TIA Report

May 2025



CTA Consultants Limited 志達顧問有限公司



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Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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1. INTRODUCTION

1.1 Background

- 1.1.1 CTA Consultants Limited was commissioned as the traffic consultant to prepare a Traffic Impact Assessment Report for proposed re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE) at various lots in D.D. 23, Tung Tsz, Tai Po, New Territories (hereafter called "proposed development").
- 1.1.2 The location of the proposed development is shown in **Figure 1.1**.

1.2 Study Objectives

- 1.2.1 The main objectives of this study are as follows:
 - To assess the existing traffic conditions in the vicinity of the proposed development;
 - To forecast traffic demands on the adjacent road network in the design year;
 - To estimate the likely traffic generated by the proposed development;
 - To assess the impacts of traffic generated by the proposed development on the adjacent road network; and
 - To recommend improvement measures, if necessary, to alleviate any traffic problems on the road network



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2. THE DEVELOPMENT

2.1 Site Location

2.1.1 The proposed development is located at various lots in D.D. 23, Tung Tsz, Tai Po which is bounded by Treasure Spot Garden II to the west as shown in **Figure 1.1**.

2.2 Proposed Development

2.2.1 Development parameters of the proposed development are summarized in **Table 2.1**.

Table 2.1 Development Parameters of the Proposed Development

Site Location	At various lots in D.D. 23, Tung Tsz, Tai Po, New Territories		
Site Area	1,494.67 m ²		
No. of Blocks	1		
No. of Storeys	10		
No. of Staff	120 (60 per shift)		
	Total: 276 beds		
No. of Suites and Beds	(29 nos. of suites, 225 nos. of beds and 22 isolated rooms ⁽¹⁾)		

Note:

- (1) Isolated rooms will be for contingency use only, normally will not be in used.
- 2.2.2 It is anticipated that the proposed development will be completed by 2030 tentatively. Therefore, design year 2033 (i.e. 3 years after the planned commencement year of the proposed development) is adopted assessments.

2.3 Proposed Access Road and Vehicular Access

2.3.1 With consideration of existing road configuration, no proper footpath from the proposed development to Tung Tsz Road, minor road improvement of 3.5m wide single track for two-way traffic with widening at turning area together with a



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minimum 1.3m wide footpath is proposed, drawing on the proposed minor road improvement of access road between the Site and Tung Tsz Road, and the relevant swept path are shown in **Figure 2.1** and **Figure SP-01**.

2.3.2 The proposed vehicular access of ~8m wide is located at the southwest of the proposed development. Location and the sightline assessment of the proposed vehicular access is shown diagrammatically in **Figure 2.2** and **Figure 2.3** respectively, and **Figure SP-02** to **Figure SP-05** demonstrating vehicles can be manoeuvred within the site. Since the visibility splay for southbound vehicles does not fulfil TPDM requirement of 60m sight distance, it is proposed to add flashing alarm lights as safety measure to alert pedestrians and drivers that vehicle is going out.

2.4 Internal Transport Facilities Provision

2.4.1 It is noted that the requirement of provision of internal transport facilities for "Residential Home for Elderly" are not specified in Hong Kong Planning Standards and Guidelines (HKPSG). Therefore, parking provision has been reference to other existing RCHE and summarized in **Table 2.2**.

Table 2.2 Examples of Existing RCHE

Table 2.2 Examples of Existing NOTE					
RCHE	Location	No. of beds	No. of Staff	Parking Facilities Category ⁽¹⁾⁽²⁾	Observed no. of Parking Provision
Assemblies of God Holy Light Church Aged Home	91 Sung Ching Sun Tsuen, Tai Tong Road, Yuen Long	60	19	Category 1	Nil
Chinese Christian Worker's Fellowship Wah Hei Elderly Home (Comet Mansion	G/F & M/F, Shop 27, Comet Mansion, 45-67 Fung Cheung Road, Yuen Long	105	29	Category 1	Nil
T.W.G.Hs. Y. C. Liang Memorial Home for the Elderly	G/F & 1/F, Yiu Yat House, Tin Yiu Estate, Tin Shui Wai	88	47	Category 1	Nil
Salvation Army Kam Tin Residence for Senior Citizens (The)	103 Kam Tin Road, Yuen Long	150	81	Category 2	1 car parking space + 1 light bus parking space
Pok Oi Hospital Yeung Chun Pui Care and Attention Home	58 Sha Chau Lei Tsuen, Ha Tsuen, Yuen Long	143	92	Category 2	2 car parking spaces + 1 light bus parking space



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RCHE	Location	No. of beds	No. of Staff	Parking Facilities Category ⁽¹⁾⁽²⁾	Observed no. of Parking Provision
Ching Chung Taoist Association of Hong Kong Limited Ching Chung Care and Attention Home for the Aged	57 Sha Chau Lei Chuen, Ping Ha Road, Yuen Long	120	61	Category 2	1 car parking space + 1 light bus parking space
Approved Y/YL-NTM/9	81 San Tam Road, Yuen Long	142	-	Category 2	2 car parking spaces + 1 light bus parking space + 1 LGV + 1 Ambulance

Notes:

- (1) Category 1 refers to RCHE without parking provision within the Site and there is no public car parking spaces can be found in the close proximity.
- (2) Category 2 refers to RCHE with parking provision within the Site.
- 2.4.2 With reference to **Table 2.2**, only one to two private parking spaces are provided by other RCHE for max 150 beds. Taking into consideration that a total of 276 nos. of beds will be provided in our development, the proposed provision is summarized in **Table 2.3**, which should be sufficient for the daily operation needs of the proposed development.
- 2.4.3 The ground floor layout plan of the proposed development showing the internal transport provision is shown in **Figure 2.2** and **Figure SP-02** to **Figure SP-05** demonstrating vehicles can be manoeuvred within the site.

Table 2.3 Proposed Parking Provision

Parking Spaces	Dimensions	Proposed	
Motorcycle	2.4m(L) x 1m(W)	1 no.	
Private Car	5m(L) x 2.5m(W)	3 nos.	
Private Car for Accessible	5m(L) x 3.5m(W)	1 no.	
Loading/Unloading	Dimensions	Proposed	
LGV	7m(L) x 3.5m(W)	1 no.	
Light bus	8m(L) x 3m(W)	1 no.	
Ambulance	9m(L) x 3m(W)	1 no.	



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2.4.4 2 private car parking spaces at the site would be for staff and 2 private car parking spaces for visitors or persons with disabilities, booking in advance is required.

2.5 Public Transport Services in the Vicinity

2.5.1 Numerous road-based public transport services are provided in vicinity of the proposed development. Details of the current services of franchised buses and GMB routes are listed in **Table 2.4** and the service points are demonstrated in **Figure 2.4**. It is revealed that the site is well-served by public transport services in the vicinity.

Table 2.4 Road-Based Public Transport Services in the Vicinity

Service	ervice Route Origin – Destination		Frequency (Mins)
	73P ⁽¹⁾	Nina Tower – Tai Mei Tuk	From Nina Tower: 2 Dep; From Tai Mei Tuk: 2 Dep
	74E ⁽¹⁾	Kwun Tong – Tai Mei Tuk	From Kwun Tong: 3 Dep; From Tai Mei Tuk: 3 Dep
Franchised	75K	Tai Mei Tuk – Tai Po Market Station	10-20
Bus	275R	Wu Kau Tang – Tai Po Market Station	10-20
	72C ⁽¹⁾	Tai Mei Tuk – Tai Po Market Station	1 Dep
	75P ⁽¹⁾	Tai Mei Tuk – Tai Po Market Station	1 Dep
	20B	Tung Tsz – Tai Po Market Station	10-20
	20C	Tai Mei Tuk – Tai Po Market Station	4-10
		Tai Mei Tuk – Tai Po Market Station (via Tai Po Tai Wo Road)	12-15
	20C ⁽²⁾	Tai Mei Tuk – Tai Po Market Station (via Shan Liu Road)	4-10
GMB	20E ⁽³⁾	Shan Liu Road, Elle Villas – Tai Po Market Station	30
	20R	Wu Kau Tang – Tai Po Market Station	60
	20T ⁽⁴⁾	Tsz Shan Monastery – Tai Po Market Station	From Tai Po Market Station: 9:15am - 9:45am and 1:15pm - 1:45pm; From Tsz Shan Monastery: 11:30am - 1:30pm and 3:30pm - 5pm



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Notes:

- (1) Peak hour service only.
- (2) Special Route during special traffic and transport arrangements (STTA) days.
- (3) Circular Route.
- (4) Weekday service only.

2.6 Shuttle Service to be Provided and Visit-by-Appointment System Enforcement

- 2.6.1 The public could access to Tung Tsz Road by NT GMB Route Nos. 20B and 20T, and to Ting Kok Road by NT GMB Route Nos. 20C, 20E, 20R and Bus Route Nos. 73P, 74E, 75K and 275R, and then walk for about 8 min to the Site. The operator will arrange the working hours of the staff such that public transport during daytime will not be affected, shuttle service may be arranged for staff depending on actual operation.
- 2.6.3 Being a RCHE, the number of visitors is very little. However, to avoid many visitors to arrive at the same period of time, the policy of limiting 2 visitors per bed under Visit-by-Appointment System will be implemented. The booking in advance by telephone/whatsapp is required. Visitors are not allowed to enter the proposed development without booking or outside the operation hour. The number of visitors allowed per hour and per day will be 19 (i.e. maximum capacity of light bus) and 152 (i.e. 19 visitor/hr x 8hr) respectively.
- 2.6.4 In general, booking in advance is required for parking provision for visitors or persons with disabilities.



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3. THE EXISTING TRAFFIC CONDITIONS

3.1 Critical Junctions

3.1.1 As shown in **Figure 3.1**, 11 junctions were identified to be critical for assessment of traffic impact due to the proposed development. They are listed in **Table 3.1** and their existing junction layout arrangements are shown in **Figures 3.2** to **3.11** respectively.

Table 3.1 Identified Critical Junctions

Ref.	Junction Me		Figure No.
A	Tung Tsz Road/ Universal Gate Road	Priority	3.2
В	Ting Kok Road/ Tung Tsz Road	Priority	3.3
С	Ting Kok Road/ Sam Mun Tsai Road	Signal	3.4
D	Ting Kok Road/ Lo Fai Road	Signal	3.5
Е	Ting Kok Road/ Dai Kwai Street	Signal	3.6
F	Ting Kok Road/ Dai Fat Street	Signal	3.7
G	Ting Kok Road/ Fung Yuen Road	Signal	3.8
Н	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	Signal	3.9
I	Yuen Shin Road/ Dai Fat Street	Signal	3.10
J	Yuen Shin Road/ Tai Po Tai Wo Road	Signal	3.11

- 3.1.2 In order to establish the existing traffic condition in the above-mentioned critical junctions, traffic survey in form of manual classified count was conducted during the AM and PM peak periods (7:15am to 9:15am and 5:00pm to 7:00pm) on a typical weekday, 6 December 2024. Analysis of the existing traffic data indicates that the AM and PM peak hour flows occurred from 7:45am to 8:45am and 5:15pm to 6:15pm respectively. The existing traffic flows is presented in **Figure 3.12**.
- 3.1.3 Existing operational performance of the identified critical junctions and road links were assessed. The results are summarized in **Table 3.2**, **Table 3.3** and the junction calculation sheets are attached in **Appendix A**.



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Table 3.2 Junction Performance of Identified Critical Junctions in Year 2024

Junction	Junction Location	Method of	Year 2024 RC ⁽¹⁾ /RFC ⁽²⁾	
Junction	Junction Location	Control	AM Peak	PM Peak
A	Tung Tsz Road/ Universal Gate Road	Priority	0.03	0.03
В	Ting Kok Road/ Tung Tsz Road	Priority	0.49	0.24
C	Ting Kok Road/ Sam Mun Tsai Road	Signal	>100%	>100%
D	Ting Kok Road/ Lo Fai Road	Signal	56%	84%
Е	Ting Kok Road/ Dai Kwai Street	Signal	19%	22%
F	Ting Kok Road/ Dai Fat Street	Signal	20%	47%
G	Ting Kok Road/ Fung Yuen Road	Signal	21%	23%
Н	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	Signal	31%	33%
I	Yuen Shin Road/ Dai Fat Street	Signal	25%	48%
J	Yuen Shin Road/ Tai Po Tai Wo Road	Signal	27%	44%

Note:

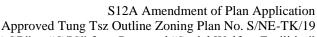
3.1.4 The assessment results in **Table 3.2** indicate that all critical junctions are at present operating within their capacities during peak hours.

Table 3.3 Traffic Flows and Volume to Capacity (V/C) Ratio Assessment of Identified Road Links in Year 2024

						Year 2024	Existing	
Road Link	Dir	Road Type	No. of Lanes	Capacity (pcu/hr)	AM I	Peak	PM Peak	
Itouu Ziiiii	Di.				Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Tong Ton Dood	EB	LD	1	460	260	0.57	130	0.28
Tung Tsz Road	WB	LD	1	460	160	0.35	160	0.35
Ting Kok Road	EB	PD	2	3,220	1,390	0.43	1,230	0.38
(between Dai Fat Street and Fung Yuen Road)	WB	PD	2	3,220	1,490	0.46	1,110	0.34
Ting Kok Road	EB	PD	2	3,220	1,470	0.46	1,370	0.43
(between Fung Yuen Road and Dai Fuk Road)	WB	PD	2	3,220	1,580	0.49	1,200	0.37
Yuen Shin Road	EB	PD	2	3,220	730	0.23	770	0.24

⁽¹⁾ RC = Reserve Capacity for Signalized Junction

RFC = Ratio of Flow to Capacity for Priority Junction



0.48

0.50



Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE)

1,540

1,600

At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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1,210

1,550

0.38

0.48

Year 2024 Existing Road No. of Capacity **AM Peak PM Peak Road Link** Dir **Type** (pcu/hr) Lanes Flow **Flow** V/C V/C (pcu/hr) (pcu/hr) (between Dai WB Fuk Road and PD 2 3,220 1,080 0.34 970 0.30 Dai Fat Street)

3,220

3,220

Notes:

Yuen Shin Road

(between Dai Fat Street and

Tai Po Tai Wo Road)

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

2

2

PD

PD

EB

WB

- (2) PCU factor of 1.15 has been derived from the result of traffic count survey. Tung Tsz Road is is single-2-lane local road of ~7m wide, therefore capacity per direction = 800÷ 2 x 1.15 = 460 pcu/hr. Ting Kok Road and Yuen Shin Road are dual-2 primary distributor of ~8m wide, therefore capacity per direction = 2,800x1.15= 3,220pcu/hr.
- 3.1.5 The assessment results in **Table 3.3** indicate that all critical road links have adequate road link capacity during the peak hours.



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4. THE FUTURE TRAFFIC CONDITIONS

4.1 Design Year

4.1.1 The proposed development is anticipated to be completed by year 2030 tentatively. Year 2033 (i.e. 3 years after completion) is therefore adopted as the design year for assessment purpose.

4.2 Traffic Forecasts

- 4.2.1 The traffic growth can be estimated by applying growth factor, based on the following information source:
 - I. Historical traffic growth in Annual Traffic Census (ATC) published by the Transport Department (TD).
 - II. 2021-Based Territorial Population and Employment Data Matrices (TPEDM) published by the Planning Department.

Annual Traffic Census

4.2.2 Numerous traffic count stations are located in the vicinity of the proposed development and the traffic counts of the concerned stations reported in the Annual Traffic Census (ATC) between 2015 and 2023 are summarized in **Table 4.1**.

Table 4.1 Historical Traffic Data from Annual Traffic Census (ATC)

			Annual A	verage Da	ily Traffic	(AADT)		Avg.
ATC Stn	Road Name	2015	2016	2017	2018	2022	2023	Annual Growth Rate
5006	Ting Kok Rd (from Nam Wan Rd to Dai Kwai St)	26,760	29,650	30,680	30,900	30,440	29,190	1.09%
6211	Ting Kok Rd (from Dai Kwai St to Tai Mei Tuk)	25,240	27,230	27,110	29,580	28,760	28,620	1.58%
6608	Ting Kok Rd (from Tai Mei Tuk to Bride's Pool Rd)	1,350	1,390	1,520	1,400	1,290	1,240	-1.06%



S12A Amendment of Plan Application Approved Tung Tsz Outline Zoning Plan No. S/NE-TK/19 Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE)

At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

64,270

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63,290

1.25%

			Avg.					
ATC Stn	Koad Name	2015	2016	2017	2018	2022	2023	Annual Growth Rate
6619	Dai Kwai St (from Ting Kok Rd to Dai Chong St)	3,970	4,110	4,270	3,880	3,780	4,240	0.83%

62,380

57,320

Note:

63,580

65,760

Planning Data

Total

4.2.3 Reference has also been made to the 2021-Based Territorial Population Employment Data Matrices (TPEDM) planning data published by the Planning Department for projection of population and employment within the study district from years 2021 to 2031. The average annual growth rates in terms of population and employment from 2021 to 2031 are tabulated in **Table 4.2**.

Table 4.2 TPEDM Planning Data from 2021 to 2031

]	Population	<u> </u>			Employment			
Zone	2021	2026	2031	Annual Growth Rate	2021	2026	2031	Annual Growth Rate	
Tai Po	316,450	348,900	343,250	0.82%	96,600	94,800	89,800	-0.73%	

Adopted Growth Rate

- 4.2.4 A.A.D.T. of ATC indicates that the traffic flows in the local road network has an average annual growth rate of +1.25% from 2015 to 2023.
- 4.2.5 Whilst, the planning data indicates that the population and employment data of the study area are expected to grow with an average annual growth rate of +0.82% and -0.73% respectively from 2021 to 2031.

⁽¹⁾ Traffic volumes for Year 2019 to Year 2021 may be suppressed by the special working arrangement implemented during the COVID-19 outbreak period and/or social event outbreak, therefore AADT from Year 2019 to Year 2021 are not adopted.



