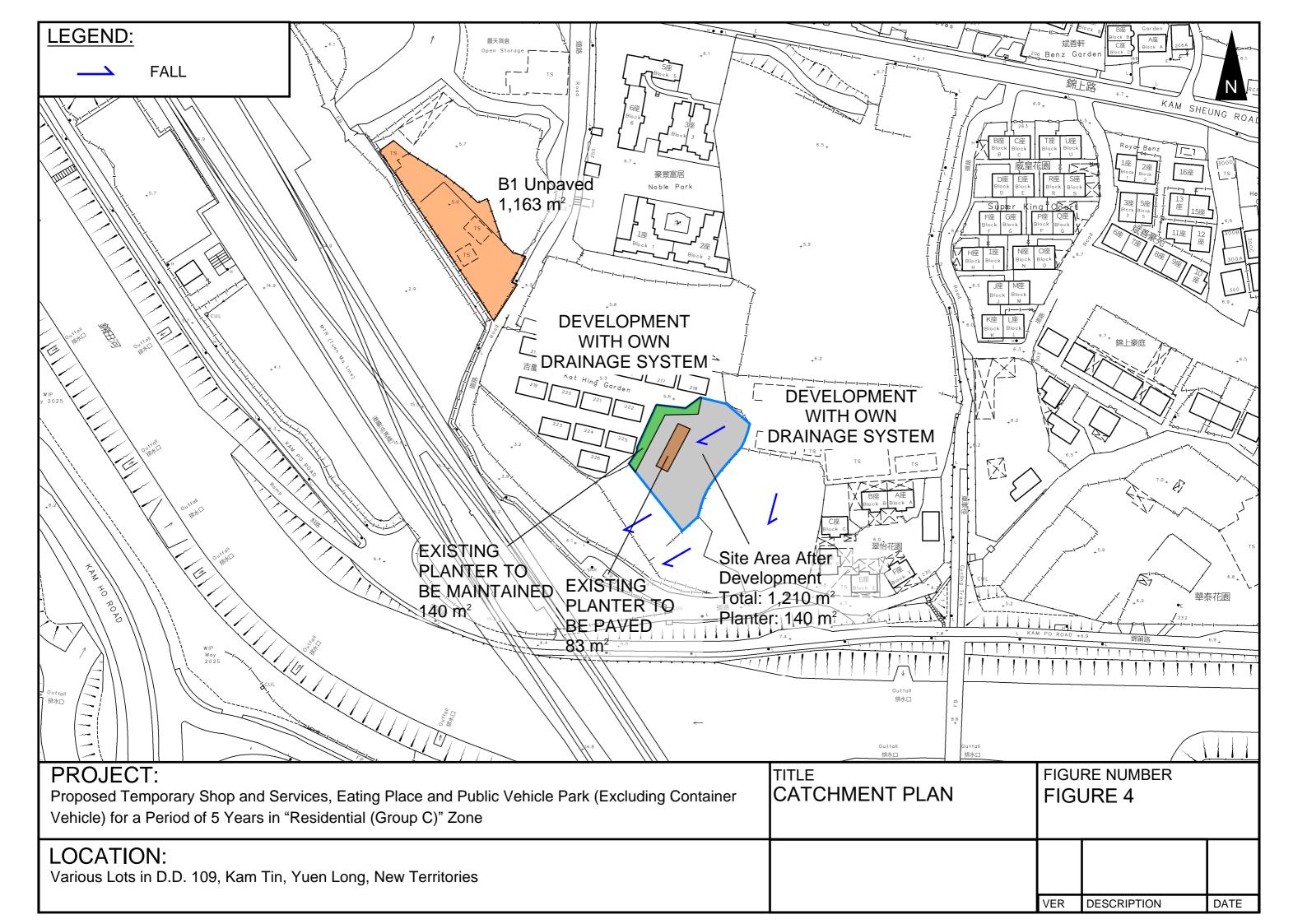
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