☐Urgent ☐Return receipt ☐Expand Group ☐Restricted ☐Prevent Copy ☐Confidential

From: [redacted]
Sent: Thursday, February 5, 2026 8:07 PM
To: tpbpd/PLAND <tpbpd@pland.gov.hk>
Cc: [redacted]
Subject: A/YL-PH/1075 進一步資料

敬啟者

此電郵取代今日 17:35 及 19:05 發出的電郵。

就上述檔案，現提交進一步資料。

規劃署

營運方面，場地營運時間為星期一至星期日及公眾假期，上午八時至下午六時。場地除上述時間之外，不會進行任何活動或夜間活動。

車位方面，申請地點設有 2 個私家車泊車位及 3 個小巴泊車位，2 個私家車泊車位，是供員工及訪客使用。小巴泊車位是指小型旅遊巴的泊車位，小巴泊車位是我們提供的小巴接送服務的專用車位，面積為 8 米 x 3 米，而每輛小型旅遊巴的實際尺寸是 7 米 x 2.5 米（可參考以下照片），因此小型旅遊巴是適合停泊於此車位。另外，小巴接送服務只會於舉辦婚禮活動時才會有。

交通需求方面，場地的確一日最多可容納 200 人，但我們主要舉辦的是小型婚禮，每場婚禮人數約有 100 人。若以 200 人來計算，且他們都不乘搭交通工具來計算的話，小巴載客量約 23-28 人，即每輛小巴最大載客量為 28 人，場地有 3 個小巴泊車位及 2 個私家車泊車位，三輛小巴載客量已是 84 人，走兩程的載客量為 168 人。期餘 32 人，可讓其中一輛小巴再走多一程，剩下 4 人可讓私家車接送。

交通安排方面，申請地點設有 2 個私家車泊車位及 3 個小巴泊車位是供客人及員工使用，小巴泊車位方面，是我們會提供小巴接送服務，小巴載客量約 23-28 人。此外，客人亦可自行乘搭公共交通工具到達申請地點，例如巴士。 接送服務方面，錦上路西鐵站停車場為上下車地點，以便客人及司機。班次方面，08:00 - 08:30 會有一班車由停車場開出，09:00 - 09:30 會有兩班車由停車場開出。回程方面，15:30 - 16:00 會有一班車由申請場地開出，16:30 - 17:00 會有兩班車由申請場地開出。因人數、車輛數目、班次的限制，所有乘客必須提早向場地預約，表示需要此項服務。任何遲到早走人士需自行乘搭交通工具。

電力供應方面，場地的太陽發電裝置均傳送電力給中電，而非申請場地。場地的電力供應則由電錶提供。另外，場地太陽能板的支架高約 1 米。

填土方面，填土總面積約 5446 平方米，填土厚度約 0.2 米，填土物料為混凝土。由 38.9mPD 填高至 39.1mPD。

補給物資方面，通常包括兩個主要類別，首先是裝飾物資，這些包括花卉佈置（如鮮花和假花）、桌布、椅套以及裝飾帶等，這些元素能夠為婚禮增添美感和氛圍。此外，燈飾等照明設備也常被使用，以營造浪漫的環境。其次是餐飲物資，場地提供代客預訂到會服務，需準備好餐具，如盤子、刀叉和杯子等。

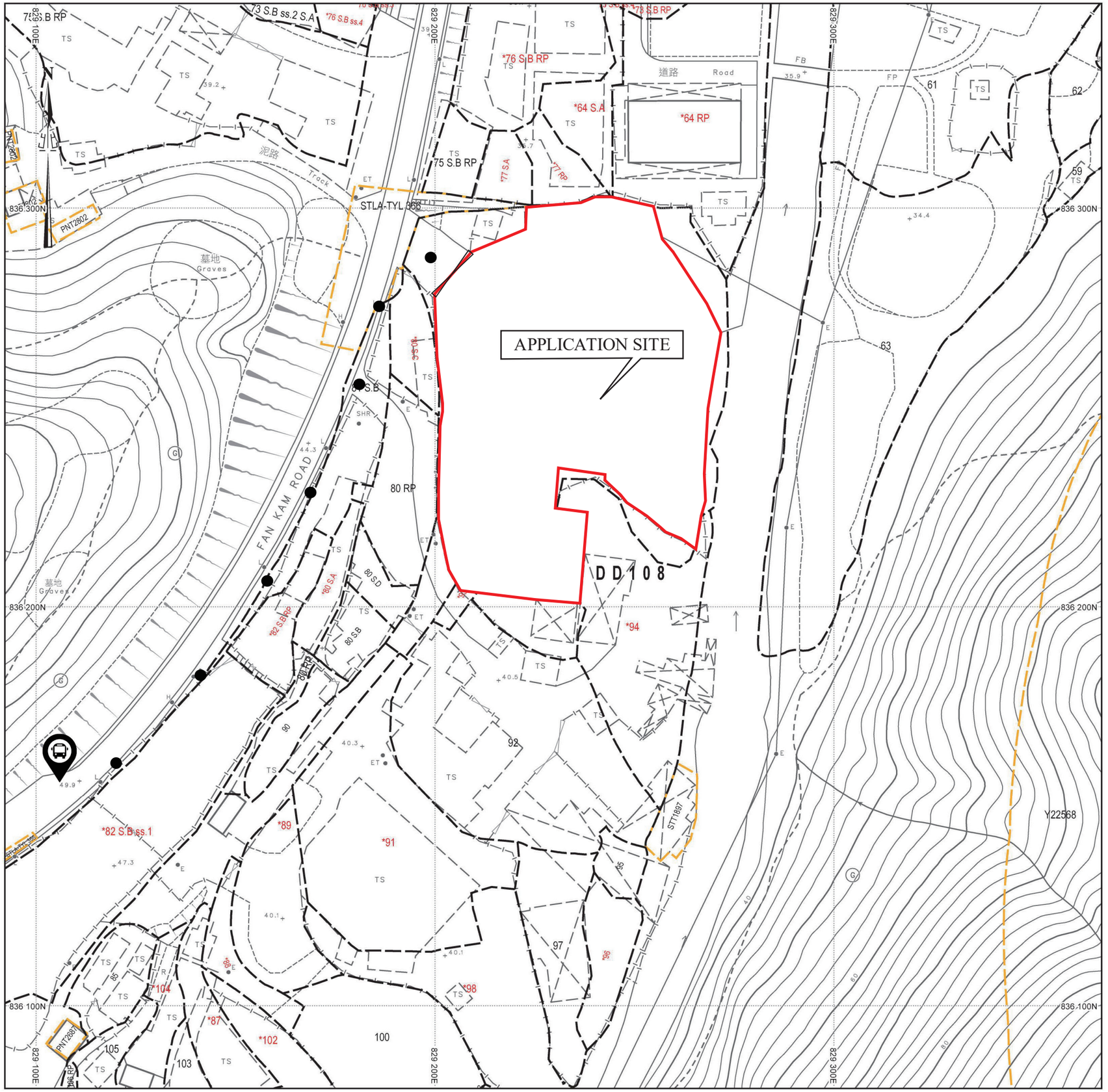
音響方面，場地有室外及室內的部分，考慮到音質、方便性、耐用性，團隊會使用 JBL PartyBox 300 ，尺寸約 12.6 x 11.3 x 24.5 吋 ，聲量會控制至約 55-60 分貝，避免滋擾距離約 290 米打石湖村的居民。