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4.2.6 As a conservative approach, annual growth rate of $\pm 1.25\%$ p.a. is adopted.

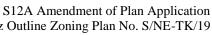
4.3 Traffic Generations of Adjacent New Developments

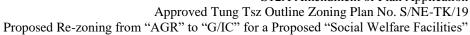
4.3.1 To fully reflect the growth traffic, trip generation of the future vicinity developments have been taken into consideration. The major planned development is detailed in **Figure 4.1** and the estimated trip rate with reference to TPDM and trips of the adjacent planned developments are shown in **Table 4.3** and **Table 4.4** respectively.

 Table 4.3
 Estimated Trip Rates of Planned Adjacent Developments

Approved						Trip	Rates	
Planning Application	Location	Use	Assumed GFA & Flat no.	Units	AM	Peak	PM	Peak
No.					Gen.	Att.	Gen.	Att.
A/TP/672	Governme nt land at Area and Chung	Public Housing	~316, 519m ² 7,431flats (av. flat size: 40m ²)	pcu/hr/flat		Already near to full population in-take		
N/11/0/2	Nga Road East, Tai Po, New Territories	Retail / Shopping Complex	~5,160m ²	pcu/hr/100 sq m GFA	the time of survey (Appendix B).			
	Chung Nga Road West	Public Housing	1,292 flats (av. Flat size: 40m ²)	pcu/hr/flat	0.0432 ⁽¹⁾	0.0326 ⁽¹⁾	0.0237 ⁽¹⁾	0.0301 ⁽¹⁾
A/TP/700		Retail / Shopping Complex	~489m²	pcu/hr/100 sq m GFA	0.2296	0.2434	0.31	0.3563
		Primary School	-	pcu/hr/ classroom	0.5670 ⁽²⁾	1.000 ⁽²⁾	0.333 ⁽²⁾	0.167 ⁽²⁾
		Public Housing	1,290 flats	-	-	-	-	-
LSPS/001	Lo Fai Road and	Retail	~1,000m ²	-	-	-	-	-
LSI 5/001	Ting Kok Road	Community Facilities	-	-	-	-	-	-
		Private Housing	460 flats					
-	Area 33, Tai Po	Construction Industry Council Training	-	-	-	-	-	-

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Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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Approved						Trip	Rates	
Planning Application	Location	Use	Assumed GFA & Flat no.	Units	AM	Peak	PM	Peak
No.			CC 2 AUG 2200		Gen.	Att.	Gen.	Att.
		Academy Tai Po Training Ground						
-	Tai Po Town Lot 246 (Ex- Shuen Wan Landfill Site)	Golf Course	-	-	-	-	-	-
-	Area 33, Tai Po	Football-cum- rugby pitch/underground public vehicle park 400 car spaces	-	Pcu/hr/ parking space	0.0771 ⁽⁶⁾	0.0907 ⁽⁶⁾	0.0493 ⁽⁶	0.0811 ⁽⁶⁾
-	On Pong Road	Community health centre	4,447m ²	pcu/hr/100 sq m GFA	0.235 ⁽⁷⁾	0.235 ⁽⁷⁾	0.23 ⁽⁷⁾	0.115 ⁽⁷⁾
-	Future Phase of CDA(1) Zone	Private Housing	~ 14,011 m ² 220 flats	pcu/hr/flat	0.0778 ⁽⁸⁾	0.063 ⁽⁸⁾	0.063 ⁽⁸⁾	0.0593 ⁽⁸⁾
Y/TP/38	Tai Po Town Lot 183 S.A ss.1 (Part) and 183 S.A ss.2 (Part), Various Lots in D.D. 11 and Adjoining Governme nt Land, Fung Yuen, Tai Po	Private Housing	1,759 flats Retail not more than 800 m ²	-	_(3)	_(3)	_(3)	_(3)

Notes:

- (1) Trip rates for public housing development of 40m² is adopted.
- (2) Adopted trip rate of primary school in Queen's Hill.
- (3) Adopted trip generations and attractions from TIA report of the relevant planning application.
- (4) Upper limit trip rates for private housing development of 60m² is adopted.
- Trip rates for public housing development of 50m² is adopted. (5)
- Based on surveyed trip rate at Tai Po Tung Cheong Street Sports Centre Public Vehicle (6) Park.



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- (7) Adopted trip rate of community health centre in the approved TIA report for Queen's Hill, Fanling.
- (8) Adopted trip rate of Mont Vert.
- (9) For the planned bus depot at Dai Fuk Street (A/TP/685), the approved TIA report stated that most of the buses leave and return to the depot between 12am and 6am for daily operation, therefore will not be included in the assessment during peak hours.
- (10) The transitional housing developments under applications No. A/NE-TK/702 and A/NE-TK/753 are covered by valid planning permissions until 26.3.2026 and 26.8.2027 respectively, therefore will not be included in the assessment.
- (11) LSPS/001 proposed residential developments are scheduled to be completed by year 2033 tentatively, which is same as the assessment year for this TIA, therefore will be included in the assessment as sensitivity test.

Table 4.4 Estimated Trip Generations and Attractions of Planned Adjacent Developments

Approved					Tr	ips		
Planning Application	Location	Development	Assumed GFA & Flat	AM (pcu	Peak ı/hr)	PM Peak (pcu/hr)		
No.			no.	Gen.	Att.	Gen.	Att.	
A/TP/672	Government land at Area and Chung Nga Road	Public Housing -316, 519m ² 7,431flats (av. flat size: 40m ²)		Already near to full population in-take				
	East, Tai Po, New Territories	Retail / Shopping Complex	~5,160m ²	at the ti	at the time of survey (Appendix B).			
		Public Housing	1,292 flats (av. Flat size: 40m ²)	56	43	31	39	
A/TP/700	Chung Nga Road West	Retail / Shopping Complex	~489m²	2	2	3	3	
		Primary School	pcu/hr/ classroom	18	30	10	6	
		Public Housing	1,290 flats	100 ⁽¹⁾	75 ⁽¹⁾	45 ⁽¹⁾	60 ⁽¹⁾	
LSPS/001	Lo Fai Road and Ting Kok Road	Retail	~1,000m ²	5 ⁽¹⁾	5 ⁽¹⁾	5 ⁽¹⁾	5 ⁽¹⁾	
		Community Facilities	-	30 ⁽¹⁾	30 ⁽¹⁾	25 ⁽¹⁾	30 ⁽¹⁾	

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S12A Amendment of Plan Application Approved Tung Tsz Outline Zoning Plan No. S/NE-TK/19 Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities"

Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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Amm					Tr	ips	
Approved Planning Application	Location	Development	Assumed GFA & Flat	AM (pcu	Peak	-	Peak /hr)
No.			no.	Gen.	Att.	Gen.	Att.
		Private Housing	460 flats	45 ⁽¹⁾	35 ⁽¹⁾	20 ⁽¹⁾	20 ⁽¹⁾
-	Area 33, Tai Po	Construction Industry Council Training Academy Tai Po Training Ground	-	23 ⁽²⁾	23 ⁽²⁾	23 ⁽²⁾	23 ⁽²⁾
-	Tai Po Town Lot 246 (Ex- Shuen Wan Landfill Site)	Golf Course	-	8 ⁽¹⁾	32 ⁽¹⁾	50 ⁽¹⁾	26 ⁽¹⁾
-	Area 33, Tai Po	Football-cum- rugby pitch/underground public vehicle park 400 car spaces	-	31	37	20	33
-	On Pong Road	Community health centre	4,447m ²	11	11	11	6
-	Future Phase of CDA(1) Zone	Private Housing	~ 14,011 m ² 220 flats	17	14	14	13
Y/TP/38	Tai Po Town Lot 183 S.A ss.1 (Part) and 183 S.A ss.2 (Part), Various Lots in D.D. 11 and Adjoining Government Land, Fung Yuen, Tai Po	Private Housing	1,759 flats Retail not more than 800 m ²	143 ⁽¹⁾	118 ⁽¹⁾	123 ⁽¹⁾	114 ⁽¹⁾

Notes:

(1)Based on the approved TIA.

(2)Based on previous study on Construction Industry Council Training Academy.



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4.4 Planned Junction Layout under Planned Project

- 4.4.1 Junction Fung Yuen Road / Ting Kok Road (G) will be modified according to the TIA report (January 2021) of approved A/NE-TK/702 at Wong Yue Tan, Tai Po and the latest drawing for A/NE-TK/753 at Shuen Wan, Tai Po as shown in **Figure 4.2**. It is anticipated that the planned junction layout would be in place for reference and design year 2033 (the commissioned year of the proposed development) for the assessments.
- 4.4.2 It is noted that Fung Yuen CDA (1) (planning application no. Y/TP/38) of about 1,800 units is undergoing planning application scheduled to be completed by year 2027 tentatively. The planned improvement scheme of Fung Yuen CDA (1) should be in place together with its development, therefore the planned improvement schemes of the critical junctions will be taken into account and summarised in **Table 4.5**.

Table 4.5 Planned Junction Layouts under Fung Yuen CDA (1) (Planning application no. Y/TP/38)

Ref.	Junction	Detail	Anticipated Completion Year
F	Ting Kok Road/ Dai Fat Street	As shown in Figure 4.3	By 2027
G	Ting Kok Road/ Fung Yuen Road	As shown in Figure 4.4	By 2027
Н	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	As shown in Figure 4.5	By 2027
I	Yuen Shin Road/ Dai Fat Street	As shown in Figure 4.6	By 2027
J	Yuen Shin Road/ Tai Po Tai Wo Road	As shown in Figure 4.7	By 2027

4.4.3 It is noted that Land Sharing Pilot Scheme (LSPS/001) proposed residential developments at UDWYT Lot 14 RP and Lot 11RP, Tai Po are scheduled to be completed by year 2033 tentatively, which is same as the assessment year for this TIA, therefore will be included in the assessment as sensitivity test. The planned improvement scheme of LSPS/0001 should be in place together with its development, therefore the planned improvement schemes of the critical junctions will be taken into account in the sensitivity test and summarised in **Table 4.6**.



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Table 4.6 Planned Junction Layouts under LSPS/0001

	able 4.0 Hamled Bulletion Enjours under ESI 5/0001								
Ref.	Junction	Detail	Anticipated Completion Year						
D	Ting Kok Road/ Lo Fai Road	As shown in Figure 4.8	By 2033						
Е	Ting Kok Road/ Dai Kwai Street	As shown in Figure 4.9	By 2033						
F	Ting Kok Road/ Dai Fat Street	As shown in Figure 4.10	By 2033						
G	Ting Kok Road/ Fung Yuen Road	As shown in Figure 4.11	By 2033						
Н	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	As shown in Figure 4.12	By 2033						
I	Yuen Shin Road/ Dai Fat Street	As shown in Figure 4.13	By 2033						
J	Yuen Shin Road/ Tai Po Tai Wo Road	As shown in Figure 4.14	By 2033						

4.5 Reference Traffic Flows

4.5.1 2033 reference traffic flows are then derived by the following and presented diagrammatically in **Figure 4.15**.

2033 Reference Traffic Flows (Without Proposed Development)	= (X	Adopted Growth Factor (i.e. +1.25% for 9 years)) +	Traffic Flows of Planned Adjacent Developments
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4.6 Traffic Generations and Attractions of Proposed Development

4.6.1 To estimate the trip generations of the proposed development, reference has been made to the trip generation rates of the existing Tung Wah Group of Hospitals Shuen Wan Complex for the Elderly which comprises Pao Siu Loong Care and Attention Home, Wu York Yu Care and Attention Home, and Wu Chiang Wai Fong Care and Attention Home in the same district, and sites of similar nature at remote area. The adopted trip generation rates are summarized in **Table 4.7**.

Table 4.7 Adopted Generation and Attraction Trip Rates of Proposed

Development

h	Developi					
D.C. CV	Approx.	Unit	AM I	Peak	PM Peak	
Reference Sites	No. of Beds		Generation	Attraction	Generation	Attraction
Tung Wah Group of Hospitals Shuen Wan Complex for the Elderly,	255	pcu/hr	7	10	13	9
93 Sam Mun Tsai Road, Shuen Wan, Tai Po, N.T.	233	pcu/hr/bed	0.027	0.039	0.051	0.035
Pok Oi Hospital Yeung Chun Pui Care and Attention Home, Lot No.	143	pcu/hr	2	2	3	2
2273 & Ext. in DD 125, Ping Ha Road, Ping Shan, Yuen Long, N.T.	143	pcu/hr/bed	0.014	0.014	0.021	0.014
TWGHs Wong Cho Tong Social Service Building,	278	pcu/hr	24	19	12	16
39 Sheung Shing Street, Homantin, Kowloon	270	pcu/hr/bed	0.086	0.068	0.043	0.058
Adopted Rate	-	pcu/hr/bed	0.086	0.068	0.051	0.058

4.6.2 Based on **Table 2.1** and **Table 4.7**, the estimated traffic generation and attraction due to the proposed development are summarized in **Table 4.8**.



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Table 4.8 Estimated Traffic Generation and Attraction of Proposed Development

	AM	Peak	PM Peak		
Proposed Development	Generation (pcu/hr)	Attraction (pcu/hr)	Generation (pcu/hr)	Attraction (pcu/hr)	
276 beds	24 (say 25)	19 (say 20)	15 (say 15)	17	
Visitors (1 no. of light bus/hr) ⁽¹⁾	=1x1.5pcu =1.5 (say 2)	=1x1.5pcu =1.5 (say 2)	=1x1.5pcu =1.5 (say 2)	=1x1.5pcu =1.5 (say 2)	
Total	26 (say 30)	21 (say 25)	17 (say 20)	19 (say 20)	

Note:

4.6.3 It is anticipated that the proposed development would generate and attract 30 pcu/hr and 25 pcu/hr during AM peak hour respectively, and generate and attract 20 pcu/hr and 20 pcu/hr during PM peak hour respectively.

4.7 Design Traffic Flows

4.7.1 The future traffic generations of the proposed development were then assigned onto the road network and superimposed onto the 2033 reference traffic flows (without proposed development) to derive the 2033 design traffic forecasts (with proposed development).

4.7.2 Year 2033 design traffic flows (with proposed development) are shown in **Figure 4.16**.

⁽¹⁾ From **Section 2.6**, shuttle service will be provided to visitor during non-peak hour, it is included for assessment in peak hour as a conservative approach.



5.

S12A Amendment of Plan Application Approved Tung Tsz Outline Zoning Plan No. S/NE-TK/19 Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

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TRAFFIC IMPACT ASSESSMENT

5.1 Operational Assessment

5.1.1 To assess the potential traffic impact due to the proposed development, capacity analysis of the identified critical junction and road links for both reference (without proposed development) and design scenarios (with proposed development) in year 2033 were carried out. The results are summarized in **Table 5.1**, **Table 5.2** and the junction calculation sheets are attached in **Appendix A**.

Table 5.1 Junction Performance of Identified Critical Junctions in Year 2033 (With and Without Proposed Development)

			Year 2033 RC/RFC (1)						
Ref.	Junction	Method of Control	Scer (Without	rence nario Proposed pment)	Design Scenario (With Proposed Development)				
			AM Peak	PM Peak	AM Peak	PM Peak			
A	Tung Tsz Road/ Universal Gate Road	Priority	0.03	0.05	0.07	0.09			
В	Ting Kok Road/ Tung Tsz Road	Priority	0.57	0.29	0.64	0.33			
С	Ting Kok Road/ Sam Mun Tsai Road	Signal	>100%	>100%	>100%	>100%			
D	Ting Kok Road/ Lo Fai Road	Signal	24%	48%	22%	46%			
Е	Ting Kok Road/ Dai Kwai Street	Signal	-5%	0%	-7%	-1%			
F ⁽²⁾	Ting Kok Road/ Dai Fat Street	Signal	41%	50%	40%	50%			
G ⁽²⁾	Ting Kok Road/ Fung Yuen Road	Signal	35%	47%	34%	45%			
H ⁽²⁾	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	Signal	32%	43%	30%	42%			
I ⁽²⁾	Yuen Shin Road/ Dai Fat Street	Signal	37%	46%	35%	44%			
J ⁽²⁾	Yuen Shin Road/ Tai Po Tai Wo Road	Signal	27%	43%	26%	42%			

Notes:

(1) RC = Reserve Capacity for Signalized Junction

RFC = Ratio of Flow to Capacity for Priority Junction

(2) Reference has been made to the planned junction improvement works mentioned in Section 4.4.



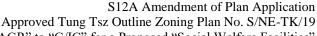
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- 5.1.2 The assessment results in **Table 5.1** revealed that all critical junctions would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2033 during the peak hours except the following junction:
 - Junction Ting Kok Road/ Dai Kwai Street (E)
- 5.1.3 Junction E will have negative RC without and with the proposed development during AM peak hour, and will RC of 0% without the proposed development and RC of -1% with the proposed development during PM peak hour.
- 5.1.4 It is anticipated that the proposed development would generate and attract 30 pcu/hr and 25 pcu/hr during AM peak hour respectively, and generate and attract 20 pcu/hr and 20 pcu/hr during PM peak hour respectively.
- 5.1.5 The peak traffic generated by the proposed development is small and would induce insignificant impact on the surrounding road network.