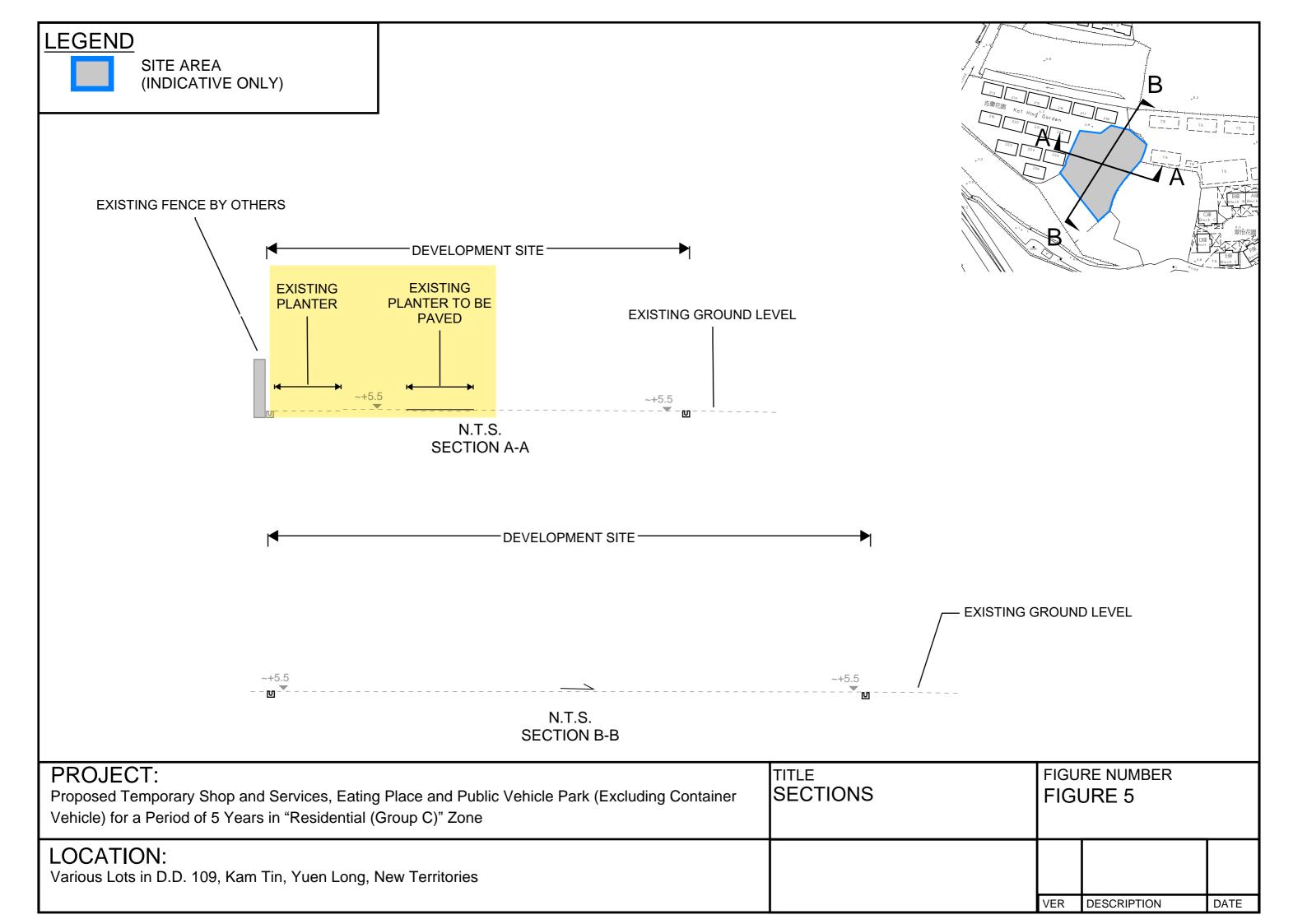
FIGURES