<u>運輸署</u>
以下附件為交通運輸圖。

<u>渠務署</u>
以下附件為渠務建議。

<u>消防處</u>
申請人希望消防建議能納入附帶條件。

交通運輸圖



打石湖村站巴士站

出入口(14米)

行人路線

SCALE 1 : 1000

SUBMISSION REPORT
FOR
DRAINAGE PROPOSAL DESIGN
FOR
TEMPORARY ACTIVITY VENUE AND PUBLIC UTILITY INSTALLATION (SOLAR PHOTOVOLTAIC SYSTEM) AND ASSOCIATED FILLING OF LAND
ON
LOT 78s.A(Part), 93(Part), 94(Part) IN D.D.108
PAT HEUNG, YUEN LONG

Date : November 2025

TABLE OF CONTENTS

APPENDIX

REFERENCES

1. Stormwater Drainage Manual, Planning Design and Management by DSD
2. Geotechnical Manual for Slopes by GEO
3. Standard Drawings by DSD

1. Introduction

This proposal is prepared for the proposed stormwater drainage works for the temporary activity venue and public utility installation (solar photovoltaic system) and associated filling land for a period of three years in "Residential (Group D)" Zone at lot 78s.A(Part), 93(Part), 94(Part) in D.D.108, Pat Heung, Yuen Long.

2. Existing Drainage Condition

A plan showing the existing catchments are enclosed in **Appendix B**. Currently, the surface runoff collected from the site is discharging to existing stream through the existing 525mm dia. pipe as shown in **Appendix A**. As per the existing site condition, an additional peripheral U-channels area is considered necessary for the proposed development. A drainage proposal is required to be carried out for the proposed development.

3. Design Parameters & Assumptions

The design criteria to be used for the modeling assessment are based on the standards set out in the Stormwater Drainage Manual, Third Edition (SDM). According to Section 6.6.1 of the SDM, the existing village drainage system in the vicinity of the development is classified as main rural catchment drainage system. Table 10 of the SDM recommends to be adopted a 50 year design return period storm event for the urban drainage branch system.

<u>Stormwater Runoff (Q)</u>

The rate of stormwater runoff used in this assessment report is estimated by the "Rational method" in which the peak runoff is calculated from the formula:

$$Q = K \times i \times A / 3600$$

where Q = maximum runoff (L/s)

i = design mean intensity of rainfall (mm/hr)

A = area of catchment (m^2)

K = runoff coefficient

Time of Concentration (tc)

The time of concentration is defined as the time required for stormwater runoff to flow from the most remote part of the catchment area to the point in the drainage system under consideration. Based on the assumptions adopted in the Rational Method, this is the time taken for the peak runoff to become established at the considered section.

The time of concentration comprises the time for water flowing within natural catchments and along the man-made drainage pipes/channels. For natural catchments, the time of concentration is estimated by the modified form of the Brandsby William's equation.

$$t_o = \frac{0.14465L}{H^{0.2} A^{0.1}}$$

Where t_o = time of concentration of a natural catchment (min.)

A = catchment area (m^2)

H = average slope (m per 100m), measured along the line of natural flow, from the summit of the catchment to the point under consideration

L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m)

Mean Rainfall Intensity (i)

Mean rainfall intensity-duration curves attached in this report are based on the Statistical analysis of long term rainfall records from the Hong Kong Observatory. A return period of 50 years is adopted.

Runoff Coefficient (K)

The value of K is taken as 0.95 for developed area. For vegetated ground, the value of K is taken as 0.3.

4. Proposed Stormwater Drainage

The proposed stormwater drainage works include surface U-channels at the peripheral of the site collecting the runoff from catchments within the site. The U-channels will connect and discharge the surface runoff to the existing stream through the existing 525mm dia. pipe. Catchpits with 300mm sump are proposed at the discharged points of proposed U-Channel to desilt the surface water before discharging to the drainage outside. The proposed stormwater drainage layout plan is shown in **Appendix A**.

