Appendix 4

Traffic Impact Assessment

Reference number: CHK50793710

PROPOSED AMENDMENT TO THE NOTES OF THE APPROVED QUARRY BAY OZP RELATING TO THE "OTHER SPECIFIED USES" ZONE ANNOTATED "CULTURAL AND/OR COMMERCIAL LEISURE AND TOURISM RELATED USES"

INLAND LOTS 8590 RP (PART) AND 8723 RP (PART) AND ADJOINING GOVERNMENT LAND AND A SHORT STRIP OF GOVERNMENT LAND CONNECTING TO QUARRY BAY PARK PHASE II, HOI YU STREET, QUARRY BAY TRAFFIC IMPACT ASSESSMENT REPORT







PROPOSED AMENDMENT TO THE NOTES OF THE APPROVED QUARRY BAY OZP RELATING TO THE "OTHER SPECIFIED USES" ZONE ANNOTATED "CULTURAL AND/OR COMMERCIAL LEISURE AND TOURISM RELATED USES" INLAND LOTS 8590 RP (PART) AND 8723 RP (PART) AND ADJOINING GOVERNMENT LAND AND A SHORT STRIP OF GOVERNMENT LAND CONNECTING TO QUARRY BAY PARK PHASE II, HOI YU STREET, QUARRY BAY

TRAFFIC IMPACT ASSESSMENT REPORT

IDENTIFICATION TABLE	IDENTIFICATION TABLE						
Client/Project owner	Marine Riches III						
Project	Proposed Amendment to the Notes of the Approved Quarry Bay OZP relating to the "Other Specified Uses" zone annotated "Cultural and/or Commercial Leisure and Tourism Related Uses" Inland Lots 8590 RP (Part) and 8723 RP (Part) and Adjoining Government Land and a short strip of government land connecting to Quarry Bay Park Phase II, Hoi Yu Street, Quarry Bay						
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T. +852 2529 7037 • F. +852 2527 8490 • E. info@mvaasia.com • www.mvaasia.com



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Proposed Amendment to the Notes of the Approved Quarry Bay OZP relating to the "Other Specified Uses" zone annotated "Cultural and/or Commercial Leisure and Tourism Related Uses"

Inland Lots 8590 RP (Part) and 8723 RP (Part) and Adjoining Government Land, Hoi Yu Street, Quarry Bay

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1. INTRODUCTION

1.1 Background

- 1.1.1 The application site is situated at the land on north of Hoi Yu Street and the Island East Corridor ("IEC") along the Quarry Bay Waterfront Promenade. It consists of the part of land lot IL8590 R.P. and IL 8723 R.P., and the adjoining government land (which currently used as a temporary open space car parking) and a short strip of government land connecting to Quarry Bay Park ("the Land" or "the Application Site").
- 1.1.2 In 2001, General Building Plans ("GBPs") were approved in compliance with the "Industrial" zone in effect at the time for an Industrial Building ("the approved industrial building") for the lot IL8590 R.P. and IL8723 R.P.. Construction of the industrial building commenced with work on foundations in 2017 and were suspended when the Development Bureau approached to owner to see if they would consider a scheme which was more appropriate to the up-to-date planning intention and public aspirations for a more vibrant waterfront. The incomplete foundation works remain on the private lots are currently largely vacant.
- 1.1.3 Following the Hong Kong Island East Harbour-front Study ("HKIEHS") commenced by the Planning Department, which aims to improve the connectivity and pedestrian accessibility of the harbour-front, it was proposed to construct an about 2km long pedestrian Boardwalk underneath the Island Eastern Corridor from Oil Street to Hoi Yu Street ("the Boardwalk") to enhance the connectivity along the North Point promenade in 2012. An investigation study for the Boardwalk was then commenced in March 2015 by the Civil Engineering and Development Department ("CEDD") to review the feasibility. Subsequently, the contract for design and construction of the Boardwalk was awarded in December 2021 and the works was planned to be completed in end 2025.
- 1.1.4 Under the latest Approved Quarry Bay Outline Zoning Plan (No. S/H21/28, September 2010), main portion of the Application site is currently zoned as "Other Specified Uses ("OU(1)") annotated "Cultural and/ or Commercial, Leisure and Tourism Related Uses" with the uses of "Eating Place, Exhibition or Convention Hall, Place of Entertainment, Place of Recreation, Sports or Culture, Hotel, Office and Shop and Services" and etc. all under Column 2.
- 1.1.5 In order to provide a better waterfront to the public and to tally with the above-mentioned enhancement proposal/ the Boardwalk, the previous owner of the Land ("the previous owner") was invited by the Government to revisit the land use of the approved industrial building.
- 1.1.6 The previous owner was willing to partly surrender the original site (i.e. the approved industrial building) in exchange for the land (including part IL 8590 R.P., part IL 8723 R.P. and adjoining Government land) zoned OU(1) to form a new site for a combined waterfront development comprising of 4 hotel blocks, an office building, some retails and food & beverage facilities, which would have a more compatible land use to the waterfront then the approved industrial building, and the adjoining area zoned as "Other Specified Uses (OU)" annotated "Elevated Walkway" ("the elevated walkway").



- 1.1.7 Therefore, a section 16 planning application for the aforesaid waterfront development had been submitted to the Town Planning Board (TPB) for consideration in 2018 and it was approved with conditions in 2019. Hereafters refer to as the "previously approved scheme".
- 1.1.8 However, with reference to the Planning Statement, the implementation of the Approved Scheme was not possible, nor was the sale of the Approved Scheme. A stalemate has therefore been reached with the Approved Scheme. As the economic outlook has significantly changed since the scheme was approved in 2019 and both office and hotels are facing a difficult time with oversupply in the areas, the new owner, Marine Riches III ("the Applicant"), would like to revisit the use of the Land.
- 1.1.9 With the use of "Flat" under Column 2 of the latest OZP, the applicant intends to provide a space for specific cultural and tourism features with residential units. A Section 12A planning application is therefore required to be submitted for TPB's consideration.
- 1.1.10 MVA Hong Kong Limited (MVA) was commissioned as the Traffic Consultant to provide traffic engineering input and prepare a Traffic Impact Assessment (TIA) Study in support of the Section 12A planning application for the proposed development.

1.2 Objectives

- 1.2.1 The main objectives of this study are as follows:
 - to review the existing traffic conditions in the vicinity of the application site;
 - to review the traffic arrangement and internal transport facilities provision of the proposed development;
 - to estimate the additional traffic generations and attractions due to the proposed development;
 - to forecast the pedestrian and vehicular traffic demands on the adjacent road network in the design year 2033 (i.e. 3 years after completion, i.e. 2030 + 3 years);
 - to assess the possible traffic impacts due to the proposed development on the adjacent road network; and
 - to recommend any traffic improvement measures to mitigate the potential traffic problems on the road network, if required.



1.3 Structure of Report

- 1.3.1 Following this introductory chapter, there are **FIVE** further chapters:
 - Chapter 2 PROPOSED DEVELOPMENT, which presents the site location, development schedules, traffic arrangements and provisions of internal transport facilities for the proposed development;
 - Chapter 3 EXISTING TRAFFIC CONDITION, which describes the existing local road network in the vicinity of the proposed development; presents details of the traffic count survey and assess the existing traffic conditions;
 - Chapter 4 FUTURE TRAFFIC CONDITION, which assesses the annual growth rate to be adopted, estimates the potential traffic generations and attractions from the proposed development and future traffic flows on the surrounding road network;
 - Chapter 5 TRAFFIC IMPACT ASSESSMENT, which presents the findings of the traffic impact assessment for the reference and design scenarios, and recommends improvement measures, if necessary;
 - Chapter 6 SUMMARY AND CONCLUSION, which summarizes the findings of this study and presents the conclusions regarding the traffic issues associated with the proposed development.



2. PROPOSED DEVELOPMENT

2.1 Overview

- 2.1.1 The application site is situated at the land on north of Hoi Yu Street and the Island East Corridor ("IEC") along the Quarry Bay Waterfront Promenade. It consists of the part of land lot IL8590 R.P. and IL 8723, and the adjoining government land (which currently used as a temporary open space car parking) and a short strip of government land connecting to Quarry Bay Park. Location of the application site is shown in **Figure 2.1**.
- 2.1.2 The "OU(1)" portion of the site has a total area of about 8,532m². A combined waterfront development comprising of 3 domestic blocks with 189 225 residential units, 1 non-domestic block for cultural, leisure and entertainment uses, and some retails and Food & Beverage facilities will be provided. The development parameters of the previously approved scheme versus the current scheme are summarised in **Table 2.1** below.

Table 2.1 Key Development Parameters

	Previously Approved Scheme	Current Scheme
Site Area	About 8,532 m²	About 8,532 m²
Uses	Hotel & Office & Shop and Services, Eating Place, and Place of Recreation, Sport, or Culture	Cultural, Leisure and Entertainment uses, Easting Place, Shops and Services, and Flat
Plot Ratio (PR)	Approx. 4.4	Approx. 4.63
Building Height	About +34.0 mPD, +39.0 mPD and +41.0 mPD	About +26.3mPD, +40.7mPD and +44mPD
No. of Storeys	10 to 13 storeys including 1- storey of podium and 2 basement levels for car park	8, 13, 14 Including 1 storey of podium (i.e. eating place and shop and services at ground floor level) and 3 basement levels
Gross Floor Area (GFA)	Total – 37,155m² Incl. Retail – 11,537m² Office – 8,070m² Hotel – 17,548m² (with 400 guest rooms)	Total – about 39,480m ² Domestic – about 26,545m ² (189 - 225 units Unit size range in 45 to 105m ²) Non-domestic – about 12,935m ² (Including about 7,703m ² for cultural facilities & about 4,617m ² for retail/ F&B & about 615 m ² for covered public open space)

2.1.3 A new waterfront cultural venue for exhibitions or multi-function events will be provided. Although no specify events is planned for the time being, such events would be some cultural digital museum or art related exhibitions, etc. with target visitor trips of about 5,000 guests per day.

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- 2.1.4 Besides, approximately over 30% area on the ground plane of the proposed development will be given over to the public with aim to provide a potential spaces and environments for leisure or opportunity to engage with some special events like concerts, performances, etc.
- 2.1.5 The proposed development is planned to be completed by Q4 Year 2029 tentatively.

2.2 Proposed Development Access and Internal Transport Facilities Provision

- 2.2.1 The existing vehicular access of the approved industrial building is located at the local street connects perpendicularly to Hoi Yu Street. A separated vehicular access is provided at Hoi Yu Street for the existing temporary parking area. With the proposed development, a combined vehicular access at Hoi Yu Street will be provided on the east side of the site for access to its basement car park.
- 2.2.2 The required car parking and servicing provisions as stipulated under the latest Hong Kong Planning Standards and Guidelines (HKPSG) and the proposed provision are summarised in **Table 2.2** below.

Table 2.2 Required and Proposed Internal Transport Facilities Provision under the Latest HKPSG Requirement

	Parameters	HKPSG Requirements	Required Provision (nos.)	Proposed Provision (nos.)
• <u>Residential</u>				
Car Parking	3 Blocks,	Global Parking Standard (GPS) x R1 x R2 x R3 (1)	99 to 173	173
Visitor parking	225 flats	Additional 5 spaces per block	15	15
Motorcycle	(size range of 45 – 105	1 space per 100-150 flats	2 to 3	3
Loading/ Unloading (L/UL)	sqm)	Min 1 bay for every 800 flats or Min 1 bay for each block	Min. 3	3
Retail and F&B	<u>Facilities</u>			
Car Parking	GFA – about	1 space per 150 - 300m ²	16 to 31	31
Motorcycle	4,617m ²	5% to 10% of total car parking	1 to 4	4
Loading/ Unloading	4,017111	1 bay per 800-1,200m ²	4 to 6	6
<u>Cultural Facilitie</u>	<u>es</u>			
Car Parking	GFA – about	(No requirement)	N/A	50
Loading/ Unloading	7,703m ²	(No requirement)	N/A	3
Coach Parking	7,703111	No requirement)	N/A	5
• <u>Total</u>	1			
Car Parking		-	115 to 204	204
Car parking for cultural		-	-	50
Visitor parking		-	15	15
Total Car Parking		-	-	<u>269</u> (2)
Motorcycle		-	3 to 7	<u>7</u>
L/UL		-	7 to 9	9 (6 in LGV, 3 in HGV)
L/UL for cultural		-	-	3 (in HGV)

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Inland Lots 8590 RP (Part) and 8723 RP (Part) and Adjoining Government Land, Hoi Yu Street, Quarry Bay

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	Parameters	HKPSG Requirements	Required Provision (nos.)	Proposed Provision (nos.)
Total L/UL		-	-	<u>12</u>
Coach Parking		-	-	<u>5</u>

Remarks: (1) GPS = 1 space per 4-7 flats; Demand Adjustment Ratio (R1) = 4.1 (average flat size between 100m² and 130m²); Accessibility Adjustment Ratio (R2) = 0.75 (within 500m-radius of rail station); Development Intensity Adjustment Ratio (R3) = 1.0 (Domestic Plot Ratio between 2 and 5).

(2) Including 4 nos. accessible parking spaces.

- 2.2.3 A total of about 50m to 55m long laybys will be provided at ground floor on Hoi Yu Street. Car parking spaces and loading/unloading bays will be provided at the basement levels. The traffic arrangement on G/F, B1, B2 and B3 under the Master Layout Plan (MLP) is shown in Figure 2.2 to 2.5.
- 2.2.4 The car parking and servicing provision for residential and retail (with F&B facilities) portions will be provided in accordance with the upper-end of the HKPSG requirement.
- 2.2.5 Owing that there is no specific requirement on transport facilities provision for "cultural" use under the latest HKPSG, additional 50 private car parking spaces and 5 bays for coaches will be provided for the cultural venue based on the estimated traffic trips by the cultural venue (with a design capacity of about 5,000 guests per day) and making reference to the "Public Transport Strategy Study, June 2017" as published by the Transport and Housing Bureau. Detail please refer to the later **Section 4.5**.
- 2.2.6 In addition, it is understood that the loading/unloading demand for the cultural venue is generally low, hence 3 nos. loading/unloading bays is expecting sufficient to cope with the operational needs of the cultural venue based on the past experiences of the potential operator (i.e. the digital museum). Besides, loading/unloading activities for the cultural venue are usually carried out during the off-peak hours. The loading/unloading bays could be share used with the residential and retail portions to increase the efficiency.
- 2.2.7 In summary, the car parking and servicing provision will be provided in accordance with the upper-end of the HKPSG requirement, even more for car parking, as listed in the following to provide a sustainable parking supply for the proposed development as well as its neighbourhood area.

Table 2.3 **Summary Table of Proposed Internal Transport Provision**

II	Transport vision	Dimension (L) x (W) x (H)	Residential Retail and Cultural F&B Facilities Facilities		Total	
Car parking			173	31 50		260
Visitor park	ing	5m x 2.5m x 2.4m	15			269
Motorcycle	parking	2.4m x 1m x 2.4m	3	4		7
Loading/	LGV	7m x 3.5m x 3.6m	2	4	-	6
Unloading	MGV/HGV	11m x 3.5m x 4.7m	1	2 3		6
Coach Parki	ng	12m x 3.5m x 3.8m	-	-	5	5

Remarks: (1) Including 4 nos. accessible parking spaces in size of 5m x 3.5m x 2.4m.



2.3 Future Pedestrian Connection

- 2.3.1 Since Hoi Yu Street is ended with a cul-de-sac at its east-end, pedestrians access the Application Site from the nearest MTR Station (i.e. Quarry Bay Exit C) or the nearby public transport services mainly via King's Road, Java Road and Hoi Yu Street at present. Besides, there is no crossing point or footpath provided on the southern part of Hoi Yu Street. Hence, all pedestrians accessing the area rely on the northern footpath of Hoi Yu Street, which the width is about 3.6m at the west-end (i.e. outside the North Point Police Station) but eventually reduced to about 1.6m near the Quarry Bay Salt Water Pumping Station and in front of the Application Site at present.
- 2.3.2 The pedestrian network in the area will undergo a major change with the Boardwalk project to be completed in end 2025. An integrated walkway system, accommodating a walkway and cycle track, running along the waterfront from the new reclamation area north (or Oil Street at the western end) to Hoi Yu Street at the eastern end will be constructed. The area adjacent to the Quarry Bay Salt Water Pumping Station will be reformed as open space and walkway to provide a continuous pedestrian connection along the waterfront promenade. In particular, the existing footpath near the Quarry Bay Salt Water Pumping Station and in front of the Application Site will be widen to minimum 3.5m by realigning the carriageway of Hoi Yu Street under the Boardwalk project.
- 2.3.3 On the other hand, the piece of land underneath the IEC and adjoining the North Point Offtake Station will be converted into some Boardwalk facilities such as management office, public toilet, bike racks, open space and etc.. A new pedestrian crossing will also be provided near the Quarry Bay Salt Water Pumping Station. It is anticipated that a new route will be created for pedestrian accessing Hoi Yu Street and the pedestrians no longer rely on the northern footpath of Hoi Yu Street only in the future.
- 2.3.4 Besides, the access road from Hoi Chak Street to the Food and Environmental Hygiene Department (FEHD) Quarry Bay Depot GLA-THK-1038 ("the FEHD Depot") runs parallel to the IEC could be retained as a pedestrian access for those pedestrians coming from the south direction or Quarry Bay MTR Exit A & B.
- 2.3.5 In view of the above-mentioned, it revealed that the pedestrian connectivity and accessibility in the area will be improved significantly in the future with the Boardwalk. The existing and future pedestrian routes to/ from the Application Site are showed in **Figure 2.6**.

2.4 The Planned Elevated Walkway

2.4.1 The elevated walkway across the IEC as has been planned as an indicative alignment under the latest Approved Quarry Bay Outline Zoning Plan (No. S/H21/28, September 2010), shown in **Figure 2.7**, and was agreed to be implemented under the S16 Approved Scheme by the previous owner at the time the aforesaid pedestrian improvement works was yet proposed by the Boardwalk project.



- 2.4.2 However, given the enhanced at-grade pedestrian facilities in the future, in particular the creation of new pedestrian routes by the Boardwalk and connections to Quarry Bay Park, a more direct and shorter pedestrian route will be provided which reduces the need for the elevated walkway from the application site to the hinterland. There is no justification for an elevated walkway above Quarry Bay Park to be implemented as part of this application.
- 2.4.3 A shorter footbridge over the IEC from the access road/ pedestrian entrance near the Quarry Bay Park Office, crossing Hoi Yu Street to the proposed development is therefore proposed and the proposed elevated walkway will be maintained and managed by the relevant Government department.
- 2.4.4 An opening will be reserved at the proposed development for future connection to the elevated walkway.



3. EXISTING TRAFFIC CONDITION

3.1 Existing Road Network

- 3.1.1 The proposed development is situated at the land on north of Hoi Yu Street and the Island East Corridor ("IEC") along the Quarry Bay Waterfront Promenade. It is mainly served by King's Road, Java Road, Hoi Chak Street and Hoi Yu Street.
- 3.1.2 King's Road is an east-west primary distributor connecting to Causeway Road to the west, and Shau Kei Wan Road to the east. It is the key distributor providing access for traffic between Quarry Bay and other area such as Causeway Bay, Chai Wan and the rest of Hong Kong Island and beyond.
- 3.1.3 Java Road is a one-way eastbound district distributor connecting Electric Road to the west and King's Road to the east. It is essentially parallel to King's Road. It connects with Hoi Yu Street adjacent to its junction with King's Road and provides the main ingress and egress traffic route to the proposed development.
- 3.1.4 Hoi Chak Street is currently one of the key local access routes serving the Quarry Bay area. It is a two-way single four-lane carriageway connecting Hoi Yu Street to the north and Hoi Tai Street to the south.
- 3.1.5 Hoi Yu Street is a single 2 lanes local distributor which operates in two-way direction and ended with a cul-de-sac at its east-end. It connects with Java Road to the West and ended at the temporary open space car park near the entrance of the Eastern Harbour Crossing to the East. It is the only vehicular access serving the Quarry Bay Waterfront area and the application site.

3.2 Existing Public Transport Services

- 3.2.1 The Quarry Bay MTR Station is located at the south-west of the proposed development with about 8 minutes walking journey from the Exit C and about 12 minutes walking journey from the Exit A/B. The alternative access from rail transport is from the Taikoo MTR Station which can be accessed from the Tai Koo Shing area via the existing footbridge network with about 20 minutes walking journey.
- 3.2.2 Currently, numerous road-based public transport services are operating within 500m catchment area of the proposed development. These include up to 54 routes for Franchised Buses, 5 routes for Green Mini-Buses (GMB) and numerous routes for Public Light Buses (PLB). Tram services are also provided on King's Road. Figure 3.1 showed the existing public transport facilities in the vicinity of the proposed development.
- 3.2.3 The proposed development is considered well served by the comprehensive public transport services within its 500m catchment area.



3.3 Existing Vehicular Traffic Condition

- 3.3.1 The key vehicular ingress and egress routes to/ from the proposed development are shown in **Figure 3.2**.
- 3.3.2 Eleven nearby road junctions, as listed in **Table 3.1**, were identified to be critical in this traffic impact assessment. Locations of these critical junctions are also indicated in **Figure 3.2**, whilst their existing junction arrangements and Method of Control (MOC) are shown in **Figures 3.3** to **3.13** respectively.
- 3.3.3 Manual classified traffic count survey was carried out at the identified critical junctions during the morning peak (07:30 to 09:30), noon (12:00-14:00) peak and evening (17:00 to 19:00) peak hours on a typical weekday in June 2025 (i.e. 26 June 2025) to obtain the up-to-date vehicular traffic pattern in the vicinity of the proposed development. Based on the observed traffic flows, it revealed that the AM peak, Noon peak and PM peak hours occurred from 08:30AM to 09:30AM, 12:00PM to 13:00 PM and 17:30PM to 18:30PM respectively.
- 3.3.4 The 2025 observed peak hour traffic flows are presented in **Figure 3.14**. Detailed calculation of the assessed existing operational performance of the identified junctions are attached in **Appendix A** and the results are summarized in **Table 3.1** below.

Table 3.1 Identified Critical Junctions

Ref.	Fig	lations	Type of		Year 2025 RC/ RFC (1)(2)	
No.	No.	Junctions	Junction	AM Peak	Noon Peak	PM Peak
J1	3.3	Hoi Yu Street/ Hoi Chak Street	Priority	0.026	0.035	0.026
J2	3.4	Java Road/ Hoi Yu Street	Signalised	23%	82%	88%
J3	3.5	King's Road/ Java Road	Signalised	>100%	>100%	>100%
J4	3.6	Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street	Signalised	68%	>100%	82%
J5	3.7	King's Road/ Finnie Street	Signalised	>100%	>100%	93%
J6	3.8	King's Road/ Model Lane	Signalised	>100%	>100%	>100%
J7	3.9	Java Road/ Man Hong Street	Signalised	>100%	>100%	>100%
J8	3.10	King's Road/ Healthy Street West/ Man Hong Street	Signalised	>100%	>100%	>100%
19	3.11	Tsat Tsz Mui Road/ Healthy Street West	Signalised	>100%	>100%	>100%
J10	3.12	Tsat Tsz Mui Road/ Tin Chiu West	Signalised	>100%	>100%	>100%
J11	3.13	King's Road/ Tin Chiu Street	Signalised	>100%	>100%	>100%

Notes: (1) Zero Reserve Capacity (RC) represents signalised junction operating at capacity. Positive RC represents signalised junction operating with spare capacity. Negative RC means signalised junction is overloaded.

Proposed Amendment to the Notes of the Approved Quarry Bay OZP relating to the "Other Specified Uses" zone annotated "Cultural and/or Commercial Leisure and Tourism Related Uses"

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⁽²⁾ Ratio of Flow to Capacity (RFC) = 1.0 represents priority junction/roundabout operating at capacity. RFC < 1.0 represents priority junction/roundabout operating with spare capacity. RFC>1.0 means priority junction/roundabout is overloaded.



3.3.5 As indicated in above **Table 3.1**, all the identified critical junctions are currently operating within their capacities during the peak hours.

3.4 Existing Pedestrian Traffic Condition

- 3.4.1 Pedestrian count survey was also carried out at the identified critical footpaths/crossing along Hoi Yu Street, Java Road and Hoi Chak Street/ Finnie Street/ Hoi Tai Street (based on the pedestrian route to/from the proposed development as shown in **Figure 2.6**) to obtain the upto-date pedestrian traffic pattern in the vicinity of the proposed development in June 2025 (i.e. 25 June 2025 and 28 Jun 2025) during the time periods below:
 - During the morning peak (07:30AM to 09:30AM), noon peak (12:00-14:00) and evening peak (17:30PM to 19:30PM) periods on a typical weekday; and
 - During the morning peak (07:30AM to 09:30AM), noon peak (12:00-14:00) and evening peak (17:30PM to 19:30PM) periods on a typical weekend.
- 3.4.2 It was observed that the AM, noon and PM peak 15-min pedestrian flows occurred from 08:45AM to 09:00AM, 12:25PM to 12:40PM and 18:00PM to 18:15PM respectively on weekday; and 09:00AM to 09:15AM, 12:40PM to 12:55PM and 17:05PM to 17:20PM respectively on weekend. The 2025 observed peak pedestrian flows are presented in **Figure 3.16**.
- 3.4.3 Currently, pedestrians accessing Quarry Bay waterfront promenade and the application site are mainly via Hoi Yu Street and some via Hoi Chak Street/ Finnie Street/ Hoi Tai Street. A Level-Of-Service (LOS) assessment was therefore carried out to access the existing performance of the identified critical sections of footpaths and crossings based on the observed peak 15-min pedestrian flows. Locations of these footpaths and crossing are shown in Figure 3.15 and the summarised in Table 3.2 and Table 3.3 below:

Table 3.2 Existing Performances of Identified Critical Sections of Footpaths/Cautionary Crossing

Critical Footpath/ Cautionary Crossing (1)	Clear Width (m)	Eff. Width (m) ⁽²⁾	Period		2-way Pedestrian Peak Flow (ped/15-min) ⁽⁴⁾	Flow Rate (ped/min/ m) ⁽⁴⁾	LOS ⁽⁵⁾
				AM Peak	30	2.5	Α
Location A			Weekday	Noon Peak	25	2.1	Α
Hoi Yu St.		0.8 ⁽³⁾		PM Peak	60	5.0	Α
Hoi fu St.	1.03	0.8	Weekend	AM Peak	60	5.0	Α
				Noon Peak	35	2.9	Α
				PM Peak	70	5.8	Α
		2.5		AM Peak	30	0.8	Α
			Weekday	Noon Peak	25	0.7	Α
Location B	3.5			PM Peak	60	1.6	Α
Hoi Yu St.	3.5	2.5		AM Peak	60	1.6	Α
			Weekend	Noon Peak	35	0.9	Α
				PM Peak	70	1.9	Α
Location C	2.8	1.8	Weekday	AM Peak	165	6.1	Α

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Critical Footpath/ Cautionary Crossing (1)	Clear Width (m)	Eff. Width (m) ⁽²⁾	Pe	riod	2-way Pedestrian Peak Flow (ped/15-min) ⁽⁴⁾	Flow Rate (ped/min/ m) ⁽⁴⁾	LOS ⁽⁵⁾
Java Road				Noon Peak	315	11.7	Α
				PM Peak	125	4.6	Α
				AM Peak	95	3.5	Α
			Weekend	Noon Peak	45	1.7	Α
				PM Peak	60	2.2	Α
				AM Peak	55	1.8	Α
			Weekday	Noon Peak	35	1.2	Α
Location D		2		PM Peak	85	2.8	Α
Java Road	3	2		AM Peak	15	0.5	Α
			Weekend	Noon Peak	25	0.8	Α
				PM Peak	35	1.2	Α
		1.8	Weekday	AM Peak	30	1.1	Α
Location E	2.8			Noon Peak	85	3.1	Α
				PM Peak	45	1.7	Α
Java Road	2.0	1.0	Weekend	AM Peak	10	0.4	Α
				Noon Peak	10	0.4	Α
				PM Peak	15	0.6	Α
				AM Peak	120	4.4	Α
			Weekday	Noon Peak	305	11.3	Α
Location F	2.8	1.8		PM Peak	115	4.3	Α
Hoi Yu St.	2.0	1.0		AM Peak	15	0.6	Α
			Weekend	Noon Peak	20	0.7	Α
				PM Peak	25	0.9	Α
				AM Peak	10	0.2	Α
			Weekday	Noon Peak	20	0.4	Α
<u>Location H</u> Hoi Chak St.	4	3		PM Peak	20	0.4	Α
	4	3		AM Peak	10	0.2	Α
			Weekend	Noon Peak	10	0.2	Α
				PM Peak	15	0.3	Α

Notes: (1) Refer to Figure 3.15.

3.4.4 The result in the above **Table 3.2** indicated that all the identified critical sections of footpaths/cautionary crossing are currently operating with a LOS level of "A".

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⁽²⁾ Effective width is clear width deducting the horizontal clearance, generally 0.5m (i.e. Dead Width) on each side.

⁽³⁾ Min. 0.8m clear width shall be adopted according to HCM 2000.

⁽⁴⁾ Observed 2-way Ped. Peak Flow rounded to the nearest 5; Flow Rate rounded to the nearest 0.1.

⁽⁵⁾ According to the Highway Capacity Manual 2000 (HCM 2000), the criteria range from LOS "A" (best) to LOS "F" (worst). In general, LOS C is considered as an optimal level of service and is preferable for all new walkways.



Table 3.3 Existing Performances of Identified Critical Sections of Crossing

Critical Crossing	Period		Eff. Crosswalk Width (m)	2-way Pedestrian Peak Flow (ped/15-min) ⁽²⁾	LOS of Cross walk	Area of Waiting Space (m)	LOS of Waiting Space ⁽³⁾
		AM Peak		25	Α		Α
Location G	Weekday	Noon Peak		30	Α	Ni a urbla i 1 O	Α
Location G		PM Peak	4	85	Α	North:18 South:16.8	Α
Java Road		AM Peak	4	15	Α	30utn:16.8	Α
	Weekend	Noon Peak		10	Α		Α
		PM Peak		15	Α		Α
		AM Peak		20	Α		Α
Location K	Weekday	Noon Peak		70	Α		В
		PM Peak	2.4	20	Α	North:11	Α
Hoi Tai Street		AM Peak	3.1	25	Α	South:15.6	Α
	Weekend	Noon Peak		25	Α		Α
		PM Peak		10	Α	, <u> </u>	Α
		AM Peak		280	F	North:12.8 South:31.5	Α
Location L Hoi Kwong Street	Weekday	Noon Peak		425	F		С
		PM Peak	4.2	280	Е		Α
	Weekend	AM Peak		45	Α		Α
		Noon Peak		150	D		Α
		PM Peak		120	С		Α
		AM Peak		80	С		Α
_	Weekday	Noon Peak		525	F	North:14.3 South:16.2	С
Location M		PM Peak		185	Е		Α
Finnie		AM Peak	2.5	60	В		Α
Street	Weekend	Noon Peak		160	Е		Α
		PM Peak		135	D		Α
		AM Peak		70	С		Α
	Weekday	Noon Peak		505	F		D
Location N		PM Peak	2.5	180	Е	North:16.2	Α
Finnie		AM Peak	2.5	30	Α	South:9.5	Α
Street	Weekend	Noon Peak		100	D		Α
		PM Peak		85	С		Α
		AM Peak		15	Α		Α
	Weekday	Noon Peak		30	Α		А
Location O		PM Peak		25	Α	North:16.2	A
Hoi Tai		AM Peak	2	55	С	South:11.6	A
Street	Weekend	Noon Peak		85	D		A
		PM Peak		75	С		Α

Notes: (1) Refer to Figure 3.15.

3.4.5 The result in the above **Table 3.3** indicated that the identified critical crossings at Java Road are currently operating with a LOS level of "B" or above. However, the identified critical crossings at Hoi Kwong Street, Finnie Street and Hoi Tai Street are currently operating at their capacities.

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⁽²⁾ Observed 2-way Ped. Peak Flow rounded to the nearest 5; Flow Rate rounded to the nearest 0.1.

⁽³⁾ According to the Highway Capacity Manual 2000 (HCM 2000), the criteria range from LOS "A" (best) to LOS "F" (worst). In general, LOS C is considered as an optimal level of service and is preferable for all new walkways.



4. FUTURE TRAFFIC CONDITION

4.1 Design Year

4.1.1 The proposed development is planned to be completed with full intended operation by Q4 Year 2029 tentatively. In order to assess the possible traffic impacts to the local road network due to the proposed development, Year 2033 (3 years after completion, i.e. 2030 + 3 years) was adopted as the design year for this study.