APPENDIX

Appendix A: Design Calculation

Zone

HKO

| Return Period | 1 in | 50 | years |
|---------------|------|----|-------|

| n | 0.014 |
|-----------|----------|
| Ks | 0.15 |
| Viscosity | 0.000001 |

| | НКО а | 505.5 |
|-------------------|-------|-------|
| Storm Constant | HKO b | 3.29 |
| | НКО с | 0.355 |

Catchment Area Table (Area in m²)

| | _ ` _ ′ _ | | | | | | | | | | | |
|-----------------|-----------|--------|---------------------------------|------------------------------|--|--|--|--|--|--|------|--|
| Catchment | A1 | B1 | Site Area Before Development | Change of Equivalent Area | | | | | | | | |
| Total Area | 1210 | 1163 | 1210 | - | | | | | | | | |
| Hard Paved Area | 1070 | 0 | 987 | - | | | | | | | | |
| Unpaved Area | 140 | 1163 | 223 | - | | | | | | | | |
| Equival. Area | 1065.5 | 407.05 | 1015.7 | 49.8 | | | | | | | | |

| Pavement Type | Hard Paved | Unpaved |
|--------------------|------------|---------|
| Runoff Coefficient | 0.95 | 0.35 |

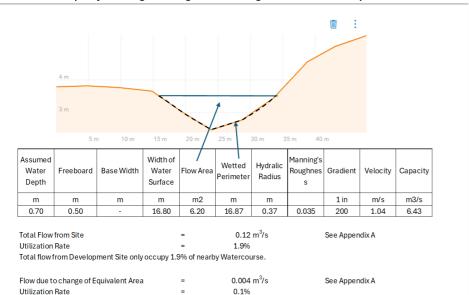
Calculation Table of Drainage System

| Calculation rable | oi Drainage System | | | | | | | | | | | | | | | | | | |
|------------------------|----------------------|-------|-------|------|----------|------|-------|-------|------------|--------|-------------------|----------|---------------------------|-----------------------|------|-----------|-------------------|--------------|--------|
| US MH/PIT | DS MH/PIT | US GL | DS GL | Size | Gradient | Туре | US IL | DS IL | U/S MH/PIT | Length | V | Capacity | Catchments | Total Equivalent Area | ToC | Intensity | Total | Utilitizatio | Remark |
| | | | | mm | 1 in | | | | TYPE | m | m/s ^{##} | m³/s | | m ² | min | mm/hr ## | Discharge m³/s | n | |
| SP01 | CP1.01 | 5.50 | 5.50 | 375 | 300 | UC | 5.13 | 5.09 | SP | 9.5 | 1.06 | 0.13 | A1 | 1065.50 | 0.40 | 353 | 0.10 | 78.7% | |
| CP1.01 | CP1.02 | 5.50 | 5.50 | 375 | 300 | UC | 5.09 | 4.94 | CP | 45 | 1.06 | 0.13 | A1 | 1065.50 | 0.55 | 348 | 0.10 | 77.6% | |
| CP1.02 | CP1.03 | 5.50 | 5.50 | 375 | 300 | UC | 4.94 | 4.84 | CP | 29.9 | 1.06 | 0.13 | A1 | 1065.50 | 1.26 | 328 | 0.10 | 73.1% | |
| CP1.03 | Existing 525 channel | | | 525 | 100 | UC | | | | | 2.30 | 0.57 | A1,B1 | 1472.55 | 1.73 | 317 | 0.13 | 22.9% | |
| Checking of Existing I | min. 375 channel | | | 375 | 200 | UC | | | | | 1.30 | 0.16 | A1 | 1065.50 | 0.40 | 353 | 0.10 | 64.2% | |
| Checking of Existing S | 525 cross road pipe | | | 525 | 200 | PIPE | | | | | 1.82 | 0.39 | A1,B1 | 1472.55 | 1.73 | 317 | 0.13 | 32.9% | |
| Flow due to change | of | | | | | | | | | | | | Change of Equivalent Area | 49.80 | 1.73 | 317 | 0.004 | | |

Equivalent Area

: With 11.1% rainfall increase as per Table 28 of SDM Corrigendum No. 1/2022.

Capacity Checking of Existing Watercourse against Flow from Development



Flow due to change of equivalent area $\,$ only occupy 0.1% of nearby Watercourse.

