寄件者:

Louis Tse

寄件日期:

2025年09月29日星期一 17:46

收件者:

tpbpd/PLAND

副本:

David Chi Chiu CHENG/PLAND; Gary Kwun Wing CHAN/PLAND; Bon Tang;

Matthew Ng; Christian Chim; Danny Ng; Grace Wong; Kevin Lam

主旨:

[FI] S.16 Application No. A/YL-SK/426 - FI to address departmental comments

附件:

FI3 for A_YL-SK_426 (20250929).pdf

類別:

Internet Email

Dear Sir,

Attached herewith the further information to address departmental comments of the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

Louis TSE | Town Planner R-riches Group (HK) Limited

R-riches Property Consultants Limited | R-riches Planning Limited | R-riches Construction Limited



Our Ref. : DD106 Lot 987 Your Ref. : TPB/A/YL-SK/426



The Secretary,
Town Planning Board,
15/F, North Point Government Offices,
333 Java Road,
North Point, Hong Kong

By Email

29 September 2025

Dear Sir,

3rd Further Information

Proposed Temporary Open Storage of Construction Materials and Vehicles and Associated Filling of Land for a Period of 3 Years in "Agriculture" Zone,

<u>Various Lots in D.D.106, Shek Kong, Yuen Long, New Territories</u>

(S.16 Planning Application No. A/YL-SK/426)

We are writing to submit further information to address departmental comments of the subject application (Appendix I).

Should you require more information regarding the application, please contact

or the undersigned at your convenience.

Thank you for your kind attention.

Yours faithfully,

For and on behalf of

R-riches Planning Limited

Louis TSETown Planner





Responses-to-Comments

Proposed Temporary Open Storage of Construction Materials and Vehicles and Associated Filling of Land for a Period of 3 Years in "Agriculture" Zone, <u>Various Lots in D.D.106, Shek Kong, Yuen Long, New Territories</u>

(Application No. A/YL-SK/426)

(i) A RtoC Table:

	Departmental Comments	Applicant's Responses
1. C	omments of the Chief Engineer/Mainland North,	Drainage Services Department (CE/MN,
D	OSD)	
(0	Contact Person: Mr. Kenneth CAN; Tel:	
(a)	Please refer to DSD's Stormwater Drainage	Noted. A drainage proposal is provided
	Manual and its corrigenda for preparation of the	by the applicant to review the drainage
	drainage assessment/proposal.	arrangement of the application site
200000 1000		(Annex I).
(b)	The ground level at the eastern and southern	
	sides of the application site are higher. The	
	applicant should review the existing site	
	condition, take into account any overflow and	
	review the total catchment area adopted in the drainage design and calculation.	
	uramage design and calculation.	
(c)	Land filling is proposed under the application In	
(-7	view of (b) above, please take into account in the	
	proposed drainage arrangement and ensure that	
	the existing drain path / overland flow from	
	adjacent land would not be affected.	
(d)	The size and gradient of the proposed/existing	
	drainage facilities to be discharge from the site	
	and/or the drainage plan. The applicant should	
	check and ensure the hydraulic capacity of the	
	existing drainage facilities would not be	
	adversely affected by the captioned	
	development.	
(e)	Reference should be made to DSD Technical Note	
(6)	No. 1 for the sizes of the proposed u-channels.	
	The first the sizes of the proposed a charmers.	
(f)	Where walls or hoarding are erected or laid along	
	the site boundary, adequate opening should be	



	provided to intercept the existing overland flow passing through the site.	
(g)	Cross sections showing the existing and proposed ground levels of the captioned site with respect to the adjacent areas should be given.	
(h)	Standard details should be provided to indicate the sectional details of the proposed u-channel and the catchpit/sand trap.	
(i)	The development should neither obstruct overland flow nor adversely affect existing natural streams, village drains, ditches and the adjacent areas, etc.	
(j)	Please provide hydraulic calculations for the proposed discharging drainage facility demonstrating its capacity to cater for the surface runoff from the entire application site.	
(k)	The existing drainage channel of the proposed discharge point is not maintained by this Department. The applicant shall resolve any conflict/disagreement with relevant lot owner(s) and seek LandsD's permission for laying new drains/channels and/or modifying/upgrading existing ones in other private lots or on Government land outside the application site.	
(1)	Connection details at discharged point (including cross section) with C.L., I.L and catchpit/channel bottom level should be shown in the drawing.	
(m)	Colour photos to indicate the current conditions of the existing drainage facilities should be included in the submission. The photos taken locations and angles should be shown on the layout plan.	



Drainage Proposal

Sep 2025



Drainage Proposal

2

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Figure 2 - Existing Drainage Plan

Figure 3- Proposed Drainage System

Table 2- Design Return Periods under SDM

Figure 4 – Catchment Plan

Figure 5 - Sections

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Appendix A – Design Calculation

Appendix B - Development Layout Plan

Appendix C - Reference Drawings

Drainage Proposal

1 Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Lots 987, 988, 989 (Part) and 990 S.C (Part) in D.D. 106, Shek Kong, Yuen Long, New Territories (the Site) for 'Proposed Temporary Open Storage of Construction Materials and Vehicles and Associated Filling of Land for a Period of 3 Years'
- 1.1.2 This report aims to support the development in drainage aspect.

1.2 Application Site

- 1.2.1 The application site is situated at the south of Shek Kong Airfield. It has an area of approx. 6,427 m². The site location is shown in **Figure 1**.
- 1.2.2 The existing site is already fully paved. The existing site levels are approx. + 13.5 mPD. No major site formation works for the application site is anticipated.
- 1.2.3 The Application Site runoff is being discharged existing watercourse at the south of the site which would eventually discharge to Kam Tin River. **Figure 2** indicates the existing drainage system of the area.