5. Effect on Drainage Characteristics and Potential Drainage Impact

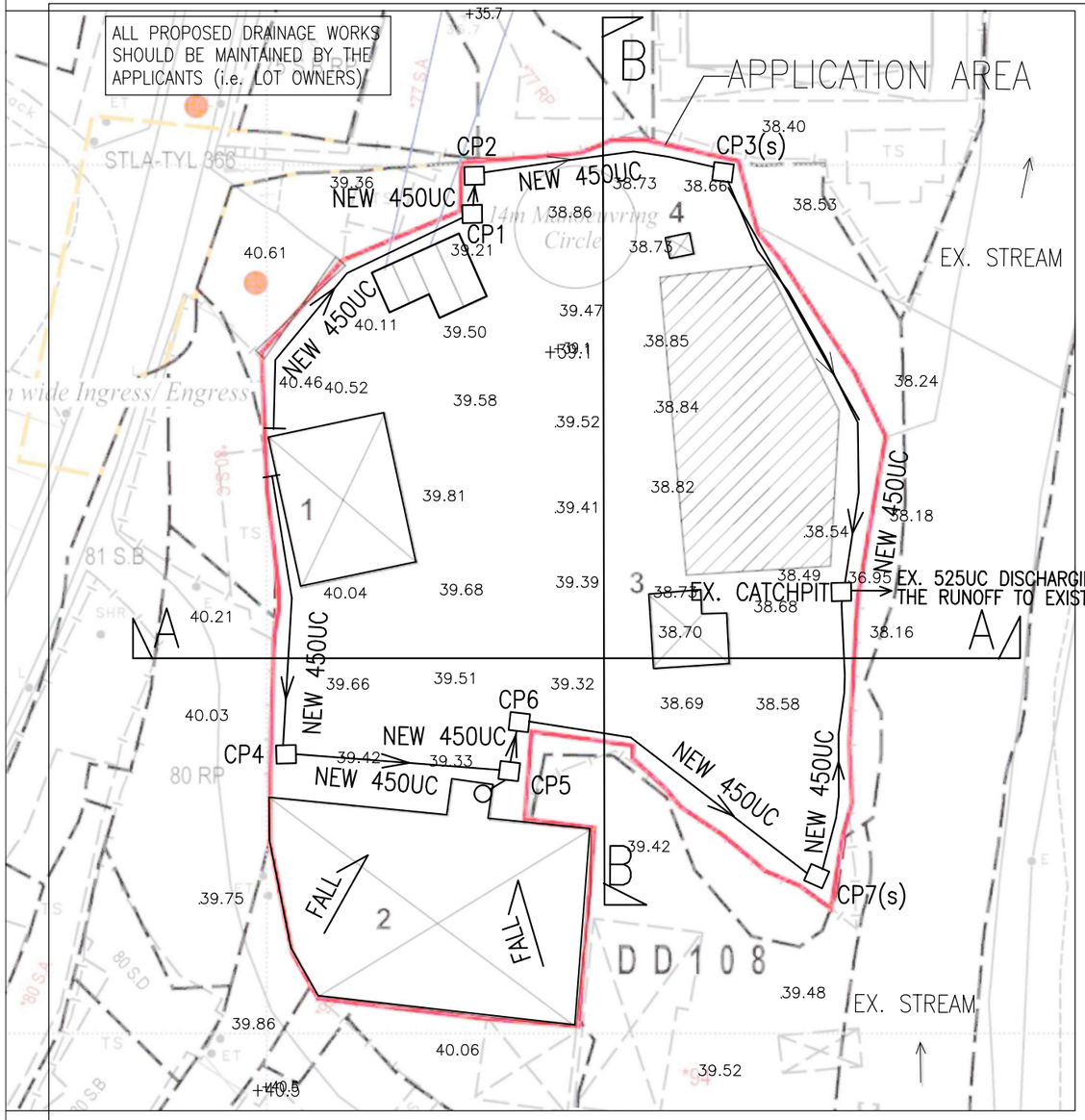
The drainage design of the proposed U-channel are presented in **Appendix B**. Since no new walls or hoarding would be erected or laid along the site boundary, the existing flow path of the site and its adjacent area would not be affected from the development.

6. Conclusion

Peripheral channels are to be provided along the site boundary where necessary to intercept runoff from crossing the site. The drainage conditions of adjacent areas will not be adversely affected.

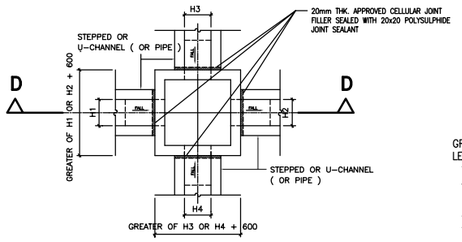
Appendix A

Stormwater Drainage Proposal Plan



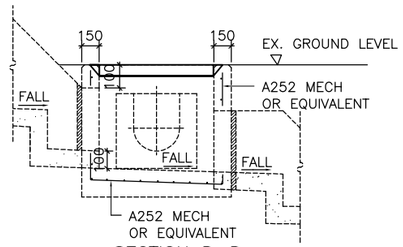
LEGEND:

NEW 225UC →	PROPOSED 225mm U-CHANNEL WITH GRATING AT FALL 1: 100 (MIN)
CP3(s) □	PROPOSED COVERED DESILTED CATCHPIT NO. CP3
CP2 □	PROPOSED COVERED CATCHPIT NO. CP2
○	50mm DIA. PVC DOWNPIPE FROM ROOF TO GROUND TO DISCHARGE RUNOFF FROM ROOF



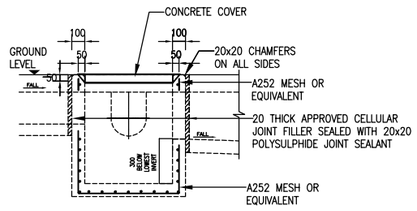
PLAN

TYPICAL DETAILS OF CATCHPIT



SECTION D–D

COMPLY WITH CEDD'S STANDARD DRAWING NOS. C2405
N.T.S.



