| | | Appendix | I |
|--------|----------|-------------|----|
| Sewera | ge Impac | t Assessmer | ۱t |

Section 12A Planning Application for Proposed Amendments to the Tung Chung Valley Outline Zoning Plan to Rezone "Residential (Group C)2" Zone to "Residential (Group B)" Zone in Support of Private Residential Development at Various Lots in D.D. 1 Tung Chung and Adjoining Government Land, Tung Chung, Lantau Island

Sewerage Impact Assessment

December 2024

Section 12A Planning Application for Proposed Amendments to the Tung Chung Valley Outline Zoning Plan to Rezone "Residential (Group C)2" Zone to "Residential (Group B)" Zone in Support of Private Residential Development at Various Lots in D.D. 1 Tung Chung and Adjoining Government Land, Tung Chung, Lantau Island

Sewerage Impact Assessment

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Sewerage Impact Assessment

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Sewerage Impact Assessment

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1. Introduction

1.1 Background

- 1.1.1 AECOM Asia Company Limited (AECOM) was commissioned by the project proponent to act as the engineering consultant to conduct a Sewerage Impact Assessment (SIA) for a private residential development in Tung Chung, Lantau Island.
- 1.1.2 The Application Site, with a total site area of 33,808m², is mostly covered by vegetation and with some open space areas and car parking areas. The Site is located at the west side of Chung Mun Road, within Ngau Au. The Application Site is also next to river outfall of Tung Chung Bay. The location of the Application Site is shown in **SIA/Figure 1**.
- 1.1.3 The Application Site is currently zoned "Residential (Group C)2" with a maximum plot ratio of 1 on the Approved Tung Chung Valley Outline Zoning Plan No. S/I-TCV/2. The Applicant now proposes to increase the domestic plot ratio to not more than 2.10 to optimise valuable land resources and infrastructural capacity.
- 1.1.4 This SIA report serves as a supporting document for rezoning the Site from "Residential (Group C)2" Zone to "Residential (Group B)" Zone.

1.2 Objective of this Submission

- 1.2.1 This report outlines the assessment results of the potential water supply impacts caused by the Application Site. The main objectives of this assessment include the followings:
 - (i) Review the existing and planned sewerage infrastructure near the Application Site;
 - (ii) Outline the methodology adopted in this assessment;
 - Identify any potential impact on the current or planned sewerage system due to the additional sewage discharge from the Application Site and the existing developments/ villages nearby;
 - (iv) Propose sewerage mitigation measures where appropriate to mitigate the potential sewerage impact.

AECOM 2024

1.3 Nomenclature

1.3.1 The following abbreviations and shortened expressions in **Table 1** are adopted in this report.

| ADWF | Average Dry Weather Flow |
|--------|--|
| AECOM | AECOM Asia Company Limited |
| CEDD | Civil Engineering and Development Department |
| CIFSUS | Commercial and Industrial Floor Space Utilization Survey (PlanD) |
| DSD | Drainage Services Department |
| EPD | Environmental Protection Department |
| GFA | Gross Floor Area |
| mPD | Metres above Principal Datum |
| PlanD | Planning Department |
| UFF | Unit Flow Factor |
| UDD | Unit Daily Demand |
| SIA | Sewage Impact Assessment |

Table 1 - Nomenclature

2. Development Proposal

2.1 The Proposed Development

- 2.1.1 The Application Site area is approximately 33,808m². The Application Site consists of 9 residential blocks, 1 covered private transport lay-by, 1 kindergarten and retail facilities. The residential blocks are ranging from 6 to 22 storeys above a 1 to 3-storey podium, providing about 1,783 units in total.
- 2.1.2 The anticipated population intake year of the Application Site is 2030.
- 2.1.3 The Master Layout Plan (MLP) of the Application Site is shown in **SIA/Figure 2**. The proposed development schedule is summarised in **Table 2** below.

| | Proposed Development | |
|--|---|--|
| Site Area | About 33,808m ² | |
| GFA - Domestic Portion - Non-Domestic Portion | About 78,292m ² About 70,997m ² About 7,295m ² | |
| Plot Ratio - Domestic Portion - Non-Domestic Portion | Not more than 2.32 Not more than 2.10 Not more than 0.22 | |
| Maximum Site Coverage | Not more than 33.3% | |
| Maximum Building Height (main roof level) - Area (a) - Area (b) - Area (c) | Not more than 50mPD Not more than 80mPD Not more than 100mPD | |
| No. of Storeys ⁽¹⁾ | 6 to 22 storeys above a 1 to 3 storey(s) podium | |
| Domestic Portion | | |
| Domestic GFA | About 70,997m ² | |
| Domestic Plot Ratio | Not more than 2.10 | |
| No. of Blocks | 9 | |
| No. of Units | About 1,783 | |
| Average Flat Size | About 39.8m ² | |
| Anticipated Population (2) | About 5,171 | |
| Private Open Space ⁽³⁾ | Not less than 5,171m ² | |
| Non-Domestic Portion – Commercial and Covered Private Transport Lay-by | | |
| Commercial GFA (4) | About 4,145m ² | |
| Covered Private Transport Lay-by GFA | About 3,150m ² | |
| Maximum Building Height | Not more than 19mPD | |
| Residents' Clubhouses (5) | | |
| Clubhouse GFA | About 3,000m ² | |
| No. of Storeys | 1 | |

Table 2 - Key Development Parameters

Sewerage Impact Assessment

Remarks:

- (1) Excluding basement floor(s) for car park and transfer plate; including above ground floors for commercial / covered private transport lay-by / ramp / E&M facilities / clubhouse / residential lobby / residential floors. The indicative typical floor-to-floor height is 3.25m which is subject to refinement at detailed design stage.
- (2) Adopting a person per flat ratio of 2.9 as per Tung Chung New Town under 2021 Population Census covering the Application Site
- (3) Not less than 1m² per person in accordance with Hong Kong Planning Standards and Guidelines (HKPSG) requirement
- (4) Commercial GFA refers to commercial uses ('Eating Place' and 'Shop and Services'), 'School' (kindergarten, nursery, language, computer, commercial and tutorial schools, art school, ballet and other types of schools providing interest / hobby related courses), 'Place of Entertainment' and 'Place of Recreation, Sports or Culture'. A kindergarten with a GFA of about 930m² is proposed.
- (5) Residents' clubhouse GFA is based on the maximum GFA concession for clubhouse according to Buildings Department's Practice Note APP-104 and shall be disregarded from the total GFA calculation

3. Assessment Methodology

3.1 Sewerage Impact Assessment Methodology

3.1.1 Capacity analysis of the sewage pipes was carried out to assess the adequacy of the proposed sewerage system. The design assumption and basis are shown in **Table 3**.

| Design Standard | DSD Sewerage Design Manual, Part 1 & 2 |
|--|--|
| Flow Formula | Colebrook-White Formula |
| Roughness | Proposed Gravity Sewer Ks = 0.3 mm (HDPE) |
| Unit Flow Factor | EPD Guideline for Estimating Sewage Flows for Sewage Infrastructure Planning (GESF): Table T-2 Proposed Development: 0.27 m³/day/head ("Domestic Private Housing (R2)") 0.28 m³/day/head ("Commercial Employee" plus "Community, Social & Personal Services" J11) 1.58 m³/day/head ("Commercial Employee" plus "Restaurant" J10) 0.28 m³/day/head ("Commercial Employee" plus "Wholesale & Retail" J4) 0.04 m³/day/head (Student) |
| Catchment Inflow Factor | EPD Guideline for Estimating Sewage Flows for Sewage Infrastructure Planning: Table T-4 1.0 (Tung Chung) |
| Peaking Factors | EPD Guideline for Estimating Sewage Flows for Sewage Infrastructure Planning: Table T-5 |
| Economic Activity and Planned Usage Type | PlanD Commercial and Industrial Floor Space Utilization Survey (CIFSUS) Table 8 Proposed Development: 5.1 employees per 100 m² ("Restaurant") 3.5 employees per 100 m² ("Retail Trade") |

Table 3 - Design Assumption and Basis

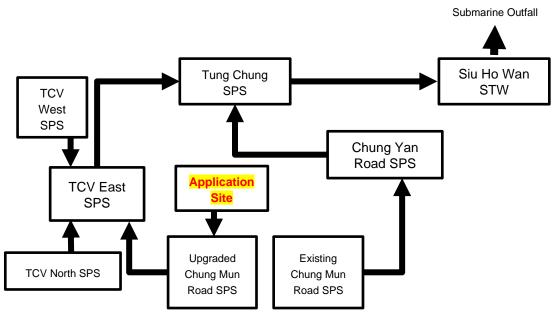
4. Review on Existing and Planned Sewerage System

4.1 Existing Sewerage System

4.1.1 According to sewerage record in GeoInfo. Map, there is no public sewerage system within the subject site. The nearest sewerage facilities are the public sewerage system along Chung Mun Road, east of the application site, discharge to existing Chung Mun Road Sewage Pumping Station (CMRSPS). Existing public sewer is available along Chung Mun Road, via a series of DN300 & DN375 sewer towards CMRSPS. The sewage is finally conveyed to Siu Ho Wan Sewage Treatment Work (SHWSTW) for treatment through two existing sewage pumping stations i.e. Chung Yan Road Sewage Pumping Station (CYRSPS) and Tung Chung Sewage Pumping Station (TCSPS).

