

# **Existing Stormwater Drainage Checking**

For

Temporary Shop and Services with Ancillary Office

at Lot Nos. 446 & 447 in D.D. 122

Ping Shan, Yuen Long, N.T.

Report No.: **LD/L446/DS01**

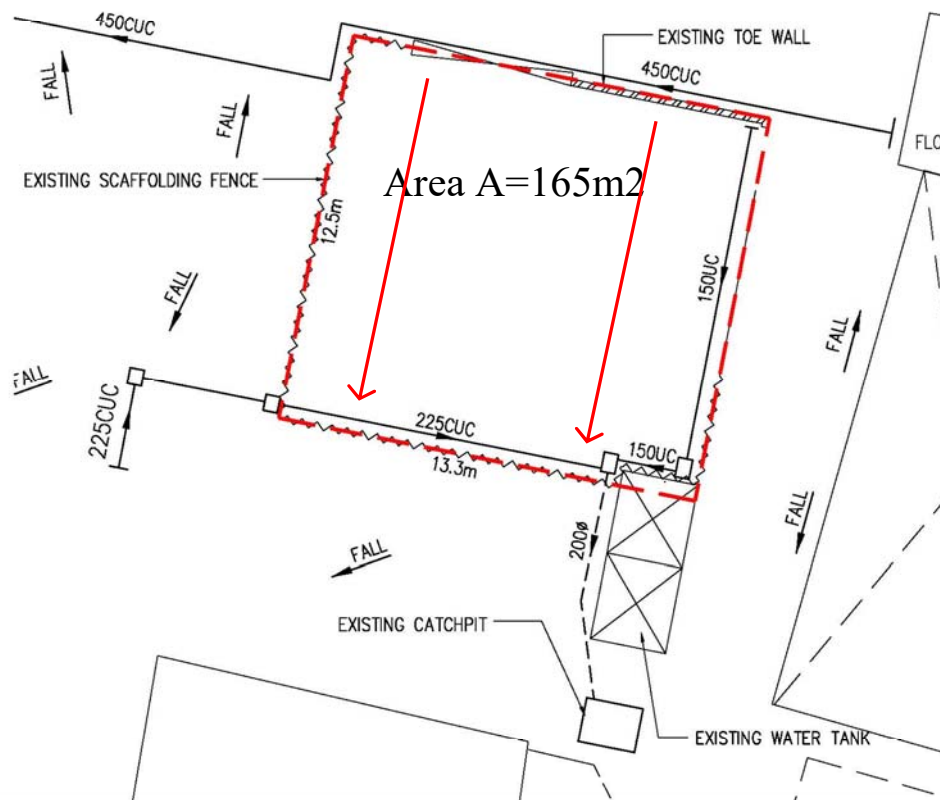
Date: **20/1/2024**

Project : Temporary Shop and Services with Ancillary Office at Lot nos.446 & 447 in DD122 for existing Drainage Checking	P. 1 of 7
<p><u>Table of Contents</u></p> <p>A. Introduction</p> <p>B. Checking for the Existing of Surface Channels</p> <p>C. Conclusion</p> <p>Existing Drainage Layout Plan</p> <p><b>A. Introduction</b></p> <p>The subject site is generally located at Lot Nos. 446 &amp; 447 in DD 122 at Ping Shan in Yuen Long N. T. and the site is surrounding by the existing drainage which has been checked in accordance with the Rational Method as prescribed in the Geotechnical Manual for Slopes. The existing drainage system has been checked to a 1 in 200 year return rainstorm, and connected to the existing catch pit system at the site. Surface runoff from the site will be collected by a existing drainage system of the existing U-channels and catchpits. The calculation for the existing drainage system is presented in the following items 2 and 3. Having reviewed the existing drainage layout plan (No. LD/L446/D01) for the adjacent Lots, the collected runoff from all of the Lots at the existing drainage was discharged into the existing catch pit via the existing U-channels.</p>	