Time of Concentration Checking

| Catchment | Flow Distance | Highest Level | LOWEST LOVE | Gradient (per 100m) = (H1-H2)/L x 100 | to (min) = 0.14465L/ (H ^{0.2} A ^{0.1}) | tc = to + tf |
|-----------|---------------|---------------|-------------|--|--|-----------------|
| Α | L | H1 | H2 | | | |
| (m2) | (m) | (mPD) | (mPD) | | (min) | (min) |
| 76 | 13.1 | 72 | 15 | 435.115 | 0.4 | 0.4 |



-APPENDIX B - PROPOSED SITE LAYOUT PLAN

| DEVELOPMENT PARAMETER | s | | STRUCTO | JRE USE | | COVERED AREA | | BUILDING HEIGHT |
|---|--|-------------------------------|---------|--|--|---|--|------------------------------|
| APPLICATION SITE AREA COVERED AREA UNCOVERED AREA | : 1,210 m ² : 158 m ² : 1,052 m ² | (ABOUT) (ABOUT) (ABOUT) |) B1 | SHOP AND SERVICE SHOP AND SERVICE | | 100 m ² (ABOUT) 58 m ² (ABOUT) | 192 m² (ABOUT)* | 7.2 m (ABOUT 7.2 m (ABOUT |
| PLOT RATIO SITE COVERAGE | : 0.25 : 13 % | (ABOUT) (ABOUT) | | | TOTAL | <u>158 m²</u> (ABOUT) | <u>304 m²</u> (ABOUT) | |
| NO. OF STRUCTURE DOMESTIC GFA NON-DOMESTIC GFA TOTAL GFA | : 2 : NOT APPLIO : 304 m ² : 304 m ² | , | • | GFA OF 1/F GFA OF G/F COVERED AREA OF STAIRG TOTAL GI | (ABOUT) (ABOUT) (ABOUT) (ABOUT) | # | GFA OF 1/F GFA OF G/F COVERED AREA OF STAIRCA TOTAL GFA | _ |