Table 5.2 Traffic Flows and Volume to Capacity (V/C) Ratio Assessment of Identified Road Links in 2033 (With and Without Proposed Development)

.												
	Dir	Dir Road Type	_	Capacity (pcu/hr)	Year 2033 Reference Scenario (Without Proposed Development)				Year 2033 Design Scenario (With Proposed Development)			
Road Link Dir					AM Pe	AM Peak		PM Peak		AM Peak		PM Peak
					Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Tung Tsz	EB	LD	1	460	290	0.63	150	0.33	320	0.70	170	0.37
Road	WB	LD	1	460	190	0.41	180	0.39	220	0.48	200	0.43
Ting Kok	EB	PD	2	3,220	1,730	0.54	1,530	0.48	1,750	0.54	1,550	0.48
Road (between Dai Fat Street and Fung Yuen Road)	WB	PD	2	3,220	1,860	0.58	1,390	0.43	1,890	0.59	1,410	0.44
Ting Kok	EB	PD	2	3,220	1,950	0.61	1,790	0.56	1,970	0.61	1,810	0.56
Road (between Fung Yuen Road and Dai Fuk Road)	WB	PD	2	3,220	2,100	0.65	1,610	0.50	2,130	0.66	1,630	0.51
Yuen Shin	EB	PD	2	3,220	970	0.30	1,000	0.31	990	0.31	1,020	0.32

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At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T



Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE)

1,910

0.59

2,030

0.63

1,930

0.60

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Dood Link	Dir Road No. of			Capacity	Year 2033 Reference Scenario (Without Proposed Development) AM Peak PM Peak			Year 2033 Design Scenario (With Proposed Development) AM Peak PM Peak				
Road Link	Dir	Туре	Lanes	(pcu/hr)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Road (between Dai Fuk Road and Dai Fat Street)	WB	PD	2	3,220	1,440	0.45	1,270	0.39	1,470	0.46	1,290	0.40
Yuen Shin Road	EB	PD	2	3,220	1,910	0.59	1,510	0.47	1,930	0.60	1,530	0.48
(between Dai												

Notes:

WB

PD

Fat Street and

Tai Po Tai Wo Road)

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

2,000

(2) PCU factor of 1.15 has been derived from the result of traffic count survey. Tung Tsz Road is is single-2-lane local road of ~7m wide, therefore capacity per direction = 800÷ 2 x 1.15 = 460 pcu/hr. Ting Kok Road and Yuen Shin Road are dual-2 primary distributor of ~8m wide, therefore capacity per direction = 2,800x1.15= 3,220pcu/hr.

0.62

5.1.6 The assessment results in **Table 5.2** indicate that all critical road links will have adequate road link capacity in 2033 during the peak hours.

5.2 Operational Assessment – Sensitivity Test for LSPS/0001

3,220

5.2.1 As mentioned in **Section 4.4**, Land Sharing Pilot Scheme (LSPS/001) proposed residential developments at UDWYT Lot 14 RP and Lot 11RP, Tai Po are scheduled to be completed by year 2033 tentatively, which is same as the assessment year for this TIA, therefore will be included in the assessment as sensitivity test. The planned improvement scheme of LSPS/0001 should be in place together with its development, therefore the planned improvement schemes of the critical junctions will be taken into account in the sensitivity test. Capacity analysis of the identified critical junction for design scenarios (with proposed development) sensitivity test for LSPS/0001 in year 2033 were carried out. The results are summarized in **Table 5.3** and the junction calculation sheets are attached in **Appendix A**.



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Table 5.3 Junction Performance of Identified Critical Junctions – Sensitivity
Test with LSPS/001 in Year 2033 (With Proposed Development)

Junction	Junction Location	Method of Control	with LS Year 2033 I Design S	itivity Test LSPS/001 33 RC/RFC (1) gn Scenario osed Development) PM Peak		
A	Tung Tsz Road/ Universal Gate Road	Priority	0.07	0.09		
В	Ting Kok Road/ Tung Tsz Road	Priority	0.64	0.33		
С	Ting Kok Road/ Sam Mun Tsai Road	Signal	>100%	>100%		
D ⁽²⁾	Ting Kok Road/ Lo Fai Road	Signal	56%	35%		
E ⁽²⁾	Ting Kok Road/ Dai Kwai Street	Signal	32%	55%		
F ⁽²⁾	Ting Kok Road/ Dai Fat Street	Signal	22%	48%		
G ⁽²⁾	Ting Kok Road/ Fung Yuen Road	Signal	42%	47%		
H ⁽²⁾	Ting Kok Road/ Yuen Shin Road/ Dai Fuk Street	Signal	20%	39%		
$\mathbf{I}^{(2)}$	Yuen Shin Road/ Dai Fat Street	Signal	37%	51%		
$\mathbf{J}^{(2)}$	Yuen Shin Road/ Tai Po Tai Wo Road	Signal	47%	69%		

Notes:

- (1) RC = Reserve Capacity for Signalized Junction RFC = Ratio of Flow to Capacity for Priority Junction
- (2) Reference has been made to the planned junction improvement works mentioned in Section 4.4.
- 5.2.2 The assessment results in **Table 5.3** revealed that all critical junctions would still operate within their capacities in design scenario (with proposed development) in 2033 during the peak hours with LSPS/001 for sensitivity test.



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6. SUMMARY AND CONCLUSION

6.1 Summary

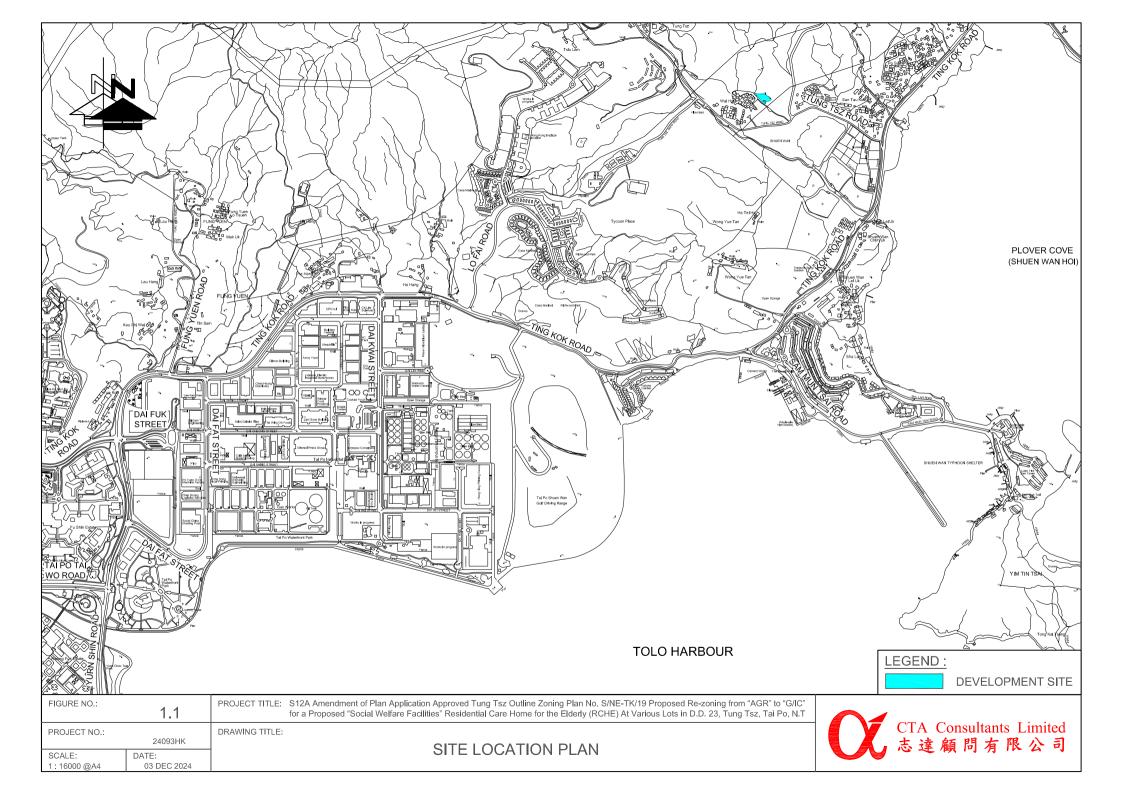
- 6.1.1 CTA Consultants Limited (CTA) is commissioned as the traffic consultant to prepare the Traffic Impact Assessment Report and provide technical justifications in supporting the proposed development from traffic engineering point of view.
- 6.1.2 To appraise the existing traffic condition, manual-classified counting surveys were conducted at critical junctions in 2024. Current operational performance of the critical junctions has been assessed. The results reveal all critical junctions are at present operating within their capacities during peak hours.
- 6.1.3 The assessment results revealed that all critical junctions would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2033 during the peak hours except the following junction:
 - Junction Ting Kok Road/ Dai Kwai Street (E)
- 6.1.4 Junction E will have negative RC without and with the proposed development during AM peak hour, and will RC of 0% without the proposed development and RC of -1% with the proposed development during PM peak hour.
- 6.1.5 It is anticipated that the proposed development would generate and attract 30 pcu/hr and 25 pcu/hr during AM peak hour respectively, and generate and attract 20 pcu/hr and 20 pcu/hr during PM peak hour respectively.
- 6.1.6 The peak traffic generated by the proposed development is small and would induce insignificant impact on the surrounding road network.

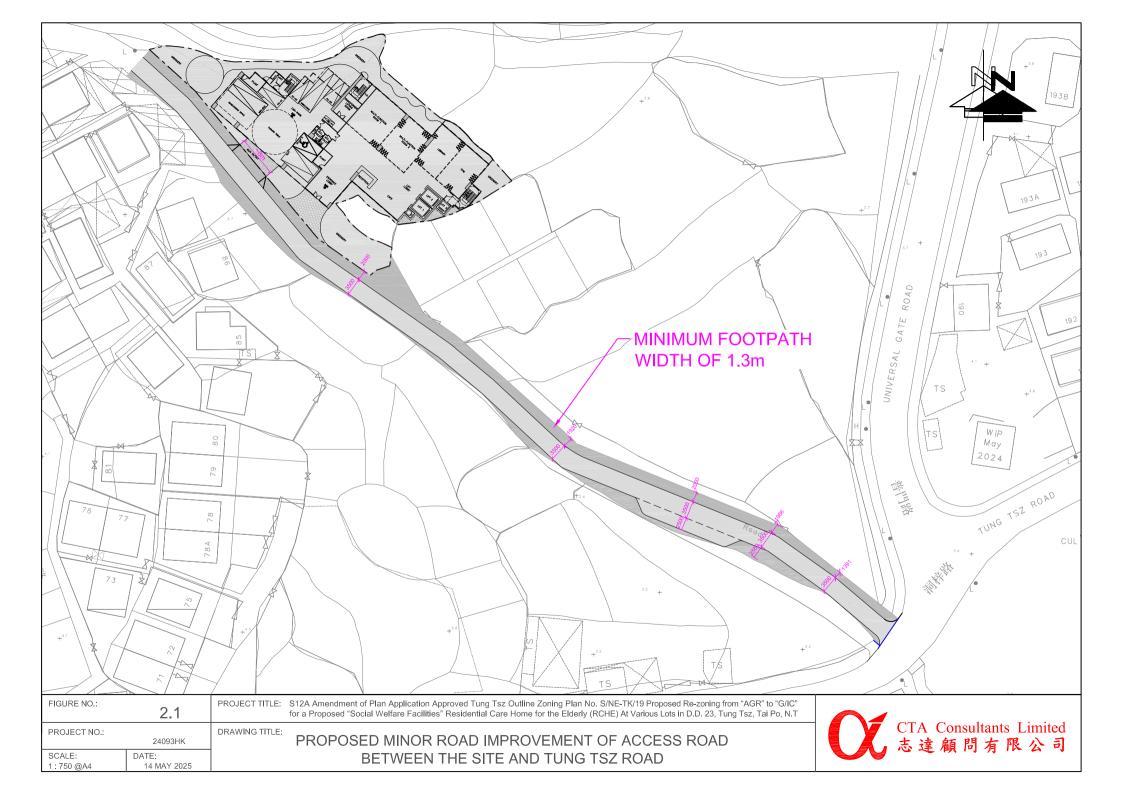


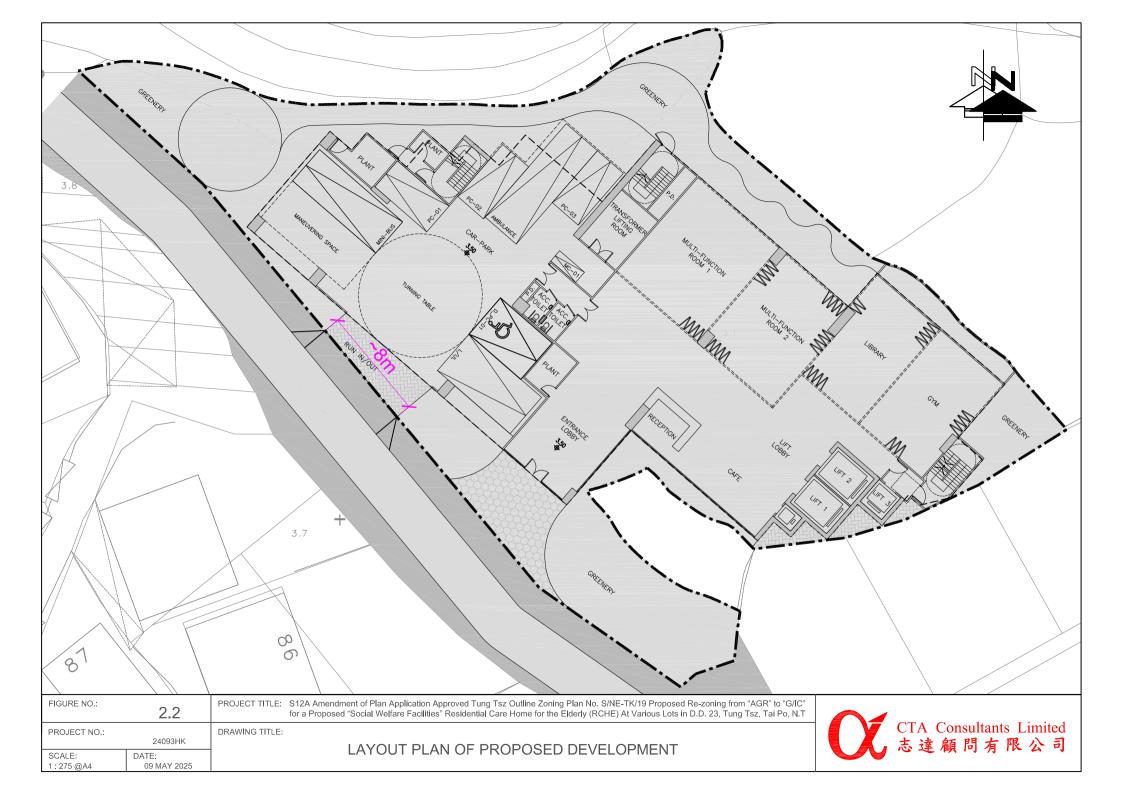
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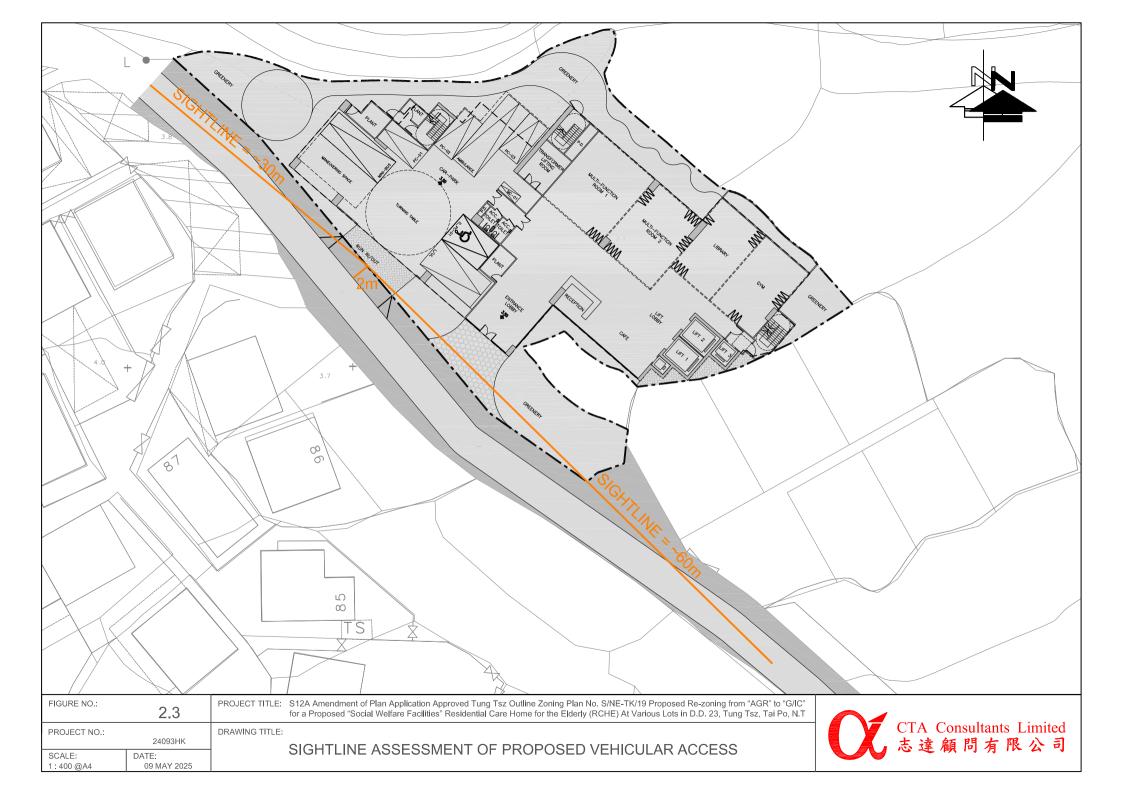
6.2 Conclusion