SECTION D – D WITH DESILTED TRAP

COMPLY WITH CEDD'S DRAWING NO. DS C2405 AND C2406I

GENERAL NOTE

1. THE PROPOSED DRAINAGE WORK, WHETHER WITHIN OR OUTSIDE THE LOT BOUNDARY, SHOULD BE CONSTRUCTED AND MAINTAINED BY THE LOT OWNER AT HIS OWN EXPENSE. FOR WORKS TO BE UNDERTAKEN OUTSIDE THE LOT BOUNDARY, PRIOR CONSENT AND AGREEMENT FROM DLO AND/OR RELEVANT PRIVATE LOT OWNER SHOULD BE SOUGHT.

CONCRETE STRENGTH AND STEEL REINFORCEMENT SPECIFICATION FOR DRAINAGE DETAILS

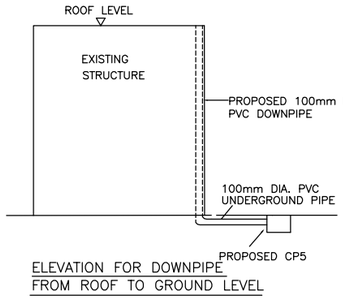
1. CONCRETE GRADE FOR CATCHPITS AND U–CHANNEL SHALL BE 30D DESIGN IN COMPLIANCE WITH CS1 : 2010
   FOR BLINDING LAYER SHALL BE 15D, DESIGN COMPLY WITH CS1–2010.
2. ALL MAIN BARS TO BE HOT ROLLED HIGH YIELD STEEL DEFORMED BAR COMPLM WITH CS2 : 2012
   Y – HIGH YIELD BAR 500 MPa
   M – MILD STEEL BAR 250 MPa
3. CONCRETE COVER TO MAIN REINFORCEMENT TO BE 50mm.
4. LAP LENGTH FOR ALL BARS TO BE 46x DIAMETER OF LARGER BAR TO BE LAPPED.
5. REACTIVE ALKALI CONTENT EXPRESSED IN SODIUM OXIDE PER CUBIC METER OF CONCRETE SHOULD NOT EXCEED 3KG AS PER PNAP APP–74.

HALF ROUND, U, AND STEPPED – CHANNELS

1 ALL DIMENSIONS ARE IN MILLIMETERS
2 CONCRETE SURFACE FINISHING SHALL BE CLASS U2 OR F2 AS APPROPRIATE
3 FOR HALF ROUND AND U – CHANNEL, SPACING OF EXPANSION JOINT IN CHANNELS, BERMS AND APRON TO BE 10m MAXIMUN. FOR STEPPED CHANNELS, EXPANSION JOINTS TO BE PROVIDED AT A MAXIMUN SPACING OF 10m.
4 DIMENSIONS FOR HALF ROUND AND U–CHANNELS SEE TABLE 1.
5 THE COVER FOR U–CHANNELS AND CATCHPIT SHALL COMPLY WITH CEDD'S STANDARD DRAWINGS NO. C2405 TO C2407 AND C2412.
6 ALL PROPOSED U–CHANNELS SHALL BE COVERED WITH GRATING

TABLE 1 : DIMENSION OF U–CHANNEL AND HALF–ROUND CHANNEL

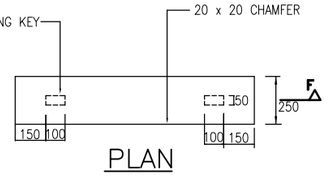
NORMAL SIZE H	T	B	REINFORCING
<300	100	100	NIL
375 – 675	150	150	NIL
750 – 900	175	175	A252 MESH PLACED CENTRALLY



ELEVATION FOR DOWNPIPE FROM ROOF TO GROUND LEVEL

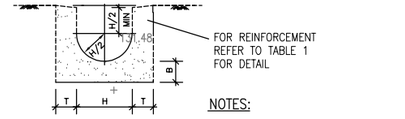
CATCHPITS

1 ALL DIMENSIONS ARE IN MILLIMETRES
2 CONCERTE SURFACE FINISH SHALL BE CLASS U2 OR F3 AS APPROPRIATE



PLAN

N.T.S.



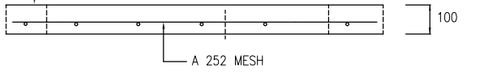
U–CHANNEL

COMPLY WITH FIG 8.11 OF GEOTECHNICAL MANUAL FOR SLOPES

NOTES:
1. H = NOMINAL CHANNEL SIZE.

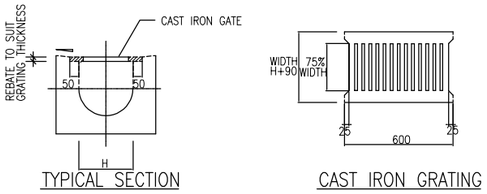
PROPOSED CATCHPIT SCHEDULE

CATCHPIT NO.	C.L. (mPD)	I.L. (mPD)
CP1	39.2	38.65
CP2	39.2	38.62
CP3(s)	38.66	38.11
CP4	39.42	38.87
CP5	39.33	38.63
CP6	39.32	38.59
CP7(s)	39.40	38.23



PRECAST CONCRETE COVERS FOR CATCHPIT

N.T.S.