4.2 Planned Sewerage System

- 4.2.1 CEDD will construct a planned sewerage network connecting the "Upgraded CMRSPS" (CEDD assigned name of a new sewage pumping station to be constructed next to the original CMRSPS) under Yu Tung Road and Chung Mun Road. Sewerage manhole (FMH-H05) will be constructed at the junction of Yu Tung Road and Chung Mun Road to convey sewage to the Upgraded CMRSPS via proposed 560 mm outer diameter (OD560) sewer (refer to SIA/Figure 4).
- 4.2.2 CEDD planned sewerage network will construct a plug-end sewer (OD355) at the boundary of Area 60, reserved for sewage discharge from future development.
- 4.2.3 According to latest Overall Schematic of Proposed Sewerage Flow for Tung Chung West (TCW) Development from CEDD, the Upgraded CMRSPS would serve Areas 33, 36A, 38A, 38B, 38C, Sha Tsui Tau, MTR TCW, and the Application Site. As advised by CEDD, the design peak flow of the Upgraded CMRSPS is 8,300 m³/day (0.0961 m³/s). The Upgraded CMRSPS will discharge to TCV East SPS via new twin DN250 rising mains laid along Chung Mun Road. TCV East SPS will deliver flow with discharge to the TCSPS via new twin DN450 rising mains laid along Chung Mun Road and Yu Tung Road. Refer to **Design Flow Diagram** below.



Design Flow Diagram

4.2.4 The Overall Schematic of Proposed Sewerage Flow for the proposed public sewerage infrastructure adopted in this SIA was obtained from CEDD and is enclosed in **Annex 1**.

4.3 Application Site Sewage Estimation

4.3.1 The estimated Average Dry Weather Flow (ADWF) from the Application Site is approximately 1,712 m³/day. Detailed sewage flow estimation can be referred to **Annex 2** and summarized in **Table 4**.

| Flow Type | Average Dry Daily Flow (ADWF) (m ³ /day) |
|-------------------------------|---|
| Domestic | 1396 |
| Other Non-domestic Provisions | 316 |
| Total ADWF | 1,712 |
| | |
| Peaking Factor | 4 |
| Peak Flow (m³/s) | 0.079 |

Table 4 - Summary of Sewage Flow

4.3.2 The adopted peaking factor excluding stormwater allowance is 4, in accordance with GESF, the estimated peak sewage flow is 0.079 m³/s.

5. Proposed Sewerage System

- 5.1.1 The existing Application Site is currently undeveloped with no existing sewerage system. The nearest sewage manhole available for connection is FMH-H05 to be constructed by CEDD.
- 5.1.2 The sewage generated from the Application Site would be discharged via internal sewerage networks leading to the terminal manhole FTMH-1 proposed at the eastern site boundary adjacent to Chung Mun Road. A short OD355 sewer will be required to complete the connection from FTMH-1 and the public plug-end OD355 sewer at Yu Tung Road. The sewage generated from the Application Site would be discharged to the Upgraded CMRSPS, then to TCV-East SPS, existing TCSPS and ultimately to SHWSTW. The proposed sewerage connection is indicated in SIA/Figure 4.
- 5.1.3 The capacity of the planned PE100 gravity sewer with size OD560 is able to cater for the sewage discharge from the Application Site, as well as the proposed Tung Chung West development areas. The estimated sewage flow discharge from the proposed Tung Chung West development areas refers to **Annex 3**. The detailed calculation refers to **Annex 5**.
- 5.1.4 The planned design peak flow of the Upgraded CMRSPS according to the latest information provided by CEDD is 0.0961 m³/s, which is insufficient to cater for the combined peak flow of 0.102 m³/s from the Application Site and other planned catchments. Therefore, the pumping rate of the Upgraded CMRSPS is proposed to be upgraded from 0.0961 m³/s to at least 0.108 m³/s, providing >5% contingency. The utilisation of the Upgraded CMRSPS is calculated in **Annex 6** and summarised in **Table 5**.

| | Sewage Flow |
|---|-------------|
| Sewage generated from the Application Site in ADWF (m³/day) | 1,712 |
| Sewage generated from other planned catchments in ADWF ((m³/day) (breakdown in Annex 3) | 1,829 |
| Total ADWF (m³/day) | 3,541 |
| Peaking Factor | 2.5 |
| Peak Flow (m³/s) | 0.102 |
| Planned design peak flow of the Upgraded Chung Mun Road Sewage Pumping Station (m³/s) | 0.0961 |
| Proposed upgrading of pump rate of the Upgraded Chung Mun Road Sewage Pumping Station (m³/s) | 0.108 |
| Utilization after Upgrading of Pump Rate | 94.9% |

Table 5 - Utilization of Upgraded CMRSPS

- 5.1.5 The downstream public gravity sewer connecting to TCV-East SPS has sufficient capacity to cater for the upgraded pumps. Hydraulic checking refers to **Annex 5**. The sewage flow estimation and catchment area plan can be referred to **Annex 4** and **Annex 7** respectively. The pumping rate of TCV-West SPS and TCV-North SPS adopted in the hydraulic analysis is 0.0231 m³/s and 0.0255 m³/s respectively, which are provided by CEDD.
- 5.1.6 The planned sewage discharge to TCV-East SPS in accordance with the latest Overall Schematic of Proposed Sewerage Flow for TCW Development from CEDD is summarised in **Table 6.** In accordance with the latest information provided by CEDD, the planned design peak flow of TCV-East SPS will be 33,264 m³/day (0.385 m³/s) with a peaking factor of 2, equals to ADWF of 33,264/2 = 16,632 m³/day. With the addition of sewage generated from the Application Site, the utilization of TCV-East SPS will become 67.6%.

| Development | ADWF (m³/day) |
|---------------------------------|---------------|
| Area 42 | 4,072 |
| Area 46 | 1,047 |
| Area 61A | 299 |
| Area 66A | 259 |
| Area 67 | 64 |
| Area 68A/B | 154 |
| Area 71A | 756 |
| Area 71B | 115 |
| Area 72 | 173 |
| Area 73 | 36 |
| Area 74 | 27 |
| Area 75 | 209 |
| Area 76 | 197 |
| Area 77 | 186 |
| River Park | 100 |
| From Upgraded CMRSPS | 3,541 |
| Total ADWF | 11,236 |
| Design Capacity of TCV-East SPS | 16,632 |
| Utilization | 67.6% |
| | - |

Table 6 - Utilization of TCV-East SPS

Note:

Latest design capacity and sewage flow information from Civil Engineering and Development Department – Overall Schematic of Proposed Sewerage Flow for TCW Development.

- 5.1.7 For any unforeseeable reason or difficulties, in which further upgrading of the Upgraded CMRSPS is not recommended, the development also proposed a fallback option to mitigate the sewerage issue, to be discussed in **Annex 8**.
- 5.1.8 The internal sewerage network and all relevant fittings will be determined in detailed design stage.

6. Maintenance Responsibility

- 6.1.1 The development is responsible for the construction of terminal manhole FTMH-1 and the downstream connection to the plug-end of public sewer near the Application Site Boundary. The development is also responsible for the construction of the internal sewerage infrastructure within the private development portion.
- 6.1.2 The development will bear the infrastructure cost to further upgrade the pump rate of the Upgraded CMRSPS from 0.0961 m³/s to at least 0.108 m³/s to cater for the additional sewage discharge from the Application Site. While the Upgraded CMRSPS and other public sewerage system will remain to be operated and maintained by the government.
- 6.1.3 Future development will be responsible for the maintenance of the terminal manhole FTMH-1 and its upstream sewerage infrastructure within the Application Site.
- 6.1.4 The government will take over the maintenance of the sewers and sewerage infrastructure downstream of terminal manhole FTMH-1, including the sewers between manhole FMH-H05 to FMH-I01a which are partially located within the private development portion. For the portions of sewer within the private development, a drainage reserve with a width of approximately 6.2m would be provided.