4.2 Future Local Road Network and Pedestrian Linkage

- 4.2.1 It is anticipated no major road works and infrastructure to be constructed within the study area. Hence, it is considered that the traffic pattern in the local area is expecting to be similar as existing. The existing junction layouts were therefore adopted for the traffic impact assessment in the reference and design scenarios.
- 4.2.2 According to the project webpage of "Boardwalk underneath Island Eastern Corridor", the Boardwalk is targeted to be fully completed by end 2025. Hence, the future pedestrian connection with the Boardwalk and its associated pedestrian trips were considered in the traffic impact assessment.
- 4.2.3 Meanwhile, an elevated walkway connects between the waterfront area at Hoi Yu Street and Quarry Bay Park was planned to be constructed under the latest Approved Quarry Bay OZP as mentioned in **Section 2**. Subject to future agreement between the applicant and government as to cover the cost of the bridge, it is assumed that the elevated walkway will be in placed in the reference and design scenarios.

4.3 Adopted Traffic Growth

4.3.1 In order to conduct the traffic forecast of Year 2033, a traffic growth factor was determined for the study area based on the following approaches.

Historical Traffic Growth and Planning Data

4.3.2 There are Annual Traffic Census (ATC) traffic count stations available in the vicinity of the proposed development. The annual traffic counts at these stations over the period between Year 2018 to Year 2023 reported in the latest ATC report are summarized in the following Table 4.1.



Table 4.1 Annual Traffic Census (ATC) Traffic Counts Between Year 2018 to 2023

Road Name	Stn.	Annual Average Daily Traffic (AADT)						Average Annual
	No.	2018	2019	2020	2021	2022	2023	Growth Rate
Man Hong Street (Java Road - King's Road)	1613	21,720	21,340	23,780	25,860	24,680	25,390	3.17%
Java Road (Island Eastern Corridor - King's Road)	1635	21,010	20,820	18,770	20,600	19,410	19,830	-1.15%
King's Road (Healthy St W - Java Road)	2020	19,840	19,500	18,600	19,440	19,120	18,640	-1.24%
Java Road (Tong Shui Road - Tin Chiu Street)	2040	16,970	16,830	16,000	16,320	15,920	15,520	-1.77%
Total		79,540	78,490	77,150	82,220	79,130	79,380	<u>-0.25%</u>

4.3.3 As indicated in **Table 4.1** above, the average annual traffic growth pattern in the vicinity of the proposed development is in a declining trend of -0.25% per annum over the past 6 years.

Traffic Growth Rate by Population Data

4.3.4 The 2019-based Territorial Population and Employment Data Matrix (TPEDM) planning data for the Eastern District published by the Planning Department has also been considered. The population and employment planning data in Year 2019, Year 2026 and Year 2031 in Eastern District is summarized in **Table 4.2**.

Table 4.2 2019-based Territorial Population and Employment Data Matrix

	Population	Employment	Total					
Eastern District								
Year 2019	568,150	331,300	899,450					
Year 2026	533,100	324,900	858,000					
Year 2031	506,050	317,250	823,300					
Average Annual	-0.96%	+0.36%	<u>-0.73%</u>					

Note:

(1) Source from webpage of Planning Department

 $(https://www.pland.gov.hk/pland_en/info_serv/statistic/tpedm19/2019_based_Open_TPEDM_(Data).pdf)$

- 4.3.5 As indicated in the above **Table 4.2**, Eastern District has a negative average annual growth rate of -0.73% on the population and employment for the next few years.
- 4.3.6 As a conservative approach, a +0.50% annual growth rate is adopted to develop the 2033 reference traffic flows for the assessment. It is deemed sufficient to allow for any unexpected future growth as a result of some changes in land use or new developments in the study area.

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4.4 Traffic Generations of Nearby Planned and Committed Developments

4.4.1 In addition to the annual traffic growth rate, the major planned and committed developments in the vicinity of the proposed redevelopment as listed in **Tables 4.3** below have also been considered in the reference traffic forecast. Locations of the aforesaid developments are shown in **Figure 4.1**.

Table 4.3 Major Planned and Committed Developments Nearby

Major Planned and Committed Development	Anticipated Completion Year
The Boardwalk underneath Island Eastern Corridor	End 2025
Proposed Redevelopment of AIA Tower at No. 734 King's Road	Uncertain
Proposed Residential Development at 16-94 Pan Hoi Street and 983-987A King's Road, Quarry Bay, Hong Kong [A/H21/151]	Uncertain
Proposed Residential Development at No. 10-12 & 14-16 Mount Parker Road [A/H21/157]	Uncertain
Proposed Residential Development at 56-76 Kai Yuen Street [A/H8/435]	Uncertain

- 4.4.2 It is noted that the existing AIA Tower at No. 734 King's Road is planned to be redeveloped as a comprehensive building with office and retails uses. Based on the information obtained from Bravo, i.e. the online building records by Buildings Department (BD), a General Building Plan (GBP) has been submitted to BD and approved in 2022. It is assumed that the proposed redevelopment will be in place in Year 2033 and its traffic generations were considered in the traffic impact assessment although the existing building is yet demolished today.
- 4.4.3 A rezoning planning application proposing a commercial development in the existing "Residential (Group A)" Zone and an area shown as "Road" at 16-94 Pan Hoi Street and 983-987A King's Road, Quarry Bay [A/H21/151A] was submitted to Town Planning Board (TPB)in 2020. Although the aforesaid application has been rejected by TPB, the site is always permitted to be redeveloped as residential use under the current OZP. Therefore, traffic generations for residential development have been adopted into the assessment as a conservative approach (i.e. traffic trips rate of office > traffic trips rate of residential).
- 4.4.4 Another planning application proposing a minor relaxation of building height from +120mPD to +139.75mPD for the proposed residential development at 992-998 King's Road and 2-16 Mount Parker Road, Quarry Bay was submitted to TPB and approved with conditions in December 2022. It is observed that construction works have been commenced on-site. Hence, the development is anticipated will be in place in Year 2033 and its traffic generations were considered in the assessment.

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- 4.4.5 Also, a Section 16 application proposing a minor relaxation of Plot Ratio and Building Height Restrictions at Land falling within "Comprehensive Development Area (2)" zone and an area shown as 'Road' at Kai Yuen Street, North Point was submitted to TPB and approved with conditions in February 2023. It is observed that construction works have been commenced onsite. Hence, the development is anticipated will be in place in Year 2033 and its traffic generations were considered in the assessment also.
- 4.4.6 Traffic generations and attractions of the nearby planned/ committed developments are listed in **Table 4.4** to **Table 4.8** below.

Table 4.4 Adopted Traffic Trips of the Boardwalk underneath Island Eastern Corridor

	AM	Peak	PM Peak	
	Generation	Attraction	Generation	Attraction
Expected Two-way Volumes at Hoi Yu Street	309		309 134	

Remarks: (1) Source from Agreement No. CE 41/2014(HY) Boardwalk underneath Island Eastern Corridor – Investigation Assessment on Pedestrian and Cyclist Demand for Proposed Boardwalk

Table 4.5 Adopted Traffic Trips of the Proposed Redevelopment of AIA Tower at No. 734 King's Road

			AM	Peak	PM Peak		
			Generation	Attraction	Generation	Attraction	
Land Use			Office with portion of retail				
Key Param	eters		About 27,870m ²				
Office	Trip	Rates	0.1703	0.2452	0.1573	0.1175	
(pcu/hr/100m2 GFA, mean) (1)							
Total Traffic Trips (pcu/hr)			48	69	44	33	

Remarks: (1) Refer to TPDM Volume 1 Chapter 3 Appendix 1 Table 1.

(2) Source from Bravo – online building records.

Table 4.6 Adopted Traffic Trips of the Proposed Residential Development at 16-94 Pan Hoi Street and 983-987A King's Road, Quarry Bay

	AM	Peak	PM Peak				
	Generation	Attraction	Generation	Attraction			
Land Use	Residential						
Key Parameters	900 flats, average flat size about 60m²						
Residential Trip Rates (pcu/hr/flat, Private Housing: High Density/R(A) & Avg. flat size 70m², mean) (1)	0.0718	0.0425	0.0286	0.037			
Total Traffic Trips (pcu/hr)	65	39	26	34			

Remarks: (1) Refer to TPDM Volume 1 Chapter 3 Appendix 1 Table 1.

(2) Source from Town Planning Board Statutory Planning Portal. Planning Application No. A/H21//157 refers.

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Table 4.7 Adopted Traffic Trips of the Proposed Residential Development at No. 10-12 & 14-16 Mount Parker Road

	AM	Peak	PM Peak		
	Generation	Attraction	Generation	Attraction	
Land Use		Resid	ential		
Key Parameters	592 flats, average flat size about 67m ²				
Residential Trip Rates (pcu/hr/flat, Private Housing: High Density/R(A) & Avg. flat size 60m², mean) (1)	0.0888	0.0515	0.0356	0.048	
Total Traffic Trips (pcu/hr)	53	31	22	29	

Remarks: (1) Refer to TPDM Volume 1 Chapter 3 Appendix 1 Table 1.

Table 4.8 Adopted Traffic Trips of the Proposed Residential Development at 56-76
Kai Yuen Street

	AM	Peak	PM Peak				
	Generation	Attraction	Generation	Attraction			
Land Use	Residential						
Key Parameters	1461 flats, average flat size about 85m ²						
Residential Trip Rates (pcu/hr/flat, Private Housing: High Density/R(A) & Avg. flat size 60m², mean) (1)	0.1887	0.0942	0.0862	0.1214			
Total Traffic Trips (pcu/hr)	276	138	126	178			

Remarks: (1) Refer to TPDM Volume 1 Chapter 3 Appendix 1 Table 1.

4.4.7 The 2033 reference traffic flows were therefore developed based on the 2025 observed traffic flow with the aforesaid growth rate and traffic generations of the nearby planned and committed developments adopted as shown in **Figure 4.2**.

4.5 Traffic Generations of Proposed Development

4.5.1 As mentioned in **Section 1**, the lot IL8590 R.P. and IL8723 R.P. could either be developed as an industrial building or the previously approved scheme (i.e. hotel + office + retails) together with the government lot (which is currently used as a temporary open space car parking) and a short strip of government land connecting to Quarry Bay Park. The traffic trips by the temporary open space car parking were therefore deducted in the calculation of the traffic forecast.

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⁽²⁾ Source extracted from Monthly Digest, Building Department.

⁽²⁾ Source from Town Planning Board Statutory Planning Portal. Planning Application No. A/H8/432 refers



- 4.5.2 The cultural venue/ digital museum is planned to have a servicing capacity of about 5,000 guests per day. Visitors would flow through the space with an expected stay of about 45-60 mins, subject to future operation, for each immersive experience (i.e. "show"). Hence, visitors would come and go constantly within the opening hours between 10:00AM to 19:00PM. Therefore, it was assumed that about 11.1% of the visitors would come during the PM peak hours (i.e. average of the 9 hours and nil during the AM peak as it is anticipated yet opened) and the estimated traffic generations by the cultural venue/ digital museum were superimposed onto the reference traffic flows to assess the potential traffic impact due to the proposed development.
- 4.5.3 The traffic trips to be generated by the cultural venue/ digital museum were estimated by making reference to the traffic modal split mentioned in Chapter 3 of the "Public Transport Strategy Study, June 2017" as published by Transport and Housing Bureau. The result is summarized in **Table 4.9** below.

Table 4.9 Anticipated Traffic Generation by the Cultural Venue/ Digital Museum

Transport Mode	Distribution ⁽¹⁾ (%)		Guest ople)	Additional Vehicular Traffic Trips (pcu) ⁽⁴⁾
Rail	38.7%	0 (215)		-
Ferry	0.9%	0 (5)		-
Franchised Bus	27%	0(150)	0 (450)	-
PLB	13.5%	0 (75)		-
Tram	0.9%	0 (5)		-
Private Vehicle	10%	0 (5	6) ⁽²⁾	0 (28)
SPB	1.8%	0 (10)(3)		0 (1)
Taxi	7.2%	0 (40)(2)		0 (20)
Total	100%	0 (556)		0 (49)

Remarks: (1) According to Paragraph 3.12 in Chapter 3 of the "Public Transport Strategy Study, June 2017";

Vehicular Traffic

4.5.4 The estimated vehicular traffic trips to be generated by the proposed development during the peak hours are summarized in **Table 4.10** below.

⁽²⁾ Assuming 2 passengers per private vehicle and 2 passengers per taxi;

⁽³⁾ Assuming 24 passengers per SPB;

⁽⁴⁾ Adopted pcu factor: 1 for private vehicle and 1.5 for SPB.



Table 4.10 Anticipated Traffic Generation of the Proposed Development

Tueffic Tuine	AM	Peak	Noon Peak		PM Peak		
Traffic Trips	Gen.	Att.	Gen.	Att.	Gen.	Att.	
Proposed Development (A	()						
Residential Units & Size				han 225 fla 45 – 105 so			
Residential Trip Rates (pcu/hr/flat, Private Housing/ Medium-Density R(B), mean) ⁽¹⁾	0.1887	0.0942	0.0862	0.1214	0.0862	0.1214	
Traffic Trips (pcu/hr)	43	22	20	28	20	28	
Retail and F&B Facilities GFA (m²)			4,6	17m²			
Retail Trip Rates (pcu/hr/100m ² GFA, mean) ⁽¹⁾	0.2296	0.2434	0.3100	0.3563	0.3100	0.3563	
Traffic Trips (pcu/hr)	11	12	15	17	15	17	
Total Traffic Trips (pcu/hr)	54	34	35	45	35	45	
Traffic Generation by the Cultura	al Venue/	Digital N	luseum	(B)			
Traffic Trips (pcu/hr)	0	0	49	49	49	49	
Existing temporary open space of	ar park at	t the Adjo	ining Gov	ernment La	ınd	(C)	
Observed Trips (pcu/hr)	20	0	0	15	0	15	
Different in Traffic Trips (A + B – C)							
Net Traffic Trips (pcu/hr)	34	34	84	79	84	79	
2-way Vehicular Trips (pcu/hr)	_	8	1	<u>63</u>	10	<u> </u>	

Remarks: (1) Refer to TPDM Volume 1 Chapter 3 Appendix 1 Table 1.

- 4.5.5 As indicated in **Table 4.10** above, it is anticipated that the proposed development would generate an additional two-way traffic trips of some 68 pcu/hr in the AM peak, some 163 pcu/hr in Noon peak, and some 163 pcu/hr during the PM peak.
- 4.5.6 Based on the anticipated traffic generations and key vehicular ingress and egress routes to/ from the proposed development as showed in **Figure 3.2**, the estimated traffic generations were then superimposed onto the year 2033 reference traffic flows to derive the year 2033 design traffic flows as shown in **Figure 4.3**. The distribution of development traffic trips is shown in **Figure 4.4**.

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⁽²⁾ As no information is provide in TPDM for noon peak, trip rate of PM peak was therefore adopted for Noon peak as a conservative approach.



<u>Pedestrian Traffic</u>

4.5.7 Pedestrian traffic generations of the proposed development were also estimated. In-house data, as summarized in **Table 4.11**, was adopted to estimate the traffic trips by the residential and retail portions and the anticipated traffic generations by the cultural venue/ digital museum were also considered.

Table 4.11 Adopted Pedestrian Generation Rates

	AM Peak		Noon	Peak	PM Peak	
	Generation	Attraction	Generation	Attraction	Generation	Attraction
Residential Trip Rates (Private Housing) (ped/hr/flat) ⁽¹⁾	0.7230	0.4400	0.3660	0.2640	0.3660	0.2640
Retail Trip Rates (ped/15 mins/100m ² GFA)	0.1005	0.1417	0.2936	0.273	0.2936	0.273

Remarks: (1) Based on in-house data.

4.5.8 The estimated pedestrian traffic trips to be generated by the proposed development during the peak hours are summarized in **Table 4.12** below.

Table 4.12 Anticipated Pedestrian Generation of Proposed Development

Tueffic Tuine	AM P	eak	Noon	Noon Peak		PM Peak		
Traffic Trips	Gen.	Att.	Gen.	Att.	Gen.	Att.		
Proposed Development (A)								
Residential Units & Size			more that ange of 4					
Residential Trip Rates (ped/15 mins/flat)	0.7230	0.4400	0.3660	0.2640	0.3660	0.2640		
Pedestrian Peak Flows (ped/ 15 mins)	41	25	21	15	21	15		
Retail and F&B Facilities GFA (m ²)	4,617m²							
Retail Trip Rates (ped/15 mins/100m ² GFA)	0.1005	0.1417	0.2936	0.273	0.2936	0.273		
Pedestrian Peak Flows (ped/ 15 mins)	1	2	4	3	4	3		
Total Pedestrian Trips (ped/ 15 mins)	42	27	25	18	25	18		
Total 2-way Pedestrian Trips (ped/15 mins)	69)	43		43			
Traffic Generation by the Cultural Venue/ Digital Museum (B)								
·		gitai ivius	1		I			
Pedestrian Trips (ped/15 mins)	0 113 113					L3		
Total Pedestrian Trips (A + B)								
• •	וט		I					
Total 2-way Pedestrian Trips (ped/15 mins)	<u>69</u>	<u>-</u>	<u>1!</u>	<u>56</u>	<u>15</u>	<u> 66</u>		

Remarks: (1) Trip rate of PM peak was adopted for Noon peak as a conservative approach.

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⁽²⁾ Trip rate of PM peak was adopted for Noon peak as a conservative approach.



- 4.5.9 It is anticipated that the proposed development would generate a 2-way pedestrian trips of 69 pedestrians per peak 15 minutes during the AM peak, 156 pedestrians per peak 15 minutes during the Noon peak, and 156 pedestrians per peak 15 minutes during the PM peak.
- 4.5.10 With the cultural venue/ digital museum, the proposed development is expecting to become one of the attractions of the Quarry Bay Waterfront Promenade. Although majority of the trips by the cultural venue/ digital museum are anticipated coming from/ travelling to the Boardwalk, it was assumed that half of the pedestrian trips would access from Hoi Yu Street/ Java Road and the other half would access via the elevated walkway as a conservative approach.
- 4.5.11 Similar to the vehicular traffic forecast, the estimated pedestrian traffic generations were then superimposed onto the year 2033 reference traffic flows to derive the year 2033 design traffic flows based on the anticipated pedestrian traffic generations as showed in **Figure 3.16**.



5. TRAFFIC IMPACT ASSESSMENT

5.1 Junction Performance Assessment

5.1.1 In order to investigate the potential traffic impacts to the local road network upon completion of the proposed development, operational performances assessment for the identified critical junctions were conducted, for both the reference and design scenarios in Year 2033. The assessment results are summarised in **Table 5.1** below.

Table 5.1 Junction Performance of Critical Junctions in Year 2033

			RC/ RFC (1)(2)							
Ref.	Junctions	Type of Junction	R Y	Design Year 2033						
			AM Peak	Noon Peak	PM Peak	AM Peak	Noon Peak	PM Peak		
J1	Hoi Yu Street/ Hoi Chak Street	Priority	0.052	0.059	0.039	0.175	0.303	0.275		
J2	Java Road/ Hoi Yu Street	Signalised	18%	74%	80%	14%	57%	72%		
J3	King's Road/ Java Road	Signalised	>100%	>100%	>100%	>100%	>100%	>100%		
J4	Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street	Signalised	61%	>100%	72%	60%	>100%	72%		
J5	King's Road/ Finnie Street	Signalised	92%	96%	40%	92%	96%	40%		
J6	King's Road/ Model Lane	Signalised	>100%	>100%	93%	>100%	>100%	84%		
J7	Java Road/ Man Hong Street	Signalised	76%	85%	82%	74%	81%	77%		
J8	King's Road/ Healthy Street West/ Man Hong Street	Signalised	95%	87%	97%	93%	84%	93%		
19	Tsat Tsz Mui Road/ Healthy Street West	Signalised	>100%	>100%	>100%	>100%	>100%	>100%		
J10	Tsat Tsz Mui Road/ Tin Chiu West	Signalised	58%	75%	83%	57%	73%	81%		
J11	King's Road/ Tin Chiu Street	Signalised	>100%	>100%	>100%	>100%	>100%	>100%		

Notes: (1) Zero Reserve Capacity (RC) represents signalised junction operating at capacity. Positive RC represents signalised junction operating with spare capacity. Negative RC means signalised junction is overloaded.

⁽²⁾ Ratio of Flow to Capacity (RFC) = 1.0 represents priority junction/roundabout operating at capacity. RFC < 1.0 represents priority junction/roundabout operating with spare capacity. RFC>1.0 means priority junction/roundabout is overloaded.



5.1.2 As indicated in **Table 5.1** above, all the identified critical junctions would still be operating within their capacities in both the reference and design scenarios in Year 2033. However, the junction performance of J2 would drop from 18% to 14% (i.e. less than 15% which is the preferable benchmark). In addition to the traffic flows to be generated by the new developments in Taikoo Place area (the newly opened Taikoo Place 2 building which is expected not yet fully occupied and the potential Pan Hoi Street redevelopment), it is anticipated that the left turn traffic of Java Road (eastbound) at the junction of Java Road/ Hoi Yu Street would become busier. In this regard, junction improvement to the Java Road/ Hoi Yu Street, in particular the left turn movement, is suggested. Details of the proposed junction improvement works are shown in **Figure 5.1**.

5.2 Proposed Junction Improvement Measure

- 5.2.1 Under the proposed junction improvement scheme, additional traffic lane (i.e. two lanes in total) would be provided for the left turn traffic of Java Road (eastbound). Alignment of the existing kerbline outside the police station is proposed to be modified slightly to provide more manoeuvring space for the traffic turning to Hoi Yu Street. With due consideration the site constraint of the existing high mast lighting at J2, it is proposed to convert the section of Hoi Yu Street (westbound) from existing 2-lane to a 1-lane carriageway. Hence the disturbance and nuisance during the construction could also be minimised.
- 5.2.2 Besides, the footpath at Hoi Yu Street outside the police station and the existing cautionary pedestrian crossing at Hoi Chak Street would also be widened under the proposed junction improvement scheme.
- 5.2.3 On the other hand, improvement to the junction of Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street, i.e. J4, is also proposed to cope with the pedestrian walking environment. Details please refer to **Section 5.4**.
- 5.2.4 Operational performances of J1 (i.e. Junction Hoi Yu Street/ Hoi Chak Street), J2 (i.e. Java Road/ Hoi Yu Street) and J4 (i.e. Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street) under the proposed improvement scheme were assessed and the result is summarized in **Table 5.2** below.

Table 5.2 Junction Performance of J1, J2 and J4 Under Proposed Improvement Schemes

				Ye	ayout With Improvement loon PM AM Noon PM Peak Peak Peak Peak Peak					
Ref. No.	Junctions	Type of Junction		Existing Layout	•	With	Improv	PM		
		Junetion	AM	Noon	PM	AM	Noon	PM		
			Peak	Peak	Peak	Peak	Peak	Peak		
J1	Hoi Yu Street/ Hoi Chak Street	Priority	0.175	0.303	0.275	0.175	0.303	0.275		
J2	Java Road/ Hoi Yu Street	Signalised	14%	57%	72%	25%	42%	41%		
J4 ⁽³⁾	Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street	Signalised	60%	>100%	72%	39%	>100%	>100%		

Notes: (1) Zero Reserve Capacity (RC) represents signalised junction operating at capacity. Positive RC represents signalised junction operating with spare capacity. Negative RC means signalised junction is overloaded.

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- (2) Ratio of Flow to Capacity (RFC) = 1.0 represents priority junction/roundabout operating at capacity. RFC < 1.0 represents priority junction/roundabout operating with spare capacity. RFC>1.0 means priority junction/roundabout is overloaded.
- (3) The junction of J4 will be modified to improve the pedestrian situation as shown in Figure 5.2.
- 5.2.5 Swept path analysis of long vehicles accessing the junctions under the proposed arrangement have also been conducted and the drawings are attached in **Appendix B** for reference.

5.3 Pedestrian Assessment

5.3.1 Based on the estimated pedestrian generations by the proposed development as summarised in **Table 4.3** and the key pedestrian routes accessing the proposed development as showed in **Figure 2.6**., performance of the identified critical sections of footpath in Year 2033 were assessed and the result is shown in **Figure 3.16** summarized in **Table 5.3** below.

Table 5.3 Year 2033 Performances of Identified Critical Sections of Footpaths/Cautionary Crossing

Critical Footpath/ Cautionary Crossing ⁽¹⁾	Clear Width (m)	Eff. Width (m) ⁽²⁾	Period		2-way Pedestrian Peak Flow (ped/15-min) ⁽⁴⁾	Flow Rate (ped/min/m) ⁽⁴⁾	LOS ⁽⁵⁾
				AM Peak	105	2.8	Α
Location A			Weekday	Noon Peak	145	3.9	Α
	3.5	2.5		PM Peak	180	4.8	Α
Hoi Yu St.	3.5	2.5		AM Peak	130	3.5	Α
			Weekend	Noon Peak	150	4.0	Α
				PM Peak	185	4.9	Α
				AM Peak	100	2.7	Α
			Weekday	Noon Peak	130	3.5	Α
Location B	3.5	2 5		PM Peak	170	4.5	A A A
Hoi Yu St.	3.5	2.5	Weekend	AM Peak	125	3.3	Α
				Noon Peak	135	3.6	Α
				PM Peak	170	4.5	Α
		1.8	Weekday	AM Peak	230	8.5	Α
				Noon Peak	410	15.2	Α
Location C	2.8			PM Peak	205	7.6	Α
Java Road			Weekend	AM Peak	155	5.7	Α
				Noon Peak	125	4.6	Α
				PM Peak	135	5.0	Α
			Weekday	AM Peak	70	2.3	Α
				Noon Peak	70	2.3	Α
Location D	3	2		PM Peak	120	4.0	Α
Java Road	3	2		AM Peak	25	0.8	Α
			Weekend	Noon Peak	55	1.8	Α
				PM Peak	65	2.2	Α
				AM Peak	35	1.3	Α
			Weekday	Noon Peak	95	3.5	Α
Location E	2.8	1.8		PM Peak	55	2.0	Α
Java Road	2.0	1.0		AM Peak	15	0.6	Α
			Weekend	Noon Peak	20	0.7	Α
				PM Peak	25	0.9	Α
	2.8	1.8	Weekday	AM Peak	125	4.6	Α

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				Noon Peak	320	11.9	Α
				PM Peak	120	4.4	Α
Location F Hoi Yu St.				AM Peak	20	0.7	Α
HOI YU St.			Weekend	Noon Peak	25	0.9	Α
				PM Peak	30	1.1	Α
				AM Peak	20	0.4	Α
			Weekday	Noon Peak	35	0.8	A A A
Location H	4	3		PM Peak	35	0.8	
Hoi Chak St.	4	3		AM Peak	20	0.4	Α
			Weekend	Noon Peak	25	0.6	Α
				PM Peak	30	0.7	Α

Notes: (1) Refer to Figure 3.15.

- (2) Effective width is clear width deducting the horizontal clearance, generally 0.5m (i.e. Dead Width) on each side.
- (3) Min. 0.8m clear width shall be adopted according to HCM 2000.
- (4) Observed 2-way Ped. Peak Flow rounded to the nearest 5; Flow Rate rounded to the nearest 0.1.
 (5) According to the Highway Capacity Manual 2000 (HCM 2000), the criteria range from LOS "A" (best) to LOS "F" (worst). In general, LOS C is considered as an optimal level of service and is preferable for all new walkways.
- 5.3.2 The result in the above Table 5.3 indicated that all the identified critical sections of footpaths/cautionary crossing would be operating with a LOS level of "A".



Table 5.4 Year 2033 Performances of Identified Critical Sections of Crossing

Critical Crossing	Period		Eff. Crosswalk Width (m)	2-way Pedestrian Peak Flow (ped/15-min) ⁽²⁾	LOS of Cross walk	Area of Waiting Space (m)	LOS of Waiting Space ⁽³⁾
		AM Peak	4	40	Α		Α
	Weekday	Noon Peak		65	Α		Α
<u>Location G</u> Java Road		PM Peak		120	В	North:18	Α
		AM Peak	4	30	Α	South:16.8	Α
	Weekend	Noon Peak		40	Α		Α
		PM Peak		45	Α		Α
		AM Peak		25	Α		A
	Weekday	Noon Peak	1	75	В		В
Location K		PM Peak	2.1	25	Α	North:11	Α
Hoi Tai Street		AM Peak	3.1	30	Α	South:15.6	Α
Street	Weekend	Noon Peak		30	Α		Α
		PM Peak		15	Α		Α
	Weekday	AM Peak	4.2	295	F	North:12.8 South:31.5	Α
Location L Hoi Kwong		Noon Peak		445	F		С
		PM Peak		295	Е		Α
	Weekend	AM Peak		50	Α		Α
Street		Noon Peak		160	D		Α
		PM Peak		130	С		Α
		AM Peak		85	С	North:14.3 South:16.2	Α
	Weekday	Noon Peak	1	550	F		С
Location M		PM Peak		195	Е		Α
Finnie	Weekend	AM Peak	2.5	65	С		Α
Street		Noon Peak		170	Е		Α
		PM Peak		145	D		Α
		AM Peak		150	Е		Α
	Weekday	Noon Peak		645	F		E
Location N		PM Peak	2.5	305	F	North:16.2	В
Finnie		AM Peak	2.5	100	D	South:9.5	Α
Street	Weekend	Noon Peak		215	Е		Α
		PM Peak		200	Е		Α
		AM Peak		90	С		Α
	Weekday	Noon Peak		150	D		В
Location O	,	PM Peak	_	145	D	North:16.2	Α
Hoi Tai		AM Peak	2	125	D	South:11.6	Α
Street	Weekend	Noon Peak		200	Е		Α
		PM Peak		190	Е		Α

Notes: (1) Refer to Figure 3.15.

5.3.3 The result in the above **Table 5.4** indicated that the identified critical crossings at Java Road would be operating with a LOS level of "B" or above. However, the identified critical crossings at Hoi Kwong Street, Finnie Street and Hoi Tai Street would be operating at their capacities.

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⁽²⁾ Observed 2-way Ped. Peak Flow rounded to the nearest 5; Flow Rate rounded to the nearest 0.1.

⁽³⁾ According to the Highway Capacity Manual 2000 (HCM 2000), the criteria range from LOS "A" (best) to LOS "F" (worst). In general, LOS C is considered as an optimal level of service and is preferable for all new walkways.



5.4 Proposed Pedestrian Improvement Measure

- 5.4.1 Owning to the constraints of limited space at the well-developed urban area, it is proposed to realign and re-arrange the pedestrian crossings at the junction of Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street as shown in **Figure 5.2** to improve the pedestrian walking environment.
- 5.4.2 Operational performances of the crossings at the junction of Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street under the proposed improvement scheme were assessed and the result is summarized in **Table.5** below.

Table 5.5 Performances of Crossing Locations M, N & O Under Proposed Improvement Scheme

			Year 2033								
Critical			Ex	xisting Layout	İ	With Improvement					
Crossing	Pe	riod	Eff.	LOS of	LOS of	Eff.	LOS of	LOS of			
(1)			Crosswalk	Crosswalk	Waiting	Crosswalk	Crosswalk	Waiting			
			Width (m)	(2)	Space ⁽²⁾	Width (m)	(2)	Space ⁽²⁾			
		AM Peak		E	Α		С	Α			
Location N/	Weekday	Noon Peak		F	E	3.5	F	LOS of Waiting Space ⁽²⁾			
<u>Location N'</u> Hoi Tai		PM Peak	2.5	F	В		E	Α			
Street	Weekend	AM Peak		D	Α		В	Α			
Street		Noon Peak		E	Α		D	Α			
		PM Peak		E	Α		С	Α			
		AM Peak		С	Α	3.5	Α	Α			
Location O'	Weekday	Noon Peak		D	В		С	Α			
Location O' Hoi Tai		PM Peak	2	D	Α		В	Α			
Street		AM Peak	2	D	Α		В	A			
Stieet	Weekend	Noon Peak		E	Α		С	Α			
		PM Peak		E	Α		С	Α			

Notes: (1) Refer to Figure 5.2.

5.4.3 As shown in Table 5.5 above, the performance of the critical crossing at Hoi Tai Street (Location N and O) could be improved but still could not reach a level of C or above under the proposed improvement scheme.

⁽²⁾ According to the Highway Capacity Manual 2000 (HCM 2000), the criteria range from LOS "A" (best) to LOS "F" (worst). In general, LOS C is considered as an optimal level of service and is preferable for all new walkways.