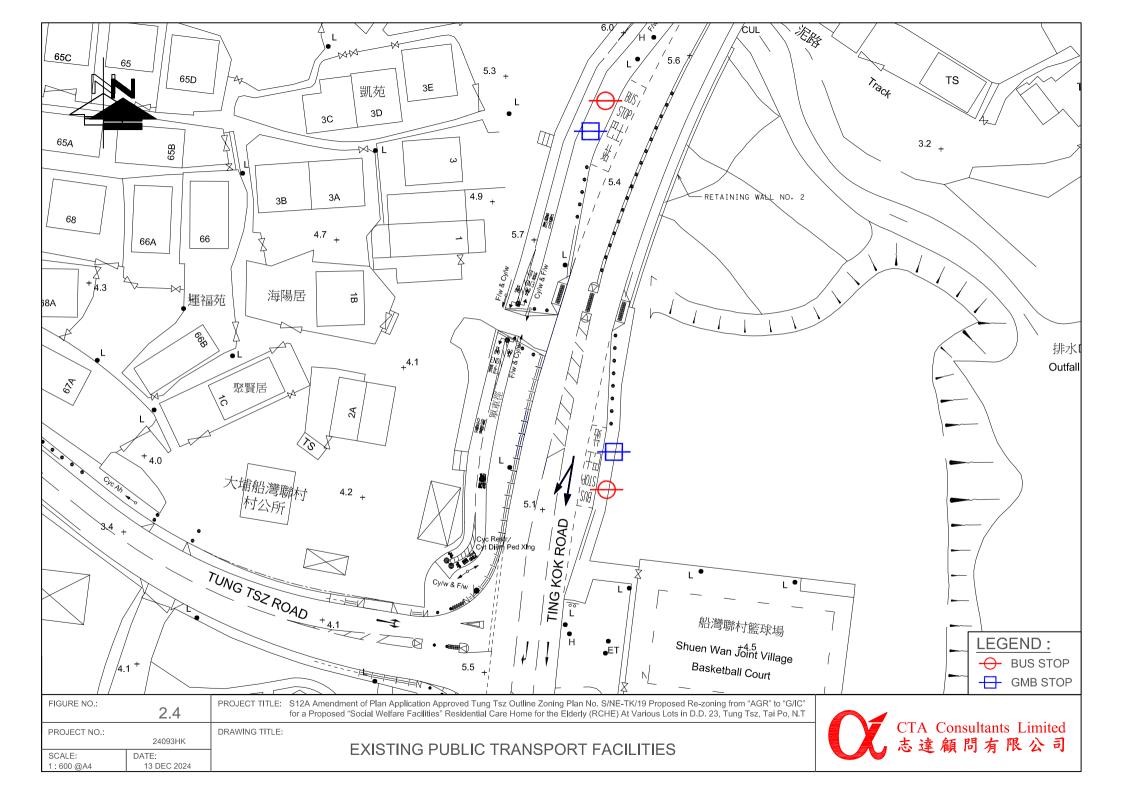
- 6.2.1 In conclusion, this Traffic Impact Assessment Report has demonstrated that the related traffic trips related to the proposed development can be absorbed by the nearby road network and no significant traffic impact will be induced.
- 6.2.2 Therefore, the proposed development is reckoned feasible from traffic engineering point of view.