7. Conclusion

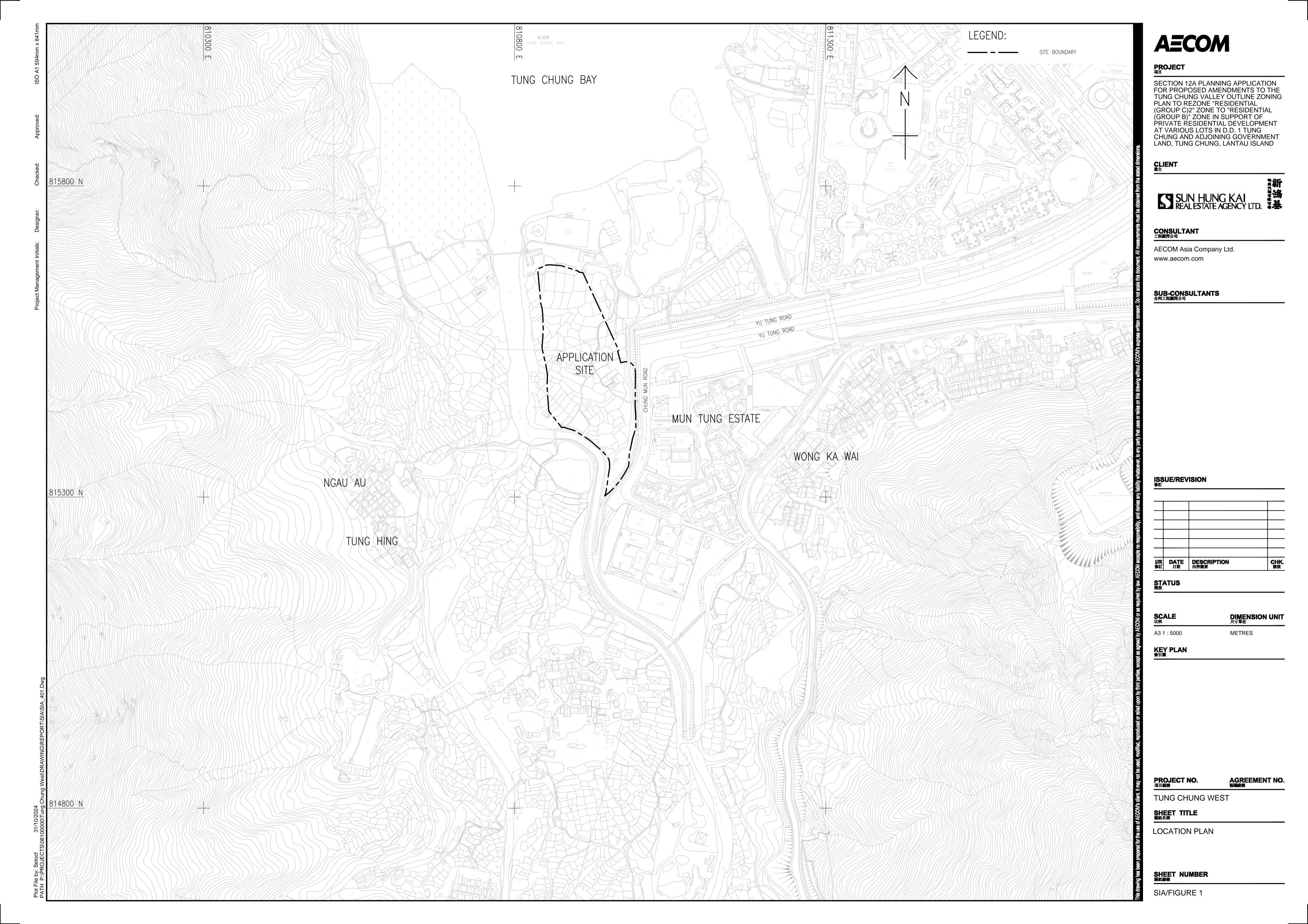
- 7.1.1 The Application Site is designated for residential use under the current OZP with a plot ratio of 1. The Applicant proposes to have a domestic plot ratio of 2.1 for the site by rezoning the site from "R(C)2" to "R(B)". The SIA has been carried out to assess the impact on the existing, planned and proposed sewerage system due to the proposed development.
- 7.1.2 The estimated total Average Dry Weather Flow (ADWF) from the Application Site is approximately 1,712 m³/day.
- 7.1.3 Planned OD560 PE100 gravity sewers will be constructed by others from planned sewerage manhole FMH-H05 to the Upgraded CMRSPS according to CEDD. The sewage from the Upgraded CMRSPS will be conveyed to the TCV-E SPS and ultimately to SHWSTW for centralized sewage treatment.
- 7.1.4 The planned OD560 PE100 gravity sewers from FMH-H05 to the Upgraded CMRSPS is designed to cater for the sewage flow from the Application Site, Sha Tsui Tau, MTR TCW and Areas 33, 36A, 38A, 38B and 38C.
- 7.1.5 The current design of the Upgraded CMRSPS could have underestimated the development potential of Tung Chung West. It is worth to review the SPS design capacity and select pump set with at least 0.108 m³/s to cope with the development need.
- 7.1.6 After successful upgrade of the Upgraded CMRSPS and completion of TCV-East SPS, the public sewerage infrastructure would have adequate capacity to cater for the proposed discharge. No adverse sewerage impact is anticipated.