6. SUMMARY AND CONCLUSION

6.1 Summary

- 6.1.1 The application site is situated at the land on north of Hoi Yu Street and the Island East Corridor ("IEC") along the Quarry Bay Waterfront Promenade. It consists of the part of land lot IL8590 R.P. and IL 8723 R.P., and the adjoining government land (which currently used as a temporary open space car parking) and a short strip of government land connecting to Quarry Bay Park ("the Land" or "the Application Site"). Location of the application site is shown in **Figure 2.1**.
- In 2001, General Building Plans ("GBPs") were approved in compliance with the "Industrial" zone in effect at the time for an Industrial Building ("the approved industrial building") for the lot IL8590 R.P. and IL8723 R.P.. However, construction works of the approved industrial building was not commenced for a decade year. In order to provide a better waterfront to the public and to tally with the harbourfront enhancement proposal/ the Boardwalk by the Development Bureau and relevant parties, the previous owner of the Land ("the previous owner") thus willing to partly surrender the Original Site in exchange for the government land zoned OU(1) to form a new Site with an area of 8,532m² for a proposed waterfront development comprising of 4 hotel blocks, an office building, and some retail and Food & Beverage facilities and OU for elevated walkway. A section 16 planning application for the aforesaid waterfront development had been submitted to the Town Planning Board (TPB) for consideration in 2018 and it was approved with conditions in 2019 ("the S16 Approved Scheme").
- 6.1.3 However, with reference to the Planning Statement, the implementation of the Approved Scheme was not possible, nor was the sale of the Approved Scheme. A stalemate has therefore been reached with the Approved Scheme. As the economic outlook has significantly changed since the scheme was approved in 2019 and both office and hotels are facing a difficult time with oversupply in the areas, the new owner, Marine Riches III ("the Applicant"), would like to revisit the use of the Land.
- 6.1.4 With the use of "Flat" under Column 2 of the latest OZP, the applicant intends to provide a space for specific cultural and tourism features with residential units. A Section 12A planning application is therefore required to be submitted for TPB's consideration.
- 6.1.5 It is proposed to provide a combined waterfront development comprising of 3 domestic blocks with 189 225 residential units, 1 non-domestic block for cultural, leisure and entertainment uses, and some retails and Food & Beverage facilities. A new waterfront cultural venue for exhibitions or multi-function events will be provided. Although no specify events is planned for the time being, such events would be some cultural digital museum or art related exhibitions, etc. with target visitor trips of about 5,000 guests per day.
- 6.1.6 The proposed development is planned to be completed by Q4 Year 2029 tentatively.



Development Access and Internal Transport Facilities Provision

- 6.1.7 The existing vehicular access of the approved industrial building is located at the local street connects perpendicularly to Hoi Yu Street. A separated vehicular access is provided at Hoi Yu Street for the existing temporary parking area. With the proposed development, a combined vehicular access at Hoi Yu Street will be provided on the east side of the site for access to its basement car park.
- 6.1.8 All the car parking and servicing provision will be provided in accordance with the upper-end of the HKPSG requirement in order to provide more parking supply to the neighbourhood area. Additional 50 private car parking spaces, 2 bays for coaches and 3 nos. loading/ unloading bays will be provided for the cultural venue based on the estimated traffic trips by the cultural venue.
- 6.1.9 In summary, the proposed car parking and servicing provision are summarised below:

Internal Transport Provision		Dimension (L) x (W) x (H)	Residential		Cultural Facilities	Total
Car parking		5m x 2.5m x 2.4m	173	31	50	269
Visitor parking		3111 X 2.3111 X 2.4111	15	-	-	209
Motorcycle	parking	2.4m x 1m x 2.4m	3	4		7
Loading/	LGV	7m x 3.5m x 3.6m	2	4	-	6
Unloading MGV/HGV		11m x 3.5m x 4.7m	1	2	3	6
Coach Park	ing	12m x 3.5m x 3.8m	-	- 5		5

Remarks: (1) Including 4 nos. accessible parking spaces in size of 5m x 3.5m x 2.4m.

Future Pedestrian Connection & The Planned Elevated Walkway

- 6.1.10 Hoi Yu Street is ended with a cul-de-sac at its east-end. At present, pedestrians accessing the Application Site from the nearest MTR Station (i.e. Quarry Bay Exit C) or the nearby public transport services mainly travel from King's Road, Java Road to Hoi Yu Street. There is no crossing point or footpath provided on the southern part of Hoi Yu Street. Hence, all pedestrians accessing the area rely on the northern footpath of Hoi Yu Street, which the width is about 3.6m at the west-end (i.e. outside the North Point Police Station) but eventually reduced to about 1.6m near the Quarry Bay Salt Water Pumping Station and in front of the Application Site.
- 6.1.11 The pedestrian network in the area will undergo a major change with the Boardwalk project to be completed in end 2025. The existing footpath near the Quarry Bay Salt Water Pumping Station and in front of the Application Site will be widen to minimum 3.5m. A new pedestrian crossing will also be provided. As such, a new route is expected to be created for pedestrian accessing Hoi Yu Street. There is also an alternative pedestrian access for pedestrians coming from the south direction or Quarry Bay MTR Exit A & B via the access road from Hoi Chak Street to the FEHD Depot which runs parallel to the IEC.
- 6.1.12 Given the enhanced at-grade pedestrian facilities in the future, in particular the creation of new pedestrian routes by the Boardwalk, there is no justification for an elevated walkway above Quarry Bay Park to be implemented as part of this application. Instead, a short elevated

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footbridge walkway over IEC is proposed subject to future agreement between the Applicant and the Government on covering the cost of the bridge. The proposed elevated walkway will be maintained and managed by the relevant Government department.

Vehicular Traffic Impact Assessment

- 6.1.13 To appraise the existing traffic conditions in the study area, classified traffic counting surveys were conducted to cover the surrounding road network affected by the proposed development. The operational performances of the identified critical junctions were assessed with the observed traffic flows collected in 2025. The result of the assessment indicated that all critical junctions identified in the study are operating within their capacities.
- 6.1.14 The year 2033 reference traffic flows were derived from the observed 2025 traffic flows adopting a conservative growth rate +0.5% per annum. The reference traffic forecast has also taken into account the major planned and committed developments in the vicinity of the proposed development.
- 6.1.15 The traffic generations and attractions of the proposed development were estimated according to the trip generation rates given from the latest TPDM. Also, the traffic trips to be generated by the cultural venue/ digital museum were estimated by making reference to the traffic modal split mentioned in Chapter 3 of the "Public Transport Strategy Study, June 2017" as published by Transport and Housing Bureau. It is anticipated that the proposed development would generate an additional two-way traffic trips of some 68 pcu/hr in the AM peak, some 163 pcu/hr in Noon peak, and some 163 pcu/hr during the PM peak.
- 6.1.16 With the proposed development, the estimated traffic flows in design year 2033 were estimated by superimposing the traffic generations from the proposed development over the year 2033 reference traffic flows on the road network covered within the study area.
- 6.1.17 The performance of those critical junctions identified in the study area would all be operating within their capacities in both the reference and design scenarios in year 2033. It revealed that the proposed development would not induce significant traffic impacts on the adjacent road network.
- 6.1.18 Hoi Yu Street is the only carriageway connects to the application site. Together with the traffic flows to be generated by the new developments in Taikoo Place area (the newly opened Taikoo Place 2 building which is expected yet fully occupied and the potential Pan Hoi Street redevelopment), it is anticipated that the left turn traffic of Java Road (eastbound) at the junction of Java Road/ Hoi Yu Street would become busier. Junction improvement measure to the Java Road/ Hoi Yu Street, in particular the left turn movement, is suggested.
- 6.1.19 Under the proposed junction improvement scheme, additional traffic lane (i.e. two lanes in total) would be provided for the left turn traffic of Java Road (eastbound). Alignment of the existing kerbline outside the police station would be modified slightly to provide more manoeuvring space for the traffic turning to Hoi Yu Street. Section of Hoi Yu Street (westbound) would also be converted from existing 2-lane to a 1-lane carriageway to maximize the overall junction capacity.

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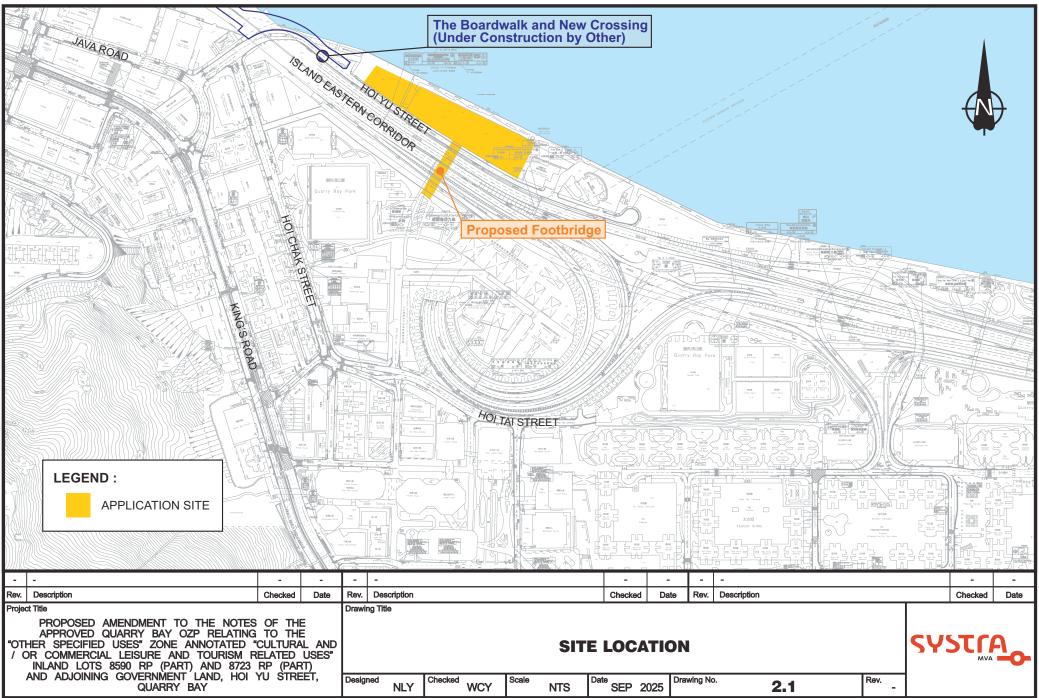


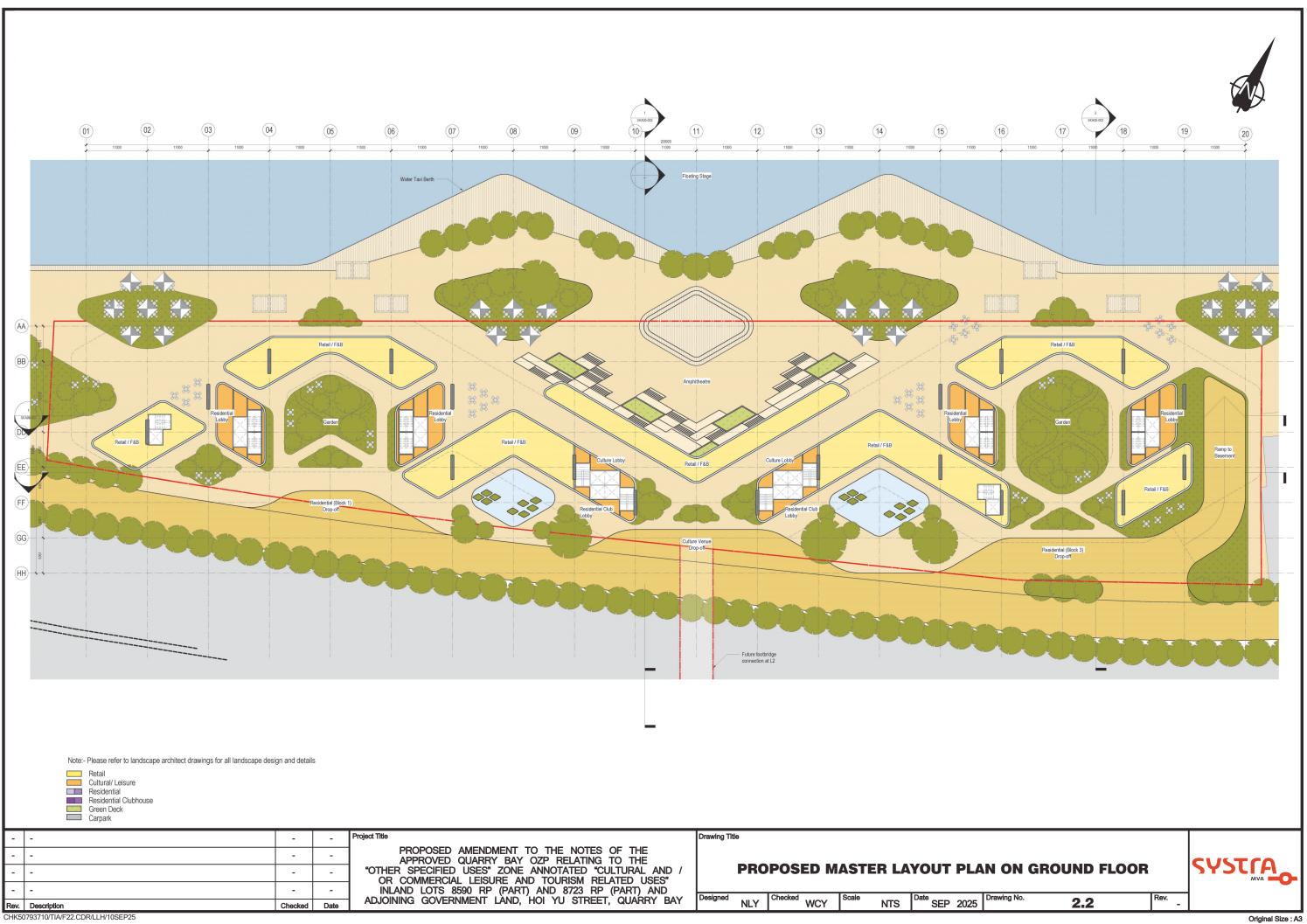
Pedestrian Traffic Impact Assessment

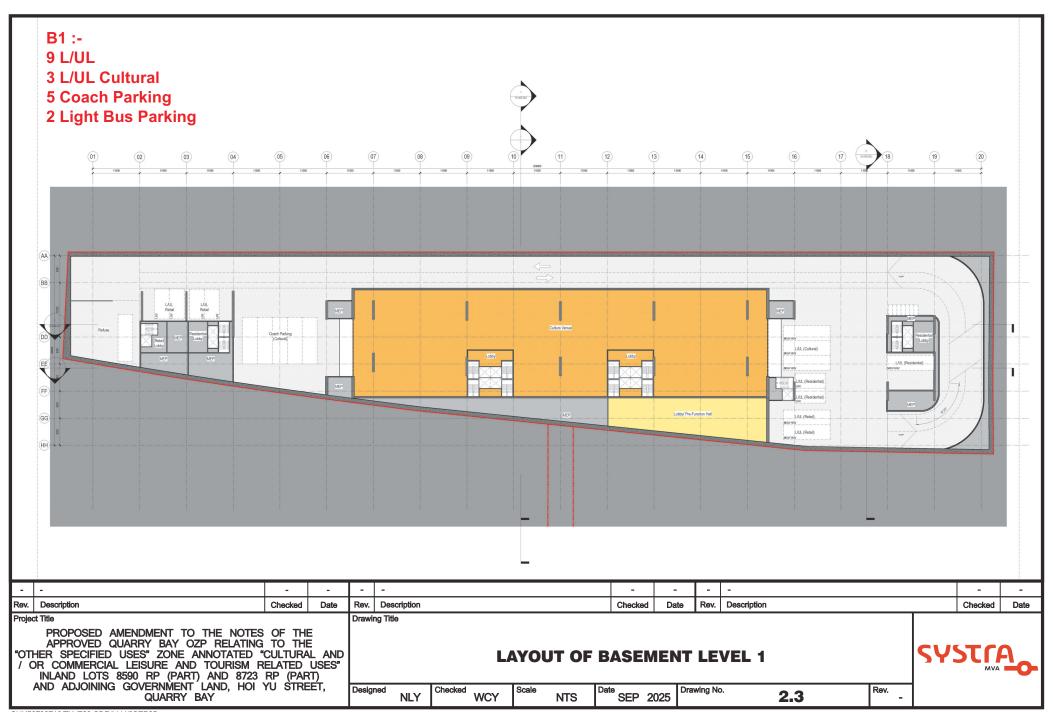
- 6.1.20 Capacities of the identified critical sections of footpaths and crossing at Hoi Yu Street, Java Road and Hoi Chak Street/ Finnie Street/ Hoi Tai Street were assessed. The performance assessment indicated that all the identified critical sections of footpaths/cautionary crossing and the identified critical crossings at Java Road would be operating with a LOS of not less than C during peak hours with the proposed development. However, the identified critical crossings at Hoi Kwong Street, Finnie Street and Hoi Tai Street would be operating at their capacities.
- 6.1.21 Improvement measure to the crossings at Hoi Chak Street/ Finnie Street/ Hoi Tai Street was therefore proposed. By converting the staggered crossings to straight crossing at the junction of Hoi Tai Street/ Hoi Chak Street/ Finnie Street/ Hoi Kwong Street, the performance of the crossings at Hoi Tai Street could be improved but still could not reach a level of C or above.

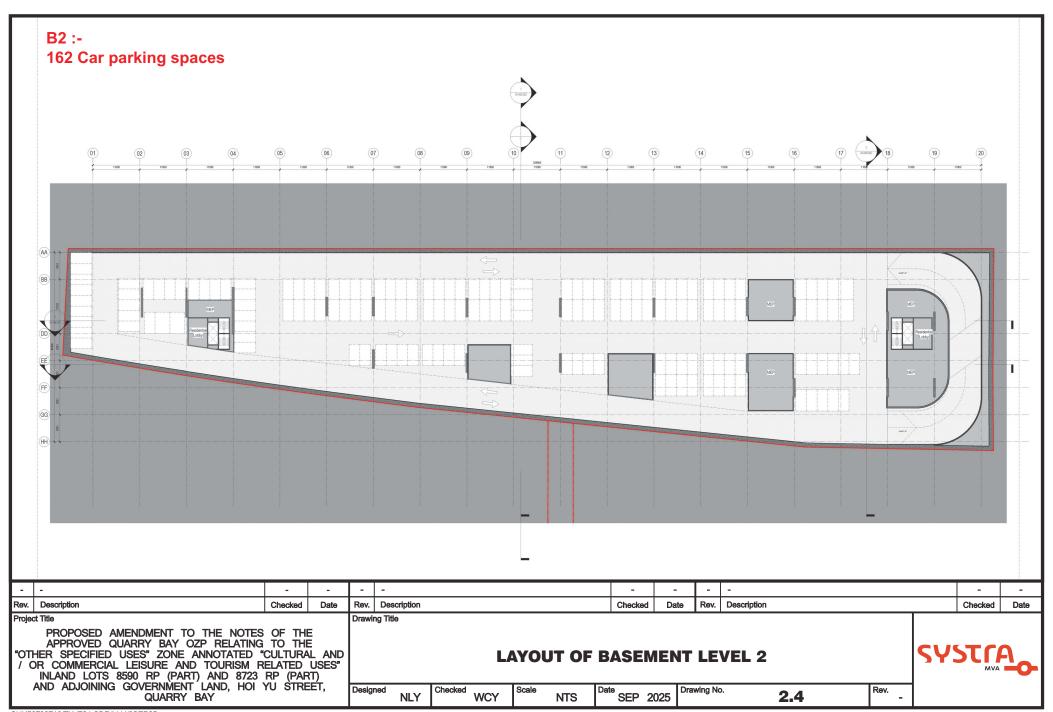
6.2 Conclusion

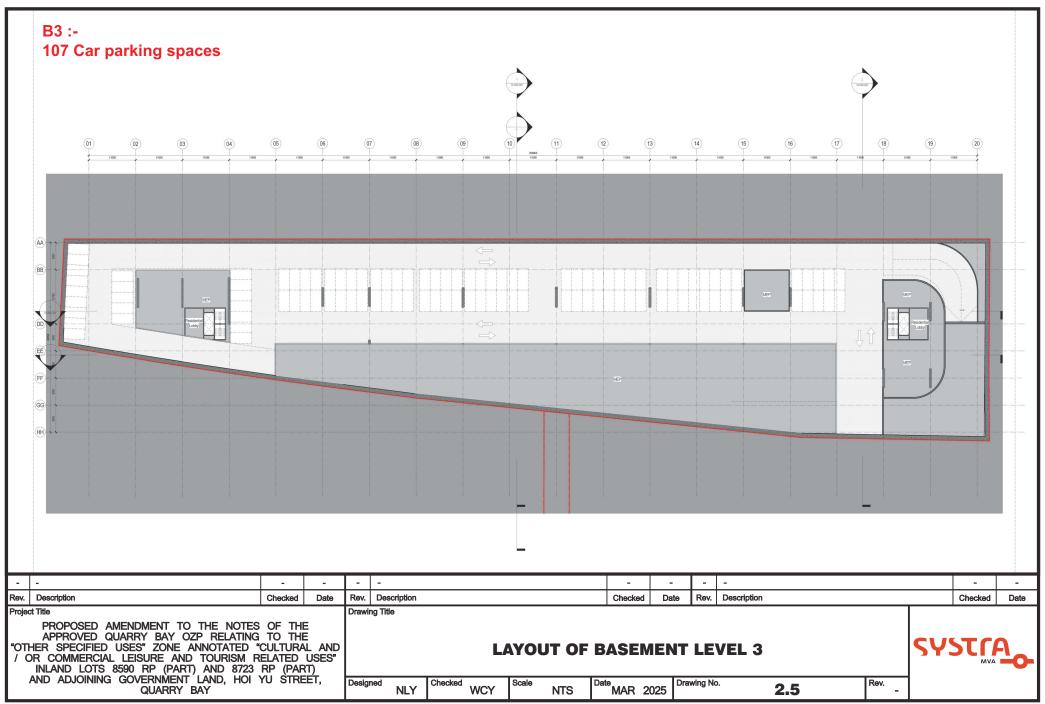
6.2.1 Based on the above-mentioned, it is concluded that the proposed development would not induce significant traffic impacts on the adjacent road network and is therefore supported from traffic engineering point of view.