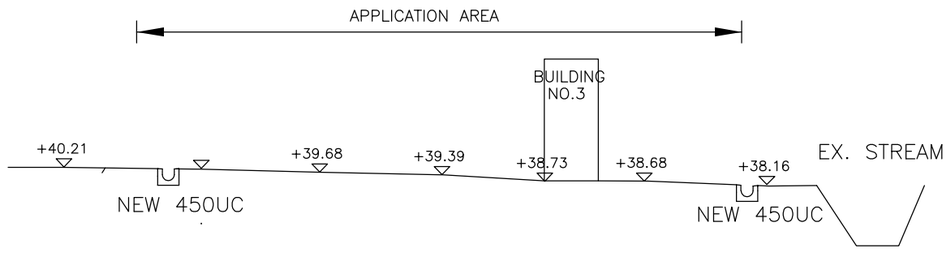


TYPICAL SECTION

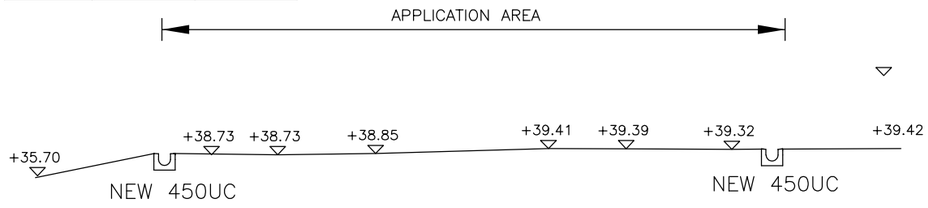
CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY. CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U–CHANNEL WITH CAST IRON GRATING



SECTION A–A
(NO EXCAVATION AND FILLINH WORKS IS PROPOSED)



SECTION B–B
(NO EXCAVATION AND FILLINH WORKS IS PROPOSED)

REV	DESCRIPTION	CHECKED	APPROVED	DWN	DATE
	DLO SUBMIT	RC	AY	RY	NOV 25

PROJECT TITLE:

STORMWATER DRAINAGE PROPOSAL
LOT NO. 78 S.A IN D.D. 108

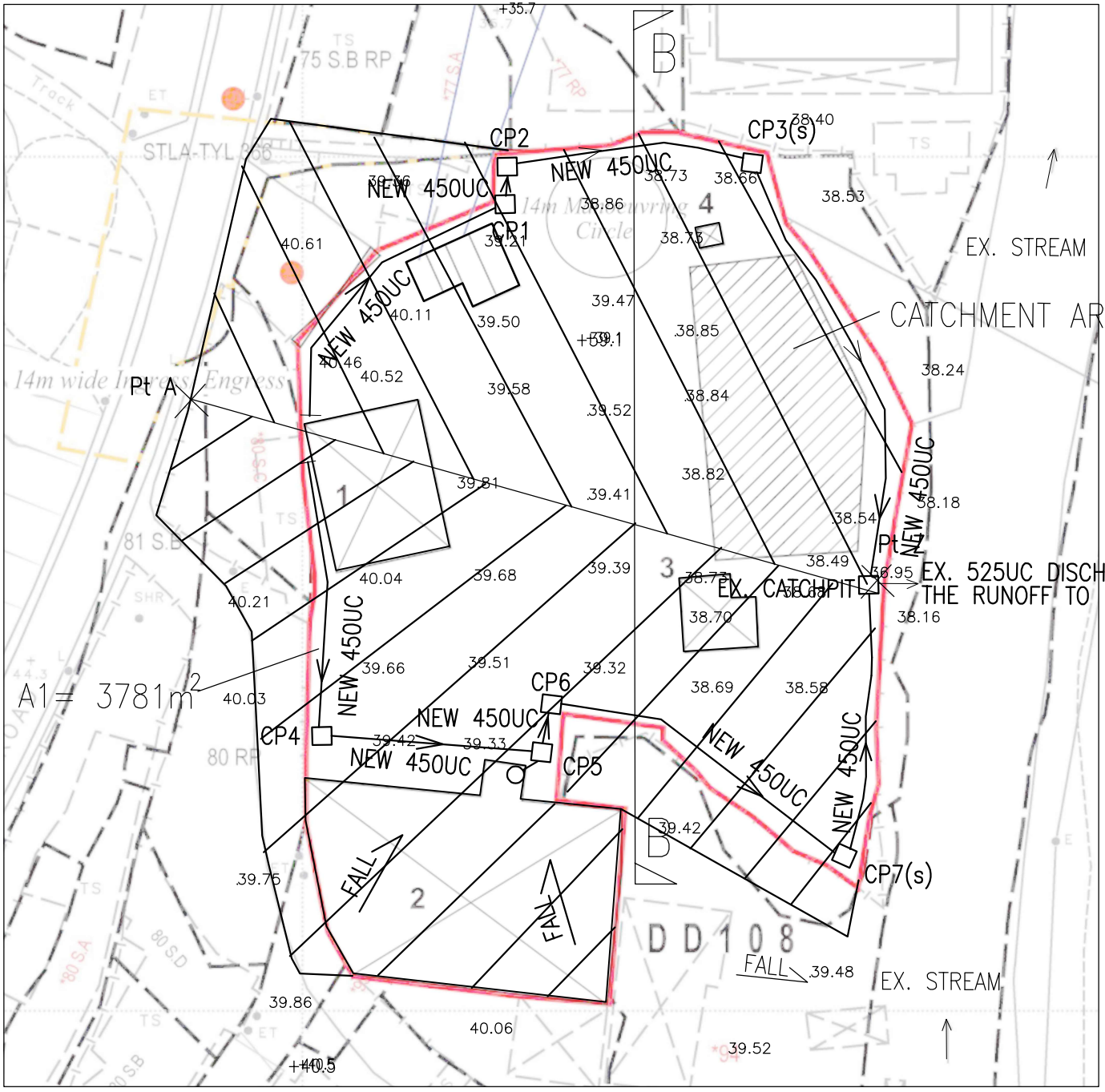
DRAWING TITLE:

DRAINAGE PROPOSAL PLAN AND TYPICAL DETAILS

SCALE :	N.T.S.		CAD FILE: CAD_REF
DRAWN	RY		DRAWING NO.
S.D	RY		SDP001
DESIGNED	RC		
CHECKED	AY		B.D. REF. NO.:

Appendix B

Surface Drainage Design



CATCHMENT AREA PLAN

Drainage Design Page no.

Project No.: Drainage Design at lot 78s.A(Part), 93(Part), 94(Part) in DD108 Date: 29-Nov-25
Prepared by: Ray Cheng

Check for the drainage capactiy of proposed 450UC

Catchment area, A1 = 3781 m^2 Assume k = 0.95 for paved surface

Use Rational Method from Geo-Manual

$Q = kiA/3600$ where,

Q = Maximum runoff (lit/sec)
k = Runoff coefficient
i = Design mean intensity of rainfall (r
A = Total catchment area (m^2)

Longest distance from summit point to outlet, Pt Z (Ld) = 164.00 m
Shortest distance from summit point to outlet, Pt Z (Ls) = 84.00 m

Elevation of remote point (Pt A) = 40.21 mPD
Elevation of outlet point (Pt Z) = 36.95 mPD

Average fall, H = $(z_1-z_2)/L_s$ x 100
= 3.88 m per 100m

T_c = $0.14465 \times L_d / (H^{0.2} \times A^{0.1})$
= 7.94 min

Assume a 1 in 50 year design rainfall return period for rural area
From SDM Corrigendum No. 1/2024

i = 202 mm/hr rainfall increase
Q = kiA/60 x 1.16
14028 lit/min

From TGN 43A1
For proposed 450 UC with 1 in 100 gradient

Maximum capacity	=	25000	lit/min	>	14028	o.k.
The corresponding velocity	=	2.25	m/s	<	4	o.k.

Drainage Design Page no.

Project No.: Drainage Design at lot 78s.A(Part), 93(Part), 94(Part) in DD108 Date: 29-Nov-25
Prepared by: Ray Cheng

Check for the drainage capactiy of proposed 450UC

Catchment area, A2 = 3074 m^2 Assume k = 0.95 for paved surface

Use Rational Method from Geo-Manual

Q = kiA/3600 where,

Q = Maximum runoff (lit/sec)
k = Runoff coefficient
i = Design mean intensity of rainfall (mm/hr)
A = Total catchment area (m^2)

Longest distance from summit point to outlet, Pt Z (Ld) = 131.00 m
Shortest distance from summit point to outlet, Pt Z (Ls) = 84.00 m

Elevation of remote point (Pt A) = 40.21 mPD
Elevation of outlet point (Pt Z) = 36.95 mPD

Average fall, H = $(z_1-z_2)/L_s$ x 100
= 3.88 m per 100m

T_c = $0.14465 \times L_d / (H^{0.2} \times A^{0.1})$
= 6.47 min

Assume a 1 in 50 year design rainfall return period for rural area
From SDM Corrigendum No. 1/2024

i = 210 mm/hr rainfall increase
Q = kiA/60 x 1.16
11856 lit/min

From TGN 43A1
For proposed 450 UC with 1 in 100 gradient

Maximum capacity	=	25000	lit/min	>	11856	o.k.
The corresponding velocity	=	2.25	m/s	<	4	o.k.

Drainage Design Page no.

Project No.: Drainage Design at lot 78s.A(Part), 93(Part), 94(Part) in DD108 Date: 29-Nov-25
Prepared by: Ray Cheng

Check for the drainage capactiy of existing 525mm dia. Pipe

Catchment area,	A1	=	3781	m^2	Assume k = 0.95 for paved surface
	A2	=	3074	m^2	
	Total area, A1+A2	=	**6855**	m^2	

Use Rational Method from Geo-Manual

$Q = kiA/3600$ where,

Q = Maximum runoff (lit/sec)
k = Runoff coefficient
i = Design mean intensity of rainfall (mm/hr)
A = Total catchment area (m^2)

Longest distance from summit point to outlet, Pt Z (Ld) = 164.00 m
Shortest distance from summit point to outlet, Pt Z (Ls) = 84.00 m

Elevation of remote point (Pt A) = 40.21 mPD
Elevation of outlet point (Pt Z) = 36.95 mPD

Average fall, H = $(z_1-z_2)/L_s$ x 100
= 3.88 m per 100m

T_c = $0.14465 \times L_d / (H^{0.2} \times A^{0.1})$
= 7.48 min

Assume a 1 in 50 year design rainfall return period for rural area
From SDM Corrigendum No. 1/2024

i = 205 mm/hr rainfall increase
Q = kiA/60 x 1.16
25810 lit/min

The capacity of the existing 525mm dia. pipe = 29100 lit/min > 25810 o.k.