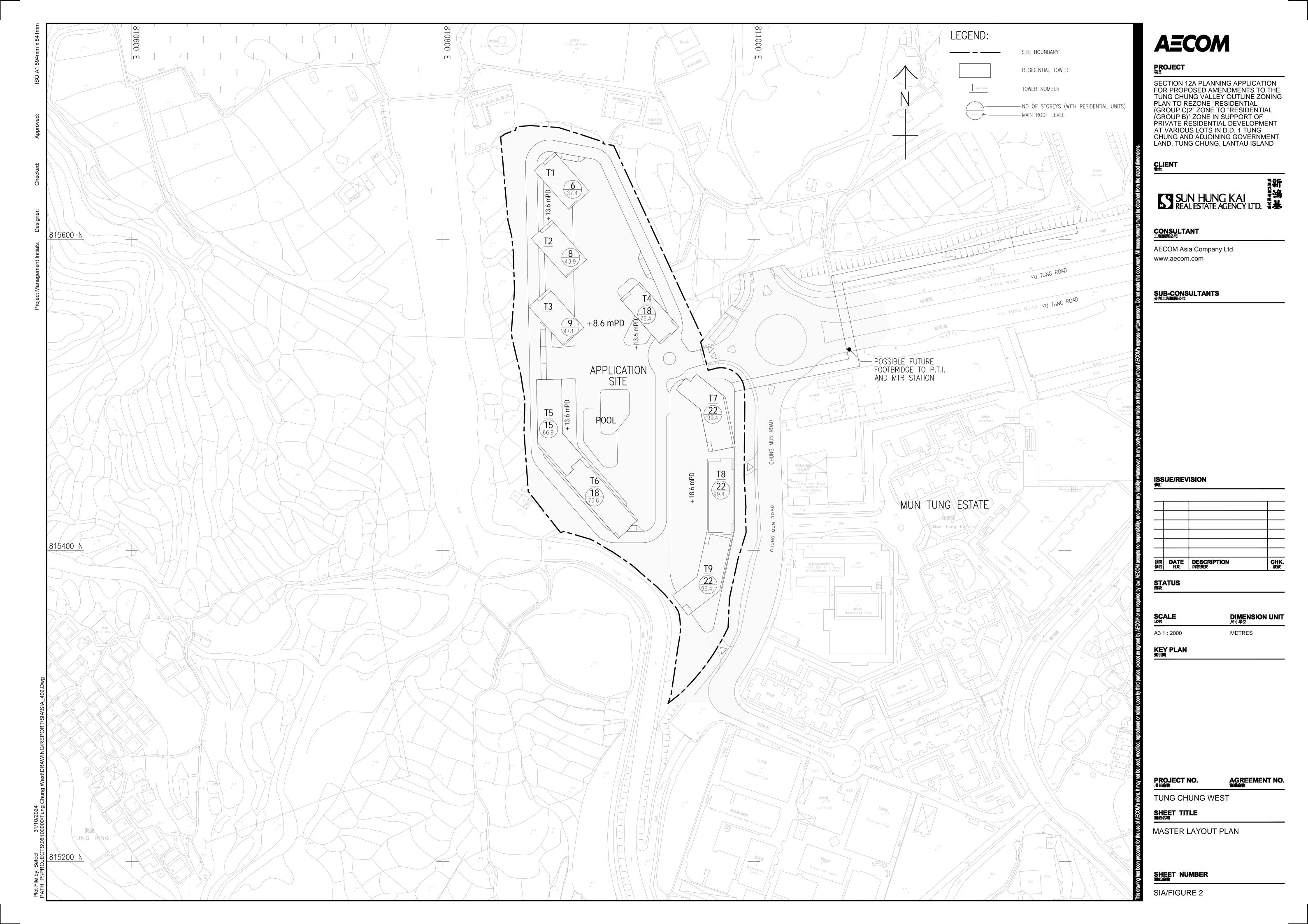
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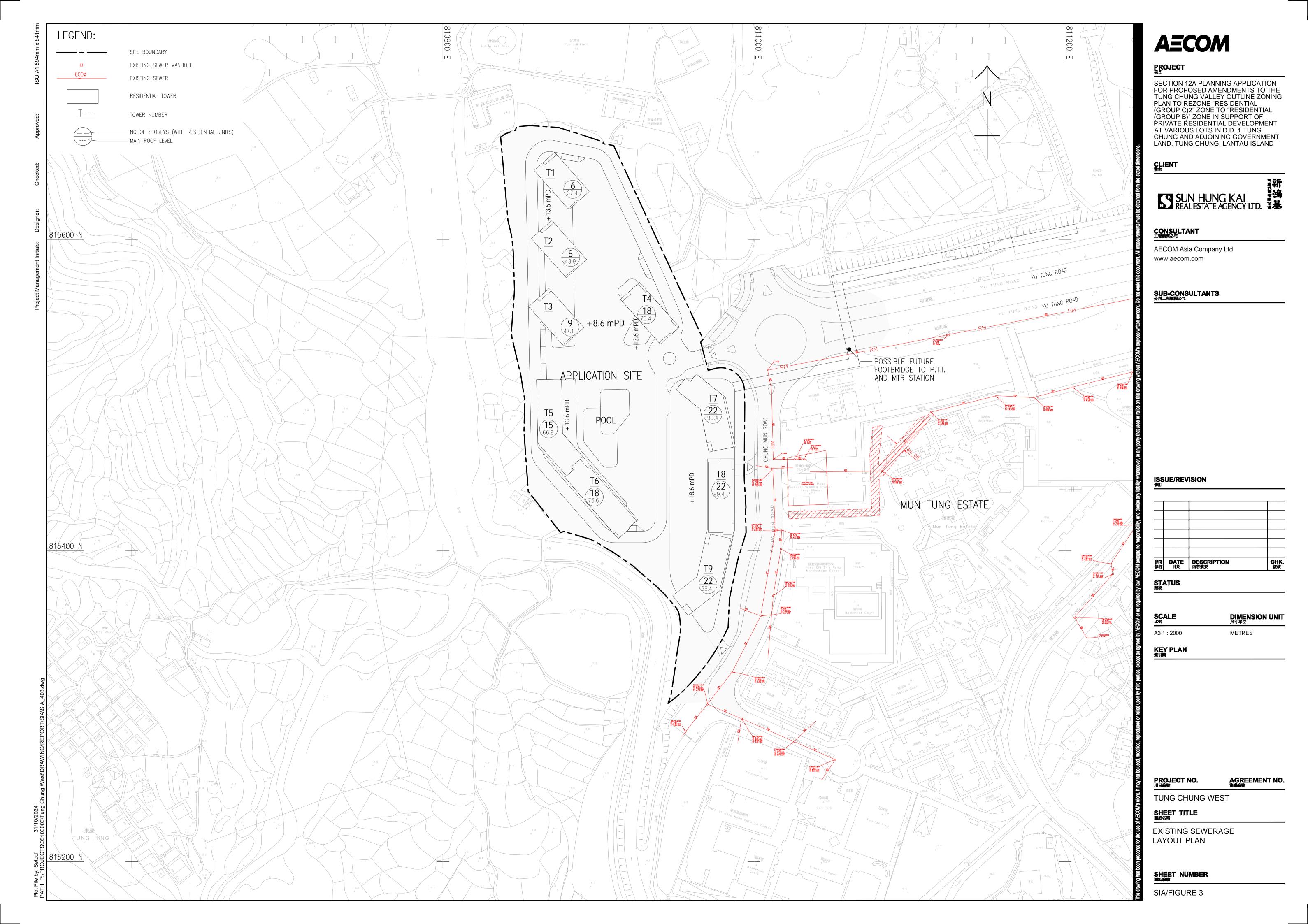
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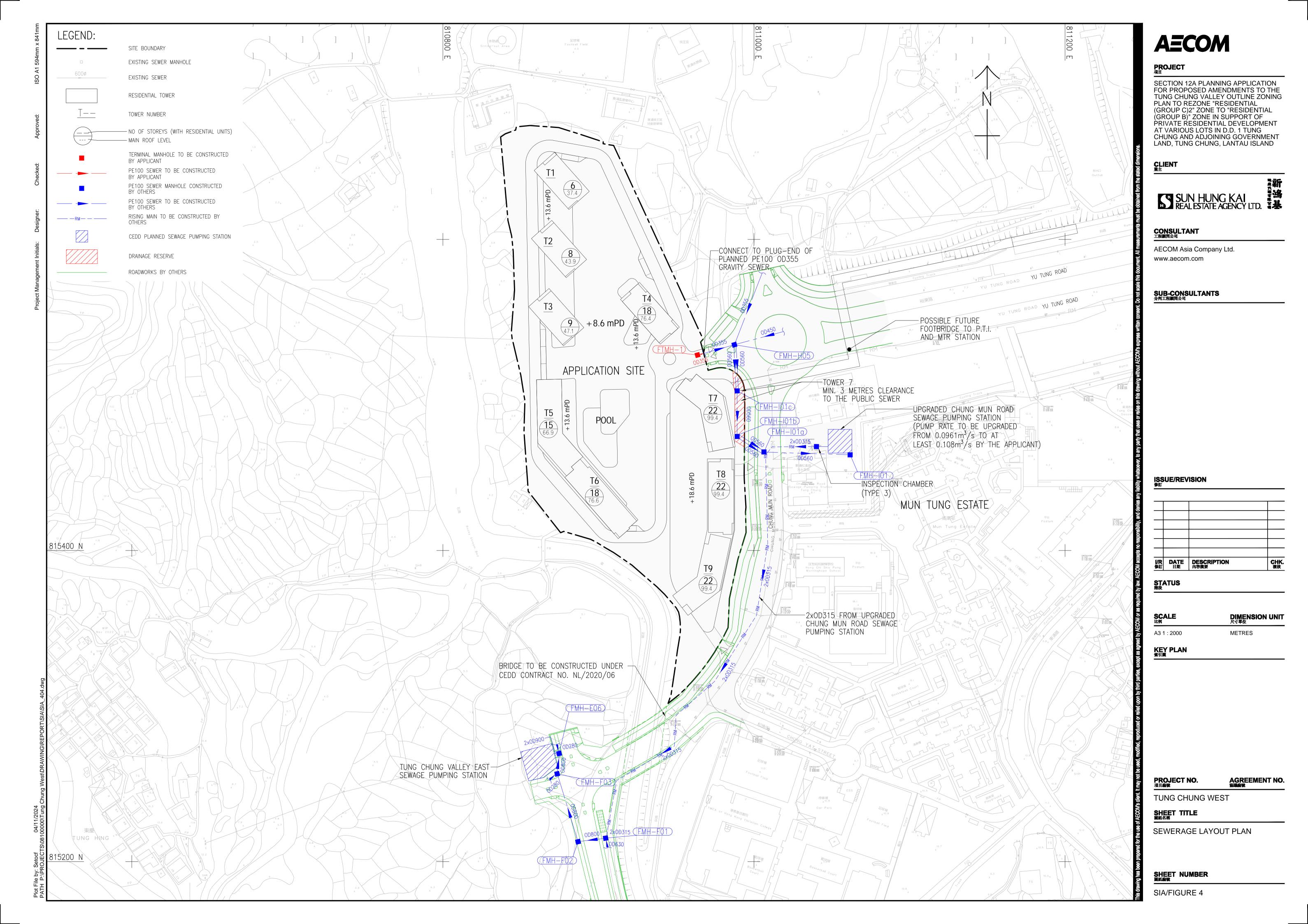
Sewerage Impact Assessment