Drainage Proposal

2 Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 6,427 m². The existing site is already fully paved before development. The catchment plan is shown in **Figure 4**.

Proposed Development Area (Approx.)					
Total Site Area (m²)	6,427				
Paved Area after Development (m²)	6,427				

Table 1 - Site Development Area

3 Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2- Design Return Periods under SDM

3.1.2 The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 50 years return period is adopted for drainage design.

Drainage Proposal

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Zone. Therefore, for 50 years return period, the following values are adopted.

(Corrigendum No.1/2024)

The development is proposed for temporary use for a period of 3 years. 11.1% rainfall increase due to climate change is considered.

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278CiA$

where Q_p = peak runoff in m³/s C = runoff coefficient (dimensionless) i = rainfall intensity in mm/hr A = catchment area in km²

- 3. The run-off coefficient (C) of surface runoff are taken as follows:
 - Paved Area: C = 0.95
 Unpaved Area: C = 0.35

Drainage Proposal

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the pipe flow (m/s)

S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:
$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V = velocity of the pipe flow (m/s)

 S_f = hydraulic gradient k_f = roughness value (m)

v = kinematics viscosity of fluid

D = pipe diameter (m) R = hydraulic radius (m)

Drainage Proposal

4 Proposed Drainage System

4.1. Proposed Channels

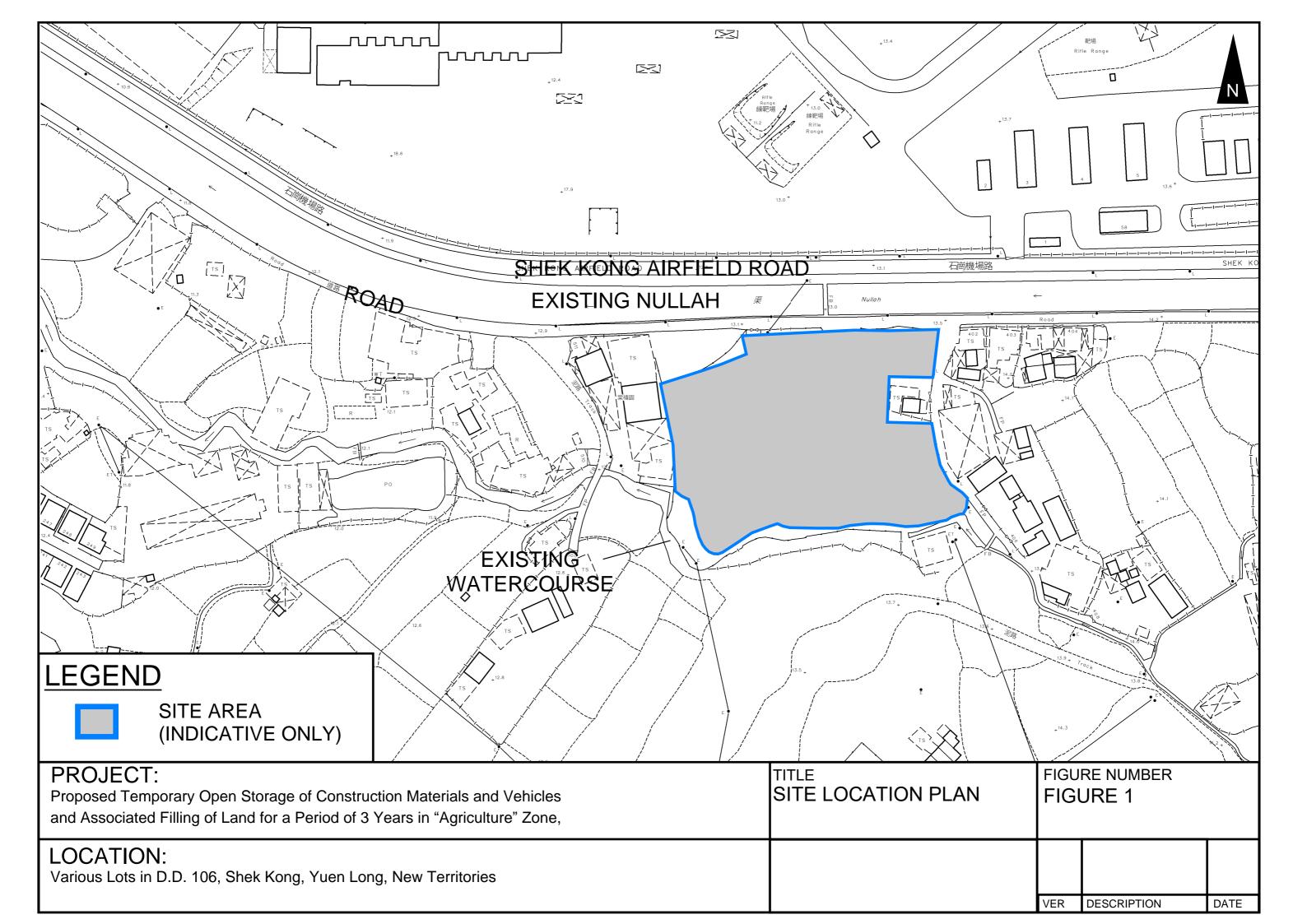
- 4.1.1 There are existing channels within site area constructed under previous planning applications no. A/YL-SK/350. According to checking in **Appendix A**, existing channels in western part of the site are proposed to be maintained. Detail shown in Appendix A.
- 4.1.2 Proposed Channels are designed for collection of runoffs for internal and external catchment. They are proposed to discharge to existing watercourse (which eventually discharge to Kam Tin River) via existing channel. The design checkings are shown in **Appendix A**. The existing site is already fully paved, no adverse drainage impact to existing drainage system is anticipated.
- 4.1.3 The design calculations of proposed UChannel are shown in **Appendix A**.
- 4.1.4 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4**.
- 4.1.5 Reference Drawings are shown in **Appendix C** for reference.