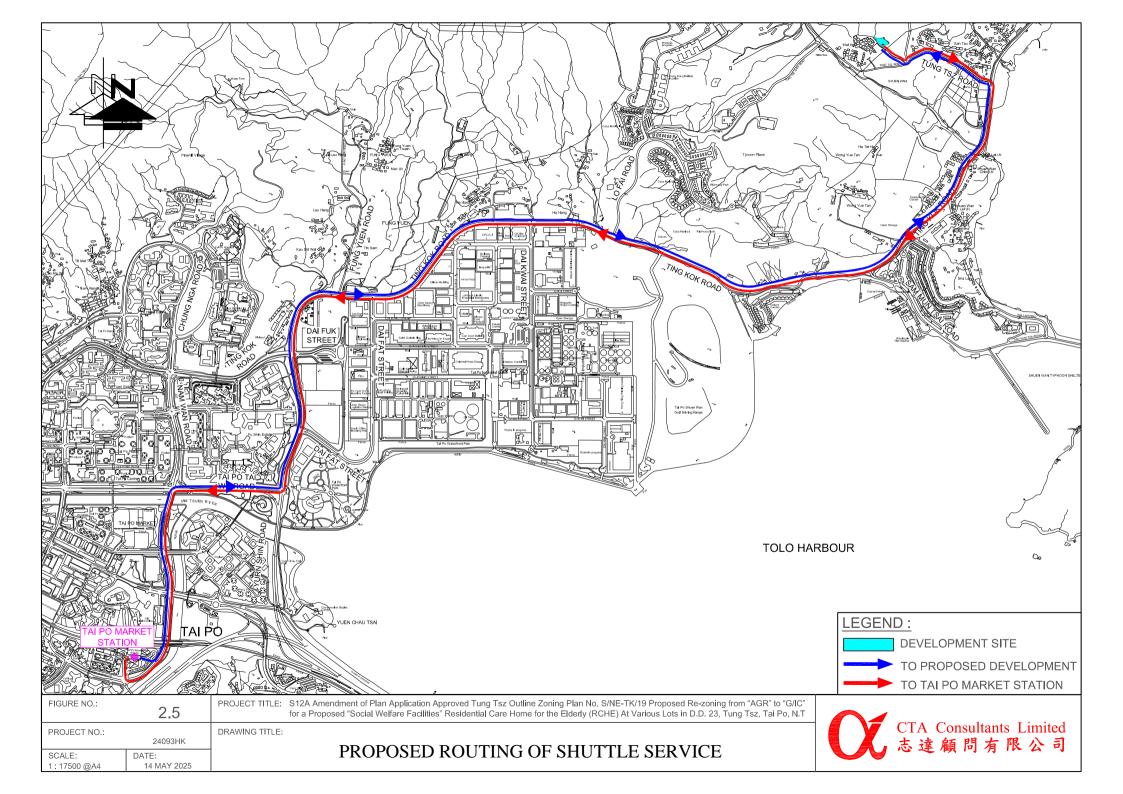


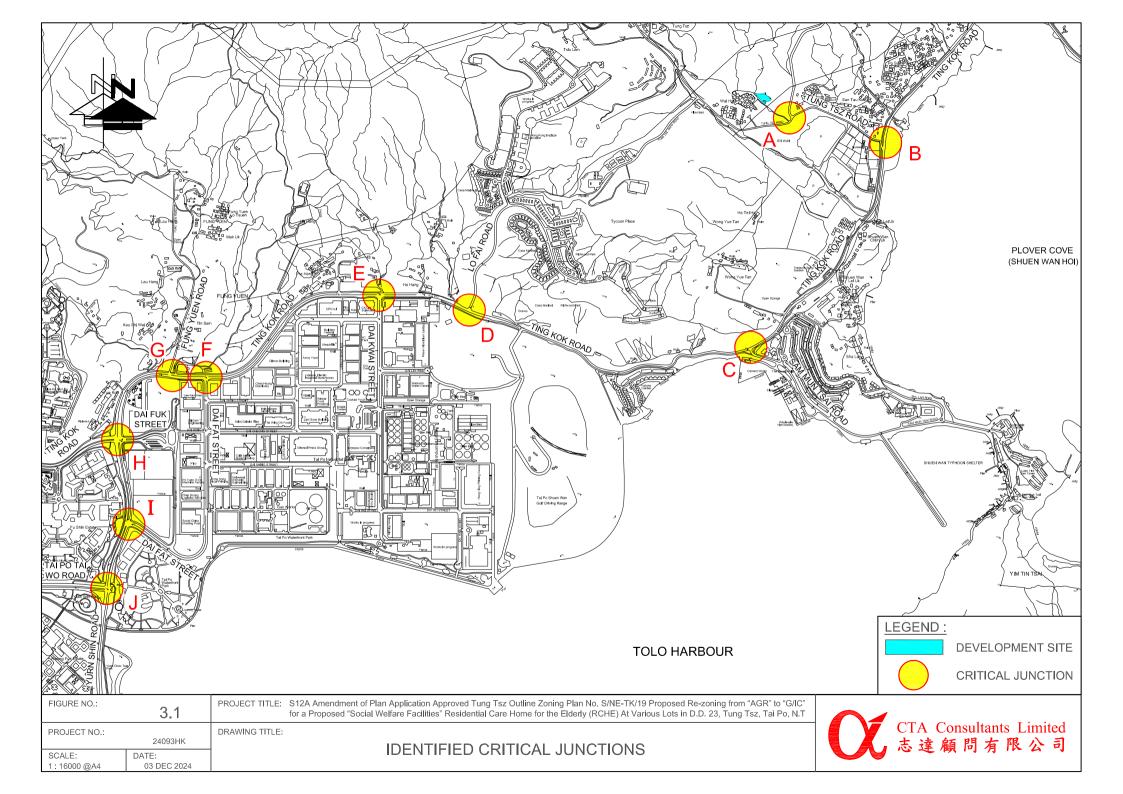


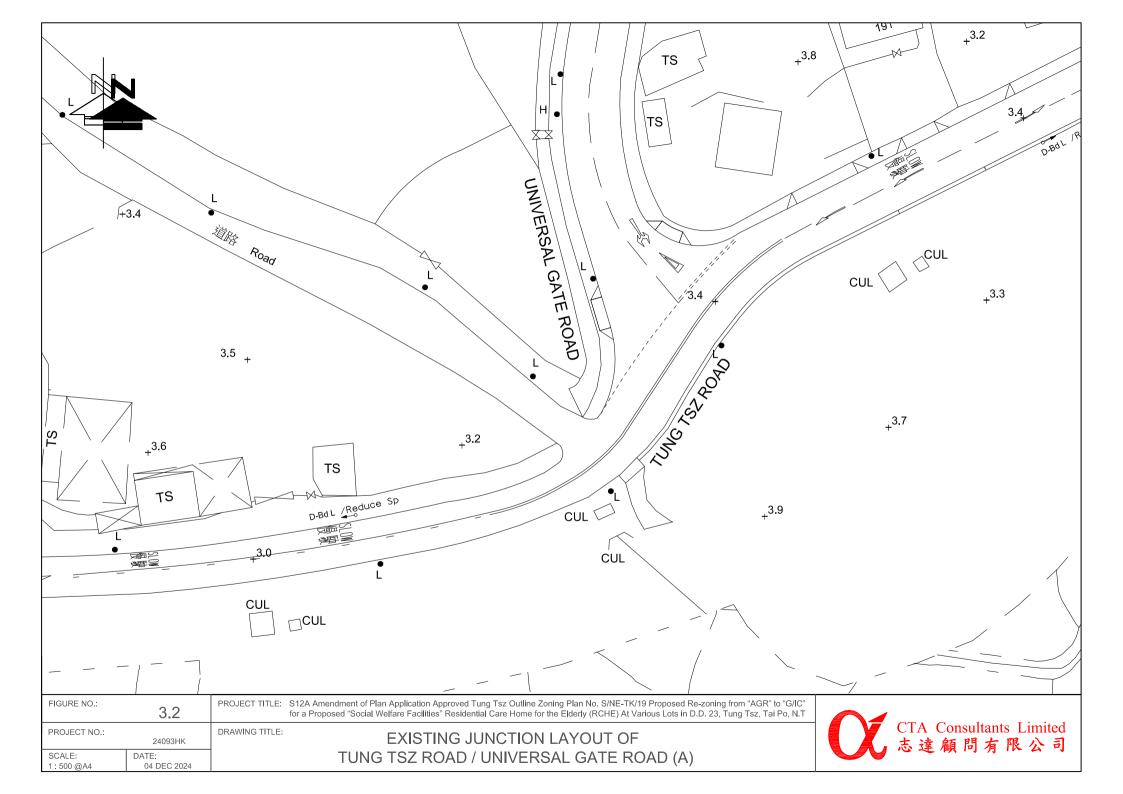


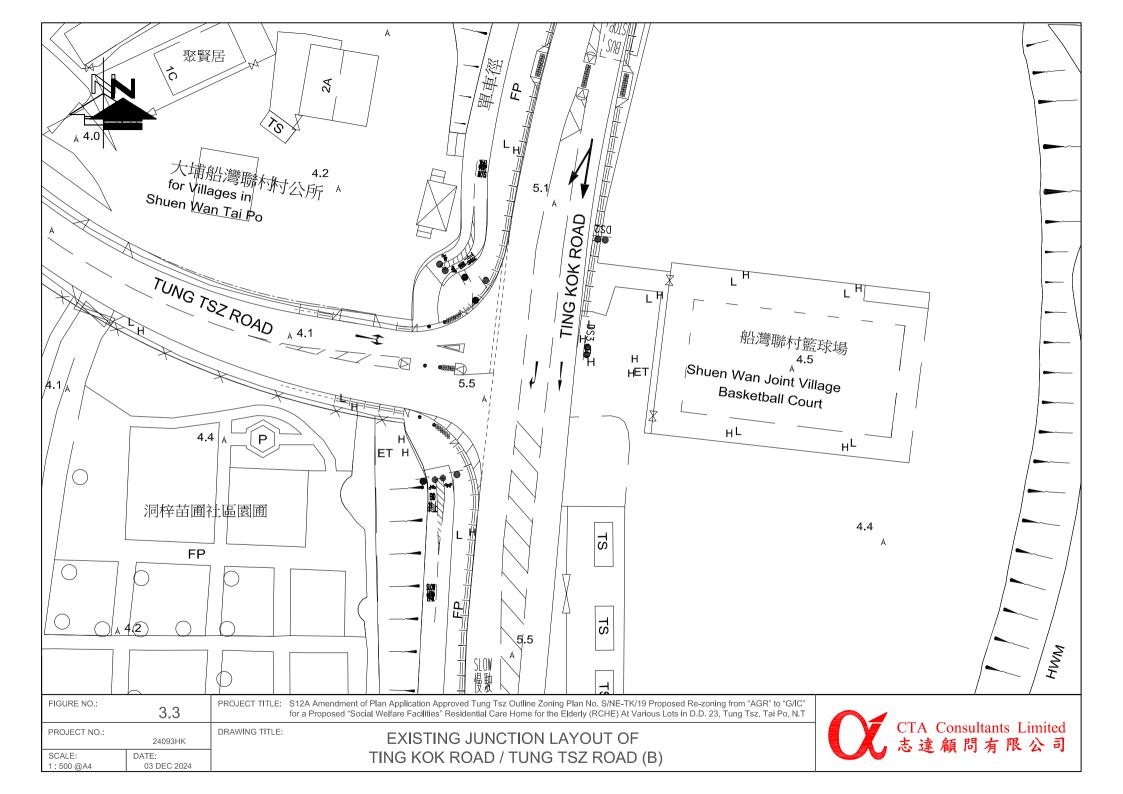


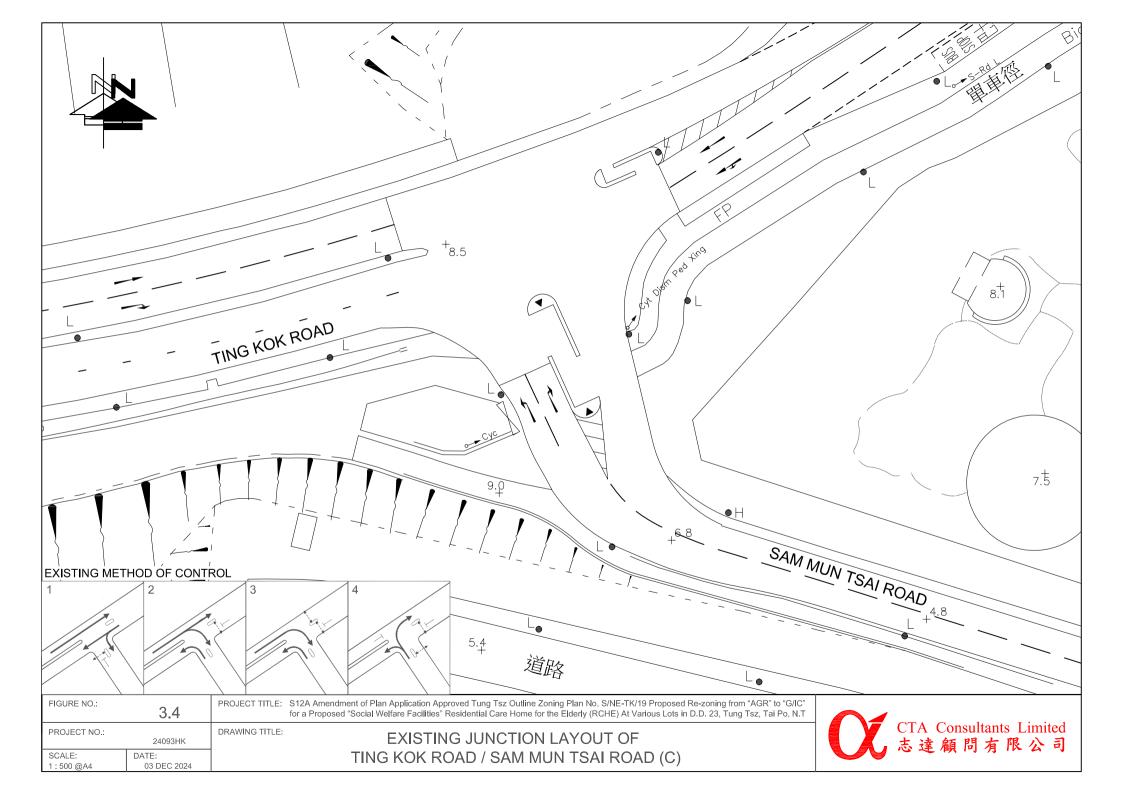


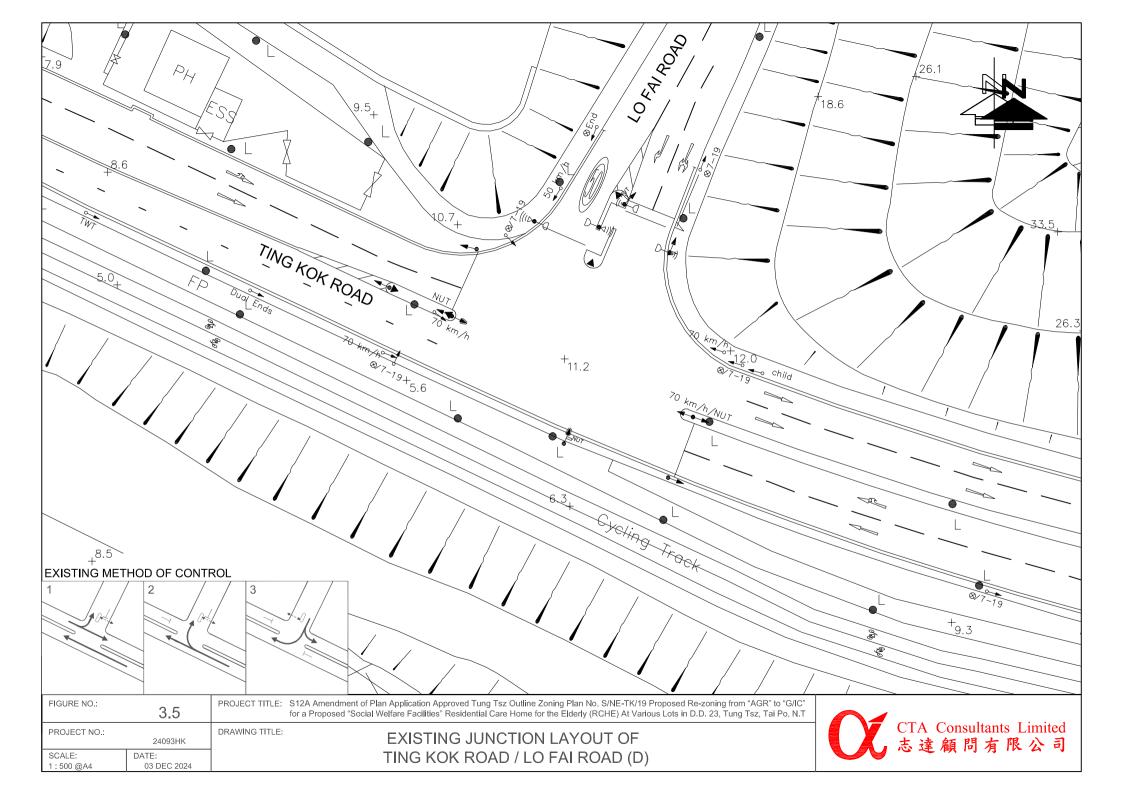


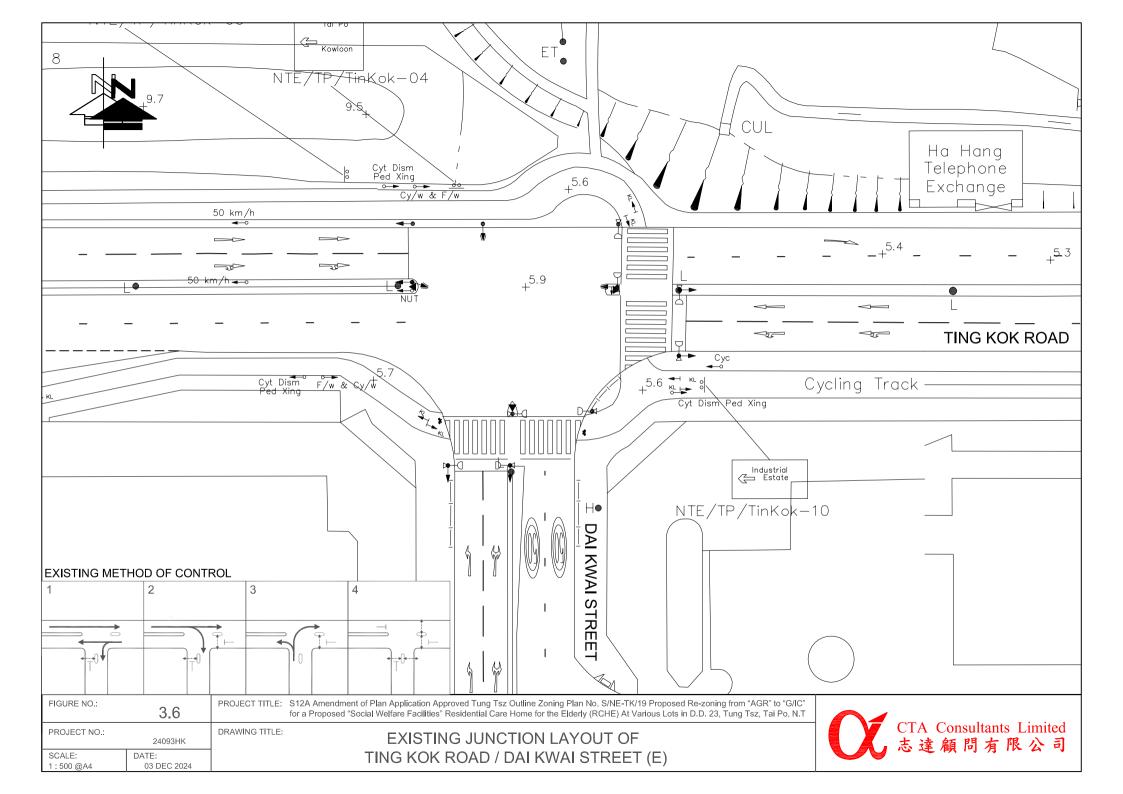


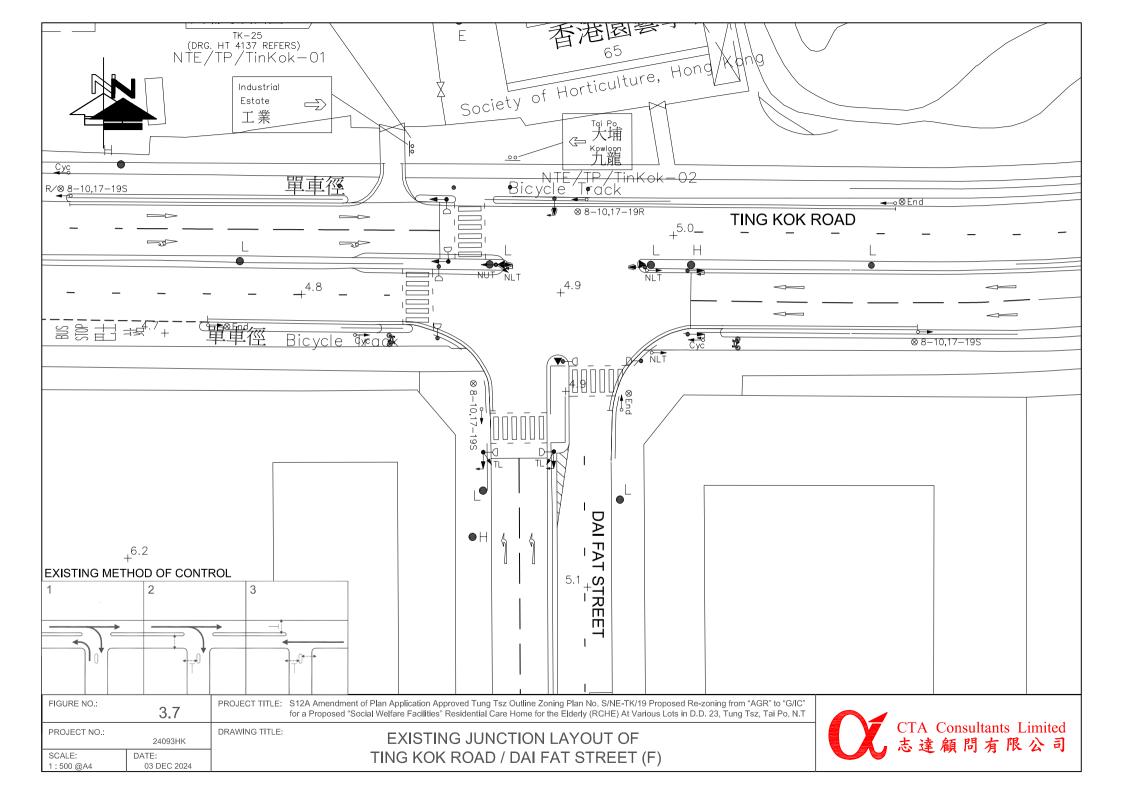


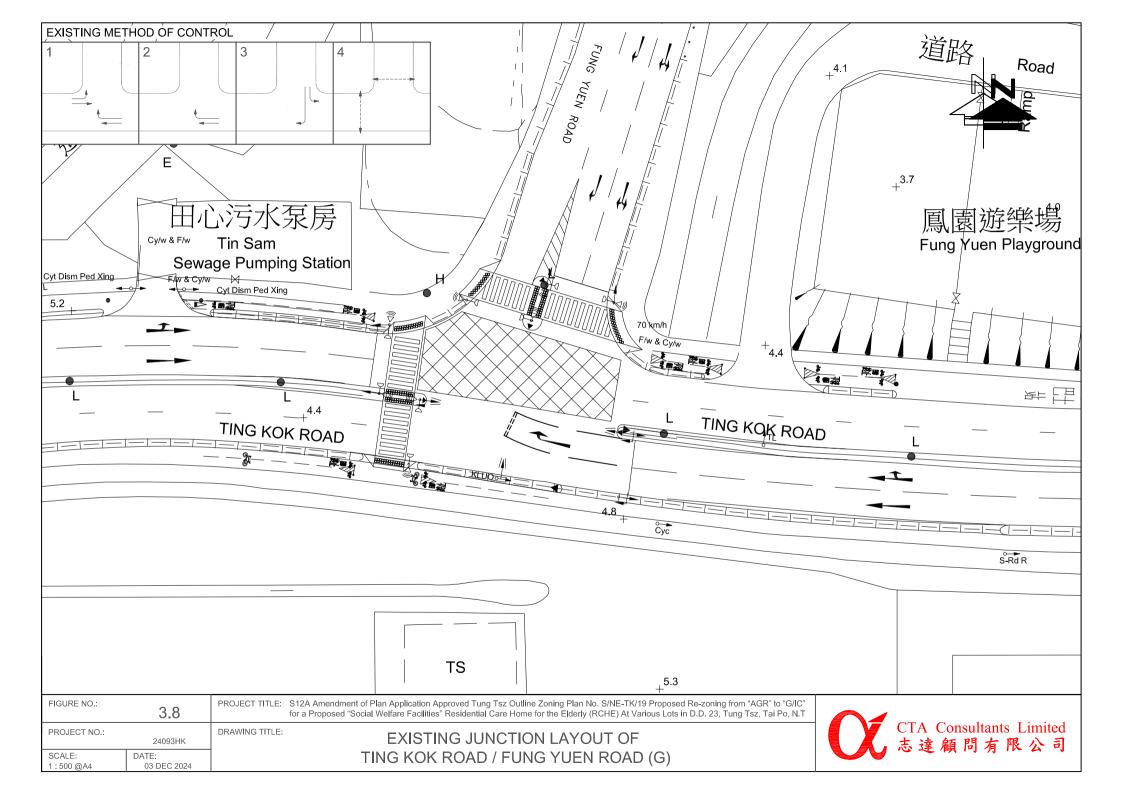


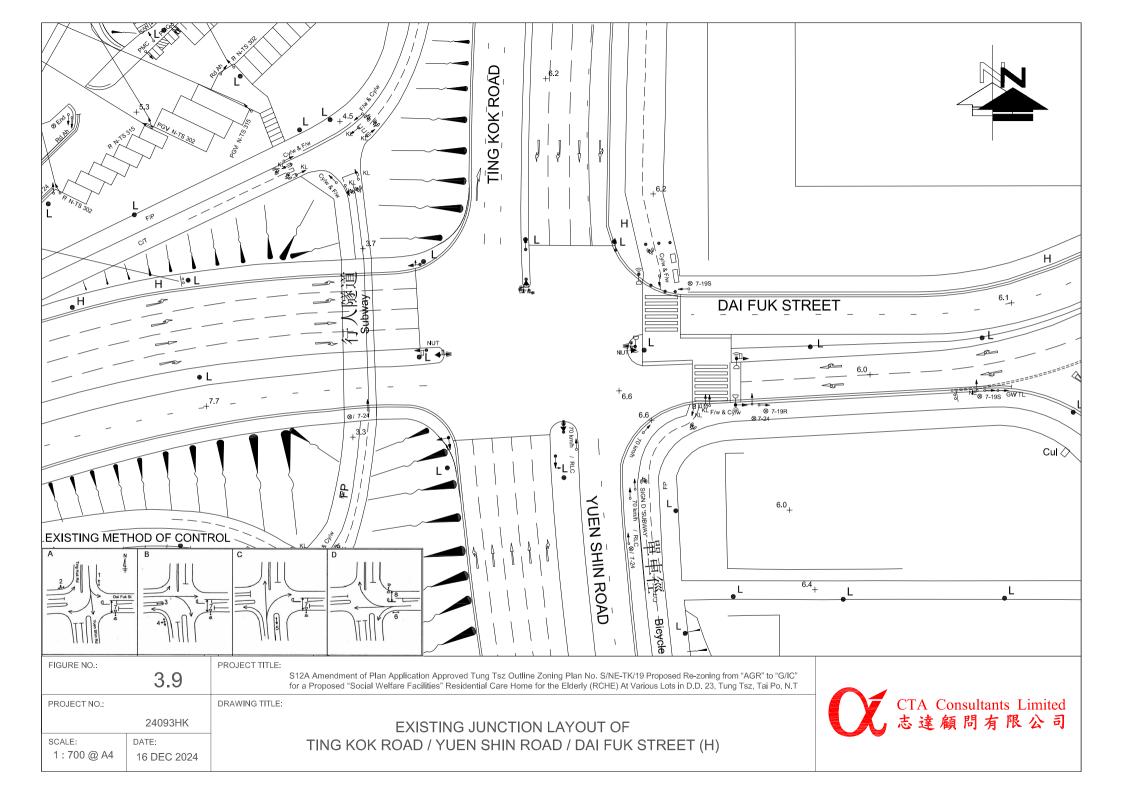