Project : Temporary Shop and Services with Ancillary Office at Lot nos.446 & 447 in DD122 for existing Drainage Checking	P. 2 of 7
<p><b>B. Checking for the Existing of Surface Channels</b></p> <p>1. <u>Equations and Assumptions</u></p> <p>1.1 Surface drainage design is in accordance with Geotechnical Manual for Slopes (2nd Edition, 1984).</p> <p>1.2 Slope drainage is designed to a frequency of 1 in 200 rainfall return period.</p> <p>1.3 Time of Concentration = time of entry + time of flow i.e. <math>t_c = t_e + t_f</math></p> <p>1.4 Time of entry is calculated based on the modified form of Bransby-Williams Equation:  <math display="block">t_e = 0.14465 \times L / (H^{0.2} \times A^{0.1})</math>           where <math>t_e</math> = time of entry (min) ,  <math>A</math> = area of catchment (<math>m^2</math>) ,  <math>H</math> = average fall (m per 100m) from the summit of catchment to the point of design,  <math>L</math> = distance in metre measured on the line of natural flow between the design section and that point of catchment from which water would take the longest time to reach the design section (m)</p> <p>1.5 Time of flow is calculated from the measured water flow length in channel divided by the assumed flow velocity.            i.e. <math>t_f = w / v</math>            where <math>t_f</math> = time of flow (min) ,  <math>w</math> = measured water flow length in channel (m) ,  <math>v</math> = assumed water flow velocity (m/s)</p> <p>1.6 Runoff coefficient for the slope is assumed to be 1.0 for <a href="#">vegetated ground surface</a>.</p> <p>1.7 Peak stormwater is determined by the "Rational Method" using the following formula:  <math display="block">Q = KiA/60</math>           where <math>Q</math> = maximum runoff (litres/min) ,  <math>K</math> = runoff coefficient (<math>K = 1.0</math>) ,  <math>i</math> = design mean intensity of rainfall (mm/hr) ,  <math>A</math> = area of catchment (<math>m^2</math>) .</p>	<p>Eqn. 8.2 Geotechnical Manual for Slopes</p> <p>Geotechnical Manual for Slopes (p. 96)</p> <p>Eqn. 8.7 Geotechnical Manual for Slopes</p>

## 2. Catchment Area

The catchment area for the existing of surface channels is shown below :



Plan of Catchment Areas  
NTS

Project : Temporary Shop and Services with Ancillary Office at Lot nos.446 & 447 in DD122 for existing Drainage Checking	P. 4 of 7
<p>3. <u>Checking of Surface Channel</u>(Assumed all rainwater will be connected by 225 CUC)</p> <p>a. Catchment Area A to Existing Drainage ( 225 CUC)</p> <p>Area A = 165 m<sup>2</sup> L = 12.5 m</p> <p><math>\delta h = 5.88 - 5.62 = 0.26</math> m</p> <p><math>H = 0.26 * 100 / 12.5 = 2.08</math> m (average fall per 100m run)</p> <p><math>t_c = 0.14465 \times 12.5 / (2.08^{0.2} \times 165^{0.1}) = 0.937</math> min</p> <p>For <math>t_f</math>, w = 13.3 m, v = 3 m/s (assumed)</p> <p><math>t_{f1} = 13.3 / (3 \times 60) = 0.074</math> min</p> <p><math>t_l = 0.937 + 0.074 = 1.011</math> min</p> <p>From rainfall curve, use t = 1.0 min</p> <p><math>i_{200} = 450</math> mm/hr K = 1</p> <p>Flow for 200 years return periods,</p> <p><math>Q_{200} = 1 * 450 \times 165 / 60 = 1238</math> litres/min</p> <p>Drop of channel = 5.540 - 5.430 = 0.11 m</p> <p>Gradient = 0.11 / 13.3 = 1 in 121</p> <p>Existing channel size = 225 UC</p> <p>Capacity = 3500 &gt; Q200 OK</p> <p>Read <math>v_{max} = 1.3</math> m/s &lt; 4 m/s OK</p> <p>Therefore, used 225mm UC is adequate for catchment Area of A.</p>	<p>Fig. 1, TGN 30</p> <p>Fig. 8.7 Geotechnical Manual for Slopes</p>

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**GEO Technical Guidance Note No. 30 (TGN 30)  
New Intensity-Duration-Frequency Curves for Slope Drainage Design**