BUILDING HEIGHT

DIMENSIONS OF PARKING SPACE

DIMENSIONS OF PARKING SPACE

NO. OF STOREY



R-RICHES
PLANNING LIMITED

CONTAINER

PROPOSED TEMPORARY SHOP AND SERVICES, EATING PLACE AND PUBLIC VEHICLE PARK

VEHICLE) FOR A PERIOD OF 5

VARIOUS LOTS IN D.D. 109, KAM TIN, YUEN LONG, NEW TERRITORIES

18.7.2025

(EXCLUDING

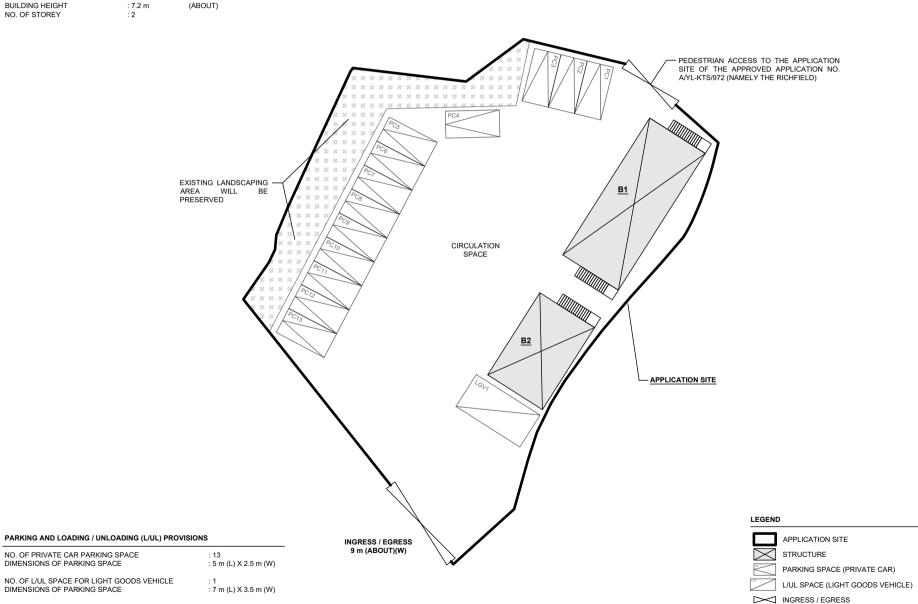
YEARS

1:350 @ A4

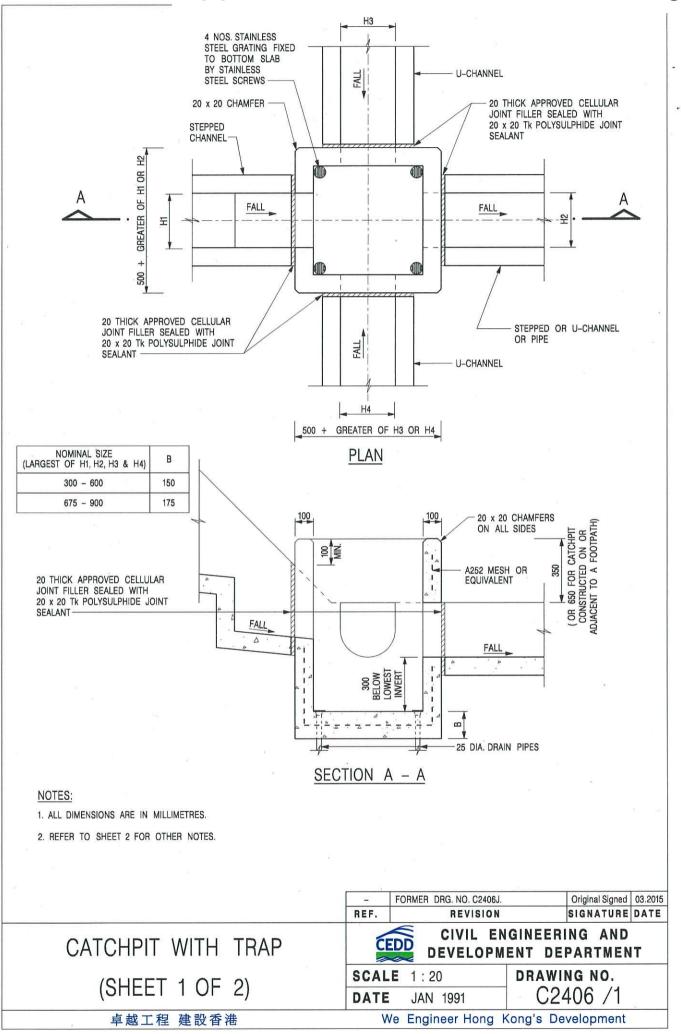
LAYOUT PLAN

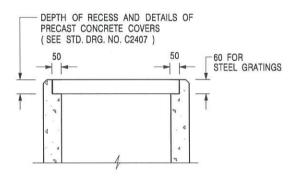
CHECKED B

PLAN 4



Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

| REF. | REVISION | SIGNATURE | DATE |
|------|-------------------------|-----------------|---------|
| - | FORMER DRG. NO. C2406J. | Original Signed | 03.2015 |
| Α | MINOR AMENDMENT. | Original Signed | 04.2016 |

CATCHPIT WITH TRAP (SHEET 2 OF 2)