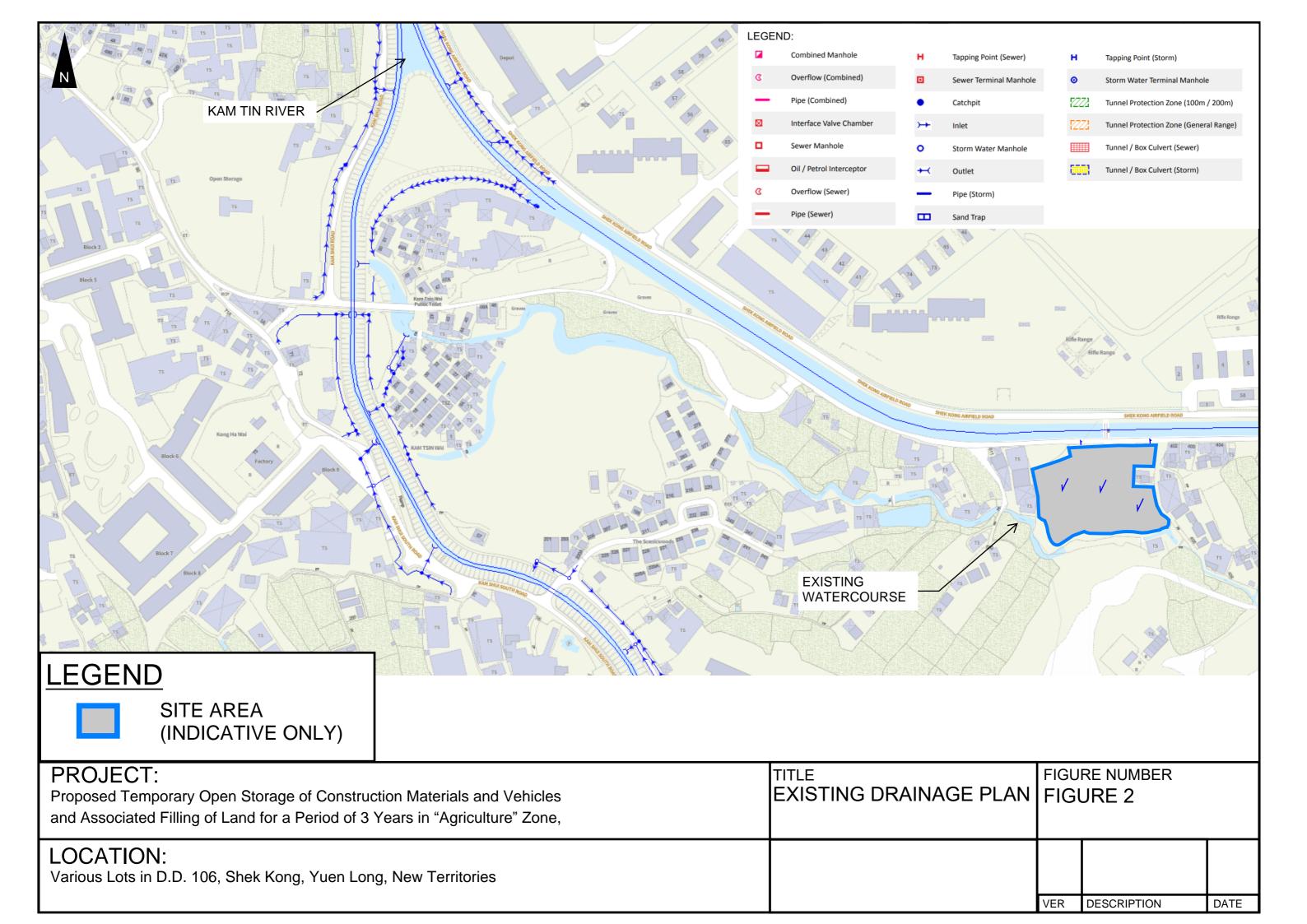
5 Conclusion

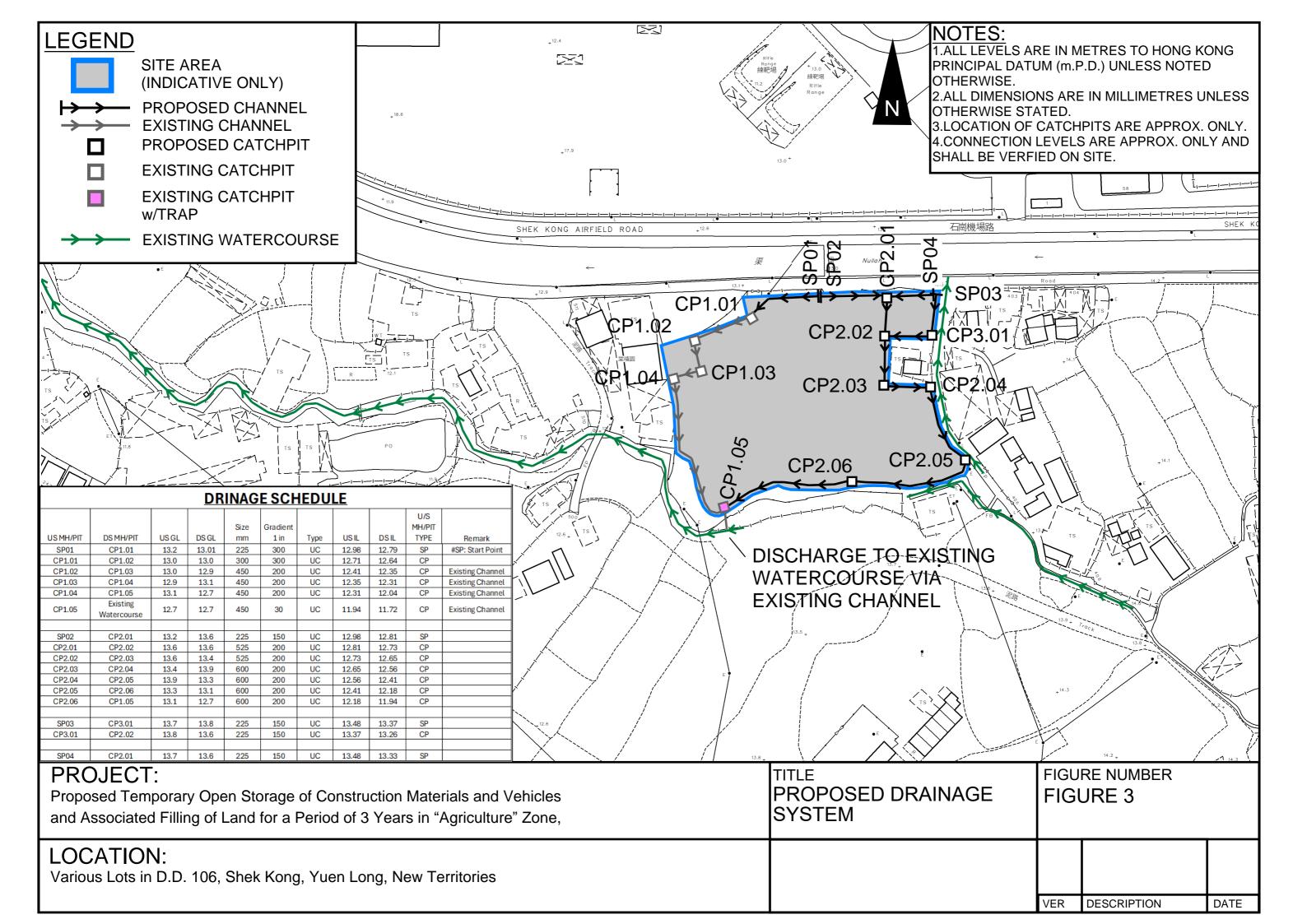
- 5.1.1 Drainage review has been conducted for the Proposed Development. The surface runoff will be collected by the existing/proposed drains and discharged to existing drainage system.
- 5.1.2 As the existing site is already paved before development, with implementation of the above drainage system, no unacceptable drainage impact is anticipated.