Issue No.: 1      Revision: -      Date: 21.3.2011      Page: 3 of 4

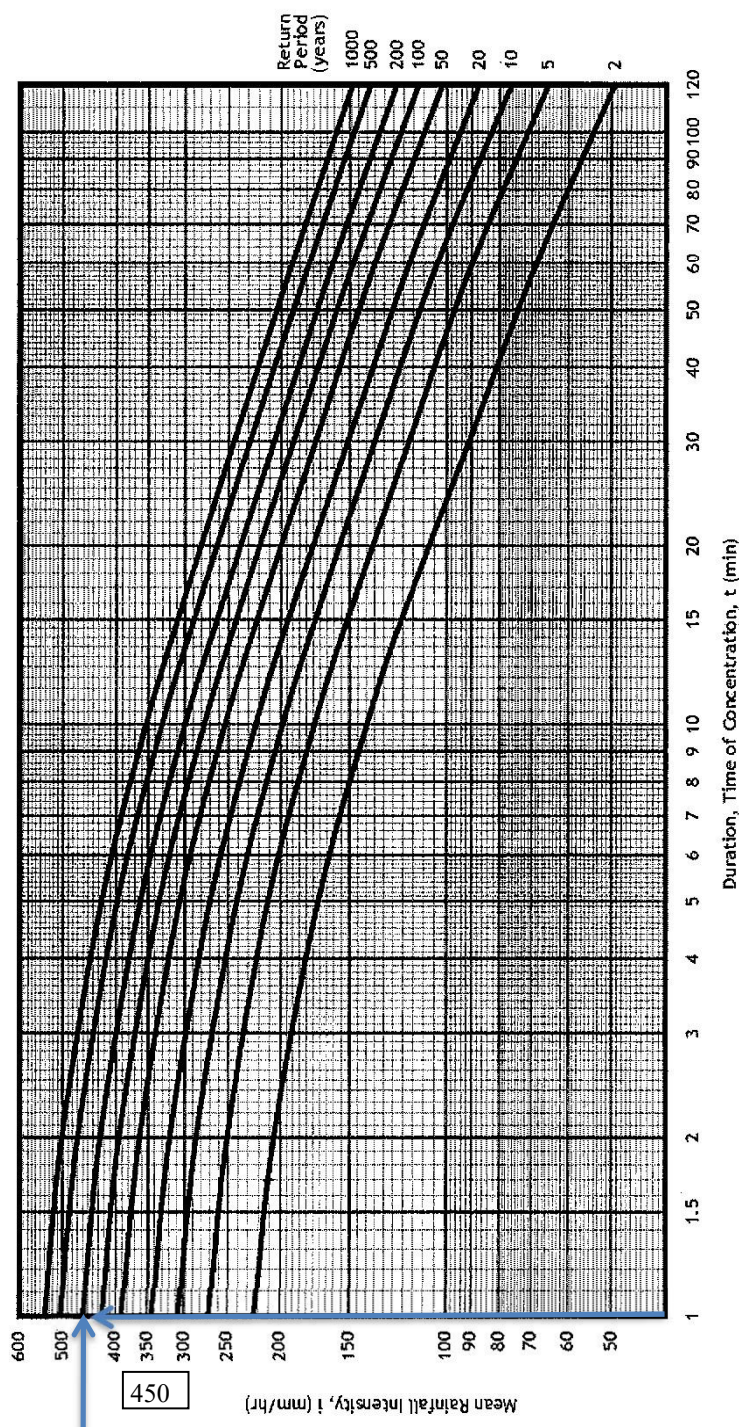


Figure 1 – New Intensity-Duration-Frequency (IDF) Curves (Tang & Cheung, 2011)

Note: These IDF curves are to supersede those given in Figure 8.2 of the Geotechnical Manual for Slopes (GCO, 1984).