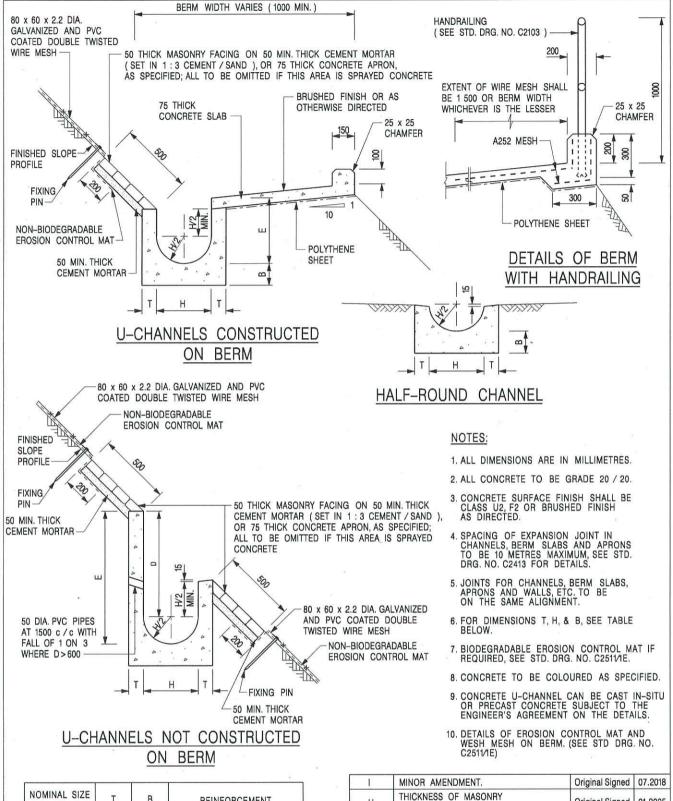


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 **DATE** JAN 1991

drawing no. C2406 /2A

卓越工程 建設香港



| NOMINAL SIZE H | T | В | REINFORCEMENT |
|-------------------|-----|-----|--------------------------------------|
| 300 | 80 | 100 | A252 MESH PLACED CENTRALLY AND T=100 |
| 375 - 600 | 100 | 150 | WHEN E>650 |
| 675 - 900 | 125 | 175 | A252 MESH PLACED CENTRALLY |

| REF. | REVISION | SIGNATURE | DATE |
|------|--------------------------------------|-----------------|---------|
| В | MINOR AMENDMENTS. | Original Signed | 3.94 |
| С | 150 x 100 UPSTAND ADDED AT BERM. | Original Signed | 6.99 |
| D | MINOR AMENDMENT. | Original Signed | 08.2001 |
| E | DRAWING TITLE AMENDED. | Original Signed | 11.2001 |
| F | GENERAL REVISION. | Original Signed | 12.2002 |
| G | MINOR AMENDMENT. | Original Signed | 01.2004 |
| Н | THICKNESS OF MASONRY FACING AMENDED. | Original Signed | 01.2005 |
| 1 | MINOR AMENDMENT. | Original Signed | 07.2018 |

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

卓越工程 建設香港

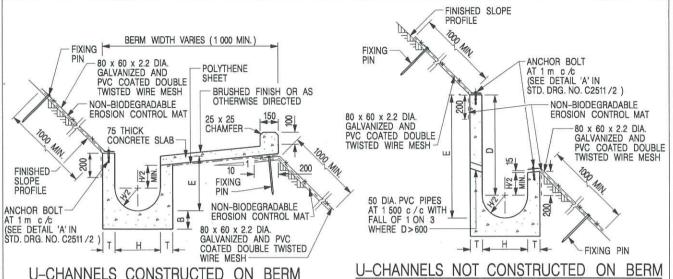
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:25

DATE JAN 1991

C2409l



U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

BIODEGRADABLE

EROSION CONTROL MAT

07.2018

12.2017

01.2005

12.2002

08 2001

6.99

3.94

10.92

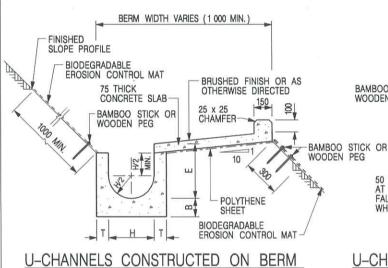
DATE

Original Signed

SIGNATURE

FINISHED SLOPE PROFILE

ш



WITH BIODEGRADABLE

EROSION CONTROL MAT

BAMBOO STICK OR WOODEN PEG

U-CHANNELS NOT CONSTRUCTED ON BERM

WITH BIODEGRADABLE

EROSION CONTROL MAT

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE TO BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- 5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- 6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- 7. FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- 8. MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- 10. THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