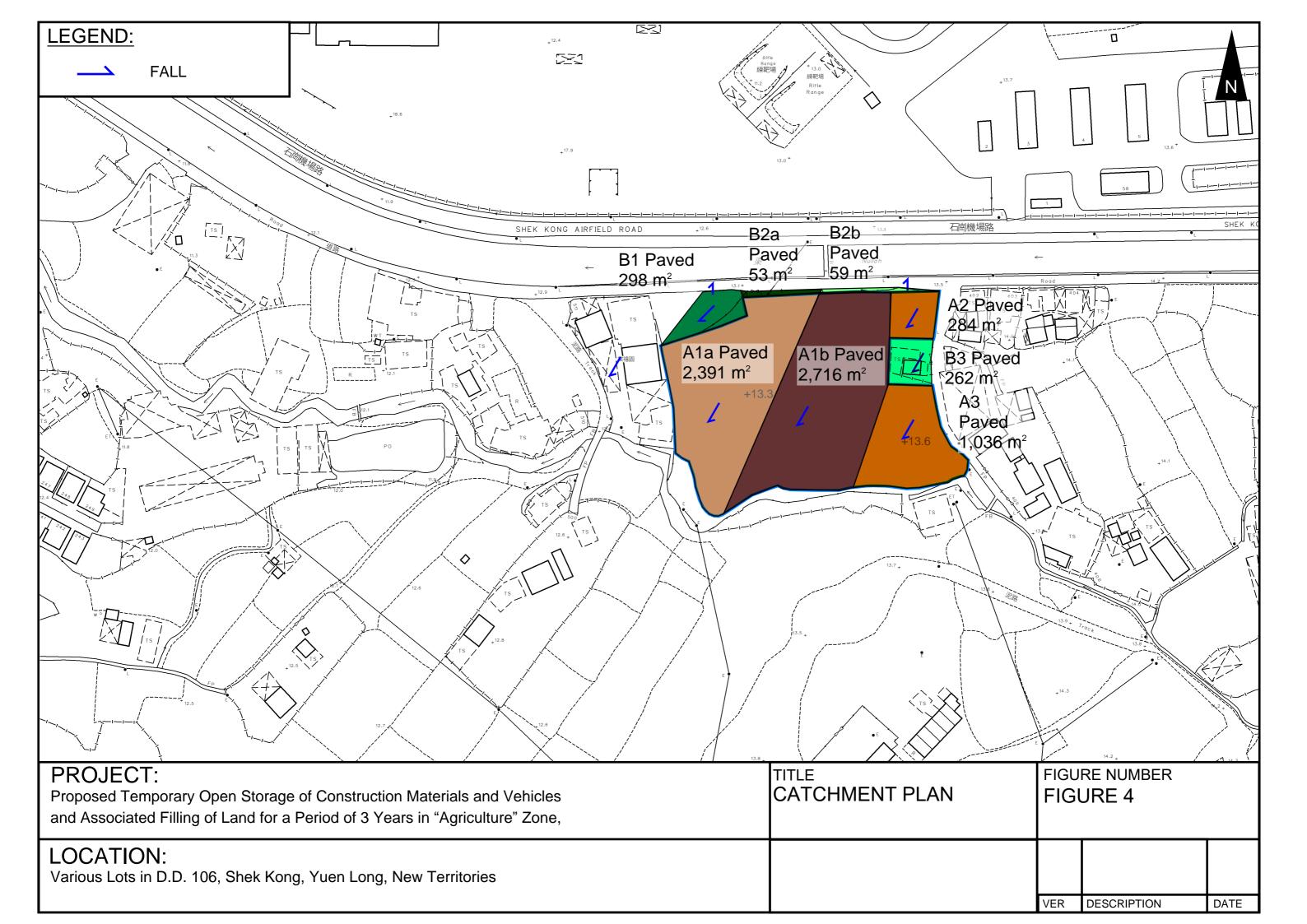
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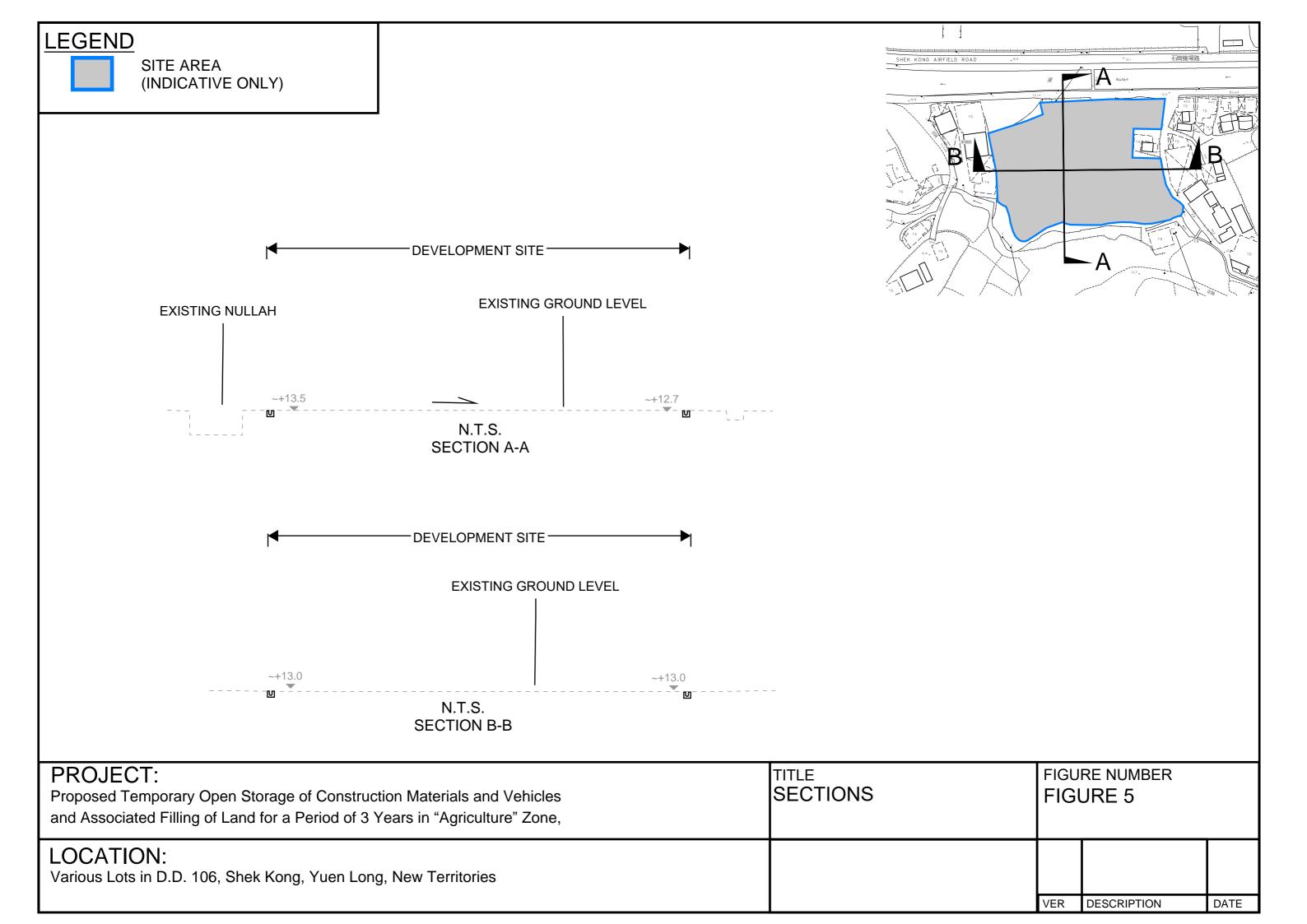
FIGURES