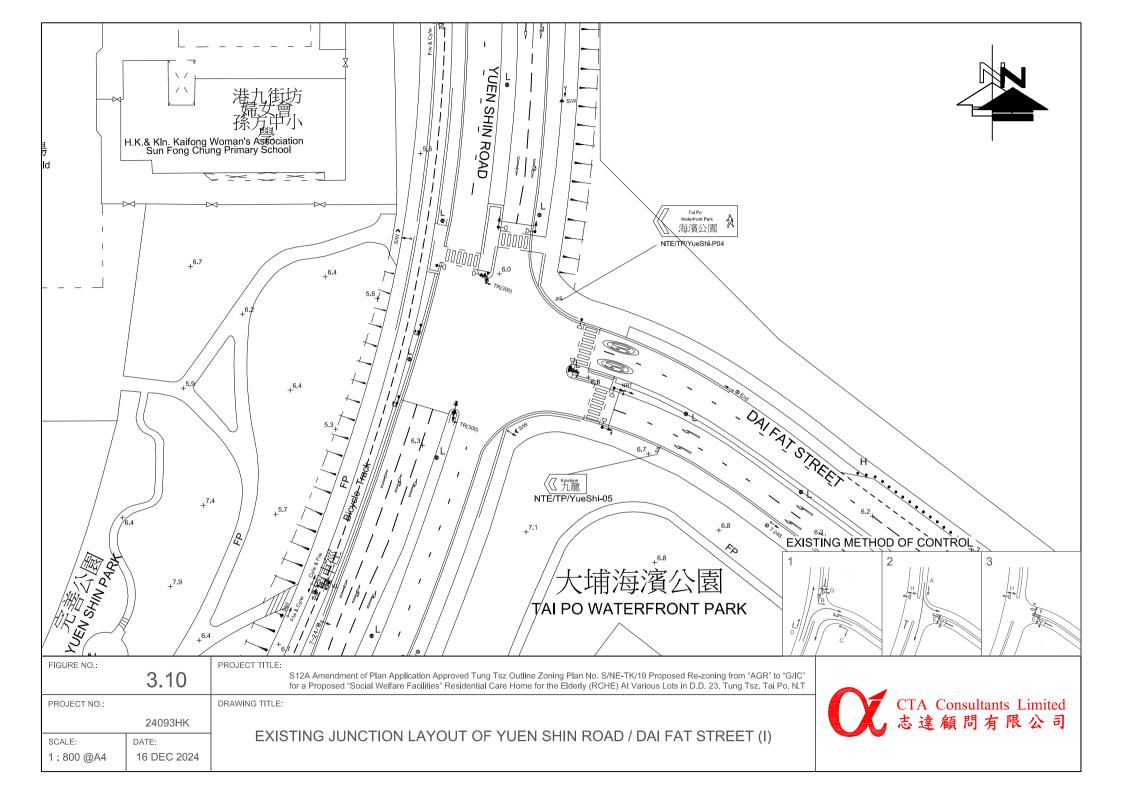


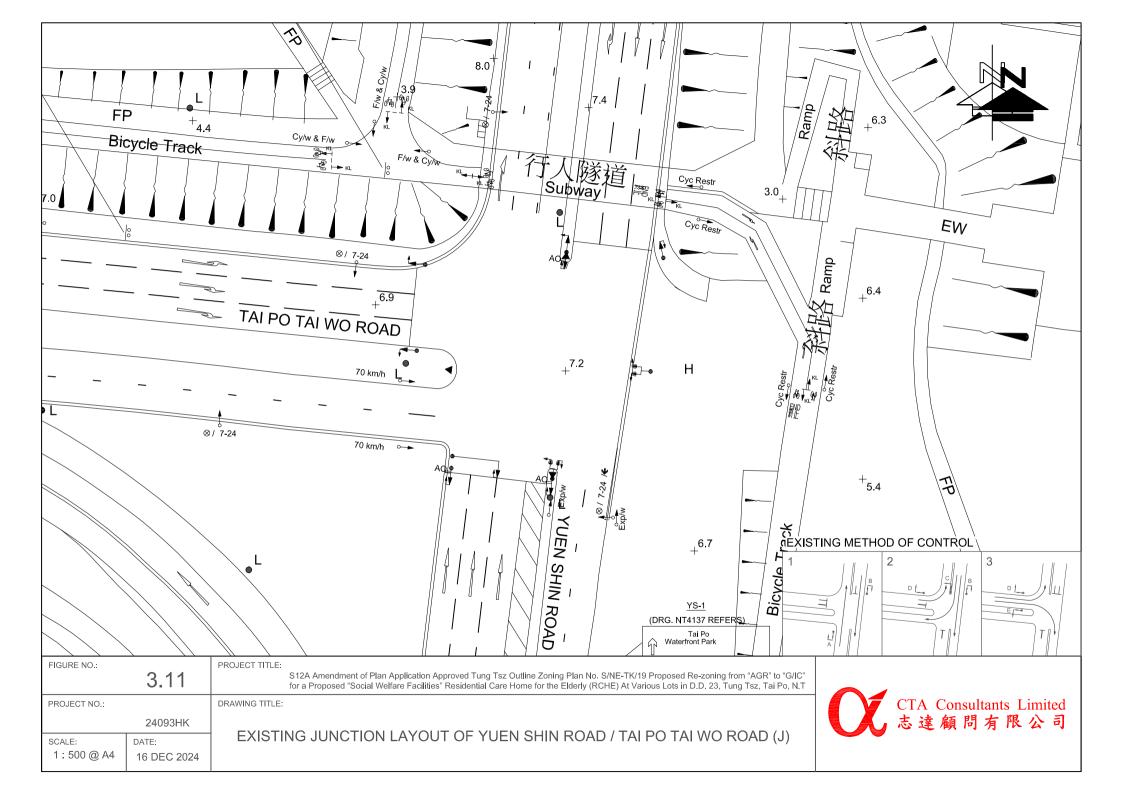


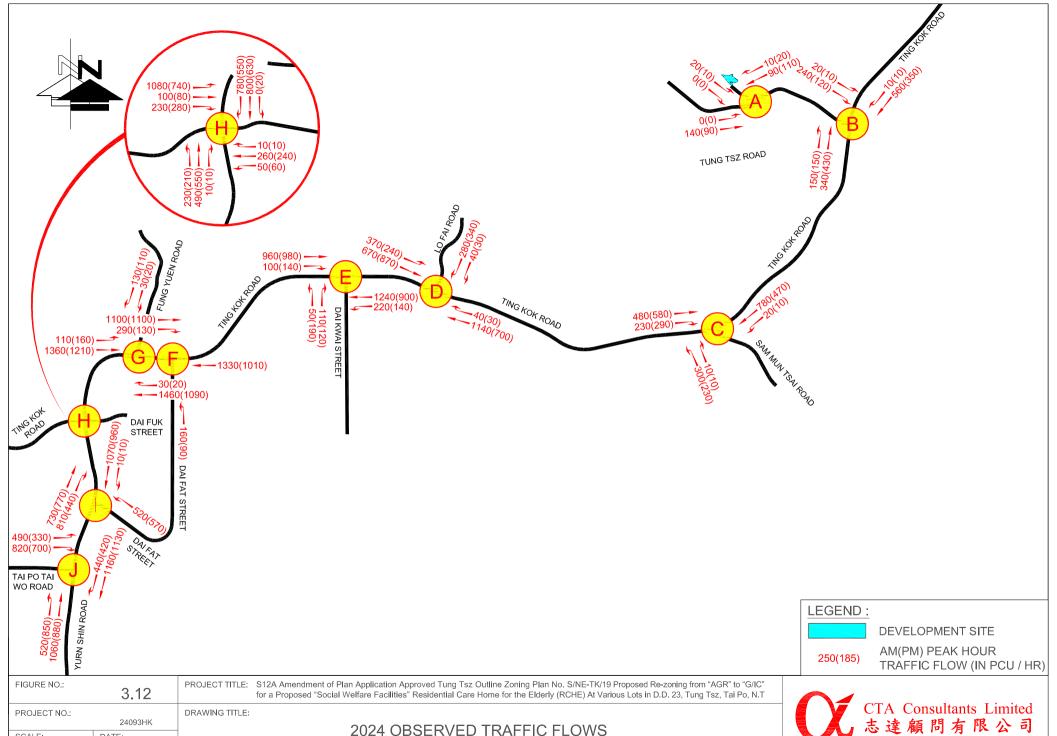










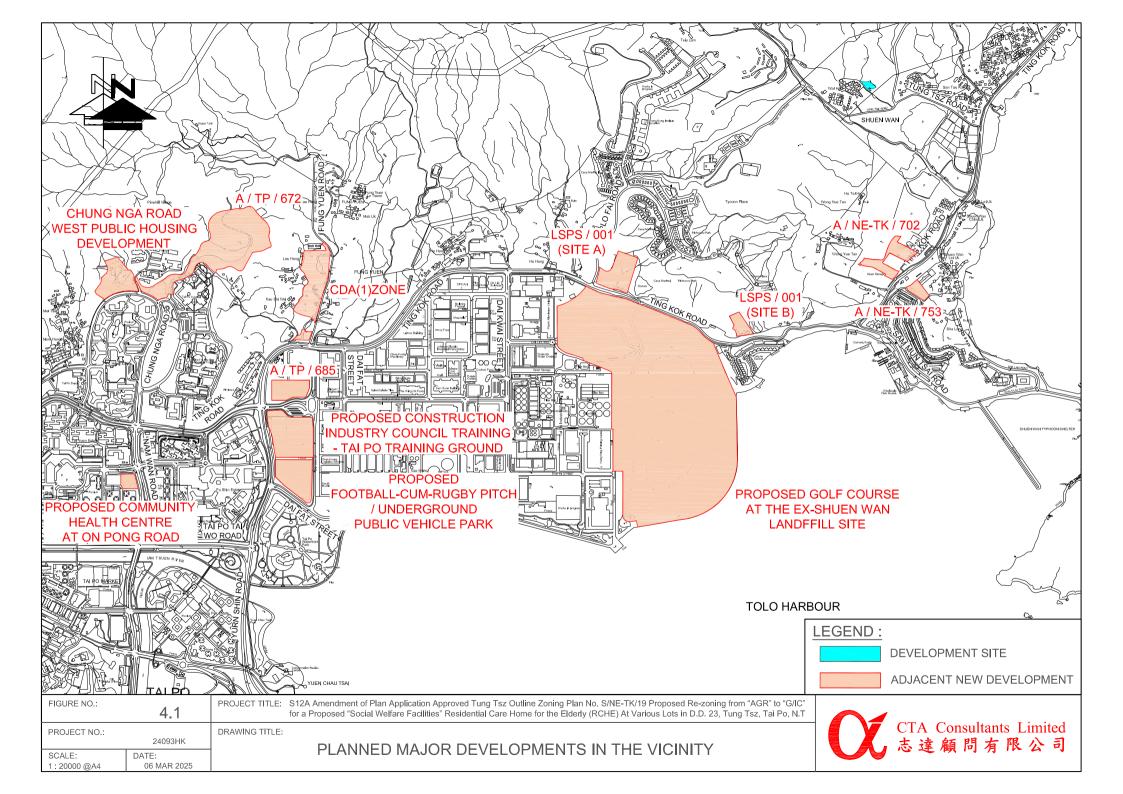


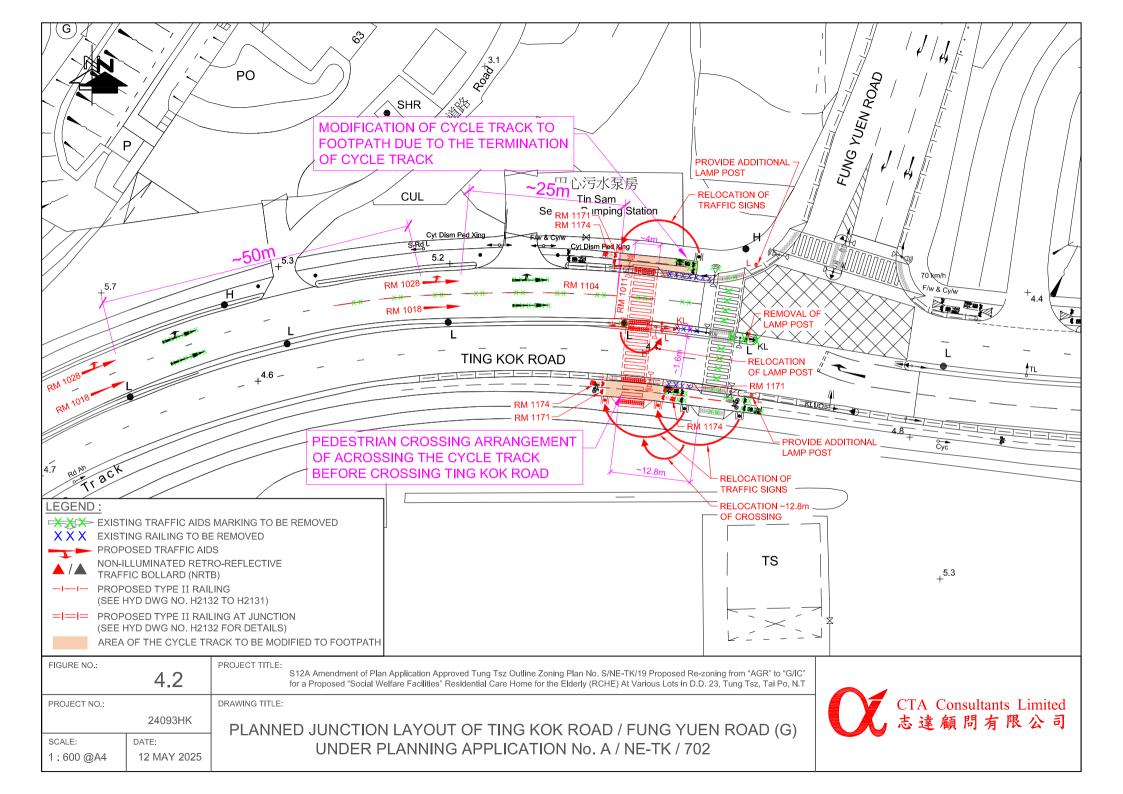
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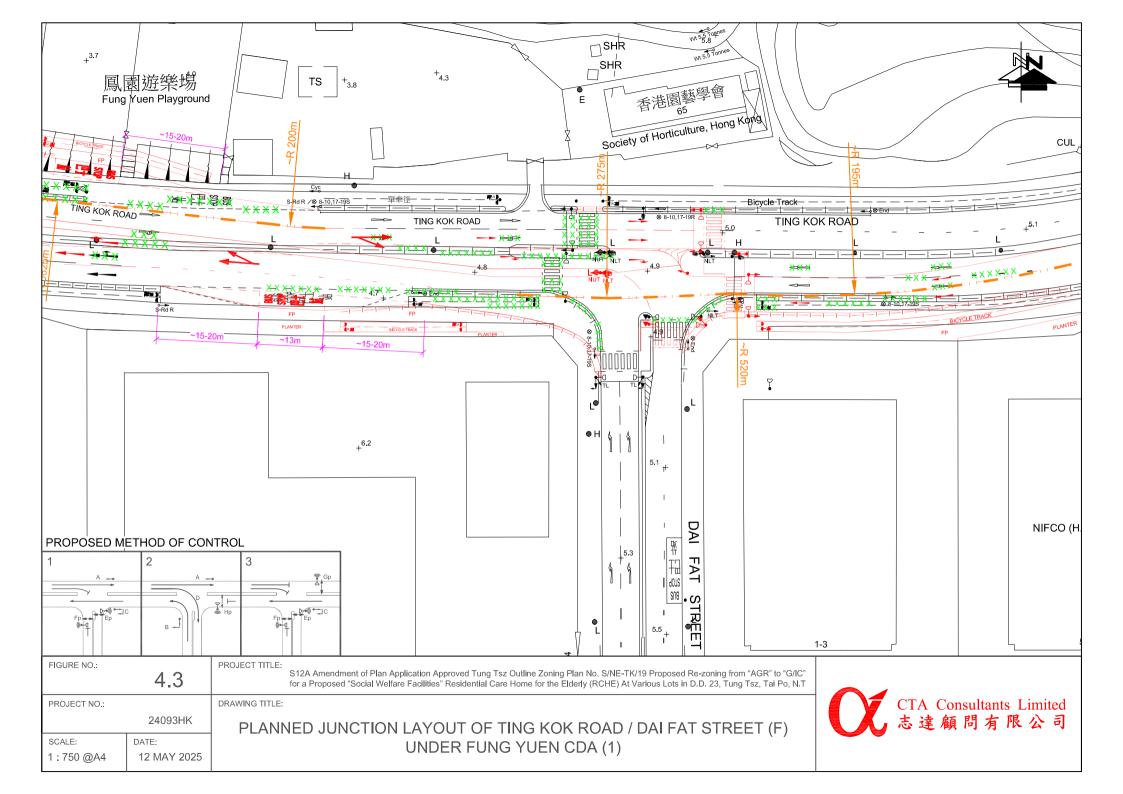
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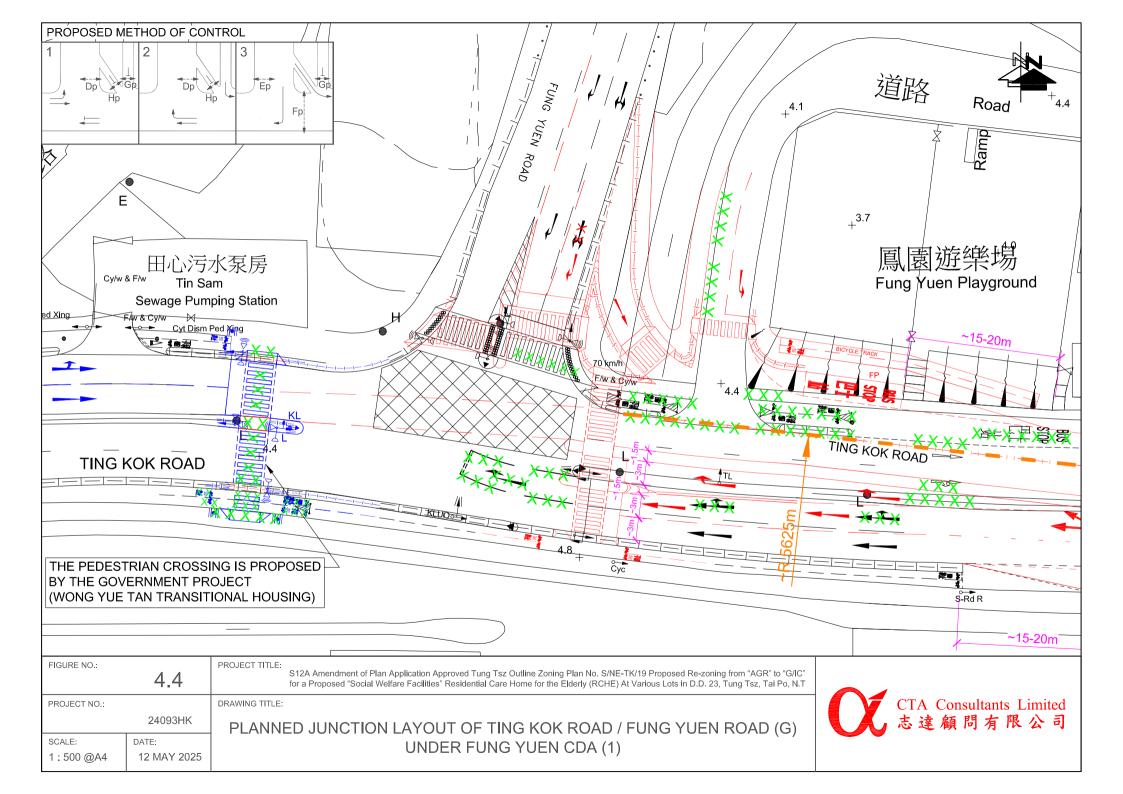
17 DEC 2024