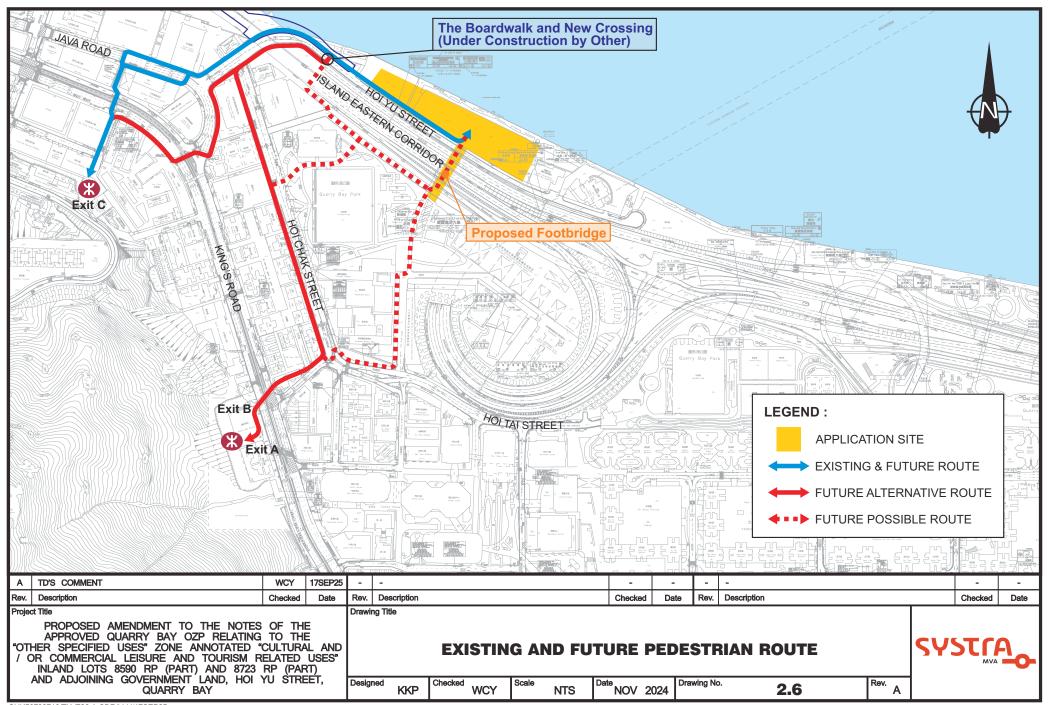




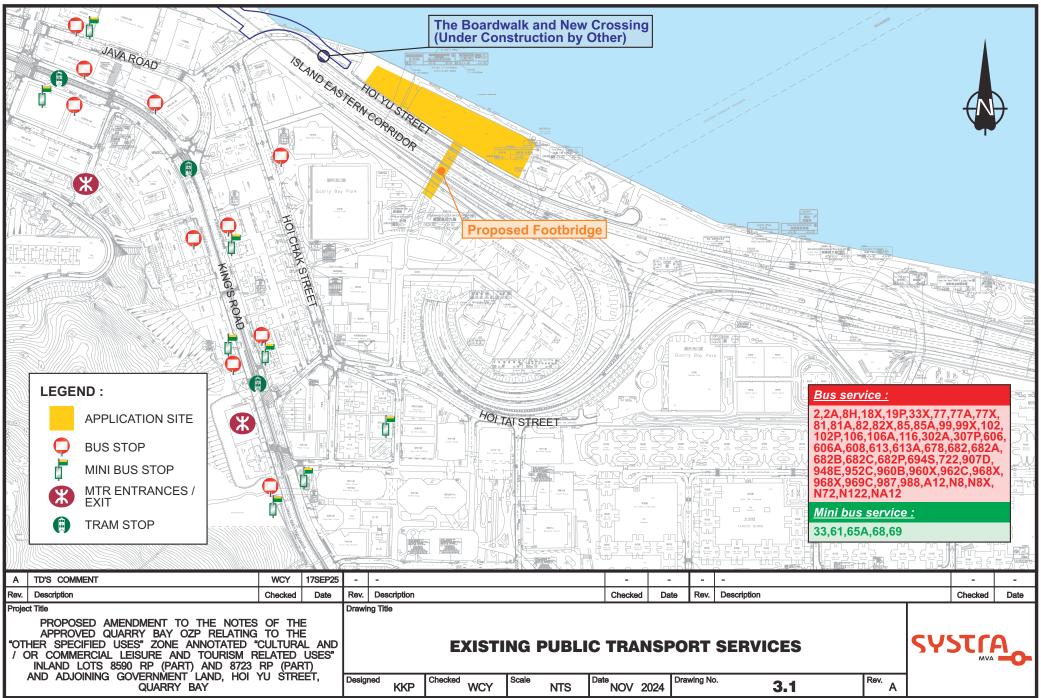


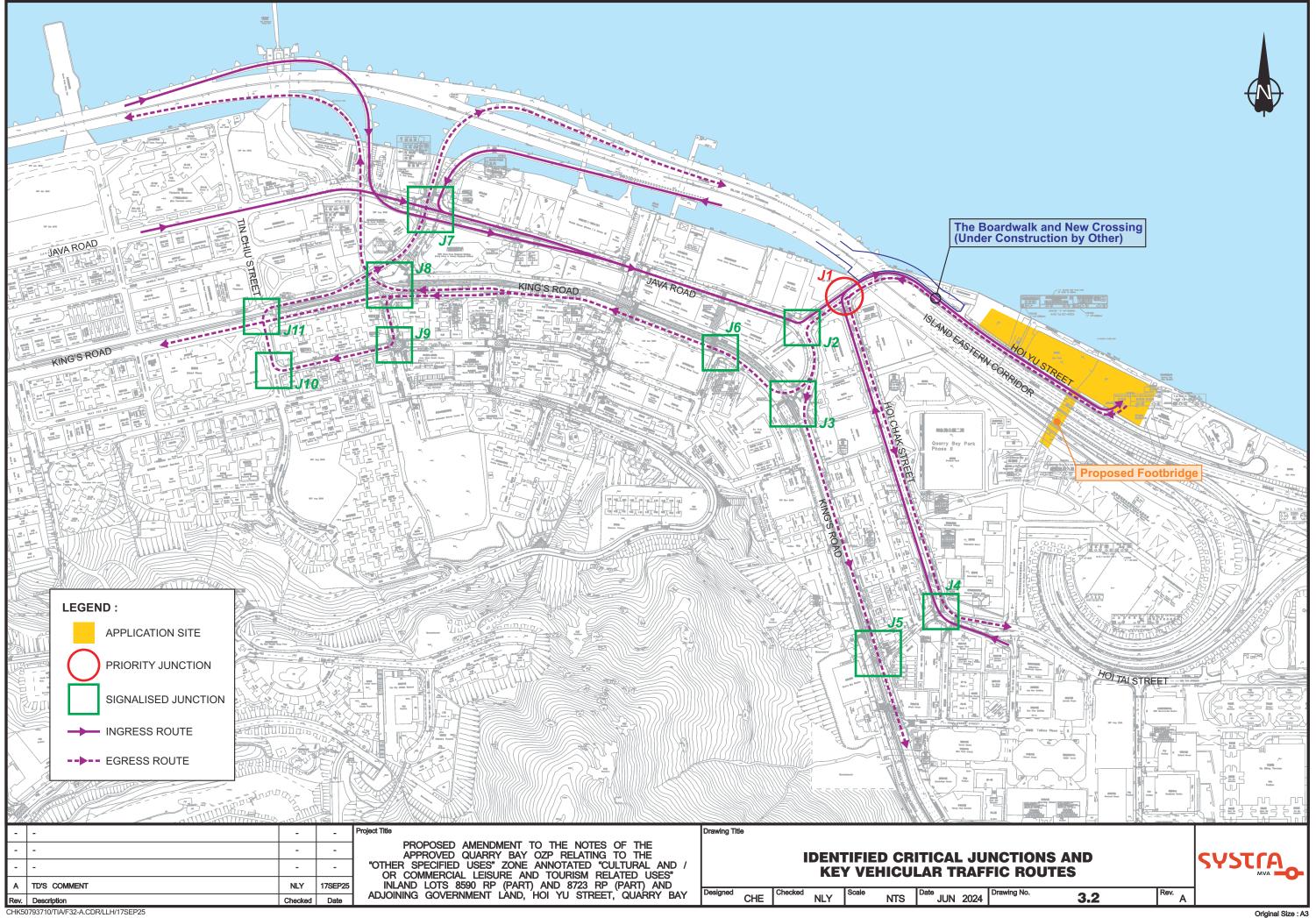


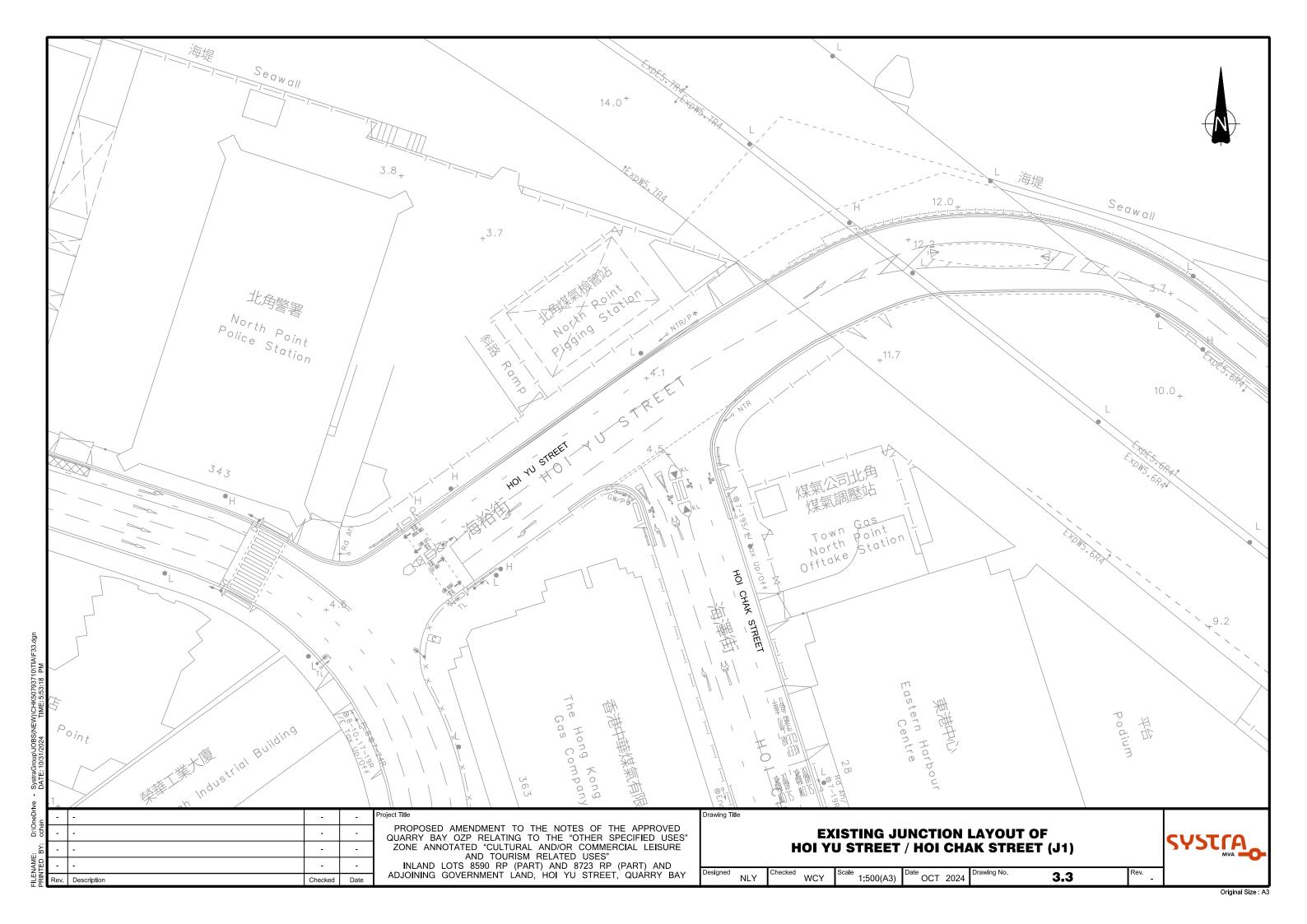


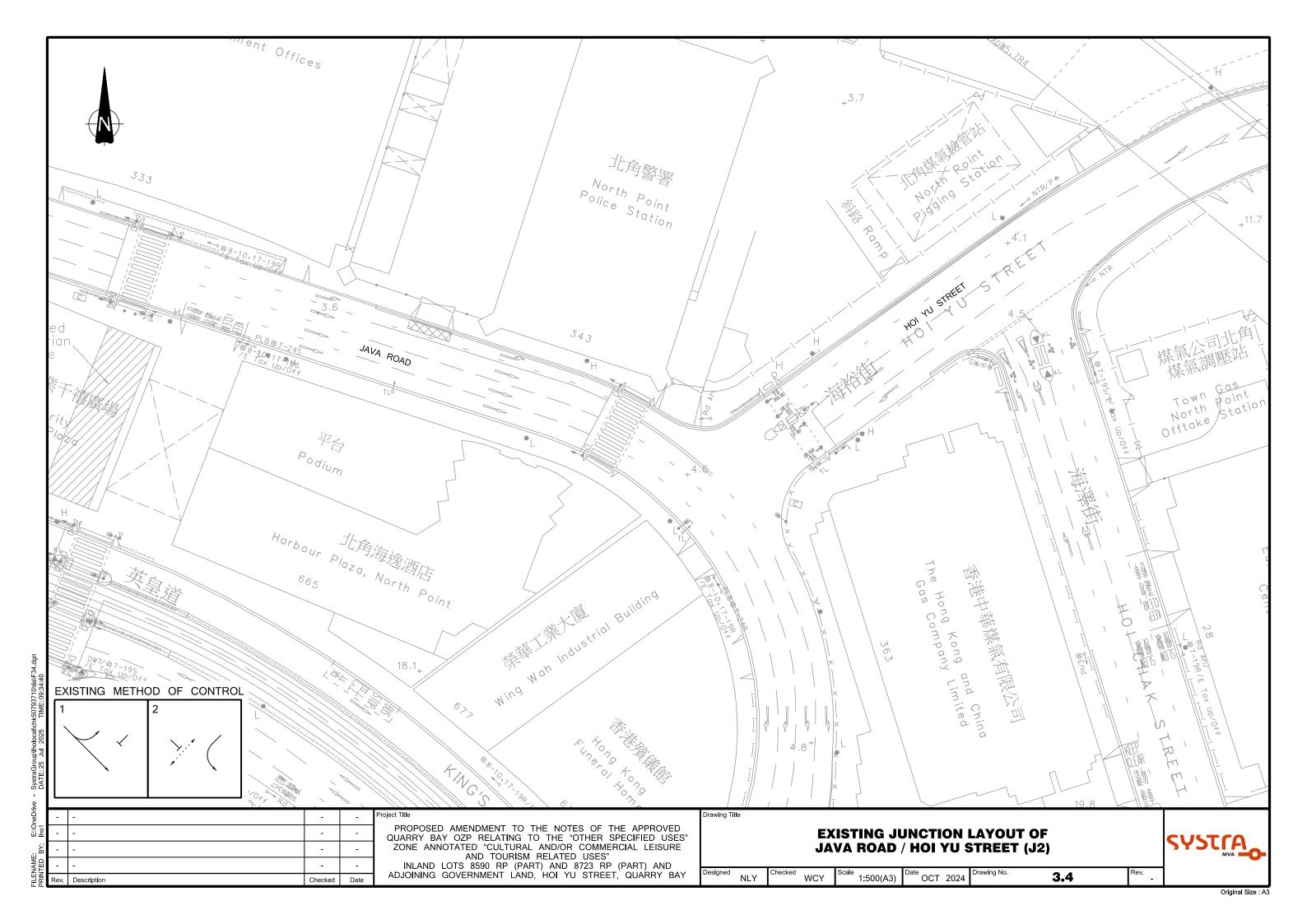


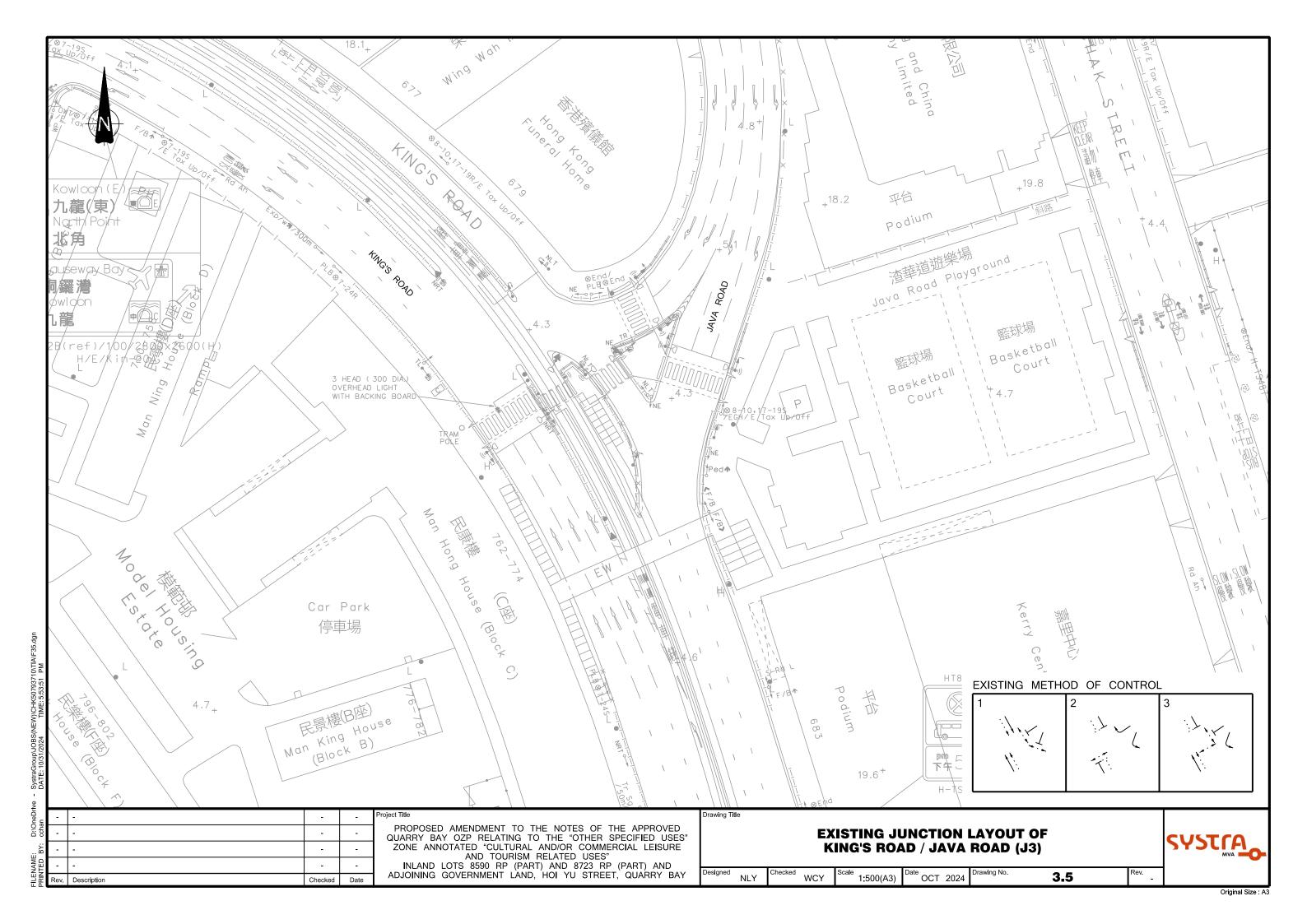


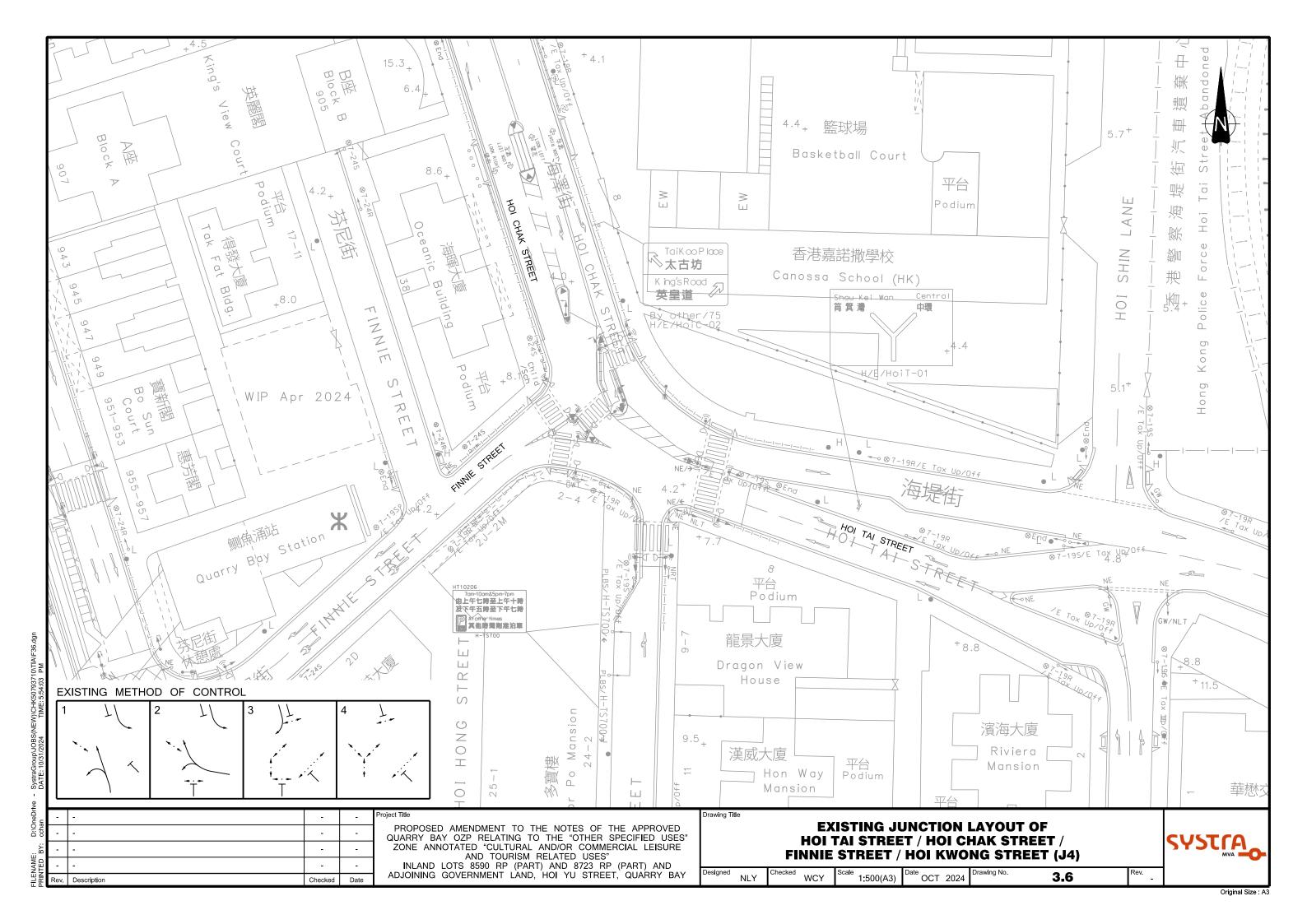


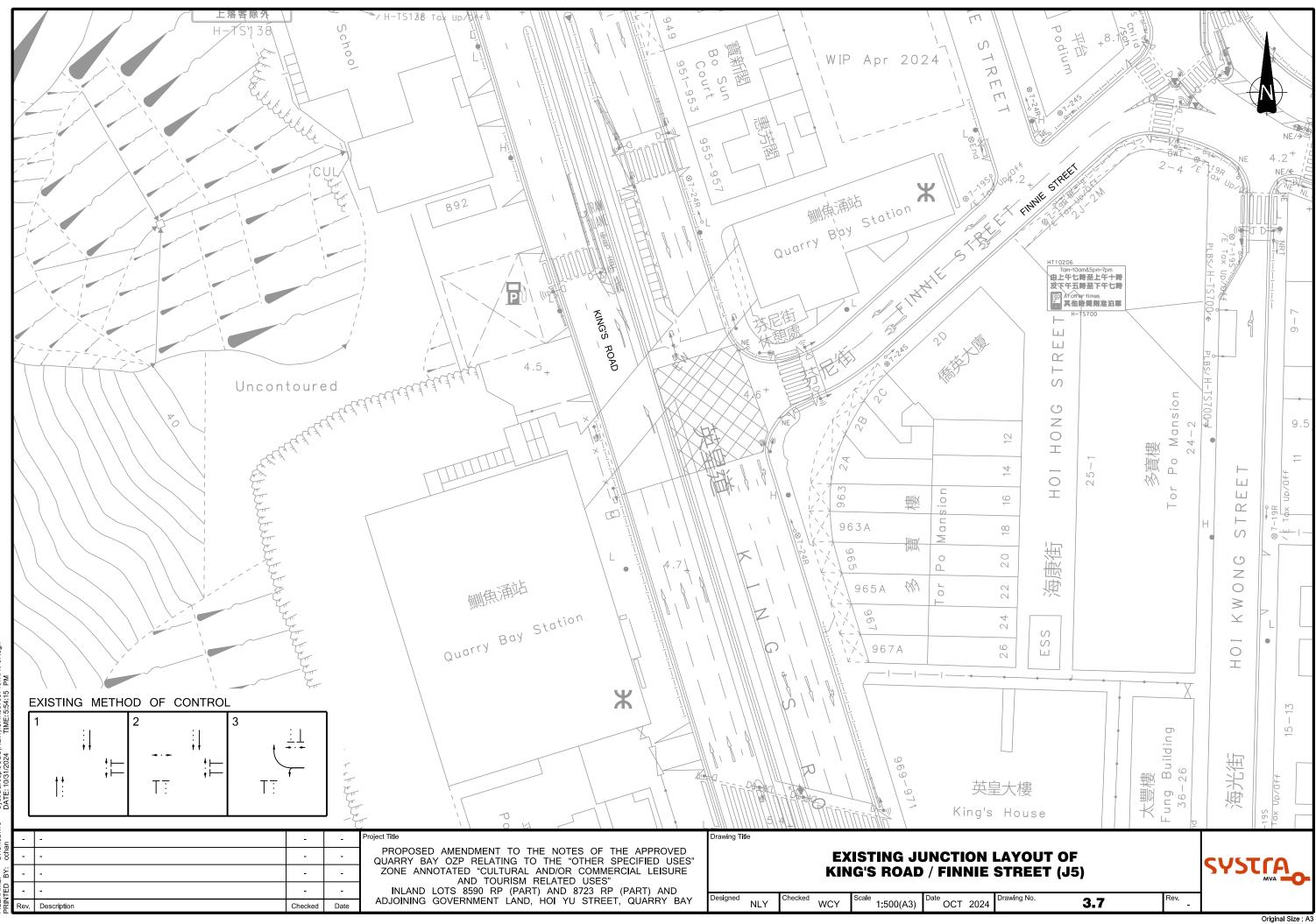


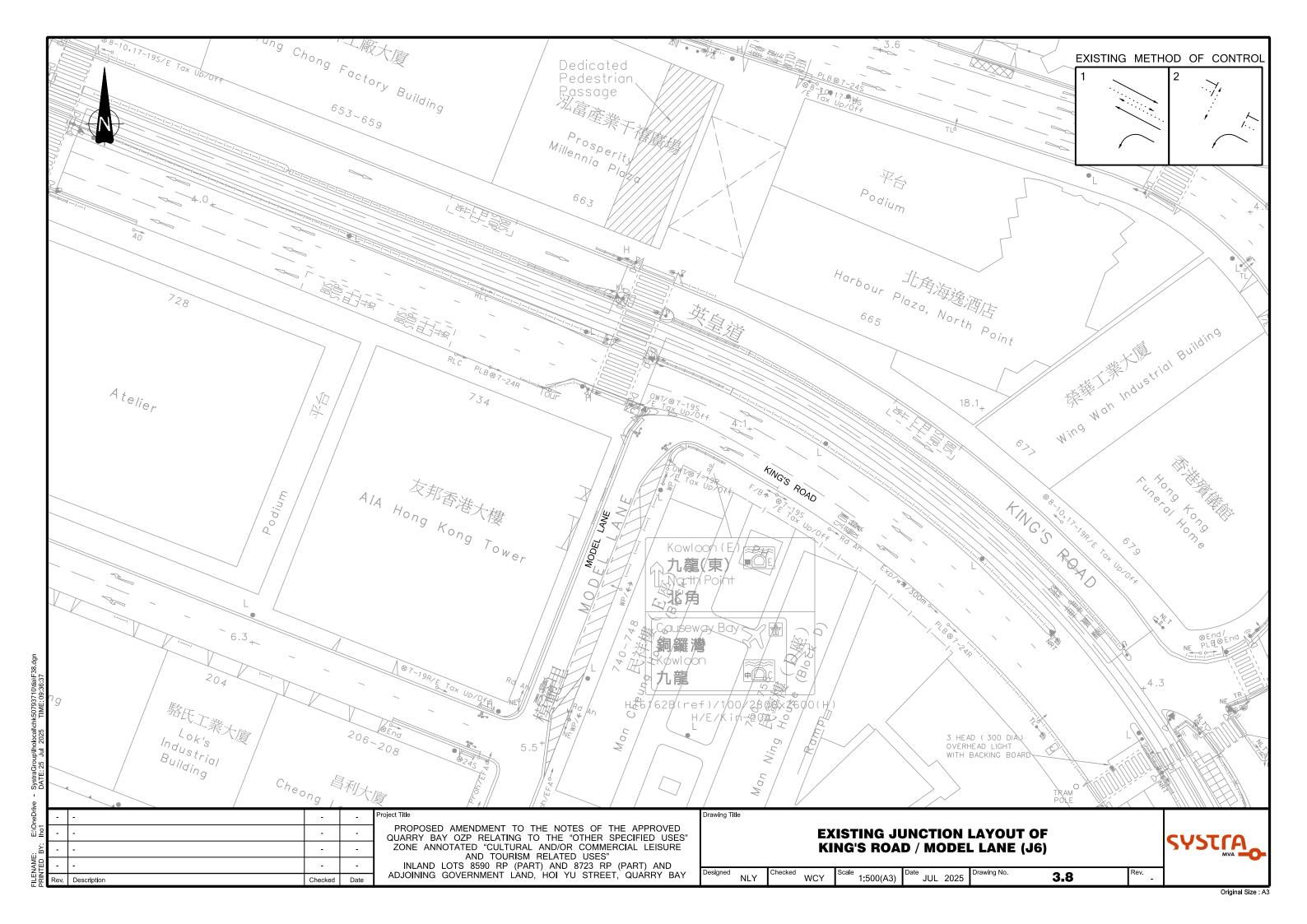


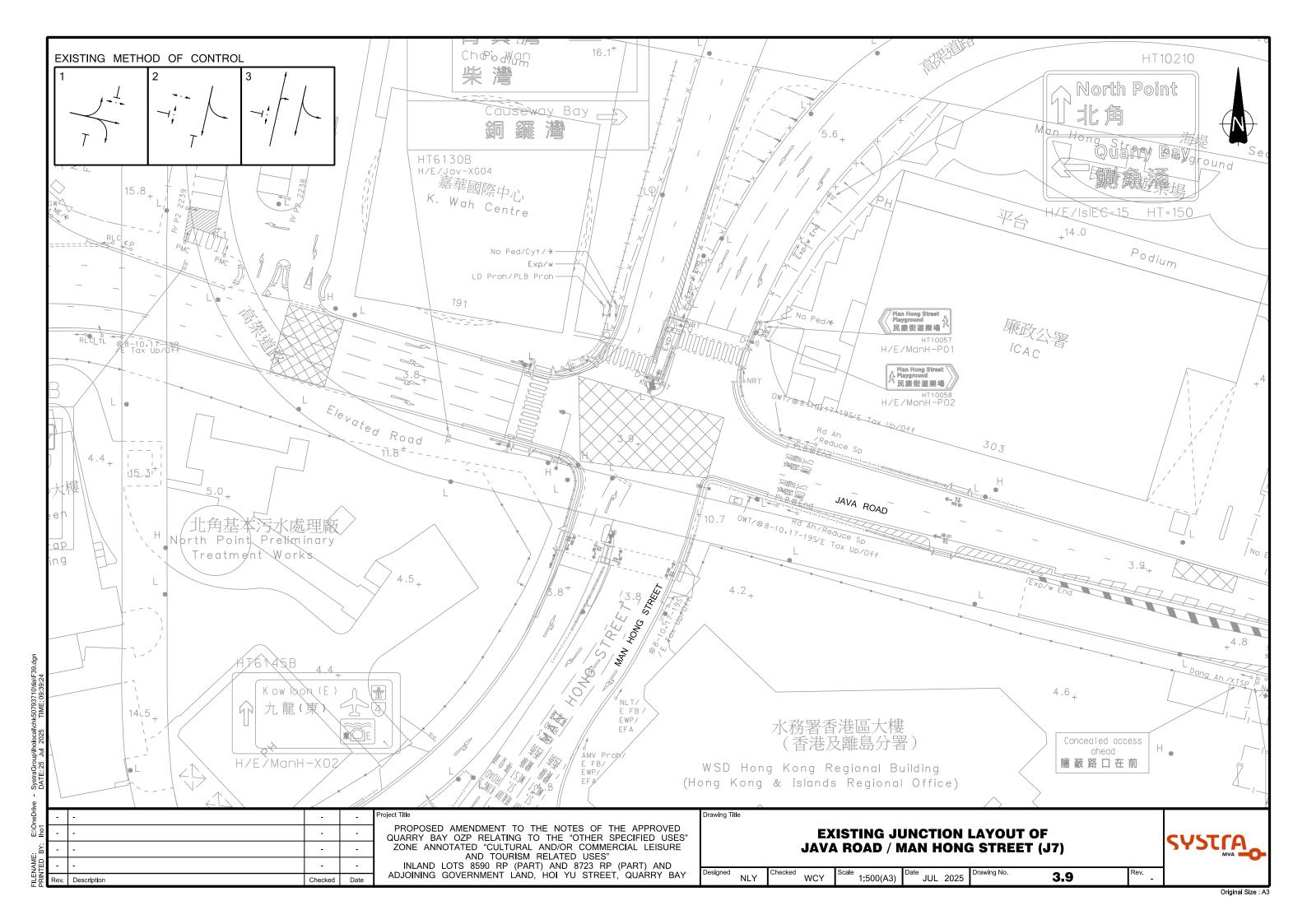


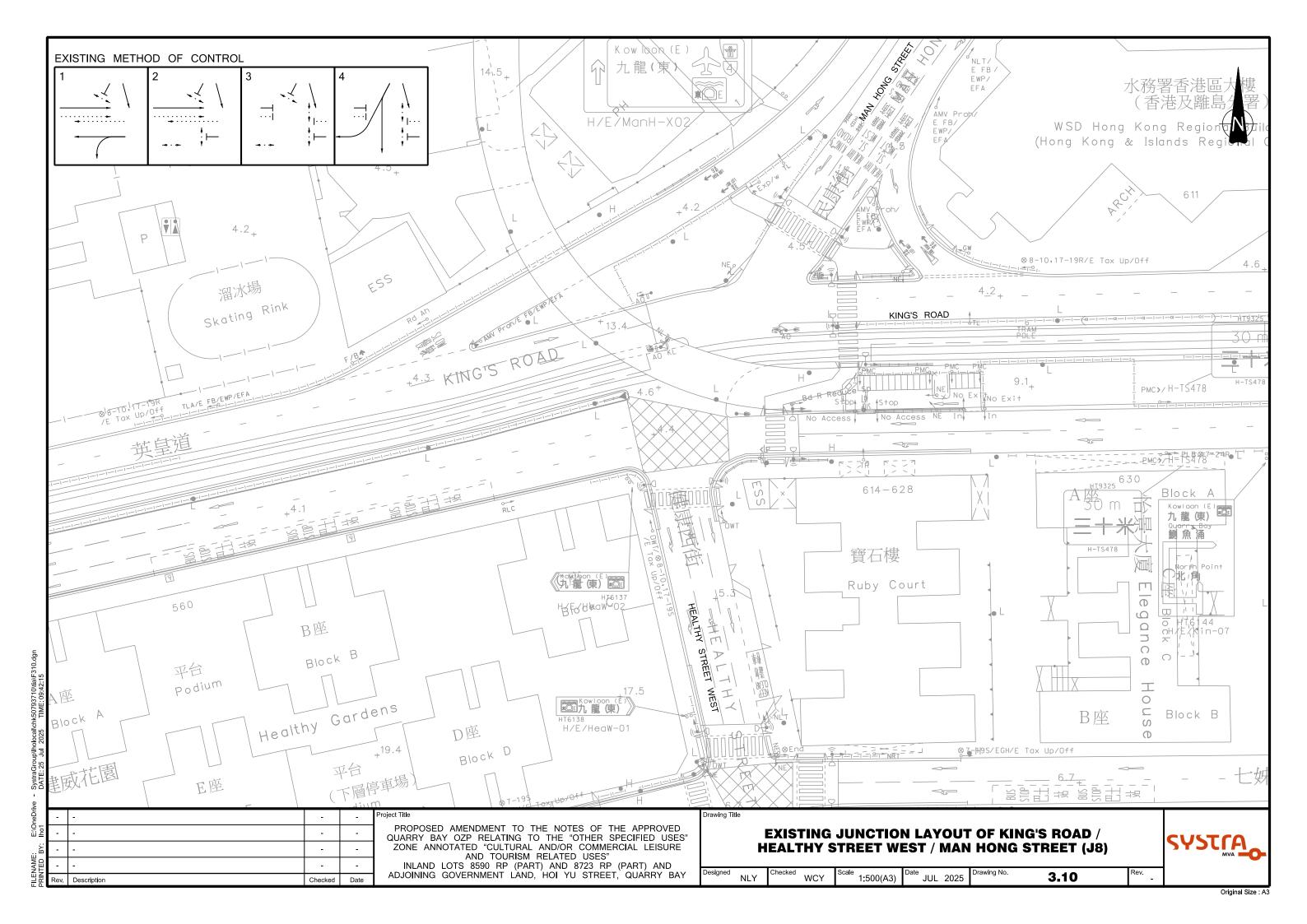


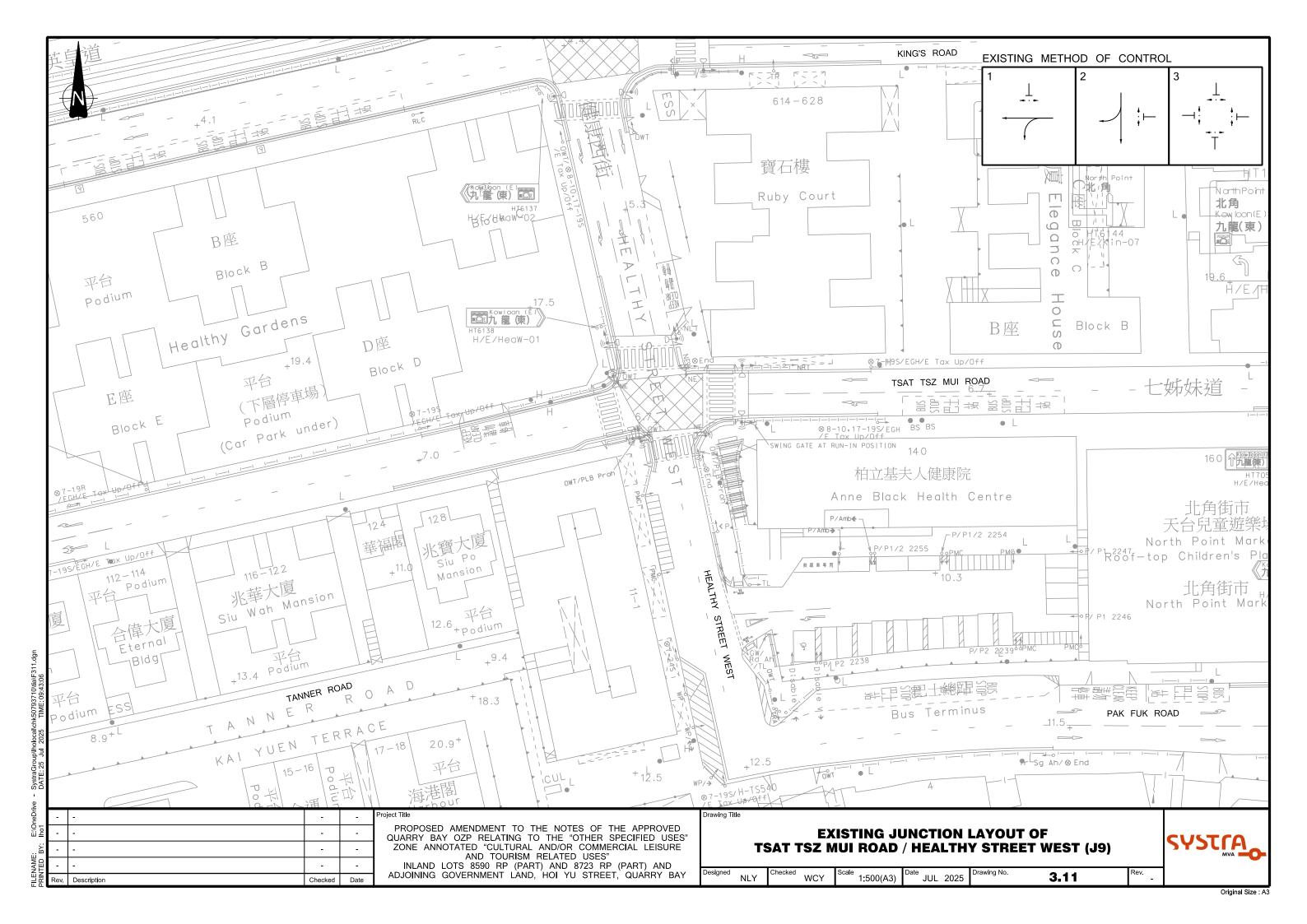


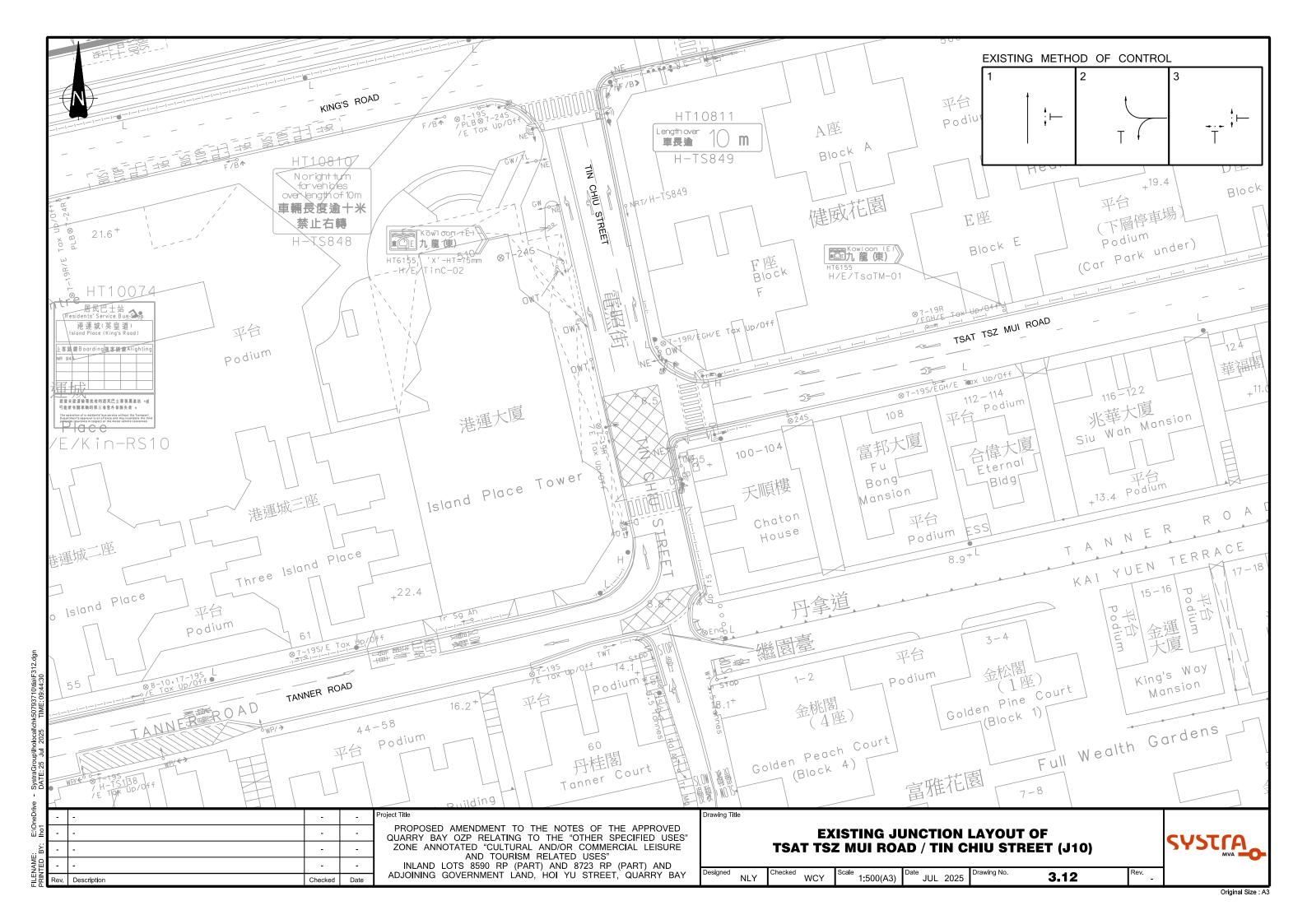


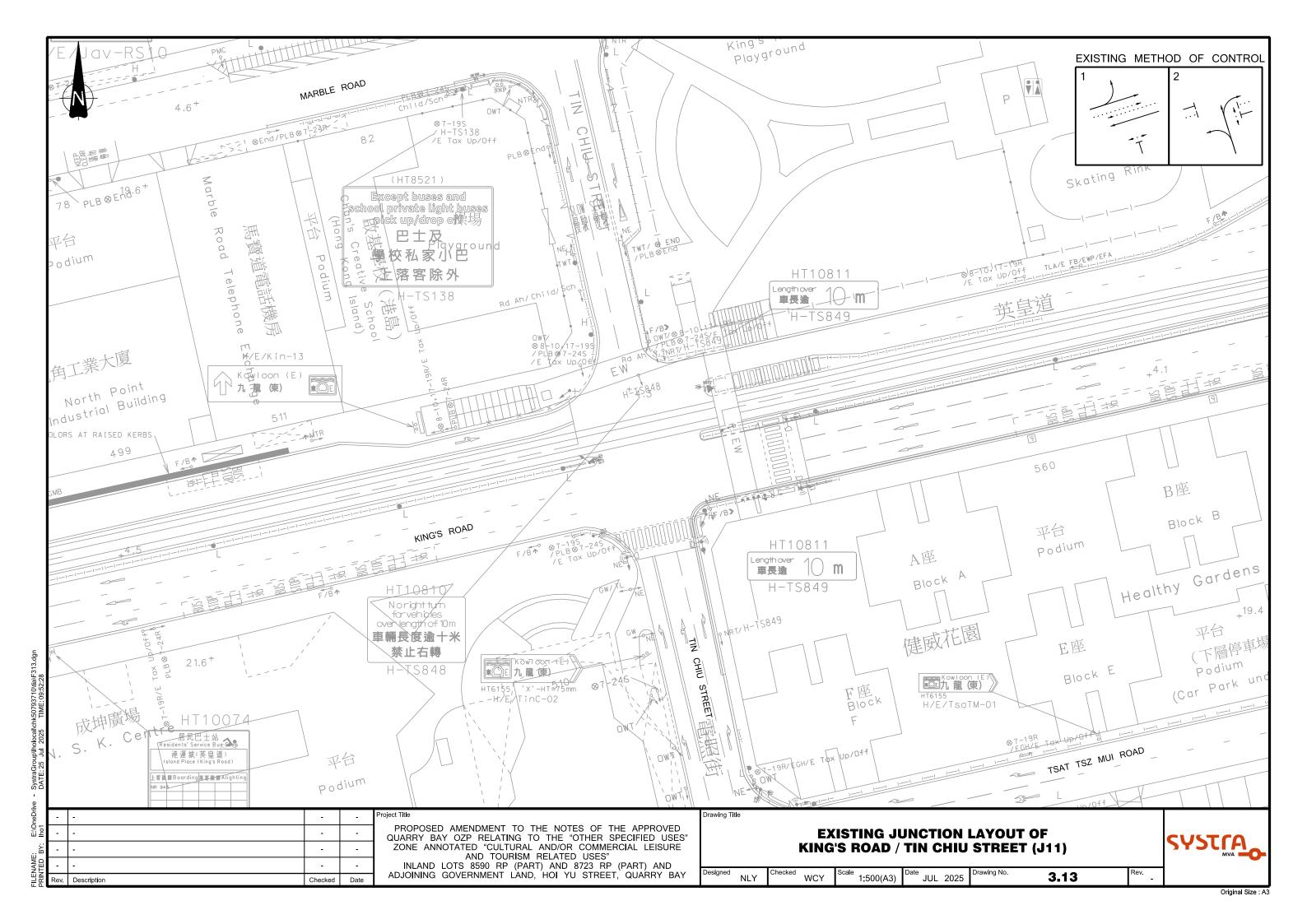


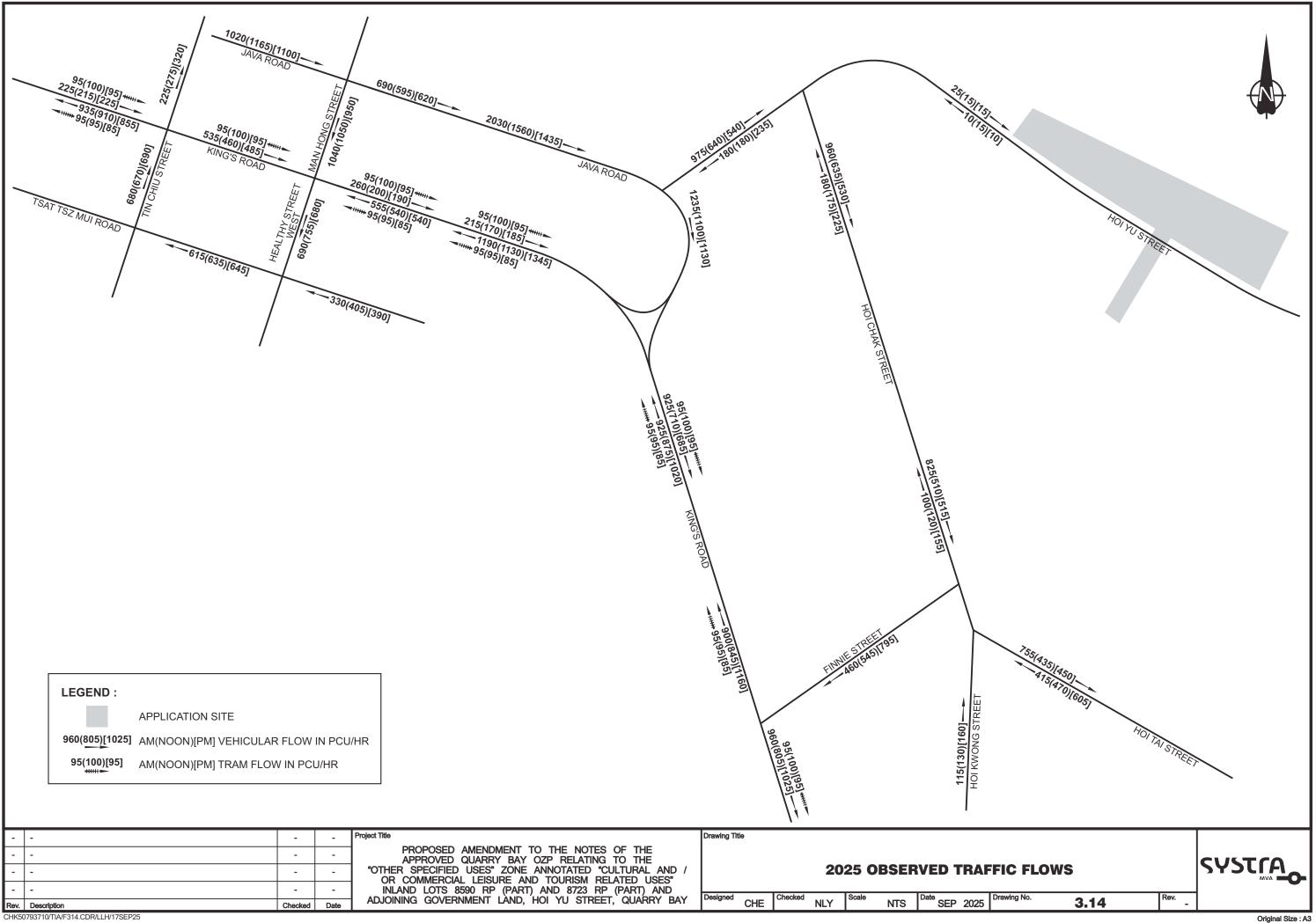


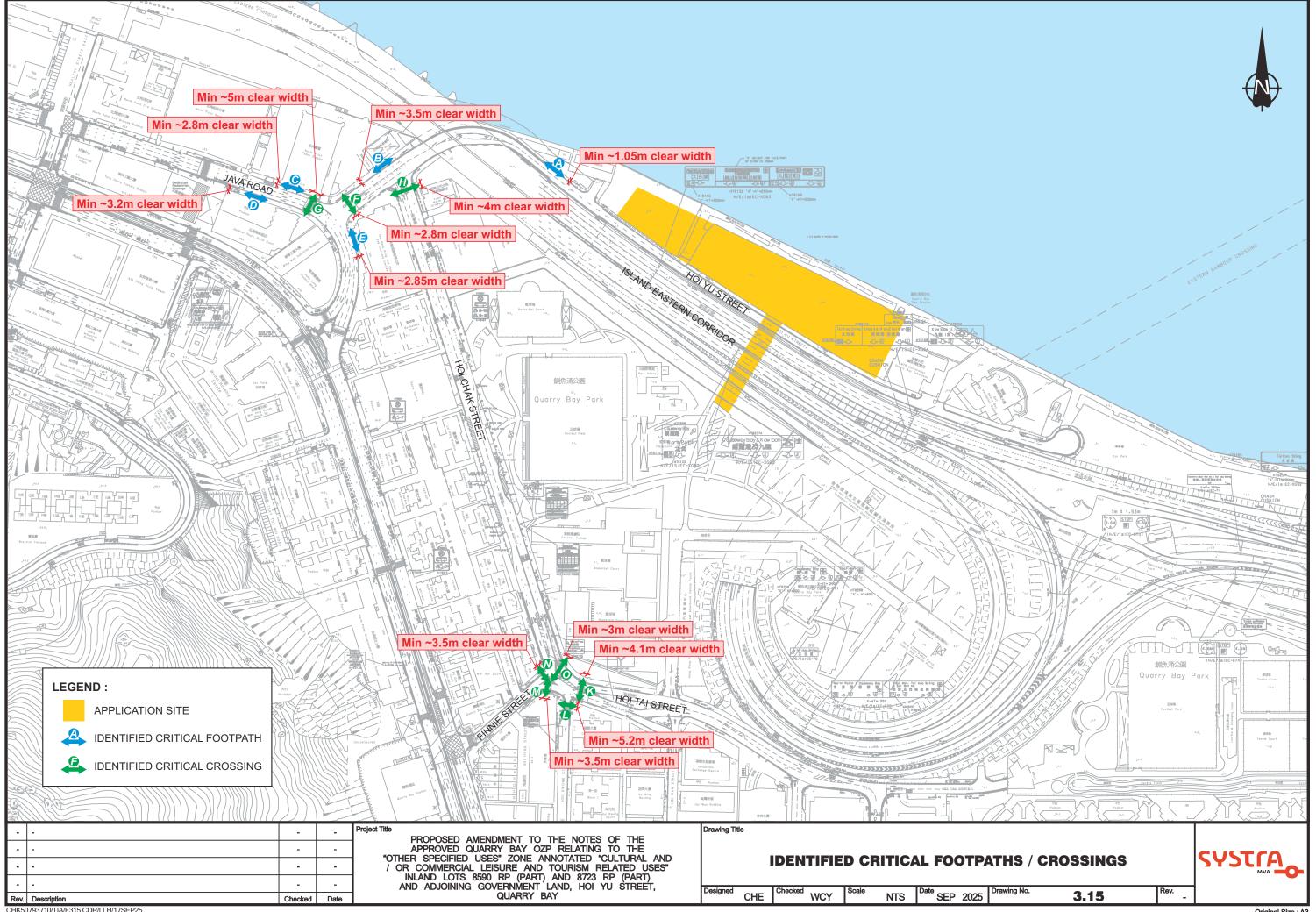


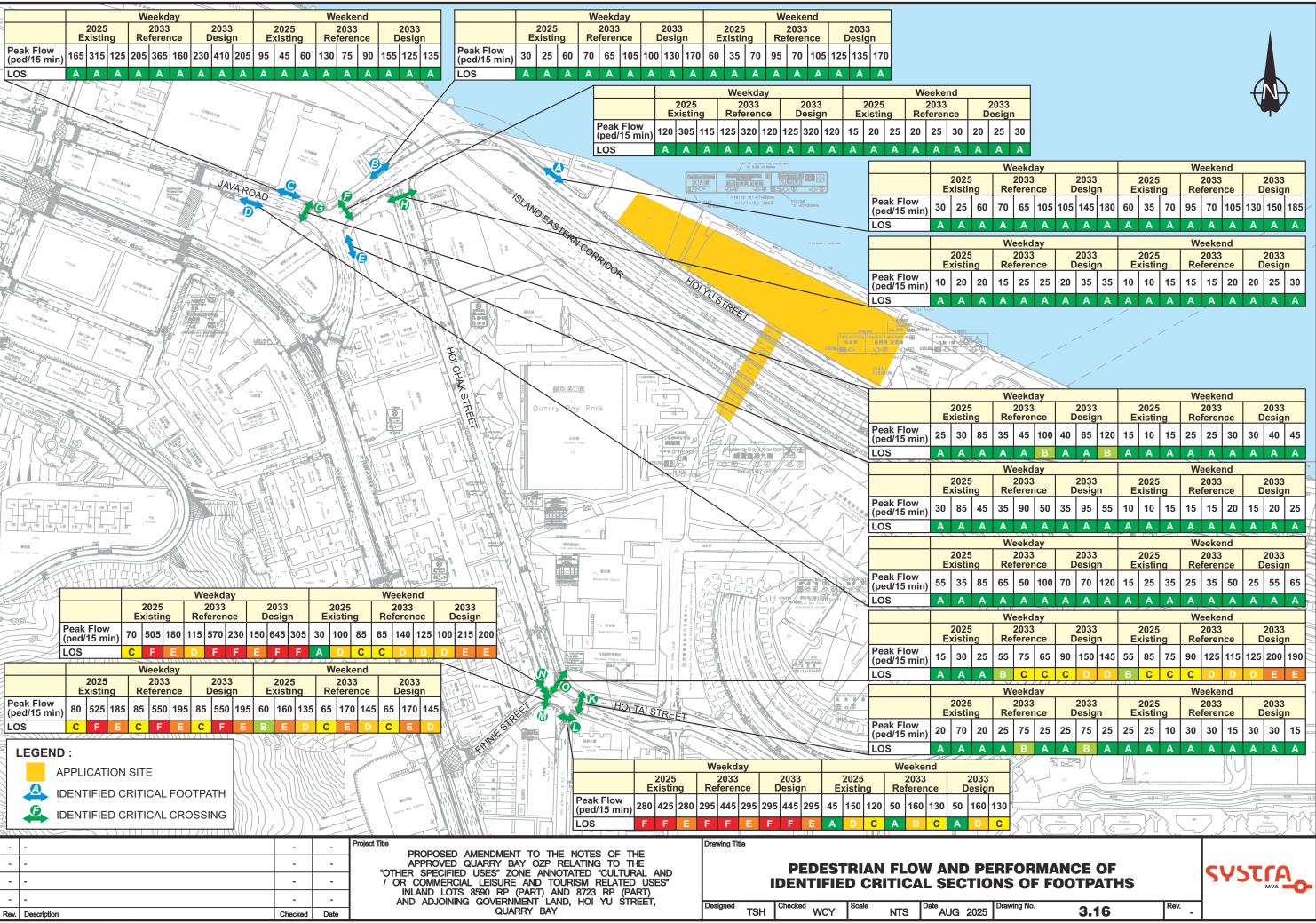


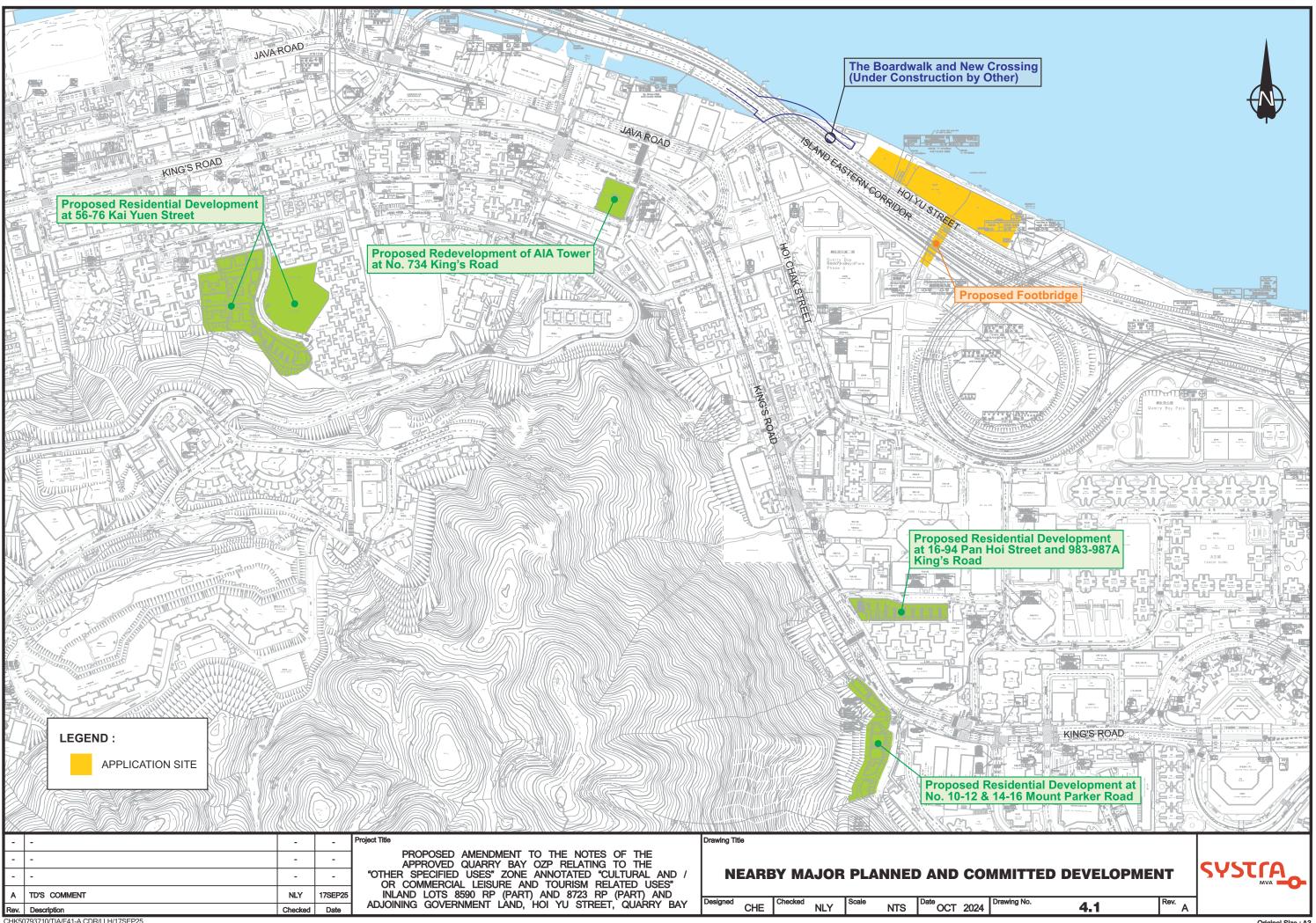


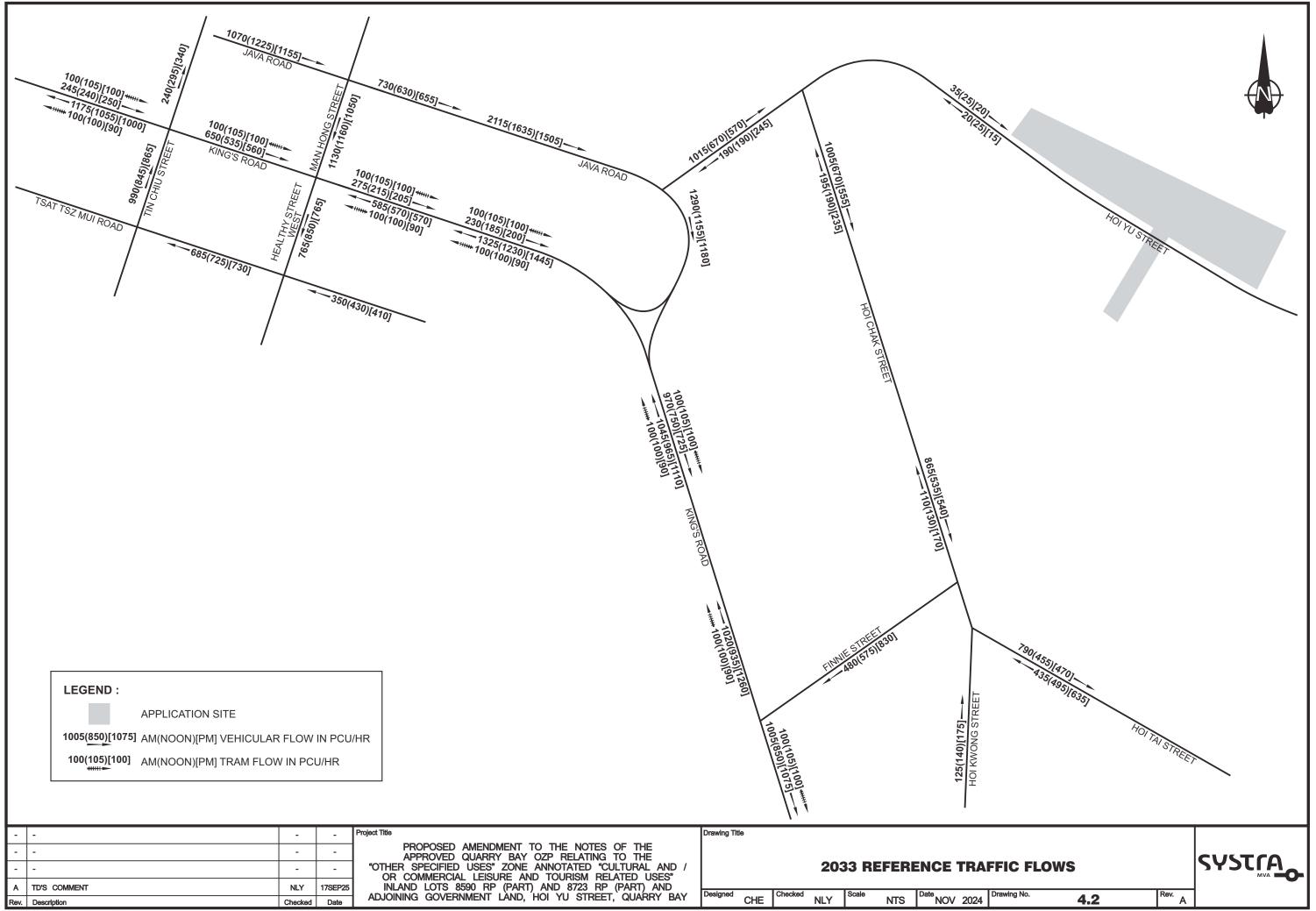


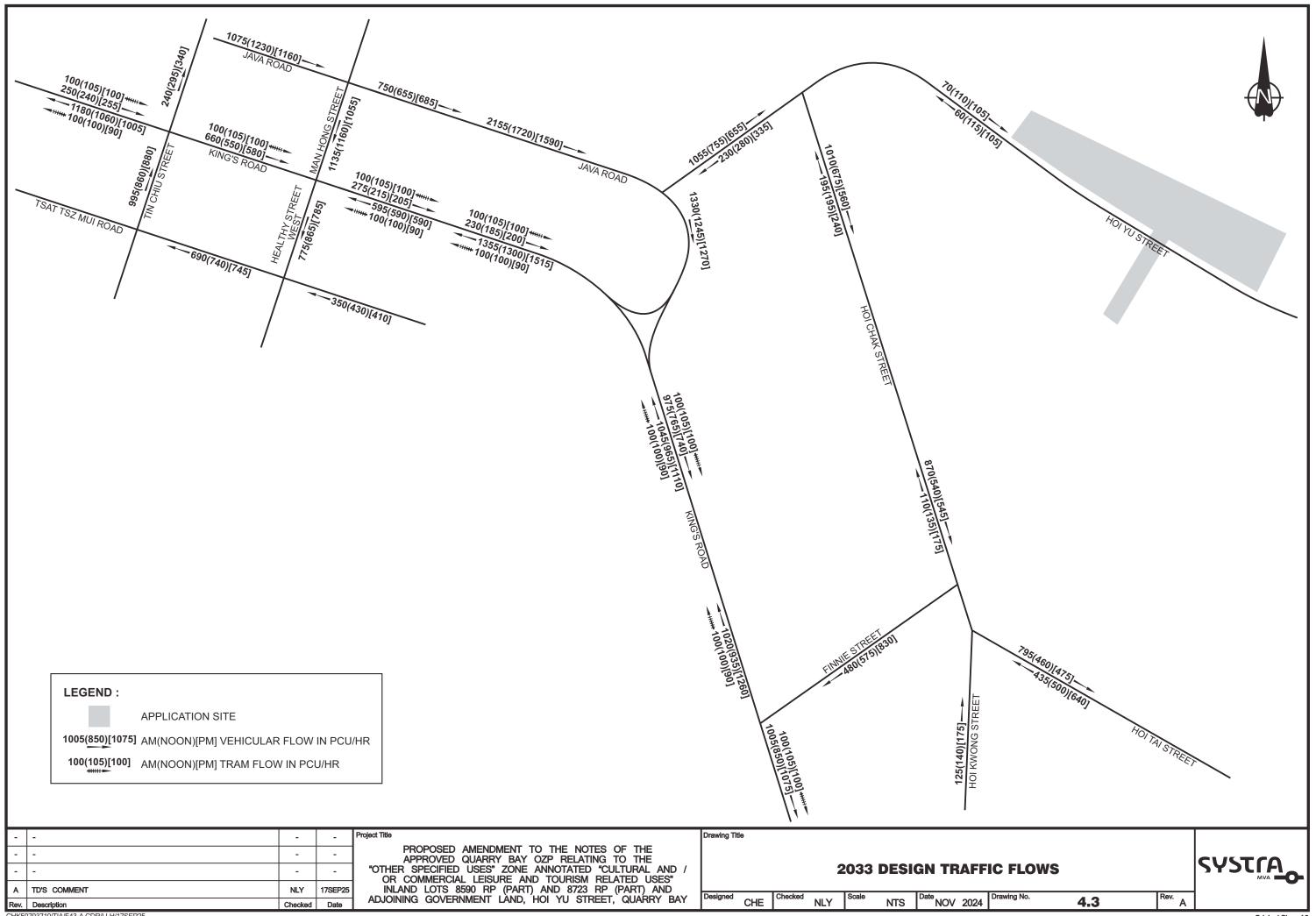


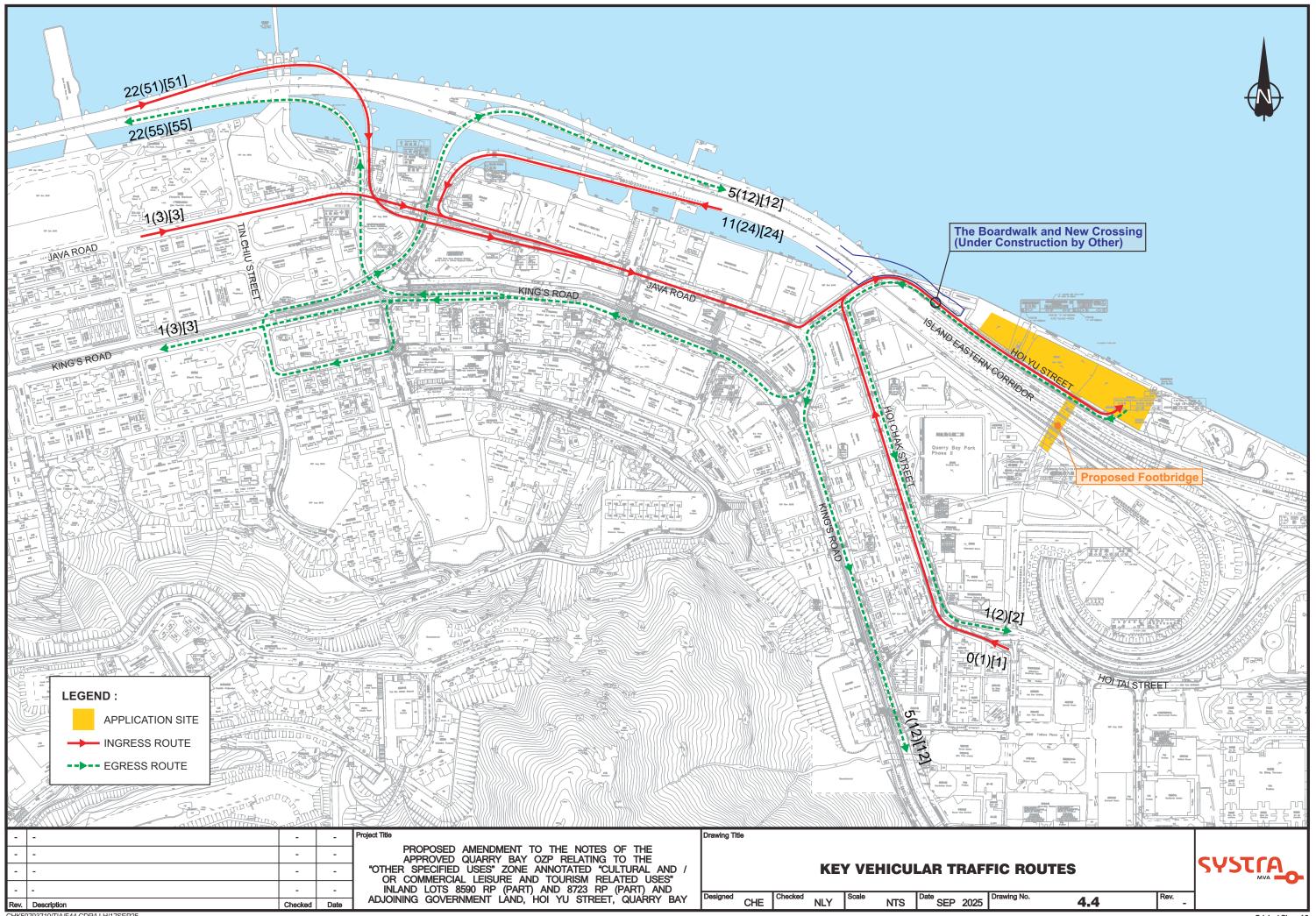


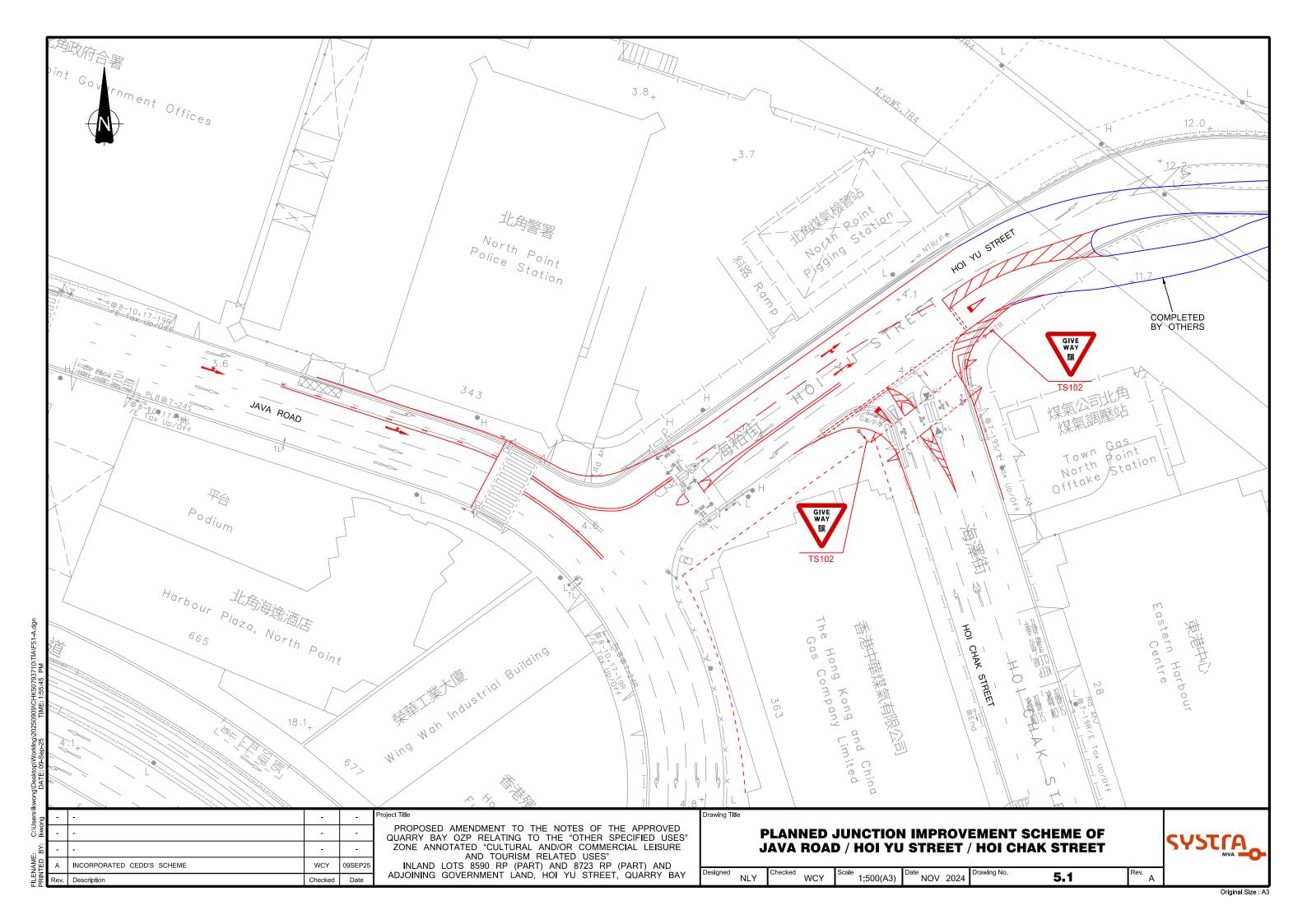


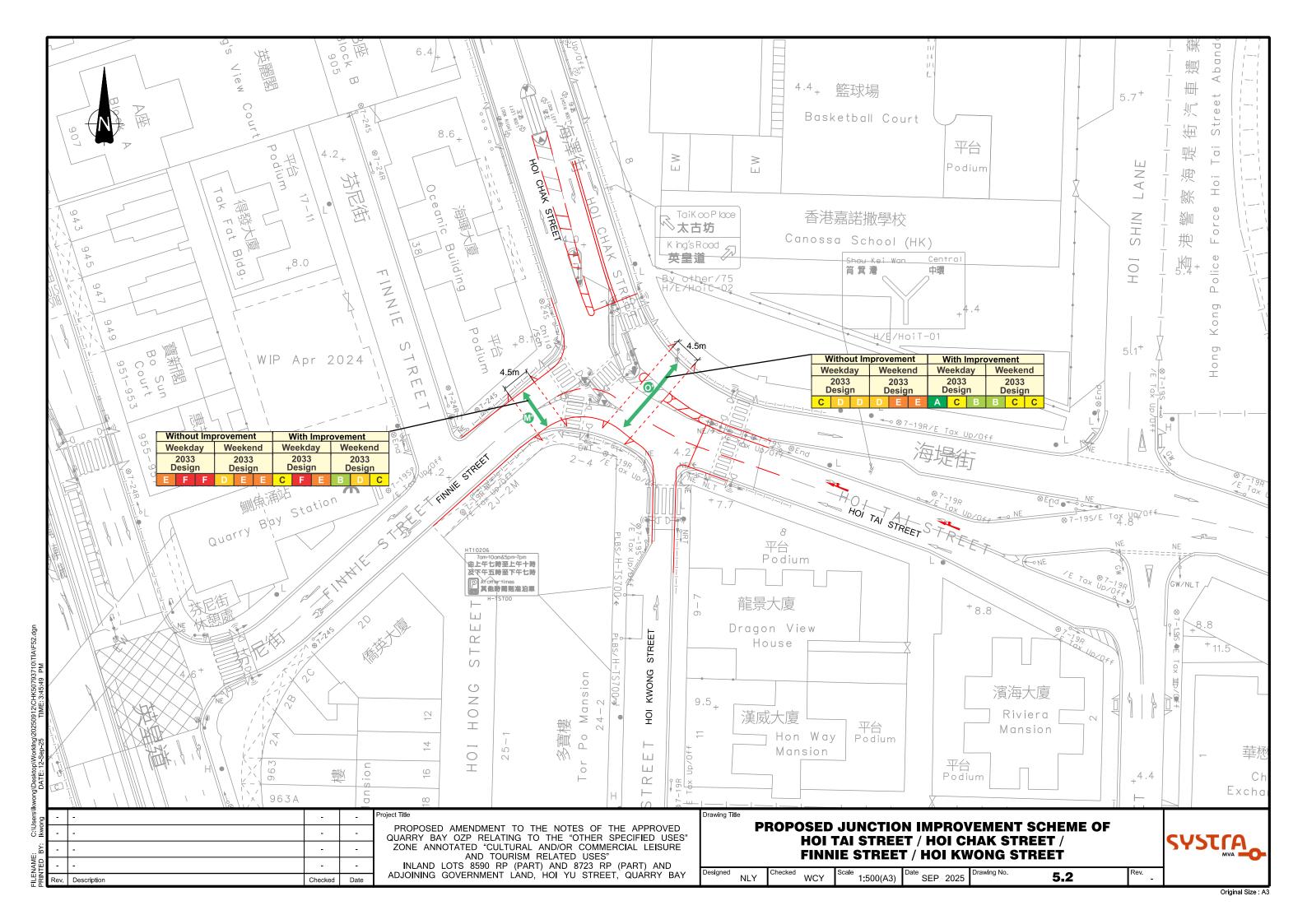










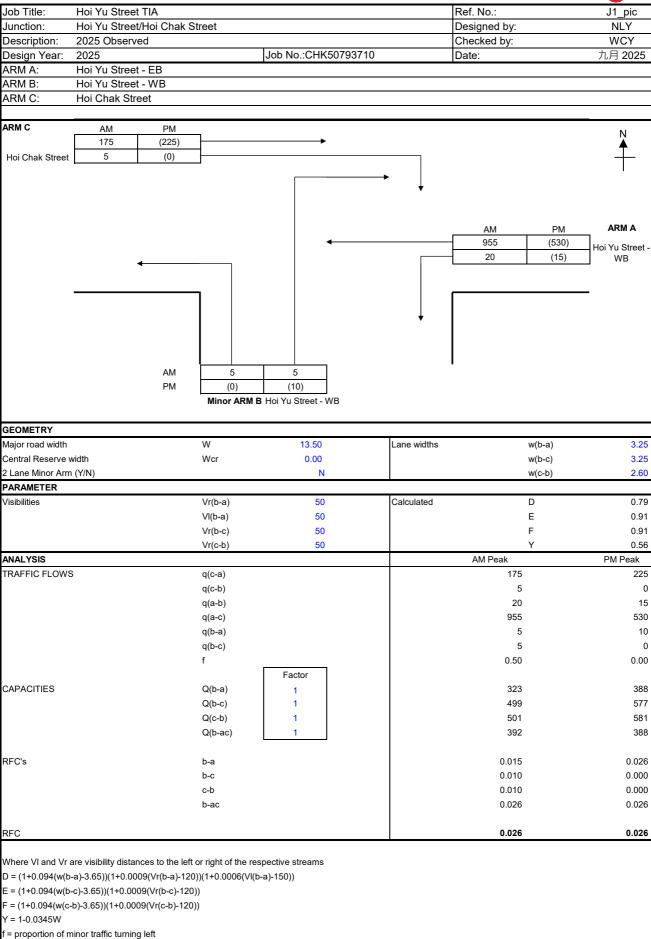


Appendix A - Junction Performance calculation

Calculation Sheets - 2025 **Existing**

Simplified picority Junction Capacity Calculation





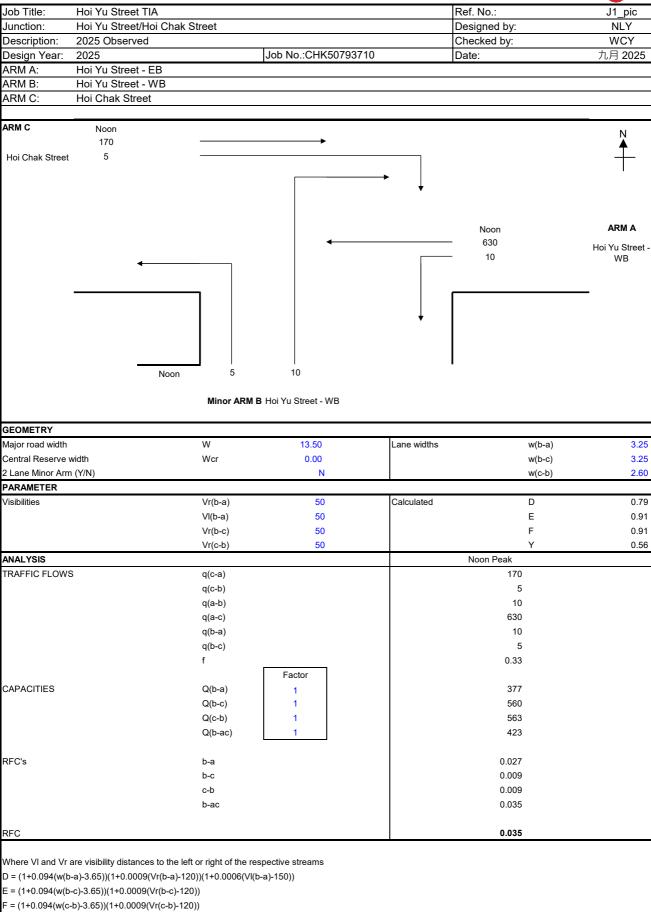
Capacity of combined streams

Q (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

Simplified picority Junction Capacity Calculation





Capacity of combined streams

Y = 1-0.0345W

f = proportion of minor traffic turning leftQ (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Hoi Yu Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Width Flow Flow Stage Approach Left y Value (pcu/hr) (pcu/hr) (m) 1725 1040 0.316 0.366 Java Road 2.4 20 1725 975 0.565 0.565 545 1040 381 0.366 3.2 449 0.432 \rightarrow Α 825 825 356 0.432 302 0.366 Α 3.1 3.1 580 580 250 0.431 0.366 Hoi Yu Street В 2 3.4 15 1905 1905 94 0.049 123 0.065 В 3.4 12.5 1745 1745 86 0.049 112 0.064 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group A,Cp 1. Site factor 0.5, 0.4 and 0.3 are 0.615 0.565 0.431 0.366 у у adopted for Java Road's second, third 180(235) and fourth lane respectively due to 10 26 10 26 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 975(545) 1055(895) C (sec) C (sec) 115 115 110 110 0.822 0.697 0.818 0.687 y pract. y pract. R.C. (%) 34% 23% R.C. (%) 90% 88% Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p В I/G= 4 I/G= 5 18 I/G= I/G= I/G= I/G= 4 I/G= 5 1/G= 1/G= I/G= Junction: 九月, 2025 lava Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Hoi Yu Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Flow Approach Left Critical y (pcu/hr) (pcu/hr) (m) 1725 1040 Java Road 2.4 20 640 0.371 0.376 3.2 391 Α 825 311 0.377 0.377 Α 3.1 3.1 580 218 0.376 Hoi Yu Street В 2 15 1905 94 0.049 В 3.4 12.5 1745 86 0.049 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group 1. Site factor 0.5, 0.4 and 0.3 are 0.426 0.377 у у adopted for Java Road's second, third 180(0) and fourth lane respectively due to 10 26 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 640(0) 920(0) C (sec) C (sec) 110 110 0.818 0.687 y pract. y pract. R.C. (%) 92% 82% R.C. (%) Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p I/G= 4 I/G= 5 18 I/G= I/G= I/G= I/G= #N/A #N/A I/G= #N/A #N/A I/G= | I/G= I/G= Junction: 九月, 2025 lava Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ Checked By: WCY 2025 Observed Designed By: NLY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 5.3 2145 2145 215 0.100 0.100 185 0.086 ΕB King's Road 2.7 2025 2025 95 0.047 0.047 EB (Tram) Java Road С 2 3.5 30 1870 1870 242 0.129 285 0.152 0.152 С 2 4.1 32.5 2070 2070 268 0.129 315 0.152 D 2.3 52.5 2025 3.3 2025 367 0.181 259 0.128 0.181 2.3 1890 1890 343 0.181 241 3.3 50 0.128 0.155 0.171 King's Road В 1,3 3.2 1935 1935 300 330 WB В 1,3 3.2 2075 2075 321 0.155 355 0.171 0.171 1,3 1965 1965 304 0.155 0.170 В 3.5 King's Road В 1,3 2.3 1845 1845 95 0.051 85 0.046 WB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 1,3 12 Еp MIN GREEN + FLASH = 9 14 Fp 5 Gp MIN GREEN + FLASH = 5 10 2.3 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group B,C Group B,C Tram 0.285 0.282 0.214 0.323 у у 215(185) 95(95) 10 12 12 10 L (sec) L (sec) 510(600) C (sec) 925(1020) 95(85) C (sec) 115 115 110 110 **≜**Tram 710(500) 0.822 0.806 0.802 0.818 y pract. y pract. R.C. (%) 189% 186% R.C. (%) 274% 153% Stage / Phase Diagrams 2. 3. 8 Java Road I/G= 5 I/G= 9 I/G= I/G= I/G= I/G= I/G= 5 I/G= 7 1/G= I/G=

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 5.3 2145 170 0.079 ΕB King's Road 2.7 2025 100 0.049 EB (Tram) Java Road С 2 3.5 30 1870 249 0.133 С 0.133 2 4.1 32.5 2070 276 0.133 D 2.3 52.5 2025 3.3 279 0.138 2.3 50 1890 0.138 3.3 261 0.146 King's Road 1,3 3.2 1935 283 WB В 1,3 3.2 2075 304 0.147 1,3 288 0.147 3.5 1965 King's Road В 1,3 2.3 1845 95 0.051 WB (Tram) Pedestrian Crossing Ер MIN GREEN + FLASH = 1,3 12 MIN GREEN + FLASH = 9 14 Fp 5 Gp MIN GREEN + FLASH = 5 10 2.3 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group Group Tram 0.217 0.280 у у 170(0) 100(0) 12 10 L (sec) L (sec) 525(0) C (sec) C (sec) 110 875(0) 110 95(0) **≜**Tram 540(0) 0.802 0.818 y pract. y pract. R.C. (%) 269% 192% R.C. (%) Stage / Phase Diagrams 3. I/G= I/G= 5 I/G= 7 I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A | I/G= I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value y Value Critical y (pcu/hr) (pcu/hr) (m) 0.069 0.253 Hoi Tai Street 3.5 2030 2030 80 0.039 140 2 1835 0.253 D 3.6 20 1835 335 0.183 465 Hoi Kwong Street 4 С 3.6 15 1825 1810 115 0.063 0.088 0.088 Hoi Chak Street В 1,2 3.1 35 1845 1845 755 0.409 0.409 450 0.244 3 3.5 30 2005 2005 70 0.035 65 0.032 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 3,4 MIN GREEN + FLASH = 16 Fp Gp 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = 12 Notes: Flow: (pcu/hr) Group B,Fp B,Gp Group C,D,Fp C,D,Gp 0.409 0.409 0.342 0.342 755(450) 70(65) 80(140) 26 27 L (sec) 33 34 L (sec) C (sec) C (sec) 110 335(465) 115 115 110 20(15) 95(145) 0.697 0.689 0.630 0.622 y pract. y pract. R.C. (%) 70% 68% R.C. (%) 84% 82% Stage / Phase Diagrams I/G= 2 I/G= I/G= 8 18 I/G= I/G= I/G= 2 I/G= 6 I/G= 10 18 I/G= I/G=