APPENDIX

Appendix A: Design Calculation

Zone								n	0.014		НКО а	505.5			
н	ко		Return Period	1 in	50	years		Ks	0.15	Storm Constant	HKO b	3.29			
								Viscosity	0.000001		НКО с	0.355			
Catchment Area Ta	able (Area in m²)														
Catchment	A1a	A1b	A2	A3	B1	B2a	B2b	B3							
Total Area	2391	2716	284	1036	298	53	59	262							
Hard Paved Area	2391	2716	284	1036	298	53	59	262							
Unpaved Area	0	0	0	0	0	0	0	0							
Equival. Area	2271.45	2580.2	269.8	984.2	283.1	50.35	56.05	248.9							

Pavement Type	Hard Paved	Unpaved
Runoff Coefficient	0.95	0.35

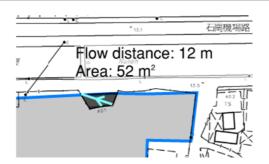
Calculation Table of Drainage System

US MH/PIT	DS MH/PIT	US GL	DS GL	Size	Gradient	Туре	USIL	DS IL	U/S MH/PIT	Length	V	Capacity	Catchments	Total Equivalent Area	ToC	Intensity	Total	Utilitizatio	Remar
				mm	1 in				TYPE	m	m/s##	m ³ /s		m ²	min	mm/hr ##	Discharge m ³ /s	n	
SP01	CP1.01	13.20	13.01	225	300	UC	12.98	12.79	SP	28	0.75	0.03	B2a	50.35	1.20	330	0.005	13.5%	
CP1.01	CP1.02	13.01	12.96	300	300	UC	12.71	12.64	CP	21.8	0.91	0.07	B1,B2a	333.45	1.82	315	0.03	39.8%	
CP1.02	CP1.03	12.96	12.92	450	200	UC	12.41	12.35	CP	11.5	1.47	0.26	A1a,B1,B2a	2604.90	2.22	306	0.22	83.8%	Existi
CP1.03	CP1.04	12.92	13.07	450	200	UC	12.35	12.31	CP	9.5	1.47	0.26	A1a,B1,B2a	2604.90	2.35	304	0.22	83.1%	Existi
CP1.04	CP1.05	13.07	12.74	450	200	UC	12.31	12.04	CP	53.9	1.47	0.26	A1a,B1,B2a	2604.90	2.46	302	0.22	82.5%	Exist
CP1.05	Existing Watercourse	12.74	12.74	450	30	UC	11.94	11.72	CP	6.8	3.78	0.68	A1a,A1b,A2,A3,B1,B2a,B2b,B3	6744.05	3.25	288	0.54	79.1%	Exist
SP02	CP2.01	13.20	13.55	225	150	UC	12.98	12.81	SP	24.5	1.07	0.05	B2b	56.05	1.20	330	0.01	10.7%	
CP2.01	CP2.02	13.55	13.63	525	200	UC	12.81	12.73	CP	15.4	1.62	0.40	A1b,A2,B2b	2906.05	1.58	320	0.26	64.7%	
CP2.02	CP2.03	13.63	13.44	525	200	UC	12.73	12.65	CP	16.9	1.62	0.40	A1b,A2,B2b,B3	3154.95	1.74	316	0.28	69.5%	
CP2.03	CP2.04	13.44	13.85	600	200	UC	12.65	12.56	CP	17.6	1.78	0.57	A1b,A2,A3,B2b,B3	4139.15	1.91	313	0.36	63.1%	
CP2.04	CP2.05	13.85	13.28	600	200	UC	12.56	12.41	CP	31	1.78	0.57	A1b,A2,A3,B2b,B3	4139.15	2.08	309	0.36	62.4%	
CP2.05	CP2.06	13.28	13.13	600	200	UC	12.41	12.18	CP	45.5	1.78	0.57	A1b,A2,A3,B2b,B3	4139.15	2.37	304	0.35	61.2%	
CP2.06	CP1.05	13.13	12.74	600	200	UC	12.18	11.94	CP	47.6	1.78	0.57	A1b,A2,A3,B2b,B3	4139.15	2.80	296	0.34	59.7%	
SP03	CP3.01	13.70	13.80	225	150	UC	13.48	13.37	SP	15.5	1.07	0.05	A2,B2b	325.85	1.20	330	0.03	62.0%	
CP3.01	CP2.02	13.80	13.63	225	150	UC	13.37	13.26	CP	16.4	1.07	0.05	A2,B2b	325.85	1.44	323	0.03	60.8%	
SP04	CP2.01	13.70	13.55	225	150	UC	13.48	13.33	SP	16.6	1.07	0.05	B2b	56.05	1.20	330	0.01	10.7%	

#SP: Start Point ## : With 11.1% rainfall increase as per Table 28 of SDM Corrigendum No. 1/2022.

Time of Concentration Checking

Catabrasat	Flow Dietones	Llighaat Laval		Gradient (per 100m)	to (min) =	tc=
Catchment	Flow Distance Highest	Highest Level	Lowest Level	= (H1-H2)/L x 100	0.14465L/ (H ^{0.2} A ^{0.1})	to + tf
A	L	H1	H2			
(m2)	(m)	(mPD)	(mPD)		(min)	(min)
52	12	13.3	13.2	0.833	1.2	1.2



<u>-APPENDIX B - PROPOSED SITE LAYOUT PLAN</u>

LEGEND

APPLICATION SITE

INGRESS / EGRESS

PARKING SPACE (PRIVATE CAR)

L/UL SPACE (LIGHT GOODS VEHICLE)