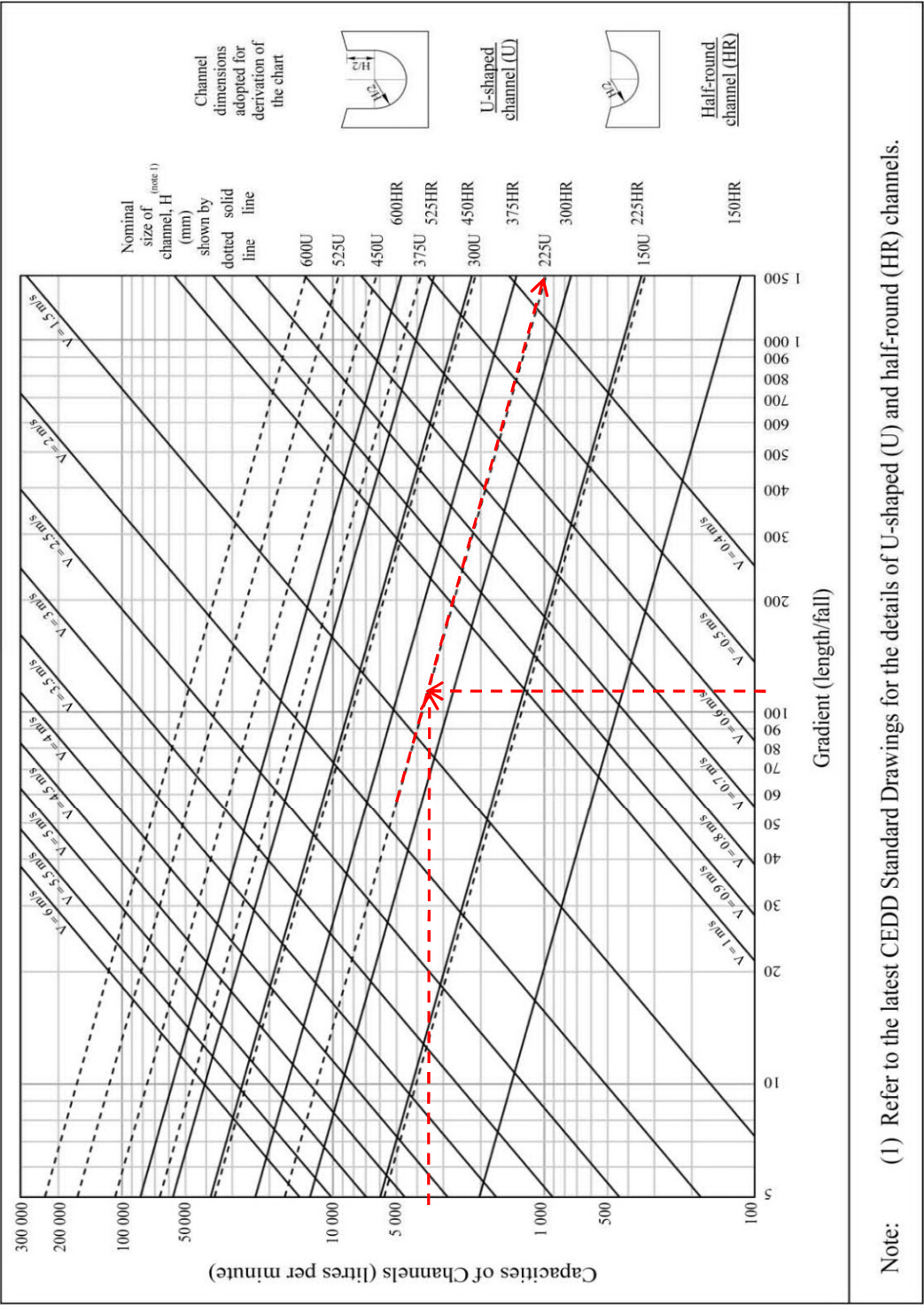


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GEO Technical Guidance Note No. 43 (TGN 43)  
Guidelines on Hydraulic Design of U-shaped and Half-round Channels on Slopes