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 2030 1835 Hoi Tai Street 3.5 0.044 0.207 D 2 3.6 20 380 0.207 Hoi Kwong Street 4 3.6 15 1835 130 0.071 0.071 Hoi Chak Street 1,2 35 1845 435 0.236 3 3.5 30 2005 75 0.037 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 3,4 MIN GREEN + FLASH = 16 Fp Gp 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = Notes: Flow: (pcu/hr) Group C,D,Fp C,D,Gp Group 0.278 0.278 у 435(0) 75(0) 90(0) 33 34 L (sec) L (sec) C (sec) 110 C (sec) 380(0) 110 100(0) 0.630 0.622 y pract. y pract. R.C. (%) 127% 124% R.C. (%) Stage / Phase Diagrams I/G= 2 I/G= 6 I/G= 10 18 I/G= #N/A #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Finnie Street Design Year: ____2025_ Designed By: NLY Checked By: WCY 2025 Observed Description:_ Revised Saturation Radius (m) Pro. Turning (%) AM Peak PM Peak % Gradient Phase Approach F (pcu/hr) (pcu/hr) King's Road 3.0 1915 1915 204 0.107 202 0.105 Ċ NB (with Tram) 3.3 2085 2085 222 0.106 221 0.106 С 3.5 2105 2105 224 0.106 0.105 Finnie Street D 20 51% / 49% 50% / 50% 1765 1765 225 0.127 0.220 22.5 0.128 0.128 0.221 0.221 King's Road 1,2 3.7 1985 1985 412 0.208 405 0.204 0.204 0.208 SB Α 1.2 3.3 2085 2085 433 0.208 425 0.204 King's Road 1.2 2015 95 0.047 85 0.042 Α 2.6 2015 SB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 12 Ер 1,2 MIN GREEN + FLASH = 11 Fp Gp MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group C,Gp,D Tram 0.234 0.335 0.327 0.425 у у 555(560) 95(85) 27 10 L (sec) 10 L (sec) 27 345(600) C (sec) 845(830) 95(85) C (sec) 115 115 110 110 **↑** Tram 115(195) 0.818 0.689 0.822 0.679 y pract. y pract. R.C. (%) 194% 145% R.C. (%) 108% 93% Stage / Phase Diagrams 3, FINNIE ST KING'S ROAD Ep D I/G= 7 I/G= I/G= 5 I/G= I/G=

I/G=

九月, 2025

I/G= Junction:

King's Road/Finnie Street

I/G= 5

I/G= 7

I/G=

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Finnie Street Design Year: ____2025_ Designed By: NLY 2025 Observed Checked By: WCY Description:_ **Revised Saturation** Radius (m) Pro. Turning (%) Noon Peak N/A % Flow (pcu/hr) Phase Approach F (pcu/hr) (pcu/hr) 0.088 King's Road 3.0 1915 169 C NB (with Tram) 3.3 2085 185 С 3.5 2105 186 0.088 Finnie Street D 20 54% / 46% 1760 267 0.152 0.152 22.5 1840 0.151 King's Road 1,2 3.7 1985 322 0.162 0.162 SB Α 1.2 3.3 2085 338 0.162 1.2 2.6 2015 0.047 King's Road Α 95 SB (Tram) Pedestrian Crossing 1,2 MIN GREEN + FLASH = 12 Ер MIN GREEN + FLASH = 11 Fp Gp MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group 0.240 0.314 у у 445() 95() 27 10 L (sec) L (sec) 400(0) C (sec) 660(0) 95(0) C (sec) 110 110 **↑** Tram 145(0) 0.679 0.818 y pract. y pract. R.C. (%) 183% 161% R.C. (%) Stage / Phase Diagrams FINNIE ST KING'S ROAD D I/G= I/G= 5 I/G= I/G= #N/A I/G= #N/A #N/A I/G= #N/A #N/A I/G= #N/A | |/G= I/G= Junction: 九月, 2025 King's Road/Finnie Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road EB 5.500 2165 2165 215 0.099 185 0.085 King's Road Α 2.200 1975 1975 95 0.048 95 0.048 EB(Tram) King's Road 3.200 2075 2075 594 0.286 0.286 671 0.323 0.323 WB 3.300 2085 2085 596 0.286 674 0.323 free flow 3.300 1640 1640 245 0.149 275 0.168 2075 0.046 0.041 King's Road 3.200 2075 95 85 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group A,Bp 0.286 0.323 у у 95(95) 215(185) L (sec) 28 L (sec) 28 C (sec) 1190(1345) 95(85) C (sec) 115 110 245(275) 0.681 0.671 y pract. y pract. 107% R.C. (%) 138% R.C. (%) Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= 4 I/G= 5 1/G= | I/G= I/G= 九月, 2025 King's Road/ Model Lan

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road EB 5.500 2165 170 0.079 King's Road Α 2.200 1975 100 0.051 EB(Tram) King's Road 3.200 2075 564 0.272 0.272 WB 3.300 2085 566 0.271 free flow 3.300 1640 270 0.165 0.046 King's Road 3.200 2075 95 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group 0.272 у у 170() 100() L (sec) 28 L (sec) C (sec) C (sec) 1130() 95() 110 Tram 270() 0.671 y pract. y pract. R.C. (%) 147% R.C. (%) Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= #N/A #N/A I/G= #N/A #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Model Land

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ Checked By: WCY 2025 Observed Designed By: Description: NLY Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left (pcu/hr) (pcu/hr) (m) 1 257 271 Java Road 3.000 12 1700 1700 231 0.136 0.136 0.151 0.151 15 95% 3.000 100% 1870 1875 254 0.136 0.144 ΕB Α * 3.000 18 3% 4% 2050 2045 278 0.136 295 0.144 Α 17 1890 1890 257 0.136 277 0.146 Man Hong Street В 2,3 3.200 2075 2075 386 0.186 328 0.158 SB В 2,3 3.400 2095 2095 389 0.186 332 0.158 В 2.3 3.800 12 1775 1775 390 0.220 300 0.169 С 2105 0.185 0.185 0.171 0.171 3 6.300 10 8% 2110 390 360 Man Hong Street NB Pedestrian Crossing 2.3 MIN GREEN + FLASH = 12 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = Fp 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group A,Fp,C Opposing factor of 0.5 has been adopted 485(515) 0.356 0.321 0.320 0.322 у у on Man Hong Street NB due to the presence of opposite movement on right 360(335) 270(295) 775(660) 16 30 16 30 L (sec) L (sec) turn C (sec) C (sec) 115 115 110 110 0.769 0.655 30(25) 390(300) y pract. 0.775 0.665 y pract. R.C. (%) 118% 107% R.C. (%) 140% 103% Stage / Phase Diagrams 3. I/G= 7 I/G= 8 I/G= 4 I/G= I/G=

1/G=

九月, 2025

I/G=

I/G= 4

I/G= 7

I/G= 8

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left (pcu/hr) (pcu/hr) (m) 1 0.155 0.155 Java Road 3.000 12 1700 264 15 0.155 3.000 78% 1905 296 EB Α Α 3.000 18 25% 2015 312 0.155 17 1890 293 0.155 Man Hong Street В 2,3 3.200 2075 336 0.162 SB В 2,3 3.400 2095 339 0.162 В 2.3 3.800 12 1775 255 0.144 С 12% 0.160 0.160 3 6.300 10 2095 335 Man Hong Street NB Pedestrian Crossing 2,3 MIN GREEN + FLASH = 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group Opposing factor of 0.5 has been adopted on Man Hong Street NB due to the 370() 0.317 0.315 у у presence of opposite movement on right 295() 300() 675() 16 30 L (sec) L (sec) turn C (sec) 110 110 C (sec) 40() 0.769 0.655 255() y pract. y pract. R.C. (%) 143% 108% R.C. (%) Stage / Phase Diagrams 3. I/G= 7 I/G= 8 I/G= 4 I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A 1/G= I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ 2025 Observed Designed By: NLY Description: Checked By: WCY Revised Saturation Pro. Turning (%) AM Peak PM Peak % Flow (pcu/hr) Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 1.2 3.300 1945 1945 70 0.036 60 0.031 2105 1.2 3.500 2105 0.036 65 0.031 EB Α 75 King's Road 1,2 2.500 2005 2005 95 0.047 95 0.047 Α EB(Tram) King's Road В 3.500 8 80% 72% 1710 1730 249 0.146 244 0.141 WB В 3.400 2095 2095 306 0.146 0.146 0.141 0.141 King's Road At 1.2 2 700 2025 2025 95 0.047 85 0.042 WB(Tram) D 3.300 1945 1945 0.153 285 0.147 Man Hong Street 4 298 2075 0.153 4 D 3.500 40 40% 28% 2085 318 0.153 304 0.146 SB 4 35 2020 2020 0.153 0.147 0.147 D 3.500 309 296 Pedestrian Crossing Ер 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 Gp 2.3.4 MIN GREEN + FLASH = 5 6 11 1.2.3 MIN GREEN + FLASH = Hр 14 Notes: Flow: (pcu/hr) Group Hp,D B,Fp,D Group Hp,D B,Fp,D 0.153 0.299 0.147 0.288 У У Tram 435(380) 490(505) Tram 24 35 24 35 L (sec) L (sec) C (sec) 115 115 C (sec) 110 110 → 95(95) 95(85) 0.712 0.614 ----- 145(125) 355(365) y pract. 0.626 y pract. 0.704 200(175) R.C. (%) 365% 109% R.C. (%) 380% 113% Stage / Phase Diagrams MAN HONG ST 3. Ğp`% Gp\% Dp Dр ⊶ At KING'S RD ⊶ At Fp Fр → Ep of → Eb of HEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G=