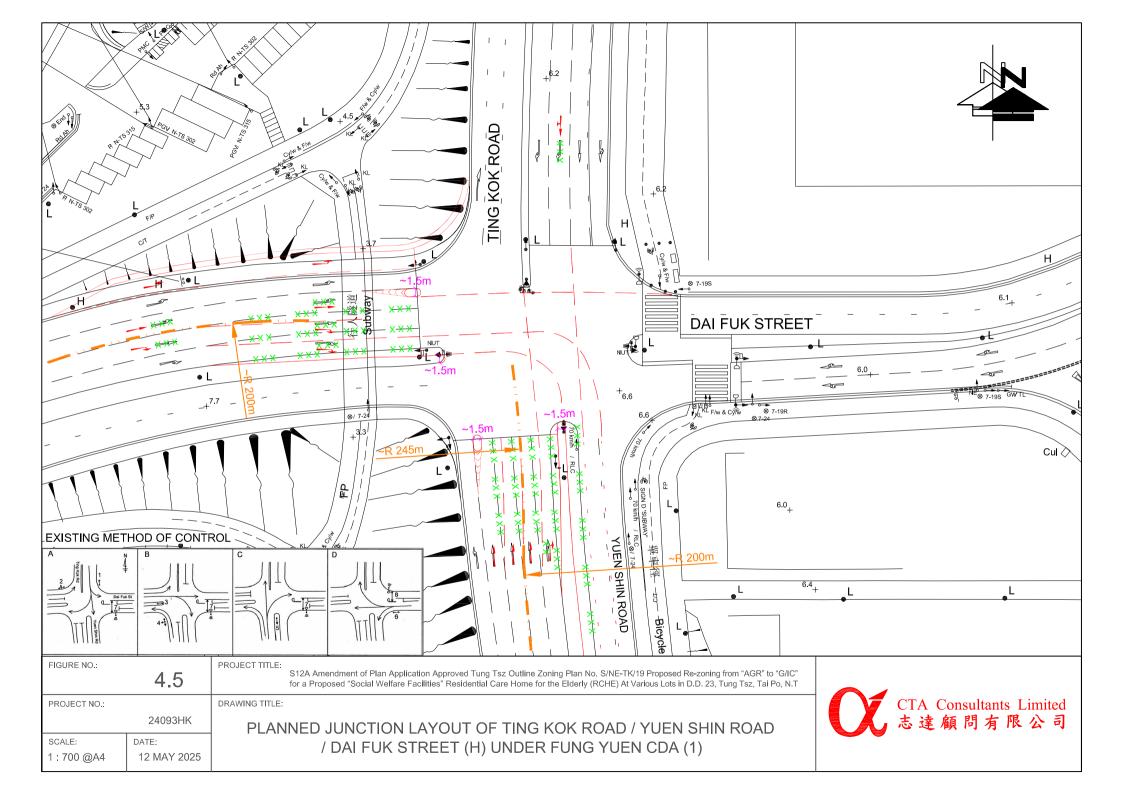


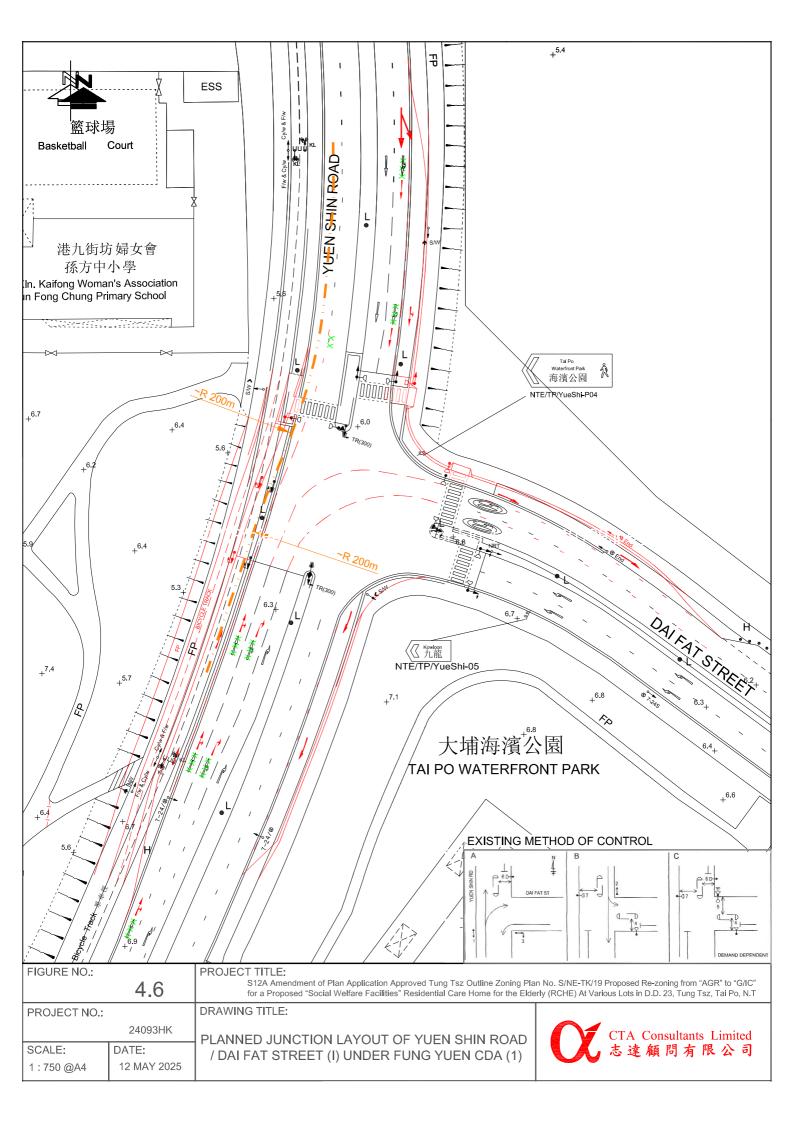


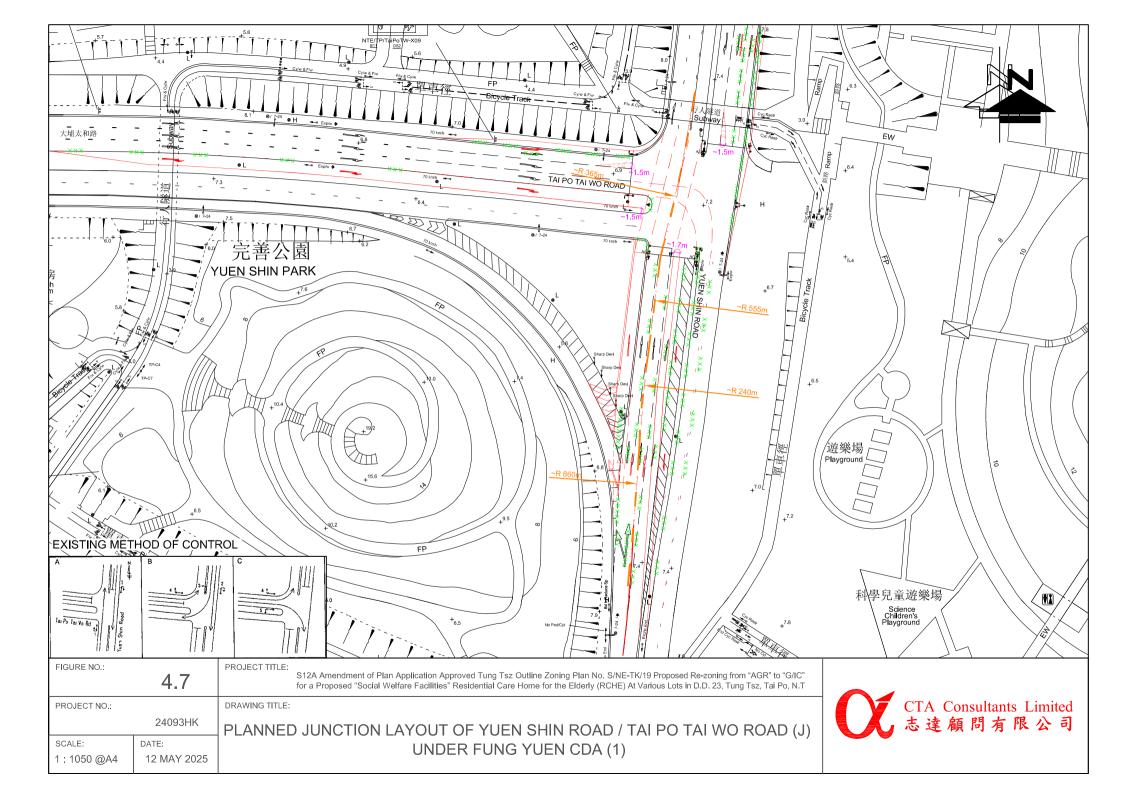


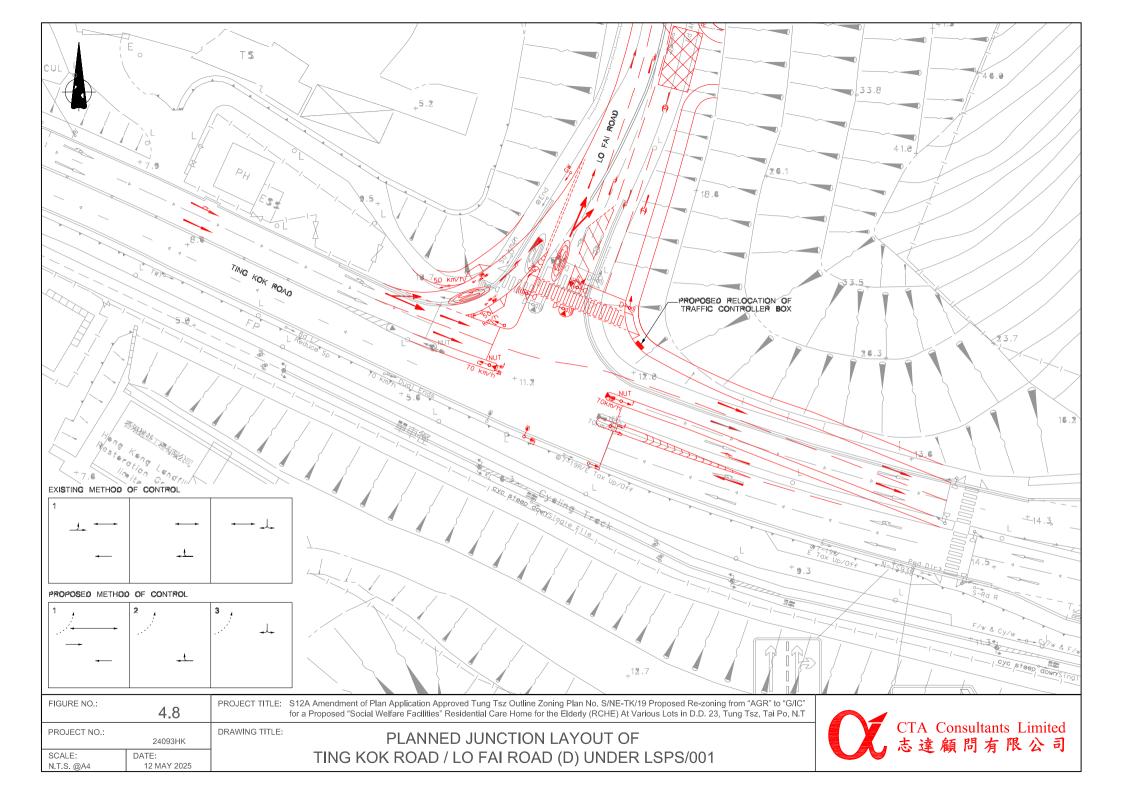


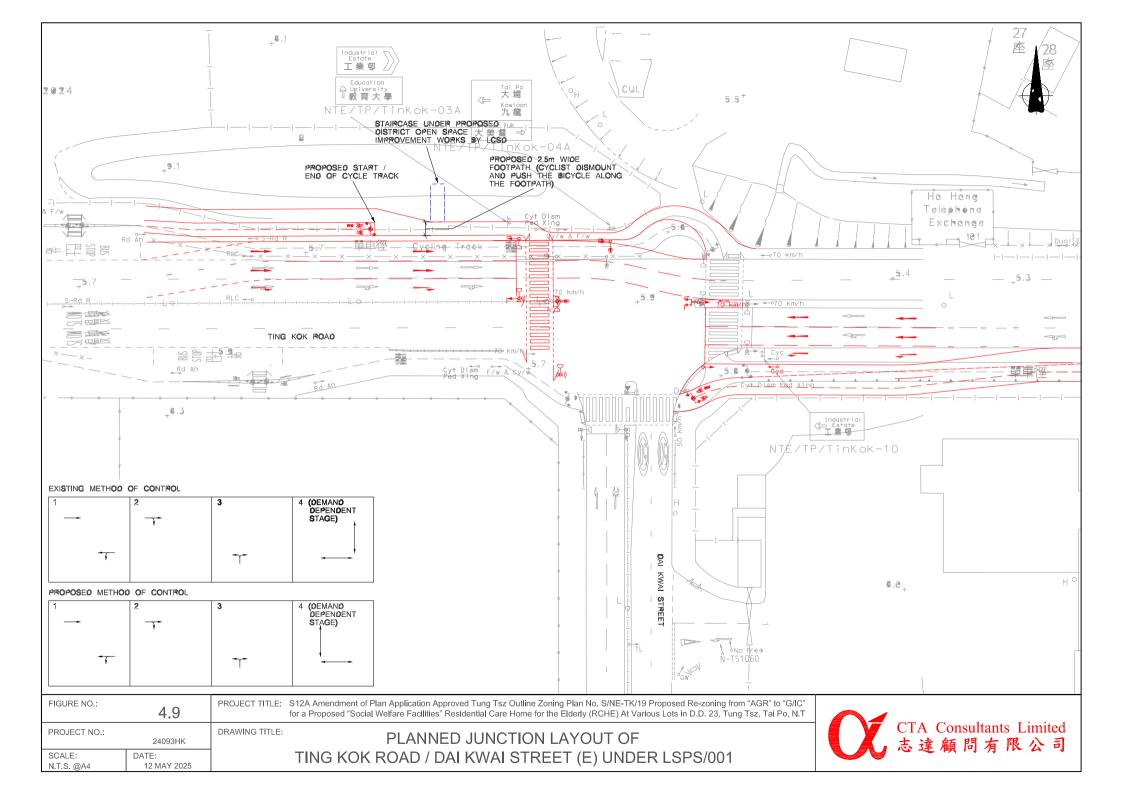


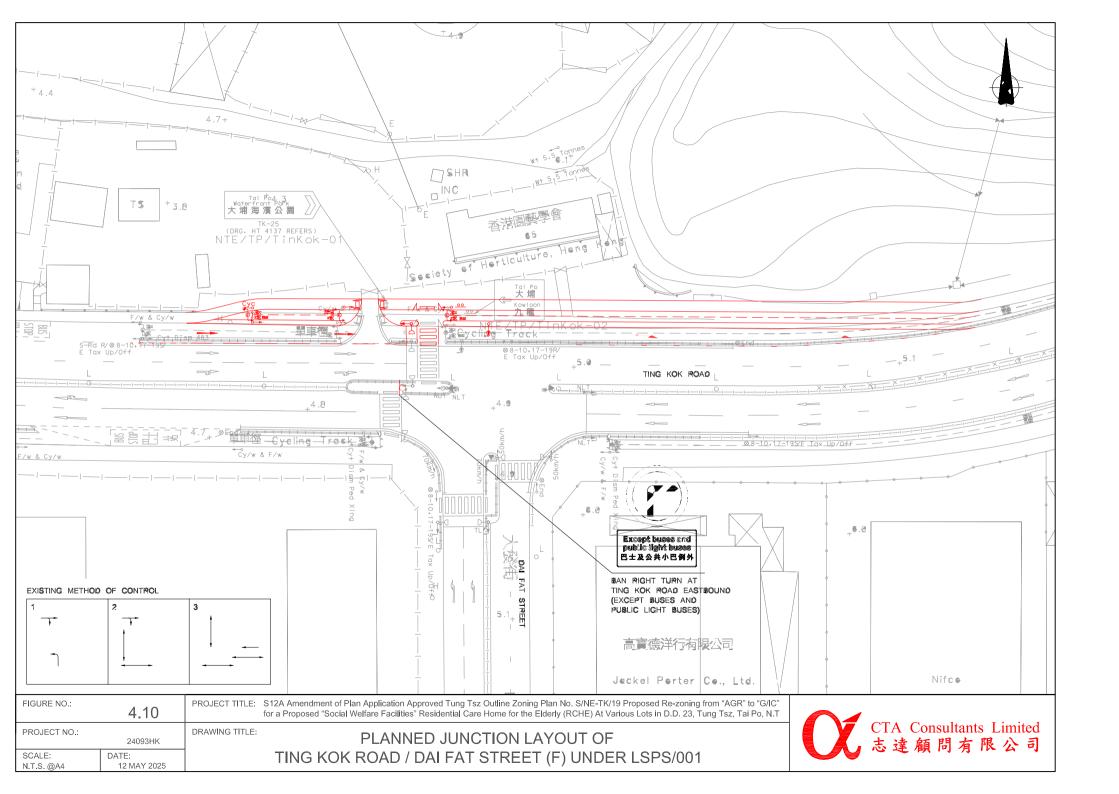


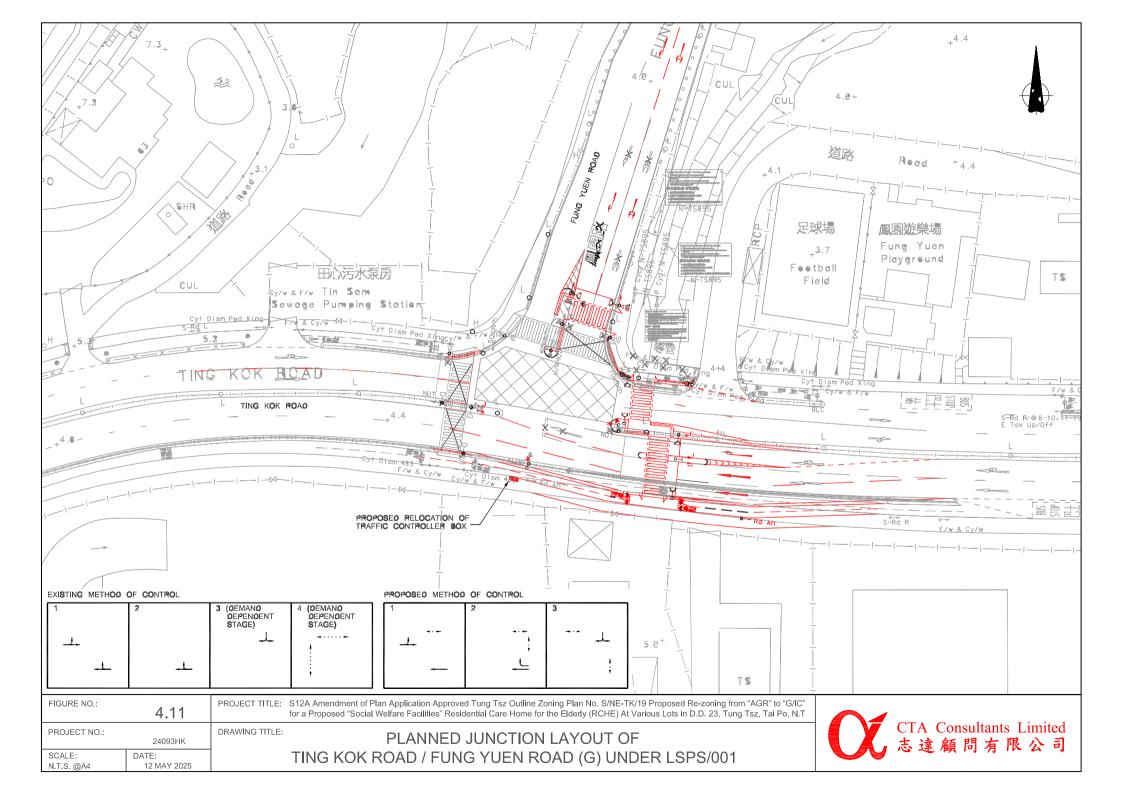


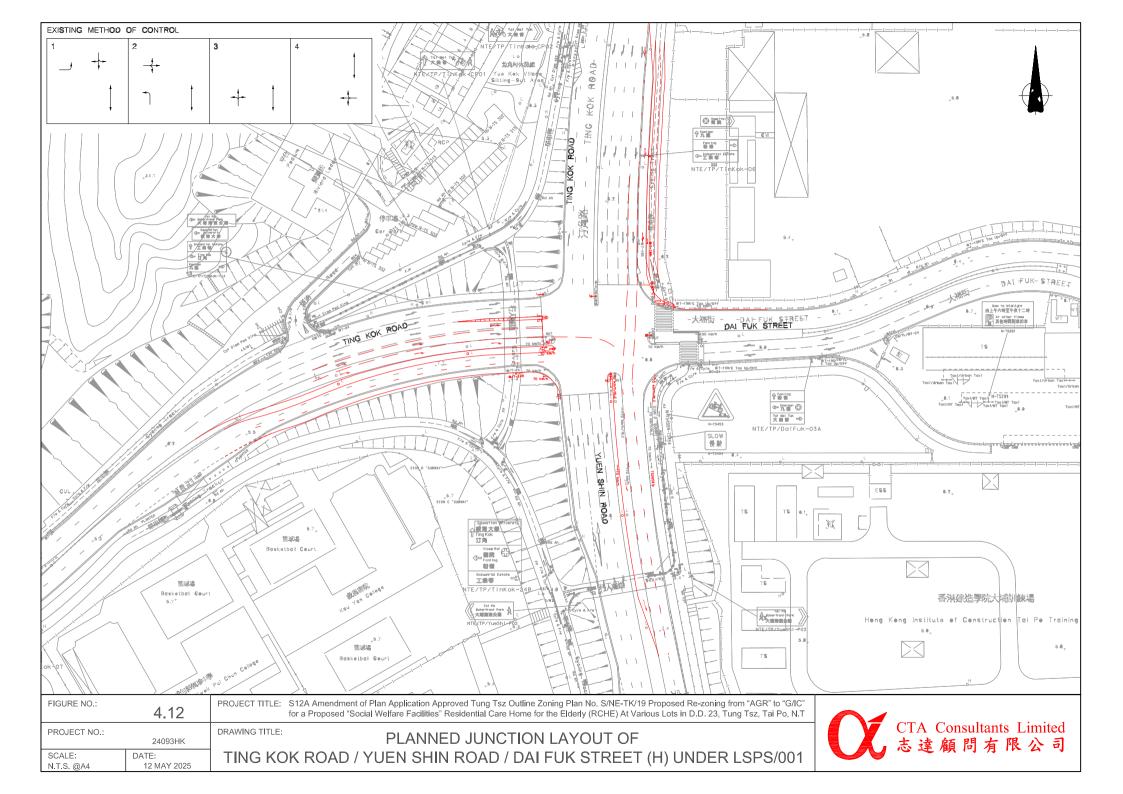


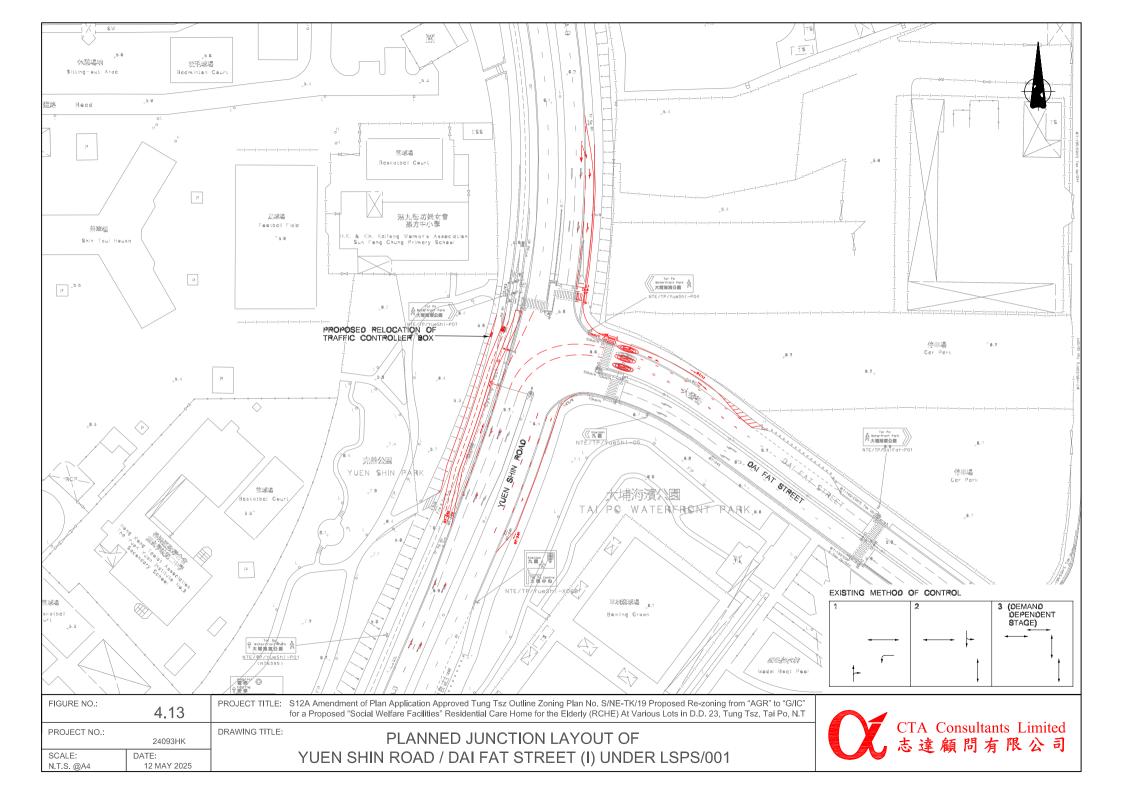