Figures



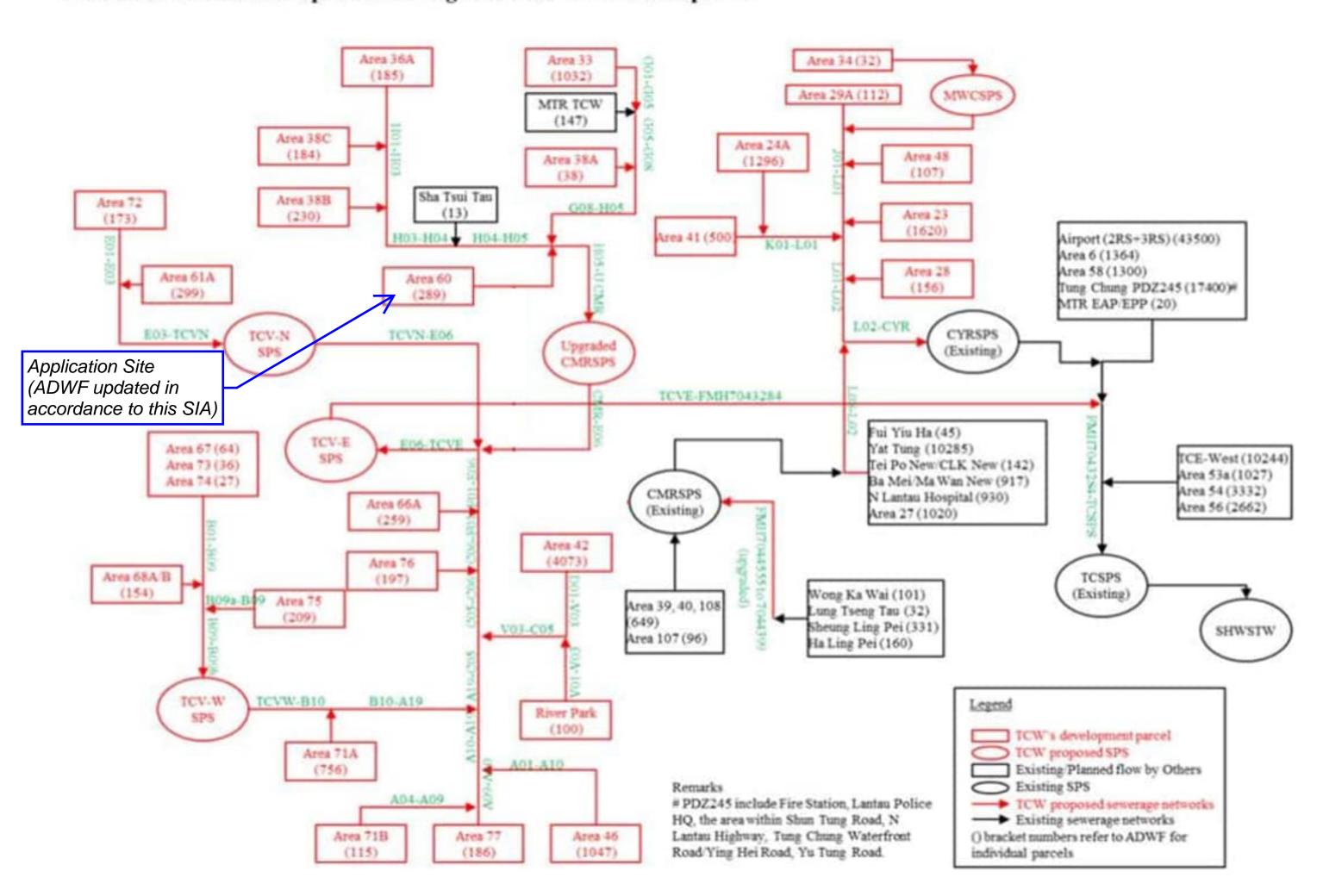






Overall Schematic of Proposed Sewerage Flow obtained from CEDD

Overall Schematic of Proposed Sewerage Flow for TCW Development



Estimated Sewage Flow Discharge from Application Site

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|---|---|
| D.D. 1 TC and Adjoining Government Land, Tung Chung, Lantau Island | - |

| Sowoan | Generation | Ectimation |
|--------|------------|------------|
| sewade | Generation | Estimation |

11/21/2024

| Development Type | GFA (m²) | Avg. Unit Size ⁽¹⁾ (m²) | No. of Units | Person/Unit ⁽³⁾ | Person/m ² | Population | Flow Type Description | Flow Type Abbreviation | UFF (m³/head/day) | Catchment Inflow Factor | ADWF (m³/day) |
|--|-----------|---------------------------------------|-----------------|----------------------------|-----------------------|------------|--|---------------------------|----------------------|----------------------------|------------------|
| Domestic | 70,997.00 | 40 | 1,783 | 2.9 | 0.073 | 5,171 | Domestic - Private R2 | R2 | 0.27 | 1.00 | 1,396.20 |
| Commercial ⁽⁷⁾ | 3,215 | | | | | | | | | | |
| Retail | 1,608 | | | | 0.035 | 57 | Commercial - Wholesale & Retail | J4 | 0.28 | 1.00 | 16.00 |
| Restaurant | 1,608 | | | | 0.051 | 82 | Commercial - Restaurants & Hotels | J10 | 1.58 | 1.00 | 129.60 |
| Kindergarten | 930 | | | | | | | | | | |
| Student | | | | | | 120 | Student | school student | 0.04 | 1.00 | 4.80 |
| Teacher | | | | | | 12 | Commercial - Community, Social & Personal Services | J11 | 0.28 | 1.00 | 3.40 |
| Clubhouse ⁽²⁾ | 3,000 | | | | | | | | | | |
| Retail | 1,500 | | | | 0.035 | 53 | Commercial - Community, Social & Personal Services | J11 | 0.28 | 1.00 | 14.80 |
| Restaurant | 1,500 | | | | 0.051 | 77 | Commercial - Restaurants & Hotels | J10 | 1.58 | 1.00 | 121.70 |
| Backwash from swimming pool ⁽⁴⁾ | | | | | | | | | | | 25.00 |
| | | | | т | otal Population | 5,572 | | Te | 1,711.50 | | |
| | | | | | | | Total ADWF without Bac | ckwash from swim | ming pool (m³/day) |) | 1,686.50 |

Note 1: Assuming the average unit size to be:

Note 2:

40.00 m

Assuming 50% clubhouse GFA be Retail and 50% be Restaurants. Person/m² information extracted from CIFSUS (PlanD, 2005)

Note 3: Based on a person per flat ratio of 2.9 by making reference to Tung Chung New Town under 2021 Population Census.

Note 4: For calculation of instant peak flow from swimming pool backwash, please refer to the attached swimming pool calculation sheet in Annex 1 - Proposed Swimming Pool - Backwash Calculation.

Note 5: Assume 30 student of 4 classes for Kindergarten

Note 6: Assuming a 1-to-11 Staff-to-Student Ratio (excluding principal), according to Education Bureau Circular No. 12/2020. The total no. of employee for kindergarten includes 1 no. of principal and 11 teachers.

Note 7: Assuming 50% Commercial GFA be Retail and 50% be Restaurants

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| Proposed Swi | imming Pool · | - Backwash (| Calculation |
|--------------|---------------|--------------|-------------|
|--------------|---------------|--------------|-------------|

Date

8/26/2024

Backwash:

Plan Area of Pool = $989 m^2$

Assuming Depth of Pool = 2.0 m

Volume of Pool = $1,979 m^3$

Turnover Rate = 6 hr Outdoor Swimming Pool

Assuming Surface Loading Rate of Filter = $20 \text{ m}^3/\text{m}^2/\text{hr}$

Filter Area Required = 1979/6/20

= 16.50 m^2

Assumed Backwash Duration = 3.00 min

Assumed Backwash Flow Rate = $30.00 \, \text{m}^3/\text{m}^2/\text{hr}$

Backwash Volume = 25.00 n

Note:

1 no. of filter is assumed

Reference:

Cap 132CA Swimming Pools Regulation

General Specifications for Swimming Pool Water Treatment Installation (ArchSD, 2012)

Estimated Sewage Flow Discharge from other planned development areas to Upgraded Chung Mun Road Sewage Pumping Station

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Sewerage Catchment

| no. | ref | ADWF (m³/day) |
|-----|--------------|------------------|
| 1 | Area 33 | 1032.00 |
| 2 | Area 36A | 185.00 |
| 3 | Area 38A | 38.00 |
| 4 | Area 38B | 230.00 |
| 5 | Area 38C | 184.00 |
| 6 | MTR TCW | 147.00 |
| 7 | Sha Tsui Tau | 13.00 |
| | Total | 1829.00 |

Note 1: Sewage Flow Estimation adopted from Overall Schematic of Proposed Sewerage Flow for TCW Development from CEDD.

Estimated Sewage Flow Discharge to Tung Chung Valley East Sewage Pumping Station

Section 12A Planning Application for Proposed Amendments to the Tung Chung Valley Outline Zoning Plan to Rezone "Residential (Group C)2" Zone to "Residential (Group B)" Zone in Support of Private Residential Development at Various Lots

Sewerage Catchment

| no. | ref | ADWF (m³/day) | Downstream Pumping Station | Pump Rate (m³/s) | | |
|-----|-----------------|------------------|-------------------------------|------------------|--|--|
| 1 | Area 67 | (iii /day) 64 | 1 dilipling Station | | | |
| 2 | Area 68A/B | 154 | | | | |
| 3 | Area 73 | 36 | TCV-W SPS | 0.0255 | | |
| 4 | Area 74 | 27 | | 0.0200 | | |
| 5 | Area 75 | 209 | | | | |
| 6 | Area 61A | 299 | | | | |
| 7 | Area 72 | 173 | TCV-N SPS | 0.0231 | | |
| 8 | Area 42 | 4073 | | | | |
| 9 | Area 46 | 1047 | | | | |
| 10 | Area 66A | 259 | | | | |
| 11 | Area 71A | 756 | | | | |
| 12 | Area 71B | 115 | TCV-E SPS | 0.385 | | |
| 13 | Area 76 | 197 | | | | |
| 14 | Area 77 | 186 | | | | |
| 15 | River Park | 100 | | | | |
| 16 | Upgraded CMRSPS | 3541 | | | | |
| | Total | 11236 | | | | |

Note 1: Sewage Flow Estimation adopted from Overall Schematic of Proposed Sewerage Flow for TCW Development from CEDD.