I/G= 2

九月, 2025

I/G=

King's Road/ Healthy Street West/ Man

1/G=

I/G= 11

I/G= 9

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ Designed By: NLY 2025 Observed Description: Checked By: WCY Revised Saturation Pro. Turning (%) Noon Peak N/A % Flow (pcu/hr) Gradient Stage Width Flow Flow Approach Left Critical y (pcu/hr) (pcu/hr) (m) King's Road 1,2 3.300 1945 60 0.031 2105 1.2 3.500 65 0.031 EB Α King's Road 1,2 2.500 2005 100 0.050 Α EB(Tram) King's Road В 3.500 74% 1725 244 0.141 0.141 WB В 3.400 2095 296 0.141 King's Road At 1,2 2.700 2025 95 0.047 WB(Tram) D 4 3.300 1945 313 0.161 Man Hong Street 22% 4 D 4 3.500 40 2090 337 0.161 0.161 SB 35 2020 325 D 3.500 0.161 Pedestrian Crossing Еp 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 Gp 2.3.4 MIN GREEN + FLASH = 5 6 11 1.2.3 MIN GREEN + FLASH = ДH 14 Notes: Flow: (pcu/hr) Group Hp,D B,Fp,D Group 0.161 0.303 У у 400() 575() Tram 24 35 L (sec) L (sec) Tram C (sec) 110 110 C (sec) → 100() 95() -----**>** 125() 360() y pract. 0.704 0.614 y pract. 180() R.C. (%) 336% 103% R.C. (%) Stage / Phase Diagrams 3. MAN HONG ST $ar{W}$ Gp\% Dp ⊶ At KING'S RD Fp Fр → Ep of → **E**p of JEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025 King's Road/ Healthy Street West/ Man

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) Tsat Tsz Mui Road 3.800 26% 33% 1920 1900 156 0.081 184 0.097 0.097 0.081 3.800 2135 2135 174 0.081 206 0.096 WB Healthy Street West 🕹 2.500 1865 1865 227 0.122 223 0.120 SB 3.300 13 51% 46% 1970 1980 239 0.121 237 0.120 0.120 3.600 10 1840 1840 224 0.122 0.122 220 0.120 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = 9 Ер 3 5 14 Fρ MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group A,C,Fp A,C,Ep 345(330) 0.203 0.203 0.217 0.217 у у 345(350) 28 29 28 29 L (sec) L (sec) C (sec) C (sec) 115 115 110 110 290(330) **√** 40(60) 0.681 0.673 0.671 0.663 y pract. y pract. R.C. (%) 235% 231% R.C. (%) 210% 206% Stage / Phase Diagrams 2. 3. Сţ Dp H TSAT TSZ MUI RD HEALTHY ST I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= 4 I/G= 6 I/G= 7 14 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value (pcu/hr) (pcu/hr) (m) Tsat Tsz Mui Road 3.800 34% 1900 191 0.101 0.101 3.800 2135 214 0.100 WB Healthy Street West 🕹 2.500 246 0.132 SB 3.300 13 24% 2030 267 0.132 3.600 10 1840 242 0.132 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = Ep 3 5 14 Fp MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group 0.232 0.232 у у 450() 28 29 L (sec) L (sec) 340() C (sec) 110 110 C (sec) **√** 65() 0.671 0.663 y pract. y pract. R.C. (%) 189% 185% R.C. (%) Stage / Phase Diagrams 2. 3, Сţ Dp I TSAT TSZ MUI RD HEALTHY ST P I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Left y Value Critical y Critical y (pcu/hr) (pcu/hr) (m) Tin Chiu Street 3.900 2005 2005 165 0.082 0.082 150 0.075 0.075 NB rsat Tsz Mui Road ₽ В 4.100 6 13 34% / 66% 34% / 66% 1745 1745 291 0.167 305 0.175 4.100 13 1940 324 0.167 0.167 0.175 0.175 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 10 19 Dρ 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group Ср,В A,B,Dp 0.167 0.249 0.175 0.250 у у 165(150) 515(540) 27 32 L (sec) 27 32 L (sec) C (sec) C (sec) 115 115 110 110 0.689 0.650 0.679 0.638 y pract. y pract. 287% 100(105) R.C. (%) 312% 161% R.C. (%) 155% Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= 4 I/G= 6 I/G= 8 | I/G= I/G= 九月, 2025 J10 Tsat Tsz Mui Road/ Tin Chiu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu West Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Width Flow Flow Left y Value (pcu/hr) (pcu/hr) (m) Tin Chiu Street 3.900 2005 180 0.090 0.090 NB rsat Tsz Mui Road ₽ В 4.100 6 13 49% / 51% 1715 298 0.174 0.174 4.100 13 0.174 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 19 Dp 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group 0.174 0.264 у у 180() 490() 27 32 L (sec) L (sec) C (sec) C (sec) 110 110 0.679 0.638 y pract. y pract. 145() R.C. (%) 291% 142% R.C. (%) Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= I/G= J10 九月, 2025 Tsat Tsz Mui Road/ Tin Chiu West

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.200 13% 11% 1905 1910 225 0.118 225 0.118 ΕB King's Road 2.700 2025 2025 95 0.047 0.047 King's Road В 3.300 1945 1945 250 0.129 0.129 235 0.121 WB В 3.500 2105 2105 270 0.128 255 0.121 0.121 В 3.500 2105 2105 270 0.128 255 0.121 King's Road 0.048 Dt 1 2.400 1995 1995 95 85 0.043 WB(Tram) Tin Chiu Street С 3.300 1730 1730 145 0.084 110 0.064 2 12 NB С 3.500 12 1965 1980 195 0.099 295 0.149 3.500 1870 1870 340 0.182 0.182 0.152 0.152 Pedestrian Crossing MIN GREEN + FLASH = 12 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group B,C 30(25) 0.300 0.310 0.270 0.274 у у 195(200) 790(745) 10 11 10 11 L (sec) L (sec) Tram 95(95) C (sec) 195(295) C (sec) 115 115 110 110 0.822 0.814 0.818 0.810 145(110) → 340(285) y pract. y pract. R.C. (%) 174% 162% R.C. (%) 203% 196% Stage / Phase Diagrams King's Rd I/G= 6 I/G= 7 I/G= I/G= I/G= I/G= 6 I/G= 7 I/G= 1/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2025 Observed Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 12% King's Road 3.200 1905 215 0.113 ΕB King's Road 2.700 2025 100 0.049 King's Road В 3.300 1945 240 0.123 WB В 3.500 2105 260 0.124 0.124 В 3.500 2105 260 0.124 King's Road 0.048 Dt 2.400 1995 95 WB(Tram) Tin Chiu Street 0.087 С 3.300 1730 150 2 12 NB С 3.500 12 63% 1950 250 0.128 3.500 1870 270 0.144 0.144 Pedestrian Crossing MIN GREEN + FLASH = 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group 25() 95() 0.257 0.268 у у 760() 10 11 L (sec) L (sec) C (sec) 110 100() 250() C (sec) 110 0.818 0.810 150() 270() y pract. y pract. R.C. (%) 218% 202% R.C. (%) Stage / Phase Diagrams I/G= 6 I/G= 7 I/G= I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