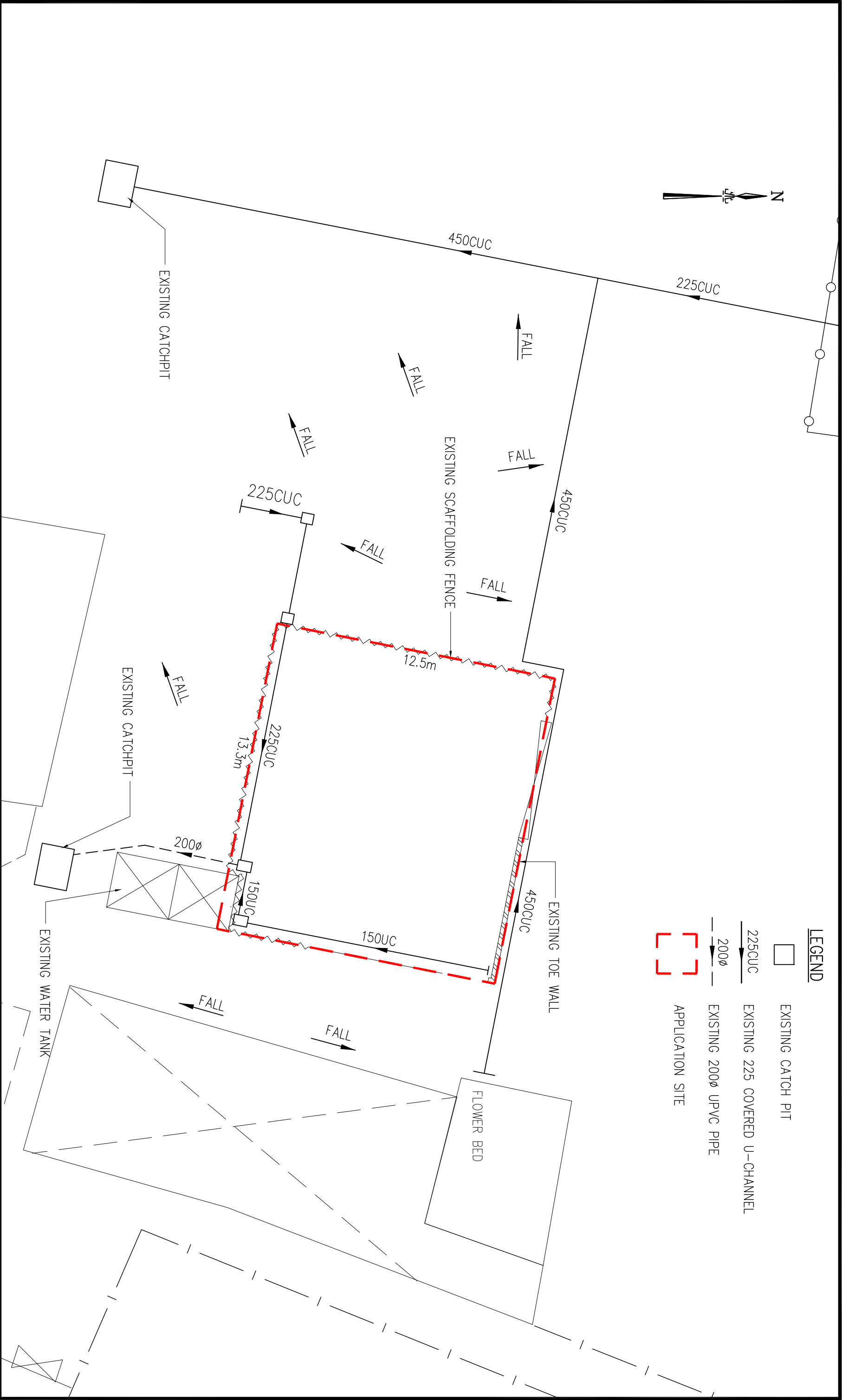
Issue No.: 1      Revision: -      Date: 05.06.2014      Page: 3 of 3

Figure 1 - Chart for the rapid design of U-shaped and half-round channels up to 600 mm



Project : Temporary Shop and Services with Ancillary Office at Lot nos.446 & 447 in DD122 for existing Drainage Checking	P. 7 of 7
<b>C. Conclusion</b>  Based on the above calculation result, existing drainage surface channel to be discharged for the catchment areas, the existing drainage system are considered to be adequate and acceptable. The runoff discharge from the application area would not cause adverse drainage impact to the surrounding area at all times.	





EXISTING DRAINAGE LAYOUT PLAN

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