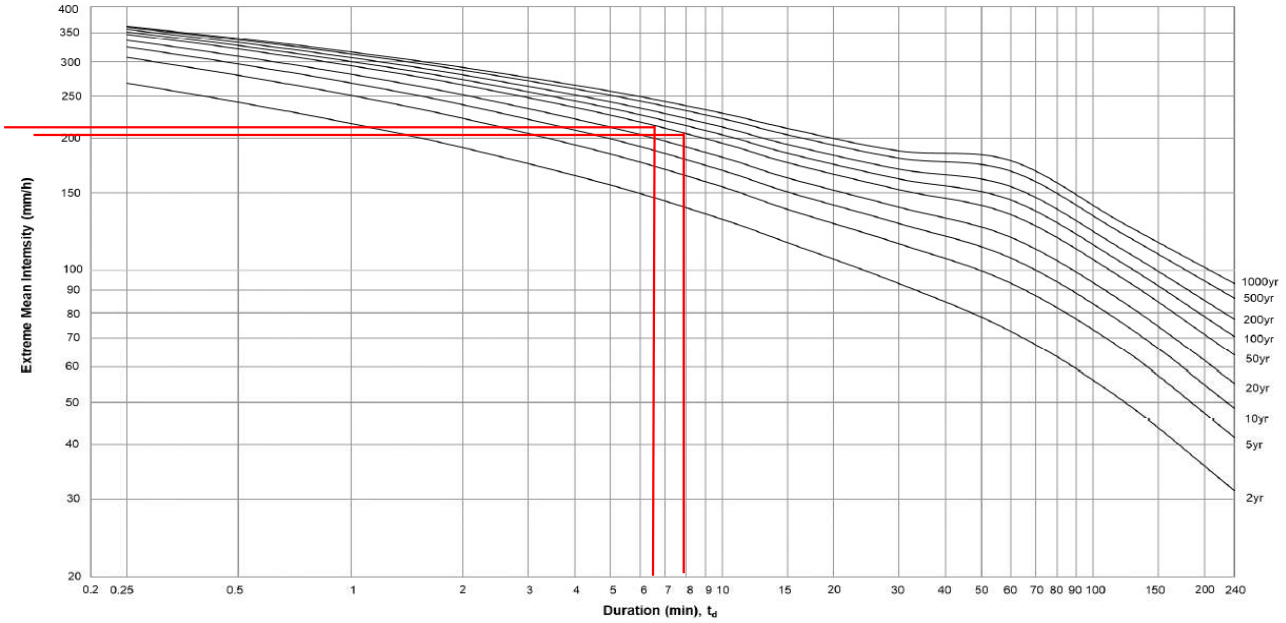


Figure 4a – Intensity-Duration-Frequency Curves of HKO Headquarters
(for durations not exceeding 4 hours)

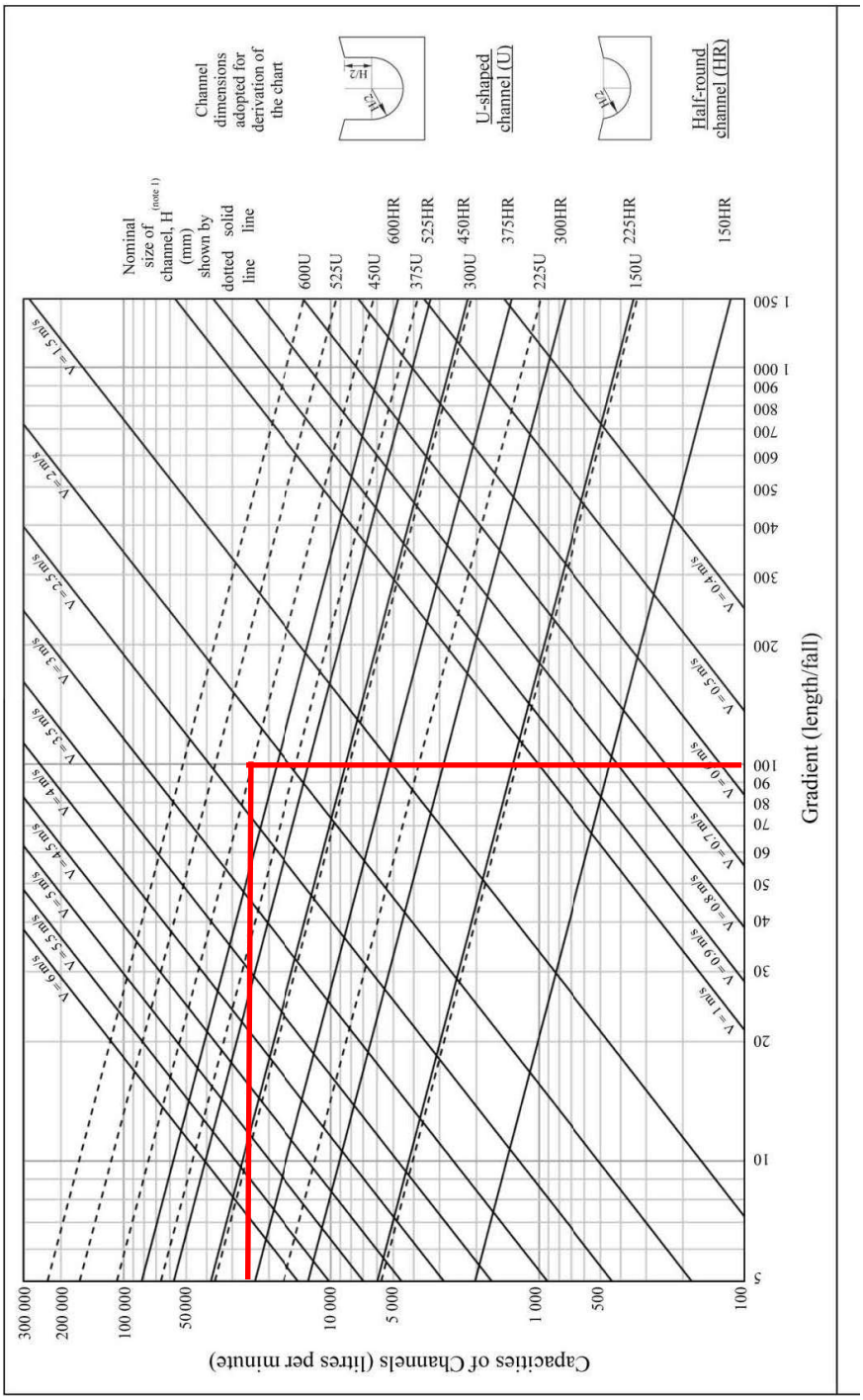


Figure 1 - Chart for the rapid design of U-shaped and half-round channels up to 600 mm