| NOMINAL SIZE H | Ţ | В | REINFORCEMENT |
|-------------------|-----|-----|-----------------------------------|
| 300 | 80 | 100 | A252 MESH PLACED |
| 375 - 600 | 100 | 150 | CENTRALLY AND T=100 WHEN E>650 |
| 675 - 900 | 125 | 175 | A252 MESH PLACED CENTRALLY |

| | DETAILS | OF | HALF-I | ROUN | D AND | |
|---|---------|------|--------|------|--------|----|
| | U-CHAN | NELS | S (TYP | ЕВ- | - WITH | |
| I | FROSION | COI | NTROI | MAT | APRON | 1) |

| 6 |
|------|
| CEDD |
| CEDD |
| nac |

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D

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В

A

REF.

BAMBOO STICK OR WOODEN PEG

50 DIA. PVC PIPES AT 1 500 c/c WITH FALL OF 1 ON 3

WHERE D>600

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC
DATE JAN 1991

MINOR AMENDMENT.

MINOR AMENDMENT

GENERAL REVISION.

MINOR AMENDMENT.

MINOR AMENDMENT.

MINOR AMENDMENT

FIXING DETAILS OF BIODEGRADABLE

150 x 100 UPSTAND ADDED AT BERM

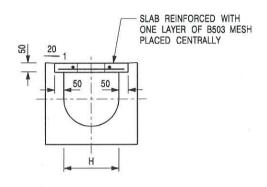
REVISION

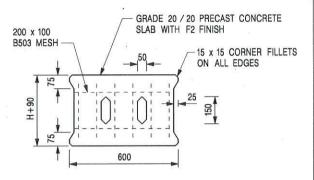
EROSION CONTROL MAT ADDED.

DIMENSION TABLE AMENDED

C2410

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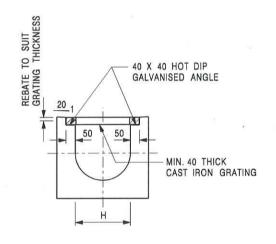


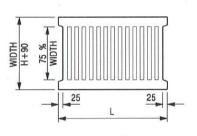
<u>PLAN OF SLAB</u>

TYPICAL SECTION

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)





L = 600mm FOR $H \le 375$ mm L = 400mm FOR H > 375mm

TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

| REF. | REVISION | SIGNATURE | DATE |
|------|--------------------------------|-----------------|---------|
| Α | CAST IRON GRATING AMENDED. | Original Signed | |
| В | NAME OF DEPARTMENT AMENDED. | Original Signed | 01.2005 |
| С | MINOR AMENDMENT. NOTE 3 ADDED. | Original Signed | |
| D | NOTE 4 ADDED. | Original Signed | |
| Ε | NOTES 3 & 4 AMENDED. | Original Signed | |

COVER SLAB AND CAST IRON GRATING FOR CHANNELS



CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:20
 DRAWING NO.

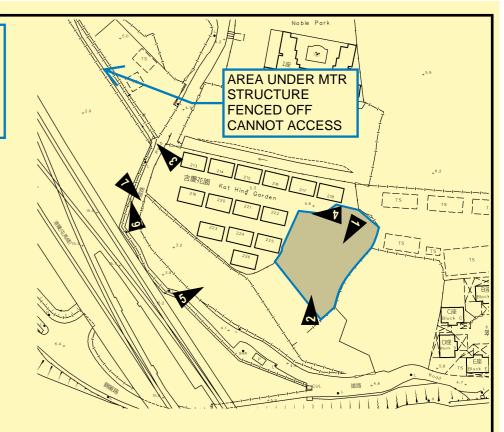
 DATE JAN 1991
 C2412E

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EXISTING MIN. 375
CHANNEL AT HIGH
LEVEL FALL TO
EXISTING 525 CHANNEL
SEE PHOTO A IN
FIGURE 3













PROJECT:

Proposed Temporary Shop and Services, Eating Place and Public Vehicle Park (Excluding Container Vehicle) for a Period of 5 Years in "Residential (Group C)" Zone

LOCATION:

Various Lots in D.D. 109, Kam Tin, Yuen Long, New Territories

SITE PHOTOS APPENDIX D

VER DESCRIPTION DATE