Sewerage Impact Assessment

Annex 5

Hydraulic Checking of Gravity Sewers

| AEC | COM | Section | 12A Plann | iing Appilo | cation for | Proposea | Amename | ents to the I | ung Cnung | y valley Ot | | | | | | hung, Lantau | | oup B)" Zone | in Support o | of Private Resid | dentiai Develop | ment at Various Lo | ots in D.D. 1 IC | Project No. | |
|----------|--------------------|-------------|-----------|-------------|------------|----------|---------|---------------|-----------|-------------|----------------|----------|---------|-------------------|-------------------|---------------------|---------------------|--------------|--------------|------------------|-----------------|--------------------|------------------|-------------|---|
| Asia (| Co. Ltd. | | | | | | | | | | | | | Propos | ed Sewer | | | | | | | | | Date | 10/30/2024 |
| | | | | | | | | | | | | | | | Ву | 0 | | | | | | | | | |
| Mar | nhole | Ground Pipe | | | | | | | | | | | | | | | | | | | | | | | |
| U/S | D/S | Cove | r Level | Size | No. of | Inver | t Level | Length | Grad | Material | k _s | Velocity | Time of | Capacity | Direct | Flow from | Total Average | Contributing | Peaking | Catchment | Upstream | Total Peak | Percentage | Factor of | Discharge |
| | | U/S | D/S | | Pipes | U/S | D/S | | | | | | Flow | | Discharge | Upstream | Flow | Population | Factor | Inflow Factor | Pump Rate | Flow | Capacity | Safety | Remarks |
| | | mPD | mPD | mm | | mPD | mPD | m | 1 in | | mm | m/s | min | m ³ /s | m ³ /s | m ³ /day | m ³ /day | | Incl. Storm | | | m ³ /s | % | | |
| | | | | | | | | | | | | | | | ADWF | ADWF | ADWF | | | | | PWWF | | | |
| FTMH-1 | FMH-H05 | 8.60 | 8.60 | 355 | 1 | 2.55 | 2.21 | 34.0 | 100.0 | PE100 | 0.300 | 1.71 | 0.33 | 0.1206 | 1711.50 | 0.00 | 1711.50 | 6339.00 | 4.0 | 1.00 | 0.00 | 0.0792 | 65.70% | 1.52 | |
| FMH-H05 | FMH-I01c | 8.60 | 6.11 | 560 | 2 | 2.21 | 1.80 | 31.0 | 75.6 | PE100 | 0.300 | 2.70 | 0.19 | 1.0621 | 1829.00 | 1711.50 | 3540.50 | 13113.00 | 3.0 | 1.00 | 0.00 | 0.1229 | 11.57% | 8.64 | |
| FMH-I01c | FMH-I01b | 6.11 | 6.30 | 560 | 1 | 1.80 | 1.71 | 30.0 | 333.3 | PE100 | 0.300 | 1.28 | 0.39 | 0.2505 | 0.00 | 3540.50 | 3540.50 | 13113.00 | 3.0 | 1.00 | 0.00 | 0.1229 | 49.08% | 2.04 | |
| FMH-I01b | FMH-I01a | 6.30 | 7.99 | 560 | 2 | 1.71 | 1.67 | 21.0 | 525.0 | PE100 | 0.300 | 1.01 | 0.35 | 0.3974 | 0.00 | 3540.50 | 3540.50 | 13113.00 | 3.0 | 1.00 | 0.00 | 0.1229 | 30.94% | 3.23 | Sewage Flow Estimation adopted from Ov Schematic of Proposed Sewerage Flow for |
| FMH-I01a | FMH-I01 | 7.99 | 8.25 | 560 | 1 | 1.67 | -0.06 | 55.0 | 31.8 | PE100 | 0.300 | 4.18 | 0.22 | 0.8216 | 0.00 | 3540.50 | 3540.50 | 13113.00 | 3.0 | 1.00 | 0.00 | 0.1229 | 14.96% | 6.68 | Development from CEDD |
| FMH-I01 | Upgraded CMRSPS | 8.25 | 8.25 | 560 | 1 | -0.06 | -0.07 | 2.0 | 300.0 | PE100 | 0.300 | 1.35 | 0.02 | 0.2643 | 0.00 | 3540.50 | 3540.50 | 13113.00 | 3.0 | 1.00 | 0.00 | 0.1229 | 46.52% | 2.15 | |
| | | | | | | | | | | | | | | | | | | | | | | | | | |
| FMH-F01 | FMH-F02 | 11.30 | 11.30 | 800 | 1 | 5.18 | 5.14 | 17.9 | 446.3 | PE100 | 0.300 | 1.33 | 0.22 | 0.4745 | 6474.00 | 0.00 | 6474.00 | 23978.00 | 3.0 | 1.00 | 0.13 | 0.3583 | 75.51% | 1.32 | Including upgraded capacity of upgrade |
| FMH-F02 | FMH-F03 | 11.30 | 11.45 | 800 | 1 | 5.14 | 5.04 | 45.9 | 458.8 | PE100 | 0.300 | 1.31 | 0.58 | 0.4679 | 0.00 | 6474.00 | 6474.00 | 23978.00 | 3.0 | 1.00 | 0.13 | 0.3583 | 76.58% | 1.31 | CMRSPS (0.108m ³ /s) and TCV-W SP |
| FMH-F03 | FMH-E06 | 11.45 | 11.30 | 800 | 1 | 5.04 | 5.01 | 12.8 | 427.3 | PE100 | 0.300 | 1.36 | 0.16 | 0.4851 | 259.00 | 6474.00 | 6733.00 | 24937.00 | 3.0 | 1.00 | 0.13 | 0.3673 | 75.72% | 1.32 | (0.0255m ³ /s) |
| FMH-E06 | TCV-E SPS | 11.30 | 11.30 | 900 | 2 | 5.18 | 5.17 | 3.7 | 500.0 | PE100 | 0.300 | 1.42 | 0.04 | 1.5153 | 0.00 | 6733.00 | 6733.00 | 24937.00 | 3.0 | 1.00 | 0.16 | 0.3904 | 25.76% | 3.88 | Including upgraded capacity of upgrade CMRSPS (0.108m³/s), TCV-W SPS (0.0255 and TCV-N SPS (0.0231m³/s) |

Notes:

^{1.} The invert level, length and gradient of public sewer between FMH-H05 and the upgraded CMRSPS is based on TCW development design drawings. They shall be verified on site.

^{2.} The invert level, length and gradient of public sewer FMH-E06 and the TCV-E SPS shall be verified on site.

Capacity Checking of Upgraded Chung Mun Road Sewage Pumping Station

Utilization of Upgraded Chung Mun Road Sewage Pumping Station

Combinations of ADWF from Different Development

Flow velocity

| Site | ADWF |
|---|--------|
| | (m³/d) |
| The Application Site (Towers T1, T2, T3, T4, T7 and associated commercial and clubhouse + kindergarten) | 1712 |
| Areas 33, 36A, 38A, 38B, 38C, MTR TCW and Sha Tsui Tau | 1829 |
| Total | 3541 |