continued

ıe hydraulic gradient = 1 in 250 to 1 in 10

velocities in m/s
discharges in m^3/s

The capacity of 525mm dia. pipe = 0.485 x 1000 x 60 = 29,100 lit/min

Gradient	Pipe diameters in mm: 350	375	400	450	500	525	600	675	700	750	800	825
0.00400 1/ 250	1.091	1.140	1.187	1.280	1.368	1.410	1.534	1.651	1.689	1.764	1.836	1.871
	0.105	0.126	0.149	0.203	0.269	0.305	0.434	0.591	0.650	0.779	0.923	1.000
0.00420 1/ 238	1.118	1.168	1.217	1.311	1.402	1.446	1.572	1.692	1.731	1.808	1.882	1.918
	0.108	0.129	0.153	0.209	0.275	0.313	0.444	0.606	0.666	0.799	0.946	1.025
0.00440 1/ 227	1.145	1.196	1.246	1.343	1.435	1.480	1.609	1.733	1.772	1.850	1.926	1.963
	0.110	0.132	0.157	0.214	0.282	0.320	0.455	0.620	0.682	0.817	0.968	1.050
0.00460 1/ 217	1.171	1.223	1.274	1.373	1.468	1.513	1.646	1.772	1.813	1.892	1.970	2.008
	0.113	0.135	0.160	0.218	0.288	0.328	0.465	0.634	0.698	0.836	0.990	1.073
0.00480 1/ 208	1.196	1.250	1.302	1.403	1.499	1.546	1.682	1.810	1.852	1.933	2.013	2.051
	0.115	0.138	0.164	0.223	0.294	0.335	0.475	0.648	0.713	0.854	1.012	1.097
0.00500 1/ 200	1.221	1.276	1.329	1.432	1.531	1.578	1.717	1.848	1.890	1.973	2.054	2.094
	0.117	0.141	0.167	0.228	0.301	0.342	0.485	0.661	0.727	0.872	1.033	1.119
0.00550 1/ 182	1.281	1.339	1.395	1.503	1.606	1.656	1.801	1.939	1.983	2.070	2.155	2.197
	0.123	0.148	0.175	0.239	0.315	0.359	0.509	0.694	0.763	0.915	1.083	1.174
0.00600 1/ 167	1.339	1.399	1.457	1.570	1.678	1.730	1.882	2.026	2.072	2.163	2.252	2.295
	0.129	0.155	0.183	0.250	0.330	0.375	0.532	0.725	0.797	0.956	1.132	1.227
0.00650 1/ 154	1.394	1.457	1.518	1.635	1.747	1.802	1.959	2.109	2.157	2.252	2.344	2.390
	0.134	0.161	0.191	0.260	0.343	0.390	0.554	0.755	0.830	0.995	1.178	1.277
0.00700 1/ 143	1.447	1.512	1.575	1.697	1.814	1.870	2.034	2.189	2.239	2.338	2.433	2.480
	0.139	0.167	0.198	0.270	0.356	0.405	0.575	0.783	0.862	1.033	1.223	1.326
0.00750 1/ 133	1.499	1.566	1.631	1.757	1.878	1.936	2.106	2.266	2.318	2.420	2.519	2.568
	0.144	0.173	0.205	0.279	0.369	0.419	0.595	0.811	0.892	1.069	1.266	1.373
0.00800 1/ 125	1.548	1.618	1.685	1.815	1.940	2.000	2.175	2.341	2.395	2.500	2.602	2.653
	0.149	0.179	0.212	0.289	0.381	0.433	0.615	0.838	0.922	1.105	1.308	1.418
0.00850 1/ 118	1.596	1.668	1.737	1.872	2.000	2.062	2.243	2.414	2.469	2.577	2.683	2.735
	0.154	0.184	0.218	0.298	0.393	0.446	0.634	0.864	0.950	1.139	1.349	1.462
0.00900 1/ 111	1.643	1.717	1.788	1.926	2.059	2.123	2.308	2.484	2.541	2.653	2.761	2.814
	0.158	0.190	0.225	0.306	0.404	0.459	0.653	0.889	0.978	1.172	1.388	1.504
0.00950 1/ 105	1.688	1.764	1.838	1.980	2.115	2.181	2.372	2.553	2.611	2.726	2.837	2.892
	0.162	0.195	0.231	0.315	0.415	0.472	0.671	0.913	1.005	1.204	1.426	1.546
0.01000 1/ 100	1.733	1.810	1.886	2.031	2.171	2.238	2.434	2.619	2.679	2.797	2.911	2.967
	0.167	0.200	0.237	0.323	0.426	0.485	0.688	0.937	1.031	1.236	1.463	1.586

Appendix C

General View of Existing 525mm dia. pipe and Stream

Photo No. V1 : Existing Catchpit and 525mm dia Pipe

Photo No. V2 : Existing Stream