Calculation Sheets - 2033 Reference Year

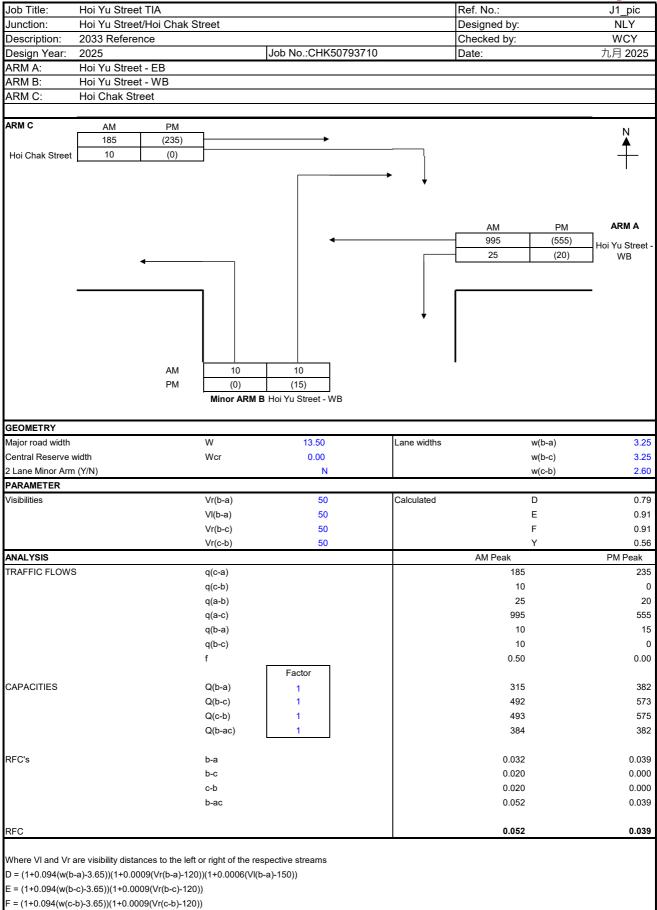
Simplified picority Junction Capacity Calculation

Y = 1-0.0345W

f = proportion of minor traffic turning leftQ (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

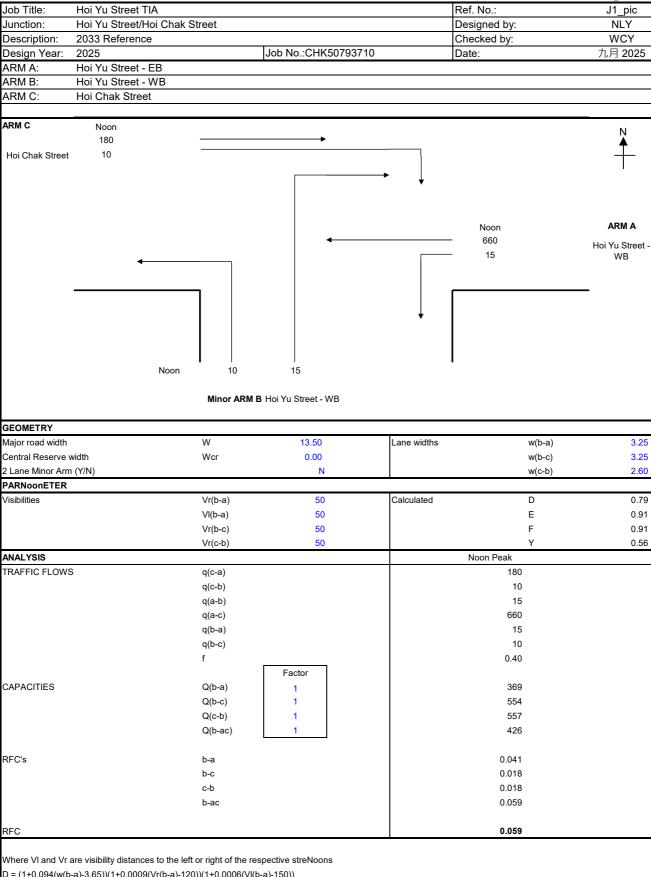




Capacity of combined streams

Simplified picority Junction Capacity Calculation





D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vl(b-a)-150))

E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))

F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))

Y = 1-0.0345W

f = proportion of minor traffic turning left

Q (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)

Capacity of combined streNoons

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Hoi Yu Street (Existing) Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Width Flow Flow Stage Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 1725 1040 0.333 0.383 Java Road 2.4 20 1725 1020 0.591 0.591 575 1040 0.450 3.2 398 Α 468 825 825 371 0.450 315 0.382 Α 3.1 3.1 580 580 261 0.450 222 0.383 0.383 Hoi Yu Street В 2 3.4 15 1905 1905 102 0.054 130 0.068 В 3.4 12.5 1745 1745 93 0.053 120 0.069 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group A,Cp 1. Site factor 0.5, 0.4 and 0.3 are 0.645 0.591 0.452 0.383 у у adopted for Java Road's second, third 195(250) and fourth lane respectively due to 10 26 10 26 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 1020(575) 1100(935) C (sec) C (sec) 115 115 110 110 0.822 0.697 0.818 0.687 y pract. y pract. R.C. (%) 27% 18% R.C. (%) 81% 80% Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p В I/G= 4 I/G= 5 18 I/G= I/G= I/G= I/G= 4 I/G= 5 1/G= 1/G= I/G= 九月, 2025 lava Road/ Hoi Yu Street (Existing)

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: ___CHK50793710 Junction: Java Road/ Hoi Yu Street (Existing) Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Approach Left Critical y (pcu/hr) (m) 1725 1040 Java Road 2.4 20 675 0.391 3.2 410 0.394 825 326 0.395 0.395 Α 3.1 3.1 580 229 0.395 Hoi Yu Street В 2 15 1905 102 0.054 В 3.4 12.5 1745 93 0.053 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group 1. Site factor 0.5, 0.4 and 0.3 are 0.449 0.395 у у adopted for Java Road's second, third 195(0) and fourth lane respectively due to 10 26 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 675(0) 965(0) C (sec) C (sec) 110 110 0.818 0.687 y pract. y pract. R.C. (%) 82% 74% R.C. (%) Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p I/G= 4 I/G= 5 18 I/G= I/G= I/G=

| I/G=

九月, 2025

I/G=

lava Road/ Hoi Yu Street (Existing)

I/G= #N/A

#N/A

I/G= #N/A

#N/A

I/G=

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ Checked By: WCY 2033 Reference Designed By: NLY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 5.3 2145 2145 230 0.107 200 0.093 ΕB King's Road 2.7 2025 2025 100 0.049 0.049 EB (Tram) Java Road С 2 3.5 30 1870 1870 254 0.136 0.136 297 0.159 0.159 С 2 4.1 32.5 2070 2070 281 0.136 328 0.158 D 2.3 52.5 2025 272 3.3 2025 383 0.189 0 134 2.3 1890 1890 357 0.189 253 0.134 3.3 50 1935 0.175 King's Road В 1,3 3.2 1935 338 359 0.186 WB В 1,3 3.2 2075 2075 363 0.175 386 0.186 0.186 1,3 1965 1965 0.175 0.175 0.186 В 3.5 365 King's Road В 1,3 2.3 1845 1845 100 0.054 90 0.049 WB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 1,3 12 Еp MIN GREEN + FLASH = 9 14 Fp 5 Gp MIN GREEN + FLASH = 5 10 2.3 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group Group B,C Tram 0.296 0.311 0.228 0.345 у у 230(200) 100(100) 12 10 12 10 L (sec) L (sec) 535(625) C (sec) 1045(1110) 100(90) C (sec) 115 115 110 110 **≜**Tram 740(525) 0.806 0.802 0.818 y pract. 0.822 y pract. R.C. (%) 172% 164% R.C. (%) 252% 137% Stage / Phase Diagrams 2. 3. S Java Road I/G= I/G= 5 I/G= 7 I/G= I/G= I/G= I/G= 5 I/G= 7 1/G= I/G= Junction:

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Approach Left y Value Critical y (pcu/hr) (m) King's Road 5.3 2145 185 0.086 ΕB King's Road 2.7 2025 105 0.052 EB (Tram) Java Road С 2 3.5 30 1870 261 0.140 С 0.140 2 4.1 32.5 2070 289 0.140 D 2.3 52.5 2025 0 144 3.3 292 2.3 50 1890 273 0.144 3.3 0.162 King's Road 1,3 3.2 1935 313 0.162 WB В 1,3 3.2 2075 335 0.161 1,3 1965 0.161 3.5 King's Road В 1,3 2.3 1845 100 0.054 WB (Tram) Pedestrian Crossing Ер MIN GREEN + FLASH = 1,3 12 MIN GREEN + FLASH = 9 14 Fp 5 Gp 2.3 MIN GREEN + FLASH = 5 10 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group Group Tram 0.231 0.301 у у 185(0) 105(0) 12 10 L (sec) L (sec) 550(0) C (sec) C (sec) 110 965(0) 100(0) 110 **≜**Tram 565(0) 0.802 0.818 y pract. y pract. R.C. (%) 248% 171% R.C. (%) Stage / Phase Diagrams 3. I/G= I/G= 5 I/G= 7 I/G= I/G= I/G= #N/A #N/A #N/A #N/A I/G= #N/A I/G= #N/A | I/G= I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value y Value Critical y (pcu/hr) (pcu/hr) (m) 0.074 0.264 Hoi Tai Street 3.5 2030 2030 0.042 150 2 1835 0.191 0.264 D 3.6 20 1835 350 485 Hoi Kwong Street 4 С 3.6 15 1830 1815 125 0.068 0.096 0.096 Hoi Chak Street В 1,2 3.1 35 1845 1845 790 0.428 0.428 470 0.255 3 3.5 30 2005 2005 75 0.037 70 0.035 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 3,4 MIN GREEN + FLASH = 16 Fp Gp 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = 12 Notes: Flow: (pcu/hr) Group B,Fp B,Gp Group C,D,Fp C,D,Gp 0.428 0.428 0.361 0.361 790(470) 75(70) 85(150) 26 27 L (sec) 33 34 L (sec) C (sec) C (sec) 350(485) 115 115 110 110 100(155) 0.697 0.689 0.630 0.622 y pract. y pract. R.C. (%) 63% 61% R.C. (%) 75% 72% Stage / Phase Diagrams I/G= 2 I/G= I/G= 8 18 I/G= I/G= I/G= 2 I/G= 6 I/G= 10 18 I/G= I/G=

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left y Value Critical y (pcu/hr) (m) 2030 1835 0.047 0.218 Hoi Tai Street 3.5 2 0.218 D 3.6 20 400 Hoi Kwong Street 4 3.6 15 75% 1835 0.076 0.076 Hoi Chak Street 1,2 35 1845 455 0.247 3 3.5 30 2005 80 0.040 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 Fp Gp 3,4 MIN GREEN + FLASH = 16 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = Notes: Flow: (pcu/hr) Group C,D,Fp C,D,Gp Group 0.294 0.294 у 455(0) 80(0) 95(0) 33 34 L (sec) L (sec) C (sec) 110 C (sec) 400(0) 110 105(0) 0.630 0.622 y pract. y pract. R.C. (%) 114% 111% R.C. (%) Stage / Phase Diagrams I/G= 2 I/G= 6 I/G= 10 18 I/G= #N/A #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Finnie Street Design Year: ____2025_ Designed By: NLY Checked By: WCY 2033 Reference Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.0 1915 1915 238 0.124 231 0.121 Ċ 0.120 NB (with Tram) 3.3 2085 2085 260 0.125 251 0.120 С 3.5 2105 2105 262 0.124 253 0.120 Finnie Street D 20 100% / 0% 100% / 0% 1710 1710 360 0.211 0.211 625 0.365 0.365 3.5 22.5 1840 1840 120 0.065 0.111 0.218 0.218 0 214 King's Road Α 1.2 3.7 1985 1985 432 424 SB Α 1.2 3.3 2085 2085 453 0.217 446 0.214 0.053 1.2 2.6 1875 1875 100 0.053 100 King's Road Α SB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 12 Ер 1,2 MIN GREEN + FLASH = Fp 11 MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group C,Gp,D Tram 0.335 0.428 0.579 0.486 у у 660(635) 100(100) 27 10 10 27 L (sec) L (sec) 120(205) C (sec) 885(870) 100(100) C (sec) 115 115 110 110 **↑** Tram 360(625) 0.818 0.689 0.679 y pract. 0.822 y pract. R.C. (%) 105% 92% R.C. (%) 41% 40% Stage / Phase Diagrams 3. FINNIE ST KING'S ROAD D I/G= 7 I/G= I/G= 5 I/G= I/G= 11 I/G= 7 I/G= 9 I/G= 2 1/G= I/G= Junction: 九月, 2025 King's Road/ Finnie Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Finnie Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Radius (m) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left y Value Critical y (pcu/hr) (m) King's Road 3.0 1915 194 0.101 C 2085 0.102 NB (with Tram) 3.3 212 0.102 С 3.5 2105 214 0.102 Finnie Street D 20 100% / 0% 1710 420 0.246 0.246 22.5 1840 155 0.084 0.171 King's Road Α 1.2 3.7 1985 339 SB Α 1.2 3.3 2085 356 0.171 0.056 1.2 2.6 1875 King's Road Α 105 SB (Tram) Pedestrian Crossing 1,2 MIN GREEN + FLASH = 12 Ер MIN GREEN + FLASH = 11 Fp MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group Tram 0.416 0.347 у у 515() 105() 10 27 L (sec) L (sec) **▲**155(0) C (sec) C (sec) 695(0) 105(0) 110 110 **↑** Tram 420(0) 0.818 0.679 y pract. y pract. R.C. (%) 96% 96% R.C. (%) Stage / Phase Diagrams 3. FINNIE ST KING'S ROAD o D I/G= 7 I/G= 9 11 I/G= 2 I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A 1/G= I/G= Junction: 九月, 2025 King's Road/ Finnie Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road EB 5.500 2165 2165 230 0.106 200 0.092 King's Road Α 2.200 1975 1975 100 0.051 100 0.051 EB(Tram) King's Road 3.200 2075 2075 661 0.319 0.319 721 0.347 0.347 WB 3.300 2085 2085 664 0.318 724 0.347 free flow 3.300 1640 1640 255 0.155 290 0.177 2075 0.048 0.043 King's Road 3.200 2075 100 90 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group A,Bp 0.319 0.347 у у 100(100) 230(200) L (sec) 28 L (sec) 28 C (sec) 1325(1445) 100(90) C (sec) 115 110 Tram 0.681 0.671 255(290) y pract. y pract. R.C. (%) 114% R.C. (%) 93% Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= 4 I/G= 5 1/G= | I/G= I/G= 九月, 2025 King's Road/ Model Lan

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ Description:_ 2033 Reference Designed By: NLY Checked By: WCY Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Approach Left y Value Critical y (pcu/hr) (m) King's Road EB 5.500 2165 185 0.085 King's Road Α 2.200 1975 105 0.053 EB(Tram) King's Road 3.200 2075 614 0.296 0.296 WB 3.300 2085 616 0.295 free flow 3.300 1640 285 0.174 2075 0.048 King's Road 3.200 100 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group 0.296 у у 28 105() 185() L (sec) L (sec) C (sec) C (sec) 1230() 100() 110 Tram 0.671 y pract. 285() y pract. R.C. (%) 127% R.C. (%) Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= #N/A #N/A I/G= #N/A #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Model Land

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ Checked By: WCY 2033 Reference Designed By: Description: NLY Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Approach Left (pcu/hr) (pcu/hr) (m) 1 0.142 0.143 Java Road 3.000 12 1700 1700 242 270 0.159 0.159 15 95% 0.143 3.000 99% 1870 1875 267 284 0.152 ΕB Α * 3.000 18 4% 5% 2050 2045 292 0.142 310 0.152 Α 17 1890 1890 269 0.142 291 0.154 Man Hong Street В 2,3 3.200 2075 2075 423 0.204 371 0.179 SB В 2,3 3.400 2095 2095 427 0.204 374 0.179 В 2.3 3.800 12 1775 1775 410 0.231 315 0.177 С 0.235 0.235 0.201 0.201 3 6.300 10 7% 2110 2110 495 425 Man Hong Street NB Pedestrian Crossing 2.3 MIN GREEN + FLASH = 12 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = Fp 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group A,Fp,C Opposing factor of 0.5 has been adopted 0.374 0.377 0.338 0.360 у у on Man Hong Street NB due to the presence of opposite movement on right 460(395) 285(310) 850(745) 16 30 16 30 L (sec) L (sec) turn C (sec) C (sec) 115 115 110 110 0.769 0.655 35(30) 410(315) y pract. 0.775 0.665 y pract. R.C. (%) 107% 76% R.C. (%) 128% 82% Stage / Phase Diagrams 3. I/G= 7 I/G= 8 I/G= 4 I/G= I/G=

1/G=

九月, 2025

I/G=

I/G= 4

I/G= 7

I/G= 8

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left (pcu/hr) (m) 1 12 15 0.163 0.163 Java Road 3.000 1700 277 1905 0.163 3.000 78% 311 EB Α Α 3.000 18 25% 2015 328 0.163 17 1890 308 0.163 Man Hong Street В 2,3 3.200 2075 381 0.184 SB В 2,3 3.400 2095 384 0.183 В 2.3 3.800 12 1775 270 0.152 С 11% 0.191 0.191 3 6.300 10 2095 400 Man Hong Street NB Pedestrian Crossing 2,3 MIN GREEN + FLASH = 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group Opposing factor of 0.5 has been adopted on Man Hong Street NB due to the 390() 520() 0.347 0.354 у у presence of opposite movement on right 355() 315() 765() 16 30 L (sec) L (sec) turn C (sec) C (sec) 110 110 270() 45() 0.769 0.655 y pract. y pract. R.C. (%) 122% 85% R.C. (%) Stage / Phase Diagrams I/G= 7 I/G= 8 I/G= 4 I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A | I/G= I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ Designed By: NLY Description: 2033 Reference Checked By: WCY Revised Saturation Pro. Turning (%) AM Peak PM Peak % Flow (pcu/hr) Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 1,2 3.300 1945 1945 74 0.038 65 0.033 2105 1.2 3.500 2105 81 0.038 70 0.033 EB Α King's Road 1,2 2.500 2005 2005 100 0.050 100 0.050 Α EB(Tram) King's Road В 3.500 80% 72% 1710 1730 263 0.154 0.154 258 0.149 0.149 WB В 3.400 2095 2095 322 0.154 0.149 King's Road At 1.2 2 700 2025 2025 100 0.049 90 0.044 WB(Tram) D 3.300 1945 1945 325 0.167 315 0.162 0.162 Man Hong Street 4 34% 2080 4 D 3.500 40 22% 2090 347 0.167 338 0.162 SB 4 35 2020 2020 338 0.167 327 D 3.500 0.167 0.162 Pedestrian Crossing Ер 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 Gp 2.3.4 MIN GREEN + FLASH = 5 6 11 1.2.3 MIN GREEN + FLASH = Hр 14 Notes: Flow: (pcu/hr) Group Hp,D B,Fp,D Group Hp,D B,Fp,D0.167 0.321 0.162 0.311 У У Tram 455(400) 555(580) Tram 24 35 24 35 L (sec) L (sec) C (sec) 115 115 C (sec) 110 110 → 100(100) 100(90) -----0.712 0.614 ----- 155(135) 375(385) y pract. 0.626 y pract. 0.704 210(185) R.C. (%) 326% 95% R.C. (%) 334% 97% Stage / Phase Diagrams MAN HONG ST 3. Gp & Gp\% Dp ⊶ At KING'S RD ⊶ At Fp Fр → Ep of → Ep of HEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G=

I/G= 2

九月, 2025

I/G=

King's Road/ Healthy Street West/ Man

1/G=

I/G= 11

I/G= 9

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ Designed By: NLY Description: 2033 Reference Checked By: WCY Revised Saturation Pro. Turning (%) Noon Peak N/A % Flow (pcu/hr) Gradient Stage Width Flow Approach Left y Value Critical y (pcu/hr) (m) King's Road 1,2 3.300 1945 65 0.033 2105 1.2 3.500 70 0.033 EB Α King's Road Α 1,2 2.500 2005 105 0.052 EB(Tram) King's Road В 3.500 74% 1725 257 0.149 WB В 3.400 2095 313 0.149 0.149 0.049 King's Road At 1,2 2.700 2025 100 WB(Tram) D 4 3.300 1945 347 0.178 Man Hong Street 0.178 4 D 4 3.500 40 16% 2090 373 0.178 SB 35 2020 360 D 3.500 0.178 Pedestrian Crossing Еp 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 2,3,4 MIN GREEN + FLASH = Gp 5 6 11 1.2.3 MIN GREEN + FLASH = ДH 14 Notes: Flow: (pcu/hr) Group B,Fp,D Group 0.178 0.328 У у 420() 660() Tram 24 35 L (sec) L (sec) Tram C (sec) 110 110 C (sec) → 105() 100() ----**>** 135() 380() y pract. 0.704 0.614 y pract. 190() R.C. (%) 294% 87% R.C. (%) Stage / Phase Diagrams 3. MAN HONG ST $ar{W}$ Gp & Gp\% Dp ⊶ At KING'S RD ⊶ At Fp Fр → Ep of → **E**p of JEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025 King's Road/ Healthy Street West/ Man

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 0.102 0.102 Tsat Tsz Mui Road 3.800 27% 34% 1915 1900 166 0.087 0.087 193 0.102 3.800 2135 2135 184 0.086 217 WB Healthy Street West 🕹 2.500 1865 1865 252 0.135 252 0.135 SB 3.300 13 59% 57% 1950 1955 264 0.135 0.135 264 0.135 3.600 10 1840 1840 249 0.135 249 0.135 0.135 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = 9 Ер 3 5 14 Fρ MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group A,C,Fp A,C,Ep 405(400) 0.222 0.222 0.237 0.237 у у 360(365) 28 29 28 29 L (sec) L (sec) C (sec) 305(345) C (sec) 115 115 110 110 **√** 45(65) 0.681 0.673 0.671 0.663 y pract. y pract. R.C. (%) 207% 203% R.C. (%) 183% 180% Stage / Phase Diagrams 2. 3. Сţ Dp H TSAT TSZ MUI RD HEALTHY ST I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= 4 I/G= 6 I/G= 7 14 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left y Value Critical y (pcu/hr) (m) Tsat Tsz Mui Road 3.800 35% 1895 202 0.107 2135 0.107 3.800 228 0.107 WB Healthy Street West 🕹 2.500 1865 278 0.149 SB 3.300 13 34% 2005 298 0.149 3.600 10 1840 274 0.149 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = Ep 3 5 14 Fp MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group 0.256 0.256 у у 475() 28 29 L (sec) L (sec) C (sec) C (sec) 360() 110 110 √ 70() 0.671 0.663 y pract. y pract. R.C. (%) 162% 159% R.C. (%) Stage / Phase Diagrams 2. 3, Сţ Dp I TSAT TSZ MUI RD HEALTHY ST P I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu West Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Left y Value Critical y (pcu/hr) (pcu/hr) (m) Tin Chiu Street 3.900 2005 2005 450 0.224 0.224 300 0.150 0.150 NB rsat Tsz Mui Road ₽ В 4.100 6 13 45% / 55% 48% / 52% 1720 322 0.187 0.187 343 0.200 0.200 4.100 13 1940 363 0.187 0.199 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 10 19 Dρ 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group Ср,В A,B,Dp 0.187 0.412 0.200 0.350 у у 450(300) 540(565) 27 32 L (sec) 27 32 L (sec) C (sec) 115 C (sec) 115 110 110 0.689 0.650 0.679 0.638 y pract. y pract. 145(165) R.C. (%) 268% 58% R.C. (%) 240% 83% Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= 4 I/G= 6 I/G= 8 | I/G= I/G= 九月, 2025 J10 Tsat Tsz Mui Road/ Tin Chiu West

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu West Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Width Flow Left y Value Critical y (pcu/hr) (m) Tin Chiu Street 3.900 2005 330 0.165 0.165 NB rsat Tsz Mui Road ₽ В 4.100 6 13 62% / 38% 1690 337 0.199 4.100 13 0.200 0.200 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 19 Dp 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group 330() 0() 0.200 0.365 у у 27 L (sec) 32 L (sec) 515() C (sec) 110 C (sec) 0() 110 0.679 0.638 0() y pract. y pract. 240% R.C. (%) 75% R.C. (%) Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= I/G= J10 九月, 2025 Tsat Tsz Mui Road/ Tin Chiu West

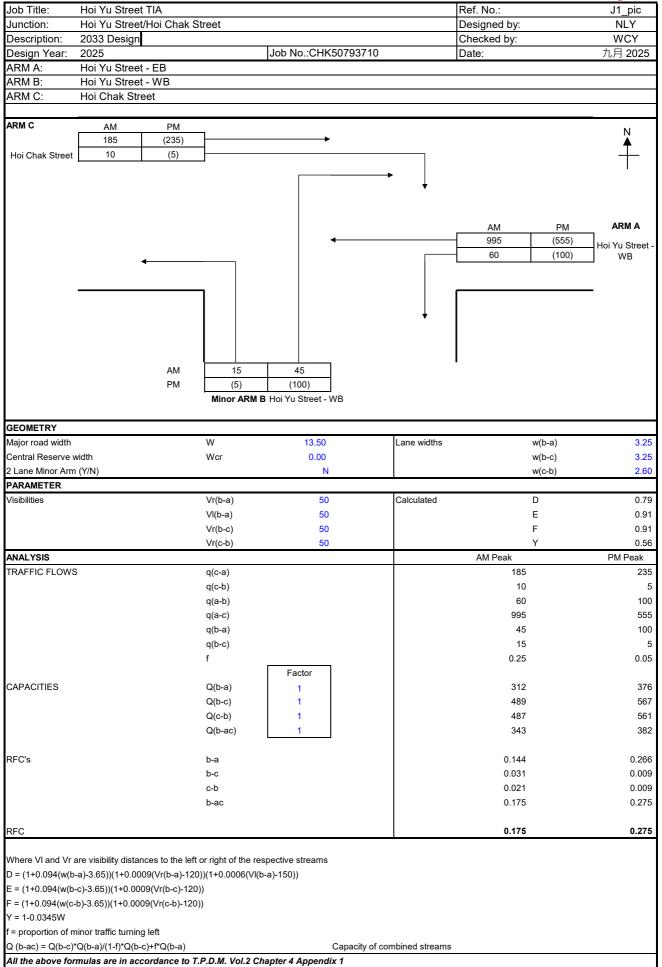
TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.200 14% 12% 1900 1905 245 0.129 250 0.131 0.131 ΕB King's Road 2.700 2025 2025 100 0.049 0.049 King's Road В 3.300 1945 1945 262 0.135 248 0.128 WB В 3.500 2105 2105 284 0.135 0.135 269 0.128 В 3.500 2105 2105 284 0.135 268 0.127 King's Road 0.050 0.045 Dt 1 2.400 1995 1995 100 90 WB(Tram) Tin Chiu Street С 3.300 1730 1730 345 0.199 215 0.124 2 12 NB С 3.500 12 48% 2010 1985 205 0.102 310 0.156 3.500 1870 1870 440 0.235 0.235 340 0.182 0.182 Pedestrian Crossing MIN GREEN + FLASH = 12 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group B,C A,C 35(30) 0.364 0.370 0.310 0.313 у у 210(220) 830(785) 10 11 11 10 L (sec) L (sec) Tram 100(100) 205(310) C (sec) C (sec) 115 115 110 110 0.822 0.814 0.810 0.818 345(215) → 440(340) y pract. y pract. R.C. (%) 126% 120% R.C. (%) 162% 161% Stage / Phase Diagrams I/G= 6 I/G= 7 I/G= I/G= I/G= I/G= 7 I/G= 5 I/G= 1/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2033 Reference Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Approach Left y Value Critical y (pcu/hr) (m) 13% King's Road 3.200 1905 240 0.126 ΕB King's Road 2.700 2025 105 0.052 King's Road В 3.300 1945 253 0.130 WB В 3.500 2105 273 0.130 0.130 В 3.500 2105 274 0.130 0.050 King's Road Dt 2.400 1995 100 WB(Tram) Tin Chiu Street С 3.300 1730 255 0.147 2 12 NB С 3.500 12 51% 1980 265 0.134 3.500 1870 325 0.174 0.174 Pedestrian Crossing MIN GREEN + FLASH = 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group 30() 100() 0.300 0.304 у у 210() 800() 10 11 L (sec) L (sec) C (sec) C (sec) 110 105() 265() 110 0.818 0.810 255() 325() y pract. y pract. R.C. (%) 173% 166% R.C. (%) Stage / Phase Diagrams I/G= 6 I/G= 7 I/G= I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

Calculation Sheets - 2033 Design Year

Simplified picority Junction Capacity Calculation





Simplified picority Junction Capacity Calculation

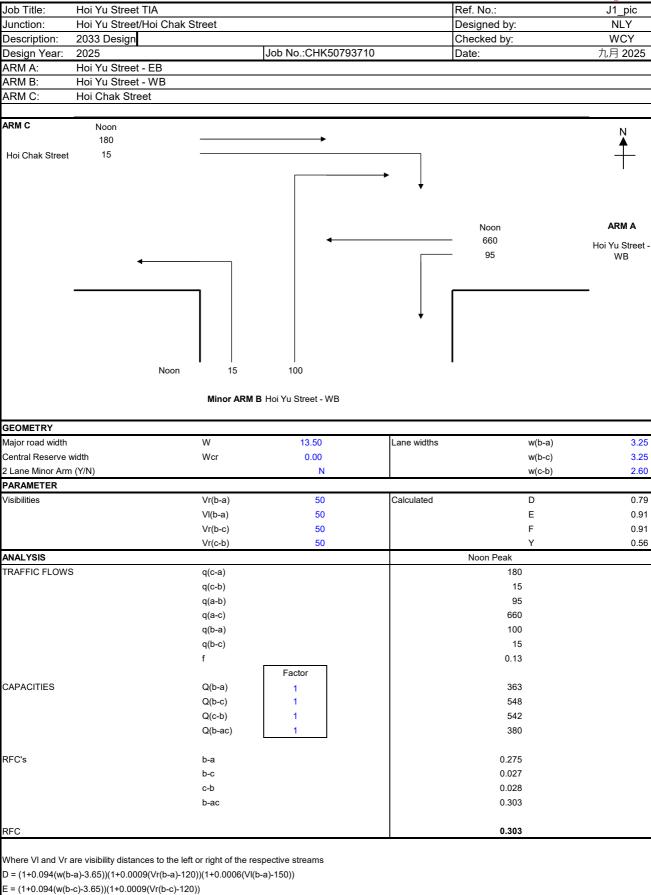
F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

f = proportion of minor traffic turning leftQ (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)

Y = 1-0.0345W





Capacity of combined streams

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Hoi Yu Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Width Flow Flow Stage Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 1725 1040 0.380 0.383 Java Road 2.4 20 1725 1055 0.612 0.612 655 1040 0.450 3.2 468 398 \rightarrow Α 825 825 371 0.450 315 0.382 Α 3.1 3.1 580 580 261 0.450 222 0.383 0.383 Hoi Yu Street В 2 3.4 15 1905 1905 120 0.063 175 0.092 0.092 В 3.4 12.5 1745 1745 110 0.063 160 0.092 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group 1. Site factor 0.5, 0.4 and 0.3 are 0.675 0.612 0.383 0.475 у у adopted for Java Road's second, third 230(335) and fourth lane respectively due to 10 26 26 10 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 1055(655) 1100(935) C (sec) C (sec) 115 115 110 110 0.822 0.697 0.687 0.818 y pract. y pract. R.C. (%) 22% 14% R.C. (%) 80% 72% Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p В I/G= 4 I/G= 5 18 I/G= I/G= I/G= I/G= 5 I/G= 7 I/G= 1/G= I/G= Junction: 九月, 2025 lava Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Hoi Yu Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Flow Approach Left Critical y (pcu/hr) (pcu/hr) (m) 1725 1040 0.438 0.394 Java Road 2.4 20 755 0.438 Α 3.2 410 3.1 825 326 0.395 Α 3.1 580 229 0.395 Hoi Yu Street В 2 15 1905 146 0.077 В 3.4 12.5 1745 134 0.077 Pedestrian Crossing Ср MIN GREEN + FLASH = 10 18 Notes: Flow: (pcu/hr) Group Group 1. Site factor 0.5, 0.4 and 0.3 are 0.514 0.438 у у adopted for Java Road's second, third 280(0) and fourth lane respectively due to 10 26 L (sec) L (sec) downstream blocking and illegal parking along kerbside observed. 755(0) 965(0) C (sec) C (sec) 110 110 0.818 0.687 y pract. y pract. R.C. (%) 59% 57% R.C. (%) Stage / Phase Diagrams 2. 3. HOI YUSI. ζ_p I/G= 4 I/G= 5 18 I/G= I/G= I/G= I/G= #N/A #N/A I/G= #N/A #N/A I/G= | I/G= I/G= 九月, 2025 lava Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ Checked By: WCY 2033 Design Designed By: NLY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 5.3 2145 2145 230 0.107 200 0.093 ΕB King's Road 2.7 2025 2025 100 0.049 0.049 EB (Tram) Java Road С 2 3.5 30 1870 1870 268 0.143 330 0.176 0.176 С 0 143 2 4.1 32.5 2070 2070 297 0.143 365 0.176 D 2.3 52.5 2025 0.190 279 3.3 2025 385 0.138 2.3 1890 1890 360 0.190 0.138 3.3 50 261 1935 0.175 King's Road 1,3 3.2 1935 338 359 0.186 WB В 1,3 3.2 2075 2075 363 0.175 386 0.186 0.186 1,3 1965 1965 0.175 0.175 0.186 В 3.5 365 King's Road В 1,3 2.3 1845 1845 100 0.054 90 0.049 WB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 1,3 12 Еp MIN GREEN + FLASH = 9 14 Fp 5 Gp MIN GREEN + FLASH = 5 10 2.3 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group Group B,C Tram 0.298 0.319 0.231 0.362 у у 230(200) 100(100) 12 10 12 10 L (sec) L (sec) 565(695) C (sec) 1045(1110) 100(90) C (sec) 115 115 110 110 **≜**Tram 745(540) 0.806 0.822 0.802 0.818 y pract. y pract. R.C. (%) 171% 158% R.C. (%) 247% 126% Stage / Phase Diagrams 2. 3. S Java Road I/G= I/G= 5 I/G= 7 I/G= I/G= I/G= I/G= 5 I/G= 7 1/G= I/G= Junction:

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Java Road Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description: Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 5.3 2145 185 0.086 ΕB King's Road 2.7 2025 105 0.052 EB (Tram) Java Road С 2 3.5 30 1870 294 0.157 С 0.157 2 4.1 32.5 2070 326 0.157 D 2.3 52.5 2025 0 148 3.3 300 2.3 50 1890 280 0.148 3.3 0.162 King's Road 1,3 3.2 1935 313 0.162 WB В 1,3 3.2 2075 335 0.161 1,3 1965 0.161 3.5 King's Road В 1,3 2.3 1845 100 0.054 WB (Tram) Pedestrian Crossing Ер MIN GREEN + FLASH = 1,3 12 MIN GREEN + FLASH = 9 14 Fp 5 Gp MIN GREEN + FLASH = 5 10 2.3 5 MIN GREEN + FLASH = 18 Notes: Flow: (pcu/hr) Group Group Tram 0.234 0.319 у у 185(0) 105(0) 12 10 L (sec) L (sec) 620(0) C (sec) C (sec) 110 965(0) 100(0) 110 **≜**Tram 580(0) 0.802 0.818 y pract. y pract. R.C. (%) 242% 156% R.C. (%) Stage / Phase Diagrams 3. I/G= I/G= 5 I/G= 7 I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A | I/G= I/G= Junction: 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value y Value Critical y (pcu/hr) (pcu/hr) (m) 0.076 0.264 Hoi Tai Street 3.5 2030 2030 0.042 155 2 1835 0.191 0.264 D 3.6 20 1835 350 485 Hoi Kwong Street 4 С 3.6 15 1830 1815 125 0.068 0.096 0.096 Hoi Chak Street В 1,2 3.1 35 1845 1845 795 0.431 0.431 475 0.257 3 3.5 30 2005 2005 75 0.037 70 0.035 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 3,4 MIN GREEN + FLASH = 16 Fp Gp 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = 12 Notes: Flow: (pcu/hr) Group B,Fp B,Gp Group C,D,Fp C,D,Gp 0.431 0.431 0.361 0.361 795(475) 75(70) 85(155) 26 27 L (sec) 33 34 L (sec) C (sec) C (sec) 110 350(485) 115 115 110 100(155) 0.697 0.689 0.630 0.622 y pract. y pract. R.C. (%) 62% 60% R.C. (%) 75% 72% Stage / Phase Diagrams I/G= 2 I/G= I/G= 8 18 I/G= I/G= I/G= 2 I/G= 6 I/G= 10 18 I/G= I/G=

九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value Critical y Critical y (pcu/hr) (pcu/hr) (m) 2030 1835 0.049 0.218 Hoi Tai Street 3.5 100 2 0.218 D 3.6 20 400 Hoi Kwong Street 4 3.6 15 75% 1835 0.076 0.076 Hoi Chak Street 1,2 35 1845 460 0.249 3 3.5 30 2005 80 0.040 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 13 3,4 MIN GREEN + FLASH = 16 Fp Gp 3,4 MIN GREEN + FLASH = 10 18 1,2,4 MIN GREEN + FLASH = Notes: Flow: (pcu/hr) Group C,D,Fp C,D,Gp Group 0.294 0.294 у 460(0) 80(0) 100(0) 33 34 L (sec) L (sec) C (sec) 110 C (sec) 400(0) 110 105(0) 0.630 0.622 y pract. y pract. R.C. (%) 114% 111% R.C. (%) Stage / Phase Diagrams I/G= 2 I/G= 6 I/G= 10 18 I/G= #N/A #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Design Year: _____2025__ King's Road/ Finnie Street Checked By: WCY 2033 Design Designed By: NLY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.0 1915 1915 238 0.124 231 0.121 0.121 Ċ NB (with Tram) 3.3 2085 2085 260 0.125 251 0.120 С 3.5 2105 2105 262 0.124 253 0.120 Finnie Street D 20 100% / 0% 100% / 0% 1710 1710 360 0.211 0.211 625 0.365 0.365 3.5 22.5 1840 1840 120 0.065 0.111 0.218 0.218 0 214 King's Road Α 1.2 3.7 1985 1985 432 424 SB Α 1.2 3.3 2085 2085 453 0.217 446 0.214 0.053 1.2 2.6 1875 1875 100 0.053 100 King's Road Α SB (Tram) Pedestrian Crossing MIN GREEN + FLASH = 12 Ер 1,2 MIN GREEN + FLASH = Fp 11 MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group C,Gp,D Tram 0.335 0.428 0.579 0.486 у у 660(635) 100(100) 27 10 10 27 L (sec) L (sec) 120(205) C (sec) 885(870) 100(100) C (sec) 115 115 110 110 **↑** Tram 360(625) 0.818 0.689 0.679 y pract. 0.822 y pract. R.C. (%) 105% 92% R.C. (%) 41% 40% Stage / Phase Diagrams 3. FINNIE ST KING'S ROAD D I/G= 7 I/G= I/G= 5 I/G= I/G= 11 I/G= 7 I/G= 9 I/G= 2 1/G= I/G= Junction: 九月, 2025 King's Road/ Finnie Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Finnie Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Radius (m) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.0 1915 194 0.101 C 2085 0.102 NB (with Tram) 3.3 212 0.102 С 3.5 2105 214 0.102 Finnie Street D 20 100% / 0% 1710 420 0.246 0.246 22.5 1840 155 0.084 0.171 King's Road Α 1.2 3.7 1985 339 SB Α 1.2 3.3 2085 356 0.171 1.2 2.6 1875 0.056 King's Road Α 105 SB (Tram) Pedestrian Crossing 1,2 MIN GREEN + FLASH = 12 Ер MIN GREEN + FLASH = 11 Fp MIN GREEN + FLASH = 11 Notes: Flow: (pcu/hr) Group C,Gp,D Group Tram 0.416 0.347 у у 515() 105() 10 27 L (sec) L (sec) **▲**155(0) C (sec) C (sec) 695(0) 105(0) 110 110 **↑** Tram 420(0) 0.818 0.679 y pract. y pract. R.C. (%) 96% 96% R.C. (%) Stage / Phase Diagrams 3. FINNIE ST KING'S ROAD o D I/G= 7 I/G= 9 11 I/G= 2 I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A 1/G= I/G= Junction: 九月, 2025 King's Road/ Finnie Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ Description:_ 2033 Design Designed By: NLY Checked By: WCY Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road EB 5.500 2165 2165 230 0.106 200 0.092 King's Road Α 2.200 1975 1975 100 0.051 100 0.051 EB(Tram) King's Road 3.200 2075 2075 676 0.326 0.326 756 0.364 WB 3.300 2085 2085 679 0.326 759 0.364 0.364 free flow 3.300 1640 1640 255 0.155 290 0.177 2075 0.048 0.043 King's Road 3.200 2075 100 90 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group A,Bp 0.326 0.364 у у 100(100) 230(200) L (sec) 28 L (sec) 28 C (sec) 1355(1515) 100(90) C (sec) 115 110 Tram 0.681 0.671 255(290) y pract. y pract. R.C. (%) 109% R.C. (%) 84% Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= 4 I/G= 5 1/G= | I/G= I/G= 九月, 2025 King's Road/ Model Lan

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Model Lane Design Year: ____2025_ Description:_ 2033 Design Designed By: NLY Checked By: WCY Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road EB 5.500 2165 185 0.085 King's Road Α 2.200 1975 105 0.053 EB(Tram) King's Road 3.200 2075 648 0.312 0.312 WB 3.300 2085 652 0.313 free flow 3.300 1640 285 0.174 2075 0.048 King's Road 3.200 100 WB(Tram) Pedestrian Crossing 2 MIN GREEN + FLASH = 13 20 Notes: Flow: (pcu/hr) Group Group 0.312 у у 105() 185() L (sec) 28 L (sec) C (sec) C (sec) 1300() 100() 110 Tram 0.671 y pract. 285() y pract. R.C. (%) 115% R.C. (%) Stage / Phase Diagrams 3. I/G= 4 I/G= 5 20 I/G= I/G= I/G= I/G= #N/A #N/A I/G= #N/A #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Model Land

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ Checked By: WCY 2033 Design Designed By: Description: NLY Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Approach Left (pcu/hr) (pcu/hr) (m) 1 0.143 0.143 0.159 0.152 Java Road 3.000 12 1700 1700 243 270 0.159 15 94% 0.143 3.000 98% 1870 1880 268 286 ΕB Α * 3.000 18 5% 6% 2045 2045 293 0.143 312 0.153 Α 17 1890 1890 0.143 292 0.154 Man Hong Street В 2,3 3.200 2075 2075 423 0.204 371 0.179 SB В 2,3 3.400 2095 2095 427 0.204 374 0.179 В 2.3 3.800 12 1775 1775 425 0.239 340 0.192 0.211 С 2105 505 0.240 0.240 445 0.211 3 6.300 10 8% 8% 2105 Man Hong Street NB Pedestrian Crossing 2.3 MIN GREEN + FLASH = 12 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = Fp 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group A,Fp,C Opposing factor of 0.5 has been adopted 0.383 0.383 0.350 0.370 у у on Man Hong Street NB due to the presence of opposite movement on right 465(410) 285(310) 850(745) 16 30 16 30 L (sec) L (sec) turn C (sec) C (sec) 115 115 110 110 40(35) 0.769 0.655 425(340) y pract. 0.775 0.665 y pract. R.C. (%) 102% 74% R.C. (%) 120% 77% Stage / Phase Diagrams 3. I/G= 7 I/G= 8 I/G= 4 I/G= I/G=

1/G=

九月, 2025

I/G=

I/G= 4

I/G= 7

I/G= 8

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Java Road/ Man Hong Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Approach Left (pcu/hr) (pcu/hr) (m) 1 0.164 0.164 Java Road 3.000 12 1700 278 15 1910 3.000 77% 312 ΕB Α Α 3.000 18 26% 2010 329 0.164 0.164 17 1890 0.164 Man Hong Street В 2,3 3.200 2075 381 0.184 SB В 2,3 3.400 2095 384 0.183 В 2.3 3.800 12 1775 295 0.166 С 11% 0.198 3 6.300 10 2095 415 0.198 Man Hong Street NB Pedestrian Crossing 2,3 MIN GREEN + FLASH = 17 Ер MIN GREEN + FLASH = 6 10 16 MIN GREEN + FLASH = 13 Notes: Flow: (pcu/hr) Group A,Fp,C Group Opposing factor of 0.5 has been adopted on Man Hong Street NB due to the 395() 520() 0.348 0.362 у у presence of opposite movement on right 370() 315() 765() 16 30 L (sec) L (sec) turn C (sec) C (sec) 110 110 45() 295() 0.769 0.655 y pract. y pract. R.C. (%) 121% 81% R.C. (%) Stage / Phase Diagrams 3. I/G= 7 I/G= 8 I/G= 4 I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A 1/G= I/G= 九月, 2025

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ 2033 Design Designed By: NLY Description: Checked By: WCY Revised Saturation Pro. Turning (%) AM Peak PM Peak % Flow (pcu/hr) Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 1,2 3.300 1945 1945 74 0.038 65 0.033 2105 1.2 3.500 2105 81 0.038 70 0.033 EB Α King's Road Α 1,2 2.500 2005 2005 100 0.050 100 0.050 EB(Tram) King's Road В 3.500 8 81% 1705 1720 267 0.157 0.157 266 0.155 WB В 3.400 2095 2095 328 0.157 0.155 0.155 King's Road At 1.2 2 700 2025 2025 100 0.049 90 0.044 WB(Tram) D 3.300 1945 1945 327 0.168 0.168 316 0.162 Man Hong Street 4 2080 4 D 3.500 40 33% 21% 2090 349 0.168 0.163 SB 4 340 35 2020 2020 339 329 0.163 0.163 D 3.500 0.168 Pedestrian Crossing Ер 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 Gp 2.3.4 MIN GREEN + FLASH = 5 6 11 1.2.3 MIN GREEN + FLASH = Hр 14 Notes: Flow: (pcu/hr) Group Hp,D B,Fp,D Group Hp,D B,Fp,D 0.168 0.325 0.163 0.318 у У Tram 455(400) 560(585) Tram 24 35 24 35 L (sec) L (sec) C (sec) 115 115 C (sec) 110 110 → 100(100) 100(90) -----0.712 0.614 ----- 155(135) 380(390) y pract. 0.626 y pract. 0.704 215(200) R.C. (%) 324% 93% R.C. (%) 332% 93% Stage / Phase Diagrams 3. MAN HONG ST Gp 🔏 Gp\% Dp ⊶ At KING'S RD ⊶ At Fр Fр → Ep of → Ep of HEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G= I/G= 11 I/G= 9 1/G= I/G= 2 I/G=

九月, 2025

King's Road/ Healthy Street West/ Man

TRAFFIC SIGNALS CALCULATION Job No.: CHK50793710 **MVA HONG KONG LIMITED** Junction: King's Road/ Healthy Street West/ Man Hong Street Design Year: ____2025_ Designed By: NLY 2033 Design Description: Checked By: WCY Revised Saturation Pro. Turning (%) Noon Peak N/A % Flow (pcu/hr) Gradient Stage Width Flow Flow Approach Left Critical y (pcu/hr) (pcu/hr) (m) King's Road 1,2 3.300 1945 65 0.033 2105 1.2 3.500 70 0.033 EB Α King's Road Α 1,2 2.500 2005 105 0.052 EB(Tram) King's Road В 3.500 77% 1715 266 0.155 0.155 WB В 3.400 2095 324 0.155 King's Road At 1,2 2.700 2025 100 0.049 WB(Tram) D 4 3.300 1945 347 0.178 Man Hong Street 0.178 4 D 4 3.500 40 16% 2090 373 0.178 SB 35 2020 360 D 3.500 0.178 Pedestrian Crossing Еp 3.4 MIN GREEN + FLASH = 13 Fp 2.3 MIN GREEN + FLASH = 5 10 15 2,3,4 MIN GREEN + FLASH = Gp 5 6 11 1.2.3 MIN GREEN + FLASH = ДH 14 Notes: Flow: (pcu/hr) Group Hp,D B,Fp,D Group 0.178 0.334 У у 420() 660() Tram 24 35 L (sec) L (sec) Tram C (sec) 110 110 C (sec) → 105() 100() ----**>** 135() 385() y pract. 0.704 0.614 y pract. 205() R.C. (%) 294% 84% R.C. (%) Stage / Phase Diagrams 3. MAN HONG ST Gp\% Dp ⊶ At KING'S RD ⊶ At Fp Fр → Ep of → **E**p of JEALTHY ST I/G= 11 I/G= 9 15 I/G= I/G= 2 I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= 九月, 2025 King's Road/ Healthy Street West/ Man

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 0.102 0.102 Tsat Tsz Mui Road 3.800 27% 34% 1915 1900 166 0.087 0.087 193 0.102 3.800 2135 2135 184 0.086 217 WB Healthy Street West 🕹 2.500 1865 1865 256 0.137 0.139 SB 3.300 13 61% 61% 1950 1950 267 0.137 271 0.139 0.139 3.600 10 1840 1840 252 0.137 255 0.139 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = 9 Ер 3 5 14 Fρ MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group A,C,Fp A,C,Ep 415(420) 0.224 0.224 0.241 0.241 у у 360(365) 28 29 L (sec) 28 29 L (sec) C (sec) C (sec) 305(345) 115 115 110 110 **√** 45(65) 0.681 0.673 0.671 0.663 y pract. y pract. R.C. (%) 204% 201% R.C. (%) 179% 175% Stage / Phase Diagrams 2. 3. Сţ Dp H TSAT TSZ MUI RD HEALTHY ST I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= 4 I/G= 6 I/G= 7 14 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Healthy Street West Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Stage Width Flow Flow Approach Left y Value (pcu/hr) (pcu/hr) (m) Tsat Tsz Mui Road 3.800 35% 1895 202 0.107 2135 0.107 3.800 228 0.107 WB Healthy Street West 🕹 2.500 283 0.152 0.152 SB 3.300 13 37% 2000 303 0.152 3.600 10 1840 279 0.152 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 14 Dp 2,3 MIN GREEN + FLASH = 8 13 MIN GREEN + FLASH = Ep 3 5 14 Fp MIN GREEN + FLASH = 13 3 Notes: Flow: (pcu/hr) Group A,C,Fp A,C,Ep Group 0.259 0.259 у у 475() 28 29 L (sec) L (sec) 360() C (sec) 110 110 C (sec) √ 70() 0.671 0.663 y pract. y pract. R.C. (%) 160% 156% R.C. (%) Stage / Phase Diagrams 2. 3, Сţ Dp I TSAT TSZ MUI RD HEALTHY ST P I/G= 4 I/G= 6 I/G= 7 14 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A 1/G= I/G= 九月, 2025 Tsat Tsz Mui Road/ Healthy Street Wes

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu West Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Left y Value Critical y (pcu/hr) (pcu/hr) (m) Tin Chiu Street 3.900 2005 2005 450 0.224 0.224 300 0.150 0.150 NB rsat Tsz Mui Road ₽ В 4.100 6 13 45% / 55% 47% / 53% 1725 1720 325 0.188 0.188 350 0.203 4.100 13 1940 365 0.188 0.204 0.204 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 10 19 Dρ 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group Ср,В A,B,Dp 0.188 0.413 0.204 0.353 у у 450(300) 545(580) 27 32 L (sec) 27 32 L (sec) C (sec) C (sec) 115 115 110 110 0.689 0.650 0.679 0.638 y pract. y pract. 145(165) R.C. (%) 266% 57% R.C. (%) 234% 81% Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= 4 I/G= 6 I/G= 8 | I/G= I/G= 九月, 2025 J10 Tsat Tsz Mui Road/ Tin Chiu West

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Tsat Tsz Mui Road/ Tin Chiu West Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Width Flow Flow Left y Value (pcu/hr) (pcu/hr) (m) Tin Chiu Street 3.900 2005 330 0.165 0.165 NB rsat Tsz Mui Road ₽ В 4.100 6 13 61% / 39% 1690 345 0.204 0.204 4.100 13 0.204 Pedestrian Crossing 1,3 MIN GREEN + FLASH = 19 Dp 3 MIN GREEN + FLASH = 16 Notes: Flow: (pcu/hr) Group Ср,В A,B,Dp Group 0.204 0.369 у у 330() 530() 27 L (sec) 32 L (sec) C (sec) C (sec) 110 110 0.679 0.638 y pract. y pract. 210() R.C. (%) 233% 73% R.C. (%) Stage / Phase Diagrams 3. В TIN CHIU ST Ср I/G= 4 I/G= 6 I/G= 8 16 I/G= I/G= I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A #N/A I/G= I/G= J10 九月, 2025 Tsat Tsz Mui Road/ Tin Chiu West

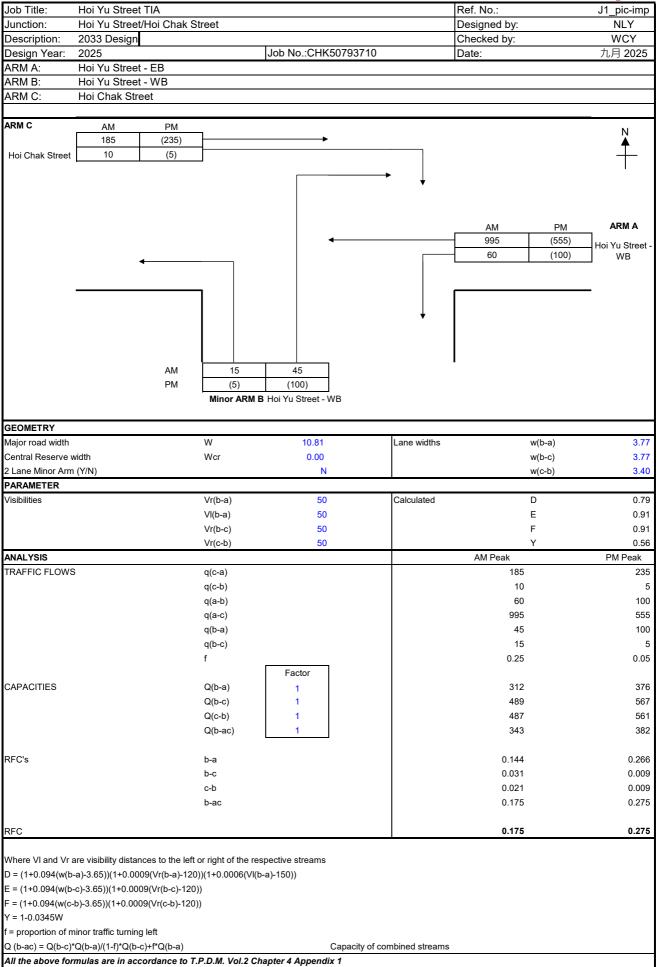
TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) King's Road 3.200 14% 12% 1900 1905 250 0.132 255 0.134 ΕB King's Road 2.700 2025 2025 100 0.049 0.049 King's Road В 3.300 1945 1945 264 0.136 250 0.129 0.129 WB В 3.500 2105 2105 285 0.135 270 0.128 0.136 В 3.500 2105 2105 286 0.136 270 0.128 King's Road 0.050 0.045 Dt 1 2.400 1995 1995 100 90 WB(Tram) Tin Chiu Street С 3.300 1730 1730 345 0.199 215 0.124 2 12 NB С 3.500 12 2010 1990 205 0.102 310 0.156 3.500 1870 1870 445 0.238 0.238 355 0.190 0.190 Pedestrian Crossing MIN GREEN + FLASH = 12 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group B,C A,C 35(30) 0.370 0.374 0.318 0.324 у у 215(225) 835(790) 10 11 11 10 L (sec) L (sec) Tram 100(100) 205(310) C (sec) C (sec) 115 115 110 110 0.822 0.814 0.810 0.818 345(215) → 445(355) y pract. y pract. R.C. (%) 122% 118% R.C. (%) 154% 153% Stage / Phase Diagrams I/G= 6 I/G= 7 I/G= I/G= I/G= I/G= 7 I/G= 5 I/G= 1/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: King's Road/ Tin Chiu Street Design Year: ____2025_ 2033 Design Designed By: NLY Checked By: WCY Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Stage Width Flow Flow Approach Left y Value Critical y (pcu/hr) (pcu/hr) (m) 13% King's Road 3.200 1905 240 0.126 ΕB King's Road 2.700 2025 105 0.052 King's Road В 3.300 1945 254 0.131 WB В 3.500 2105 276 0.131 0.131 В 3.500 2105 275 0.131 King's Road 0.050 Dt 2.400 1995 100 WB(Tram) Tin Chiu Street С 3.300 1730 255 0.147 2 12 NB С 3.500 12 51% 1980 265 0.134 3.500 1870 340 0.182 0.182 Pedestrian Crossing MIN GREEN + FLASH = 18 MIN GREEN + FLASH = 14 Notes: Flow: (pcu/hr) Group Group 30() 100() 0.308 0.313 у у 210() 805() 10 11 L (sec) L (sec) C (sec) C (sec) 110 105() 265() 110 0.818 0.810 255() 340() y pract. y pract. R.C. (%) 166% 159% R.C. (%) Stage / Phase Diagrams 3. I/G= 6 I/G= 7 I/G= I/G= I/G= #N/A #N/A I/G= #N/A I/G= #N/A I/G= | I/G= I/G= 九月, 2025 King's Road/ Tin Chiu Street

Calculation Sheets - 2033 Design Year (Junction Improvement)

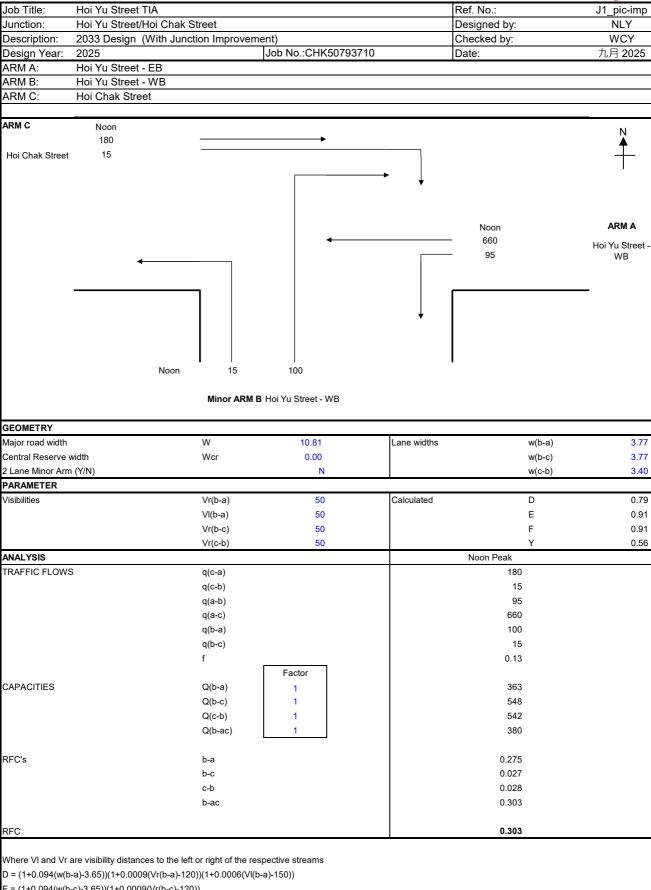
Simplified picority Junction Capacity Calculation





Simplified picority Junction Capacity Calculation





E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))

F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))

Y = 1-0.0345W

f = proportion of minor traffic turning left

Q (b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a) Capacity of combined streams

All the above formulas are in accordance to T.P.D.M. Vol.2 Chapter 4 Appendix 1

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: _ CHK50793710 Java Road/ Hoi Yu Street Design Year: ___2025 2033 Design (With Junction Improvement) Designed By: NLY Checked By: WCY Description: _ Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Gradient Phase Stage Width Flow (pcu/hr) Flow (pcu/hr) Left Approach ΑM AM y Value Critical y y Value Critical y (m) 1740 1740 0.521 0.521 Java Road 3.00 15.0 906 655 0.376 29% 1020 518 0.521 393 0.385 Δ Α 2.80 20.0 995 Α 3.10 825 825 429 0.520 319 0.387 0.387 580 580 302 0.521 223 0.384 Hoi Yu Street $\sqrt{}$ В 2 3.600 1715 1715 0.134 0.134 0.195 0.195 Pedestrian Crossing MIN GREEN + FLASH = 20 Ср Notes: Flow: (pcu/hr) **₹**N Group A,Cp A,B Group A,Cp A,B 1. Site factor 0.4 and 0.3 are adopted for 0.582 0.521 0.655 0.387 У у Java Road's third and fourth lane respectively due to downstream blocking 230(335) L (sec) 28 10 L (sec) 28 10 and illegal parking along kerbside 1055(655) 1100(935) observed. C (sec) 115 115 C (sec) 110 110 2. Site factor 1.0 is adopted for Java Road 0.681 0.822 y pract. 0.671 0.818 y pract. second lane due to proposed road widening at Hoi Yu Street. R.C. (%) 31% 25% R.C. (%) 74% 41% Stage / Phase Diagrams 2. 1. 3. 4. 5. Ср I/G= 7 I/G= I/G= I/G= I/G= 5 I/G= I/G= I/G= 5 I/G= 7 I/G= Date: Junction: 九月, 2025 Java Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: _ CHK50793710 Java Road/ Hoi Yu Street Design Year: ___2025 2033 Design (With Junction Improvement) Designed By: NLY Checked By: WCY Description: _ Revised Saturation Flow (pcu/hr) Pro. Turning (%) Noon Peak N/A % Gradient Phase Stage Width Flow (pcu/hr) Flow (pcu/hr) Left Approach y Value Critical y y Value Critical y Noon Noon (m) 1740 1740 0.414 Java Road 3.00 15.0 720 8% 1010 1020 418 0.414 Δ Α 2.80 20.0 Α 3.10 825 825 342 0.415 0.415 580 580 240 0.414 Hoi Yu Street $\sqrt{}$ 2 3.600 1715 1715 0.163 0.163 Pedestrian Crossing MIN GREEN + FLASH = 20 Ср Notes: Flow: (pcu/hr) Group A,Cp A,B Group 1. Site factor 0.4 and 0.3 are adopted for 0.415 0.578 У у Java Road's third and fourth lane respectively due to downstream blocking 280(0) 755(0) 965(0) L (sec) 28 10 L (sec) and illegal parking along kerbside observed. C (sec) 110 110 C (sec) 2. Site factor 1.0 is adopted for Java Road 0.818 0.671 y pract. y pract. second lane due to proposed road widening at Hoi Yu Street. R.C. (%) 62% 42% R.C. (%) Stage / Phase Diagrams 1. 2. 3. 4. 5. Ср I/G= 7 I/G= I/G= I/G= I/G= 5 I/G= #N/A I/G= #N/A #N/A I/G= I/G= #N/A I/G= Date: Junction: 九月, 2025 Java Road/ Hoi Yu Street

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ 2033 Design (With Junction Improvement) Designed By: NLY Checked By: WCY Revised Saturation Flow (pcu/hr) Pro. Turning (%) AM Peak PM Peak % Stage Width Flow Flow Approach Left y Value Critical y y Value Critical y (pcu/hr) (pcu/hr) (m) 0.167 0.166 Hoi Tai Street 3.500 20 1830 1830 207 0.113 305 2 40 63% / 37% 54% / 46% D 3.600 25 2010 2015 228 0.113 335 Hoi Kwong Street 4 С 3.6 15 1830 1815 0.068 0.096 Hoi Chak Street В 1,2 3.1 35 1845 1845 795 0.431 0.431 475 0.257 0.257 3 3.5 30 2005 2005 75 0.037 0.037 70 0.035 0.035 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 10 18 MIN GREEN + FLASH = 10 Fp 4 8 18 Gp MIN GREEN + FLASH = 10 18 MIN GREEN + FLASH = Нр 18 Notes: Flow: (pcu/hr) Group B,A,Hp B,A,Fp Group B,A,Hp B,A,Fp 0.468 0.468 0.292 0.292 у у 795(475) 75(70) 350(485) 31 32 L (sec) 31 32 L (sec) C (sec) C (sec) 85(155) 115 115 110 110 25(20) 100(155) 0.646 0.657 0.650 0.638 y pract. y pract. R.C. (%) 40% 39% R.C. (%) 121% 118% Stage / Phase Diagrams 2. 3. , Gр Ep-> Έp Ēp-I/G= 4 I/G= I/G= 6 I/G= 6 18 I/G= I/G= 4 1/G= I/G= 6 I/G= 6 18 I/G= 九月, 2025 Hoi Tai Street/ Hoi Kwong Street/ Finnie

TRAFFIC SIGNALS CALCULATION **MVA HONG KONG LIMITED** Job No.: CHK50793710 Junction: Hoi Tai Street/ Hoi Kwong Street/ Finnie Street/ Hoi Chak Street Design Year: ____2025_ Designed By: NLY Checked By: WCY 2033 Design (With Junction Improvement) Description:_ Revised Saturation Flow (pcu/hr) Pro. Turning (%) % Stage Width Flow Approach Left Critical y (pcu/hr) (m) 0.130 0.130 Hoi Tai Street 3.500 20 1830 238 62% / 38% 2 40 2010 D 3.600 25 262 Hoi Kwong Street 4 С 3.6 15 1835 0.076 Hoi Chak Street В 1,2 35 1845 460 0.249 0.249 30 3 3.5 2005 80 0.040 0.040 Pedestrian Crossing Еp 2,3,4 MIN GREEN + FLASH = 18 Fp Gp MIN GREEN + FLASH = 10 18 4 8 MIN GREEN + FLASH = 10 18 MIN GREEN + FLASH = Нр 18 Notes: Flow: (pcu/hr) Group B,A,Hp B,A,Fp Group 0.289 0.289 у у 460() 80() 400() 31 32 L (sec) L (sec) C (sec) C (sec) 100() 110 110 105() 0.646 0.638 y pract. y pract. R.C. (%) 123% 121% R.C. (%) Stage / Phase Diagrams 2. 3. , Gр Έp Ēp-I/G= 4 I/G= I/G= 6 I/G= 6 18 I/G= #N/A #N/A #N/A I/G= #N/A I/G= #N/A I/G= #N/A I/G= #N/A #N/A I/G= 九月, 2025 Hoi Tai Street/ Hoi Kwong Street/ Finnie

Appendix B - Swept Path Analysis