Utilization of Upgraded Chung Mun Road Sewage Pumping Station

Within desirable range of rising main stated in DSD's Sewerage Manual Part 2

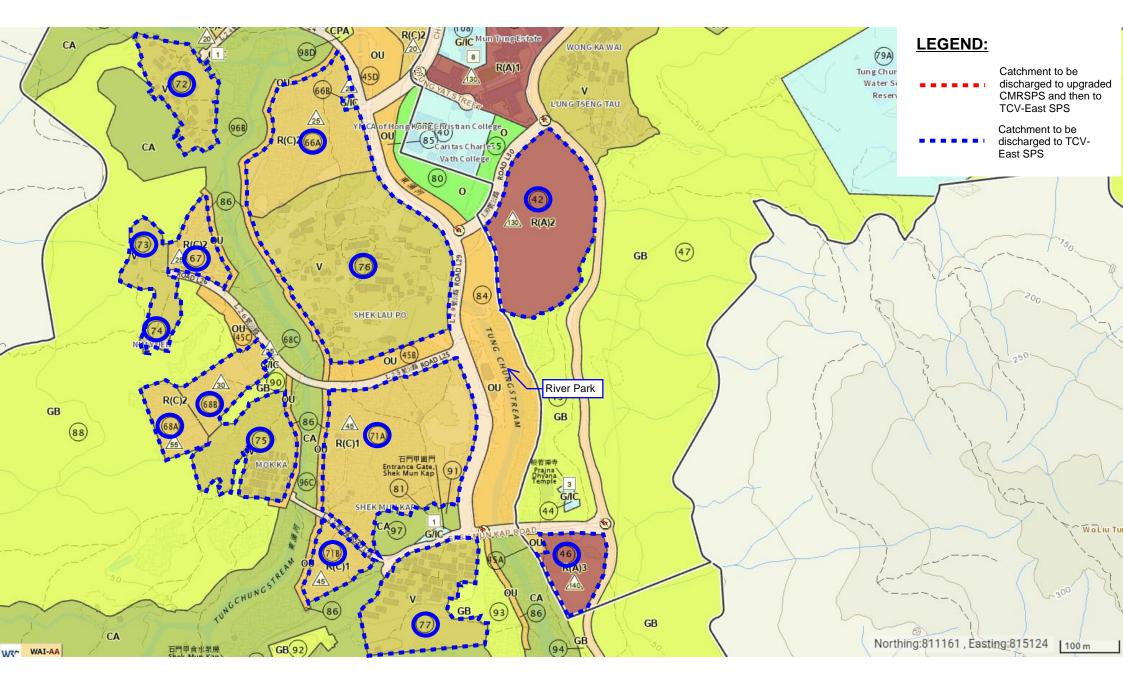
| Total ADWF to Upgraded CMRSPS | = | 3541 m³/d |
|--|--------|--|
| Contributing Population Peaking Factor | = = | 13113 2.5 |
| Peak Flow | = | 0.102 m ³ /s |
| Design information of Upgraded CMRSPS from CEDD Capacity of Upgraded CMRSPS Pump Rate of the Upgraded CMRSPS | = = | 2,767 m ³ /d 0.096 m ³ /s |
| Proposed upgrading works Proposed upgraded pump rate of Upgraded CMRSPS Design ADWF after upgrading | = = | 0.108 m ³ /s 3732 m ³ /d |
| Utilisation after upgrading | = | 94.9% |
| Velocity Checking of Twin OD315 Rising Main | | |
| Inner diameter of OD315 Flow area of twin OD315 rising main | = = | 250 mm 0.0982 m ² |

1.10 m/s

Sewage Pumping Station Service Catchments



Note: Sewage Flow Catchment Area adopted from Overall Schematic of Proposed Sewerage Flow for TCW Development from CEDD



Note: Sewage Flow Catchment Area adopted from Overall Schematic of Proposed Sewerage Flow for TCW Development from CEDD

Sewerage Proposal of Fallback Option

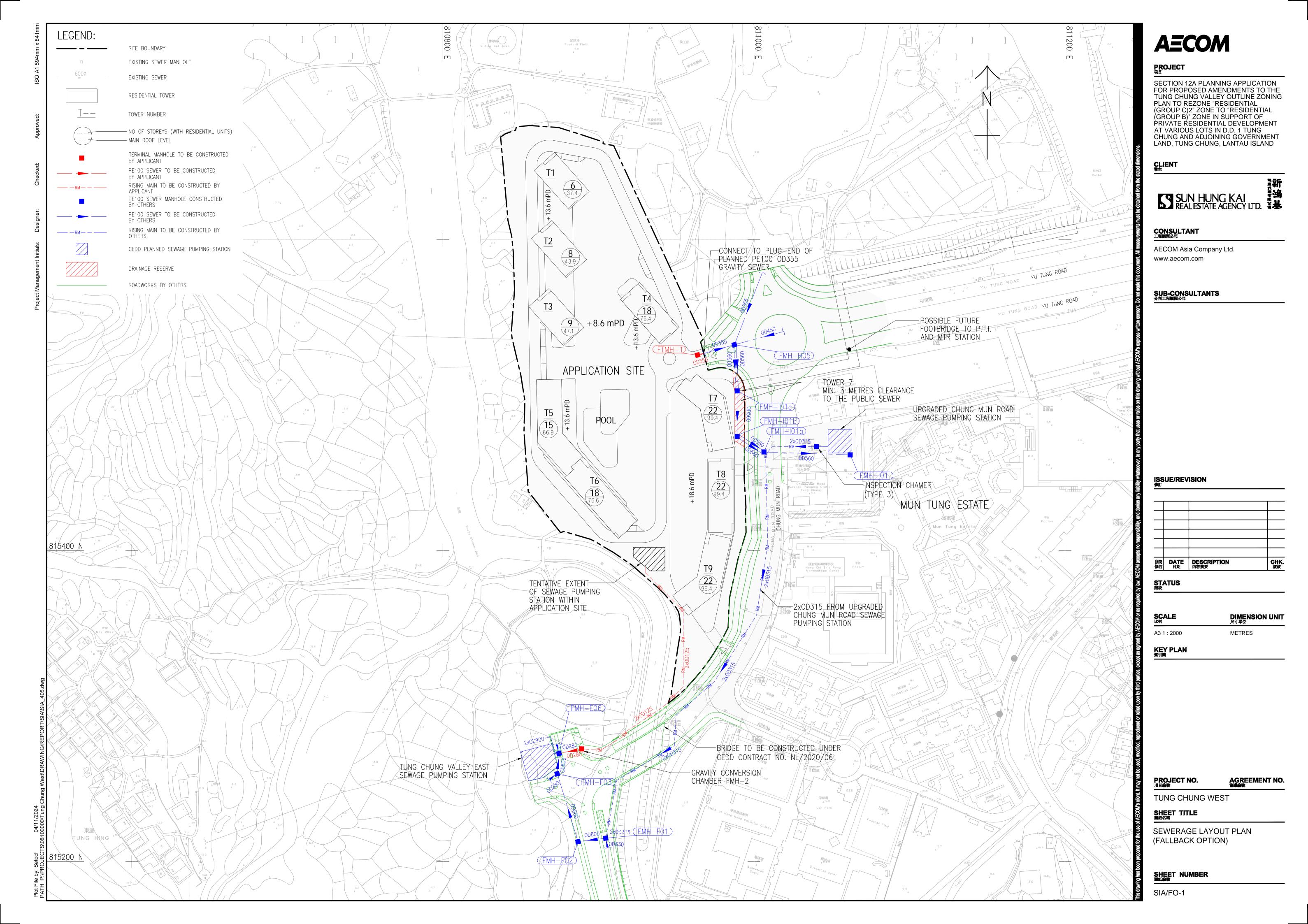
A split flow arrangement is proposed as a fallback option. A portion of the sewage generated by the development will utilize the remaining capacity of the Upgraded CMRSPS through the public plug-end sewer at FMH-H05. To avoid overloading of the Upgraded CMRSPS, the remaining portion of sewage generated from the site will be discharged to the TCV-East SPS via. twin rising mains proposed by the development exiting the southern site boundary. The allocation of split flow is summarised in **Table A**. An indicative sewerage layout of this fallback option and preliminary hydraulic checking are presented in the same Annex. The design of the private pumping station and rising main will be further developed in the detailed design stage.