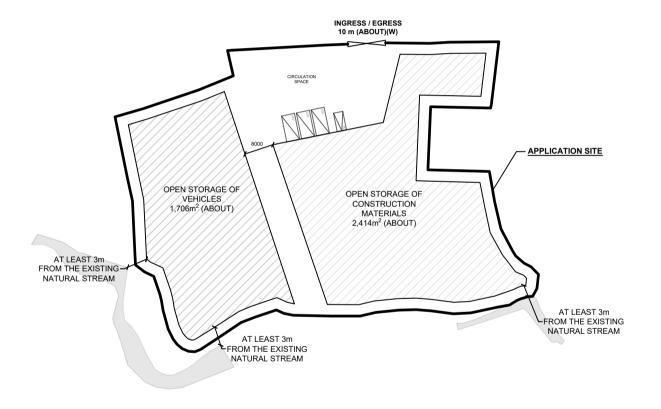
DEVELOPMENT PARAMETERS

APPLICATION SITE AREA : 6,427 m² (ABOUT) COVERED AREA NOT APPLICABLE UNCOVERED AREA : 6,427 m² (ABOUT)

: 4,120 m² OPEN STORAGE AREA (ABOUT) HEIGHT OF STACKING : NOT MORE THAN 3 m

NO STRUCTURE IS PROPOSED AT THE APPLICATION SITE.







PROPOSED TEMPORARY OPEN STORAGE OF CONSTRUCTION MATERIALS AND VEHICLES AND ASSOCIATED FILLING OF LAND FOR A PERIOD OF 3 YEARS

VARIOUS LOTS IN D.D. 106, SHEK KONG, YUEN LONG, NEW TERRITORIES

1:1000 @ A4

11.6.2025

PLAN 5

PARKING AND LOADING/UNLOADING PROVISION

NO. OF PRIVATE CAR PARKING SPACE

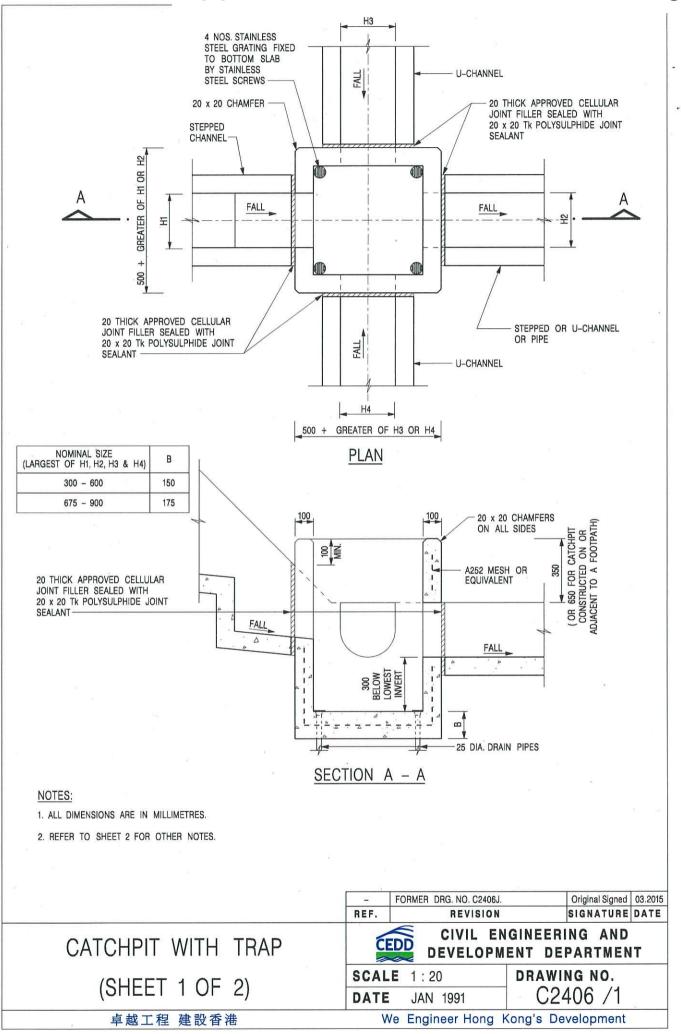
: 5 m (L) X 2.5 m (W) DIMENSION OF PARKING SPACE

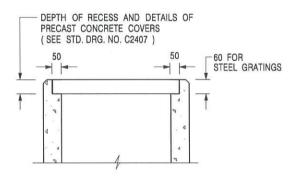
NO. L/UL SPACE FOR LIGHT GOODS VEHICLE

DIMENSION OF L/UL SPACE

: 7 m (L) X 3.5 m (W)

Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

REF.	REVISION	SIGNATURE	DATE
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
Α	MINOR AMENDMENT.	Original Signed	04.2016

CATCHPIT WITH TRAP (SHEET 2 OF 2)

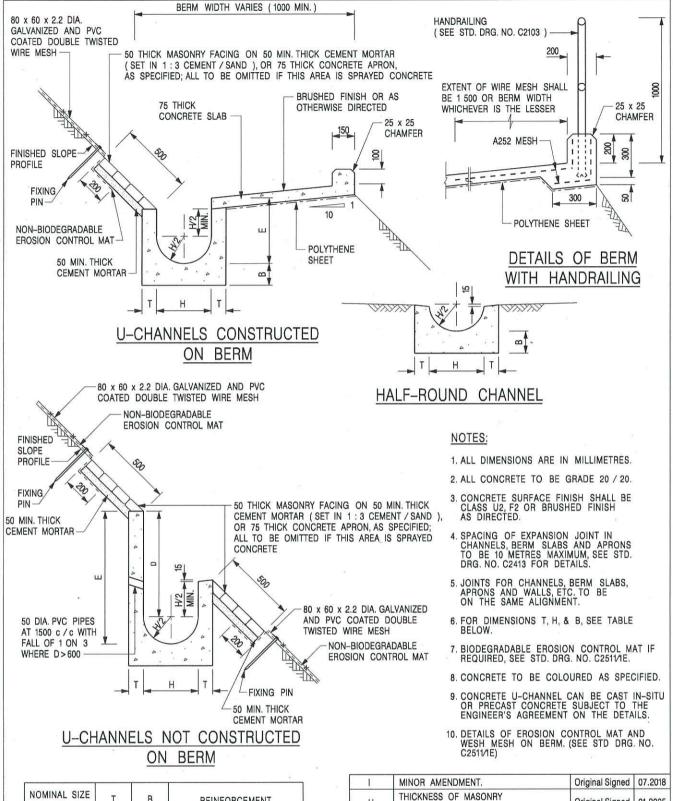


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 **DATE** JAN 1991

drawing no. C2406 /2A

卓越工程 建設香港



NOMINAL SIZE H	T	В	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100
375 - 600	100	150	WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
В	MINOR AMENDMENTS.	Original Signed	3.94
С	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
D	MINOR AMENDMENT.	Original Signed	08.2001
E	DRAWING TITLE AMENDED.	Original Signed	11.2001
F	GENERAL REVISION.	Original Signed	12.2002
G	MINOR AMENDMENT.	Original Signed	01.2004
Н	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
1	MINOR AMENDMENT.	Original Signed	07.2018

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

卓越工程 建設香港

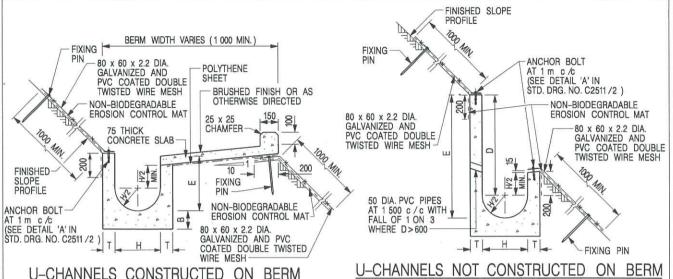
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:25

DATE JAN 1991

C2409l



U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

BIODEGRADABLE

EROSION CONTROL MAT

07.2018

12.2017

01.2005

12.2002

08 2001

6.99

3.94

10.92

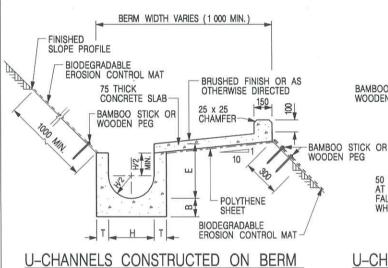
DATE

Original Signed

SIGNATURE

FINISHED SLOPE PROFILE

ш



WITH BIODEGRADABLE

EROSION CONTROL MAT

BAMBOO STICK OR WOODEN PEG

U-CHANNELS NOT CONSTRUCTED ON BERM

WITH BIODEGRADABLE

EROSION CONTROL MAT

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE TO BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- 5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- 6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- 7. FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- 8. MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- 10. THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	Ţ	В	REINFORCEMENT
300	80	100	A252 MESH PLACED
375 - 600	100	150	CENTRALLY AND T=100 WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

	DETAILS	OF	HALF-I	ROUN	D AND	
	U-CHAN	NELS	S (TYP	ЕВ-	- WITH	
I	FROSION	COL	NTROI	MAT	APRON	1)

6
CEDD
CEDD
nac

Н

G

F

E

D

C

В

A

REF.

BAMBOO STICK OR WOODEN PEG

50 DIA. PVC PIPES AT 1 500 c/c WITH FALL OF 1 ON 3

WHERE D>600

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC
DATE JAN 1991

MINOR AMENDMENT.

MINOR AMENDMENT

GENERAL REVISION.

MINOR AMENDMENT.

MINOR AMENDMENT.

MINOR AMENDMENT

FIXING DETAILS OF BIODEGRADABLE

150 x 100 UPSTAND ADDED AT BERM

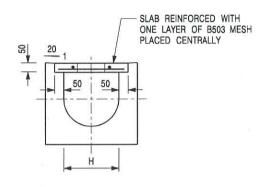
REVISION

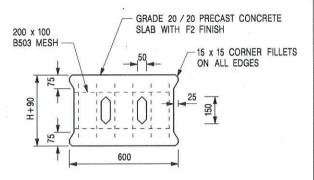
EROSION CONTROL MAT ADDED.

DIMENSION TABLE AMENDED

C2410

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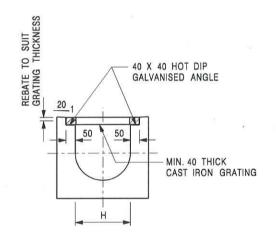


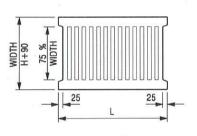
<u>PLAN OF SLAB</u>

TYPICAL SECTION

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)





L = 600mm FOR $H \le 375$ mm L = 400mm FOR H > 375mm

TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

REF.	REVISION	SIGNATURE	DATE
Α	CAST IRON GRATING AMENDED.	Original Signed	
В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	
D	NOTE 4 ADDED.	Original Signed	
Ε	NOTES 3 & 4 AMENDED.	Original Signed	

COVER SLAB AND CAST IRON GRATING FOR CHANNELS



CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:20
 DRAWING NO.

 DATE JAN 1991
 C2412E

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