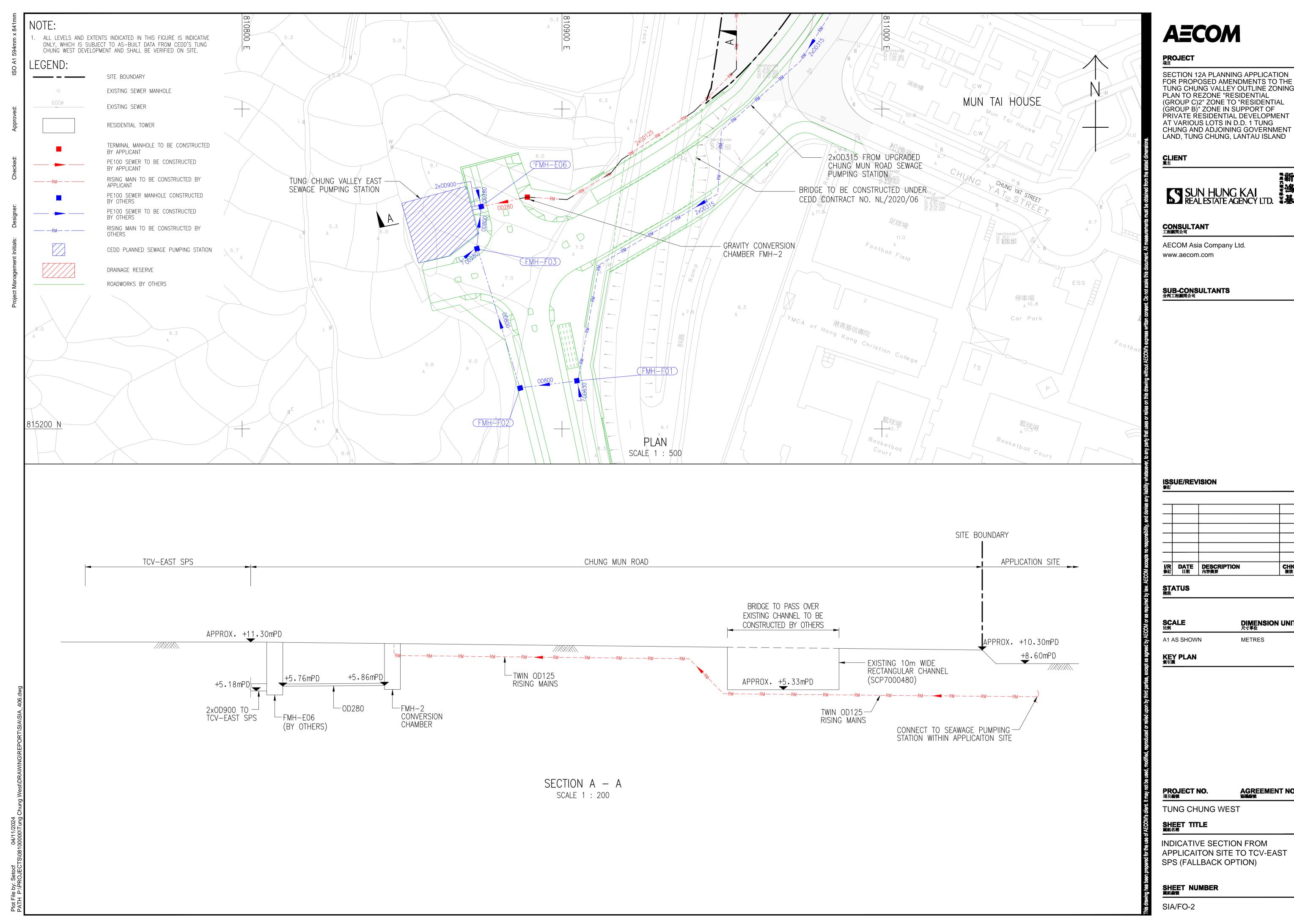
| Sewage Flow | ADWF (m³/day) |
|---|--------------------|
| Sewage discharged to Upgraded CMRSPS via. public manhole FMH-H05 | 937 ⁽¹⁾ |
| Sewage to bypass Upgraded CMRSPS to be directly discharged to TCV-East via. rising main | 775 |
| Total ADWF | 1,712 |

Table A - Split Flow Summary of Fallback Option

Remarks:

- (1) The planned design peak flow of the Upgraded CMRSPS according to the latest information provided by CEDD is 8,300 m³/day with a peaking factor of 3, corresponding to an ADWF of 2,767 m³/day. Subtracting the sewage generated from other planned catchments (1,829 m³/day), the remaining capacity of Upgraded CMRSPS that can be utilized by the Application Site is 937 m³/day.
- 2. The development will be responsible for the construction and maintenance of the sewage pumping station within the Application Site, the twin OD125 rising mains, the gravity conversion chamber FMH-2, and the downstream OD280 gravity sewer up to the connection to the public manhole FMH-E06.





TUNG CHUNG WEST

AGREEMENT NO. 協議線號

DIMENSION UNIT 尺寸單位 **METRES**

I/R DATE DESCRIPTION 内容演要

SUN HUNG KAI REALESTATE AGENCY LTD. 基

TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TUNG CHUNG AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

| Asia (| Co. Ltd. | | | | | | | | | | | | | Propos | ed Sewer | | | | | | | | | Date | 10/30/2024 |
|----------|--------------------|-------------|-------|------|--------|-------|---------|--------|-------|----------|----------------|----------|---------|-------------------|-----------|-----------|---------------------|--------------|-------------|---------------|-----------|------------|------------|-----------|--|
| | | | | | | | | | | | | | | Ву | 0 | | | | | | | | | | |
| Maı | nhole | Ground Pipe | | | | | | | | | | | | | | | | | | | | | | | |
| U/S | D/S | Cover | Level | Size | No. of | | t Level | Length | Grad | Material | k _s | Velocity | Time of | Capacity | Direct | Flow from | Total Average | Contributing | Peaking | Catchment | Upstream | Total Peak | Percentage | Factor of | Discharge |
| | | U/S | D/S | | Pipes | U/S | D/S | | | | | | Flow | | Discharge | Upstream | Flow | Population | Factor | Inflow Factor | Pump Rate | Flow | Capacity | Safety | Remarks |
| | | mPD | mPD | mm | | mPD | mPD | m | 1 in | | mm | m/s | min | m ³ /s | m³/s | m³/day | m ³ /day | | Incl. Storm | | | m³/s | % | | |
| | | | | | | | | | | | | | | <u> </u> | ADWF | ADWF | ADWF | | | | | PWWF | | | |
| FTMH-1 | FMH-H05 | 8.60 | 8.60 | 355 | 1 | 2.55 | 2.21 | 34.0 | 100.0 | PE100 | 0.300 | 1.71 | 0.33 | 0.1206 | 937.00 | 0.00 | 937.00 | 3470.00 | 5.0 | 1.00 | 0.00 | 0.0542 | 44.96% | 2.22 | Split flow from Application Site |
| FMH-H05 | FMH-I01c | 8.60 | 6.11 | 560 | 2 | 2.21 | 1.80 | 31.0 | 75.6 | PE100 | 0.300 | 2.70 | 0.19 | 1.0621 | 1829.00 | 937.00 | 2766.00 | 10244.00 | 3.0 | 1.00 | 0.00 | 0.0960 | 9.04% | 11.06 | |
| FMH-I01c | FMH-I01b | 6.11 | 6.30 | 560 | 1 | 1.80 | 1.71 | 30.0 | 333.3 | PE100 | 0.300 | 1.28 | 0.39 | 0.2505 | 0.00 | 2766.00 | 2766.00 | 10244.00 | 3.0 | 1.00 | 0.00 | 0.0960 | 38.34% | 2.61 | Sewage Flow Estimation adopted from Overal Schematic of Proposed Sewerage Flow for TC\ Development from CEDD |
| FMH-I01b | FMH-I01a | 6.30 | 7.99 | 560 | 2 | 1.71 | 1.67 | 21.0 | 525.0 | PE100 | 0.300 | 1.01 | 0.35 | 0.3974 | 0.00 | 2766.00 | 2766.00 | 10244.00 | 3.0 | 1.00 | 0.00 | 0.0960 | 24.17% | 4.14 | |
| FMH-I01a | FMH-I01 | 7.99 | 8.25 | 560 | 1 | 1.67 | -0.06 | 55.0 | 31.8 | PE100 | 0.300 | 4.18 | 0.22 | 0.8216 | 0.00 | 2766.00 | 2766.00 | 10244.00 | 3.0 | 1.00 | 0.00 | 0.0960 | 11.69% | 8.55 | |
| FMH-I01 | Upgraded CMRSPS | 8.25 | 8.25 | 560 | 1 | -0.06 | -0.07 | 2.0 | 300.0 | PE100 | 0.300 | 1.35 | 0.02 | 0.2643 | 0.00 | 2766.00 | 2766.00 | 10244.00 | 3.0 | 1.00 | 0.00 | 0.0960 | 36.34% | 2.75 | |
| | | | | | | | | | | | | | | | | | | | | | | | | | |
| FMH-2 | FMH-E06 | 11.30 | 11.30 | 280 | 1 | 5.86 | 5.76 | 14.3 | 150.0 | PE100 | 0.300 | 1.24 | 0.19 | 0.0607 | 0.00 | 0.00 | 0.00 | 0.00 | 6.0 | 1.00 | 0.0269 | 0.0269 | 44.32% | 2.26 | Including private SPS (0.027m ³ /s) |
| FMH-E06 | TCV-E SPS | 11.30 | 11.30 | 900 | 2 | 5.18 | 5.17 | 3.7 | 500.0 | PE100 | 0.300 | 0.76 | 0.08 | 0.8100 | 6733.00 | 0.00 | 6733.00 | 24937.00 | 3.0 | 1.00 | 0.17 | 0.4054 | 50.04% | 2.00 | Including capacity of upgraded CMRSPS (0.0961m³/s), TCV-W SPS (0.0255m³/s) and N SPS (0.0231m³/s) and private SPS (0.027 |

Notos:

^{1.} The invert level, length and gradient of public sewer between FMH-H05 and the upgraded CMRSPS is based on TCW development design drawings. They shall be verified on site.

^{2.} The invert level, length and gradient of public sewer FMH-E06 and the TCV-E SPS shall be verified on site.

Sizing of Rising Main

| Total ADWF for rising main | = | 775.0 m ³ /d |
|---|--------|------------------------------|
| Contributing Population Peaking factor for pumping stations (for facility with new upstream sewerage) | = = | 2870 3 |
| Design peak flow | = | $0.027 \text{ m}^3/\text{s}$ |
| Inner diameter of OD125 rising main | = | 100 mm |
| Flow area of twin OD125 rising main | = | 0.016 m ² |
| Flow velocity | = | 1.71 m/s |

Therefore, provide twin OD125 PE pipe.

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