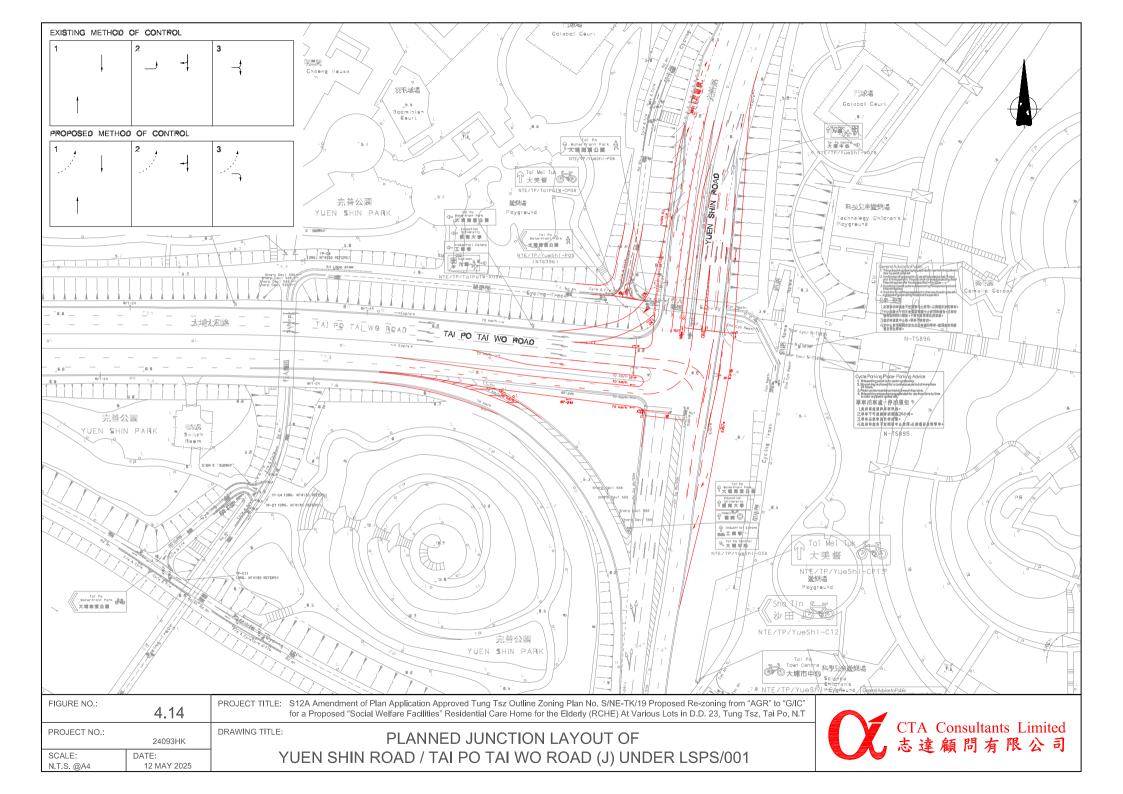


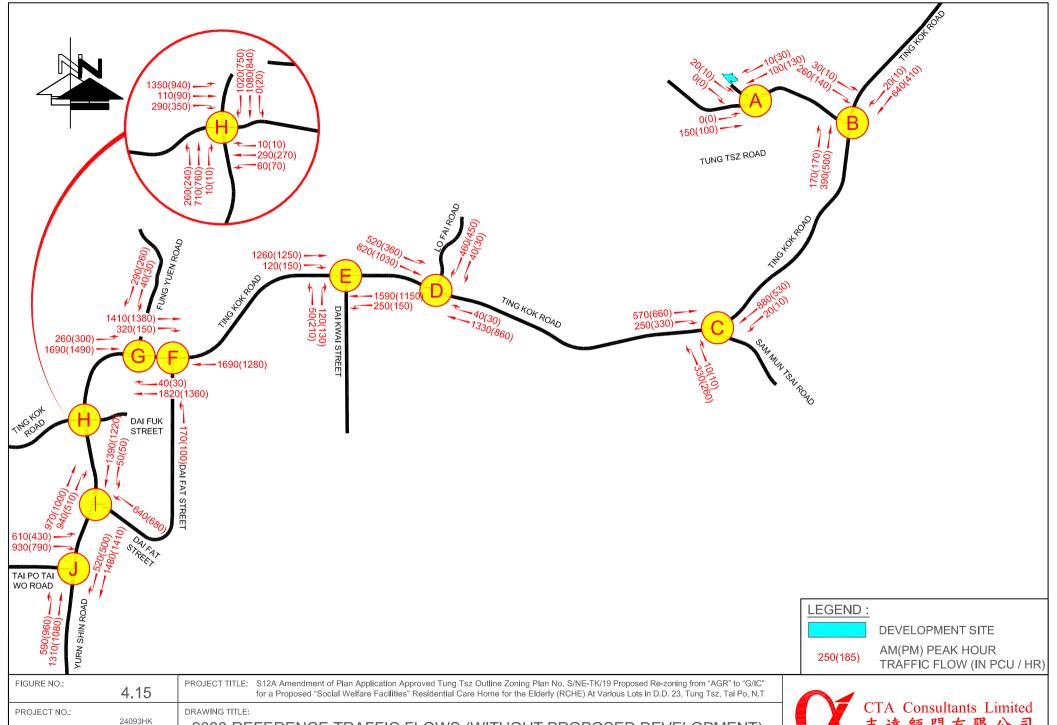












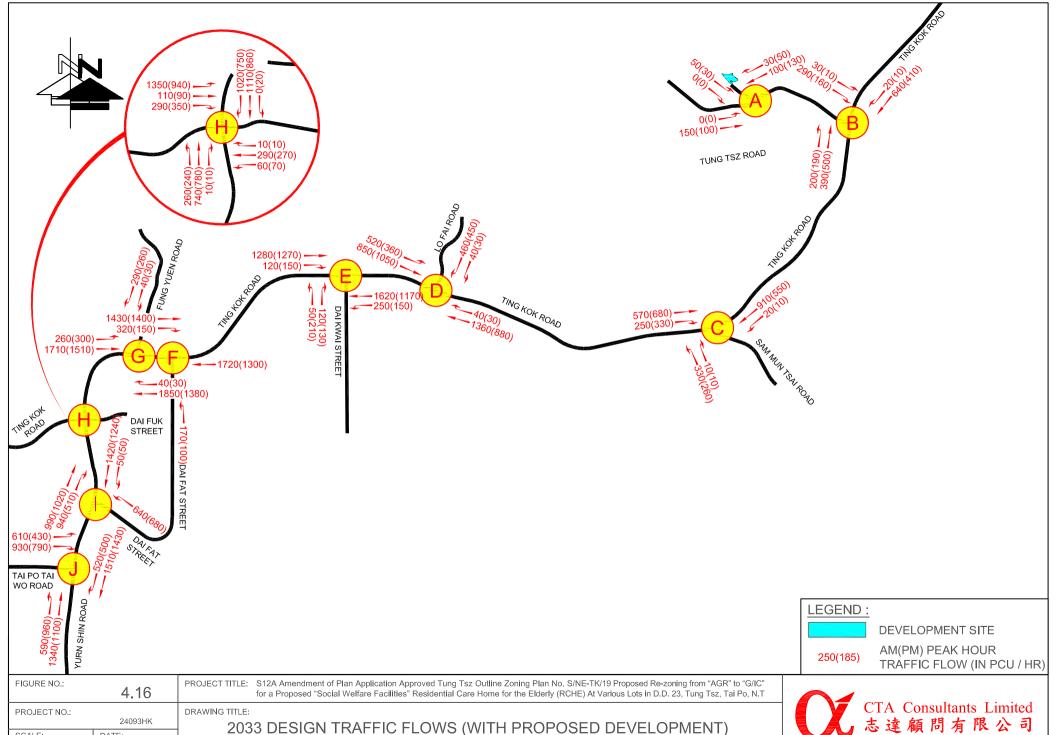
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N.T.S. @A4

14 MAY 2025

2033 REFERENCE TRAFFIC FLOWS (WITHOUT PROPOSED DEVELOPMENT)

CTA Consultants Limited 志達顧問有限公司

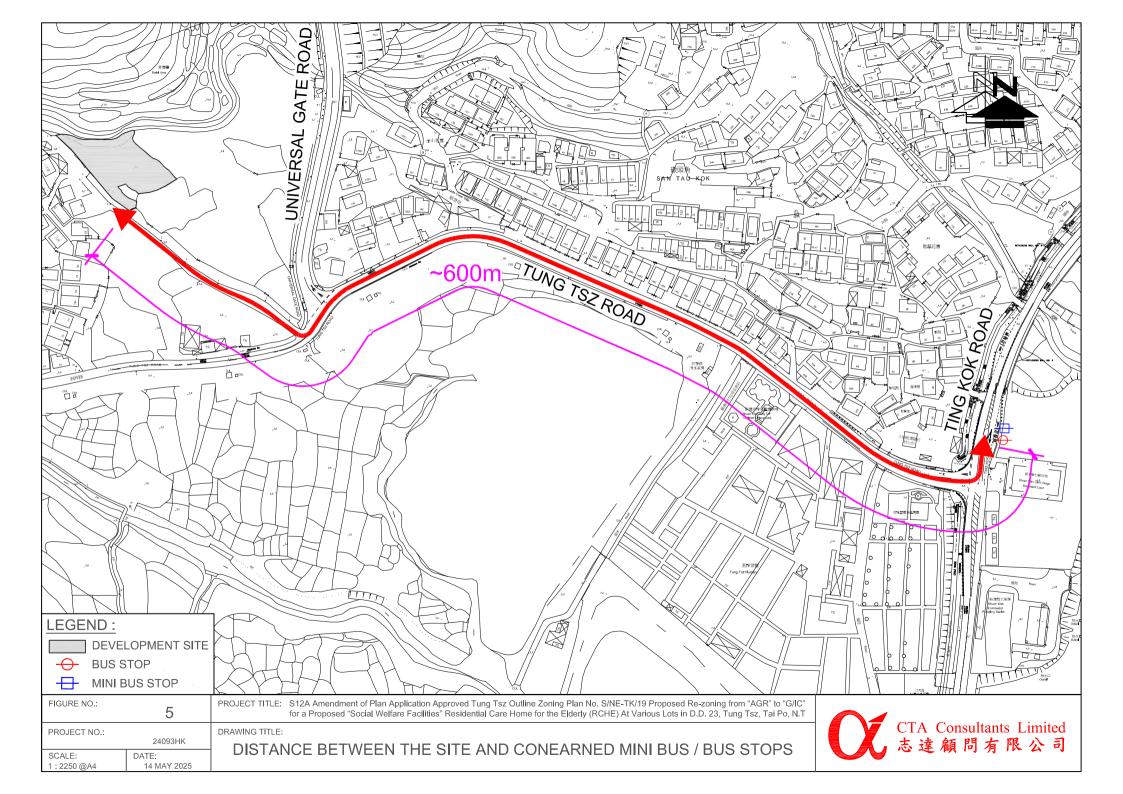


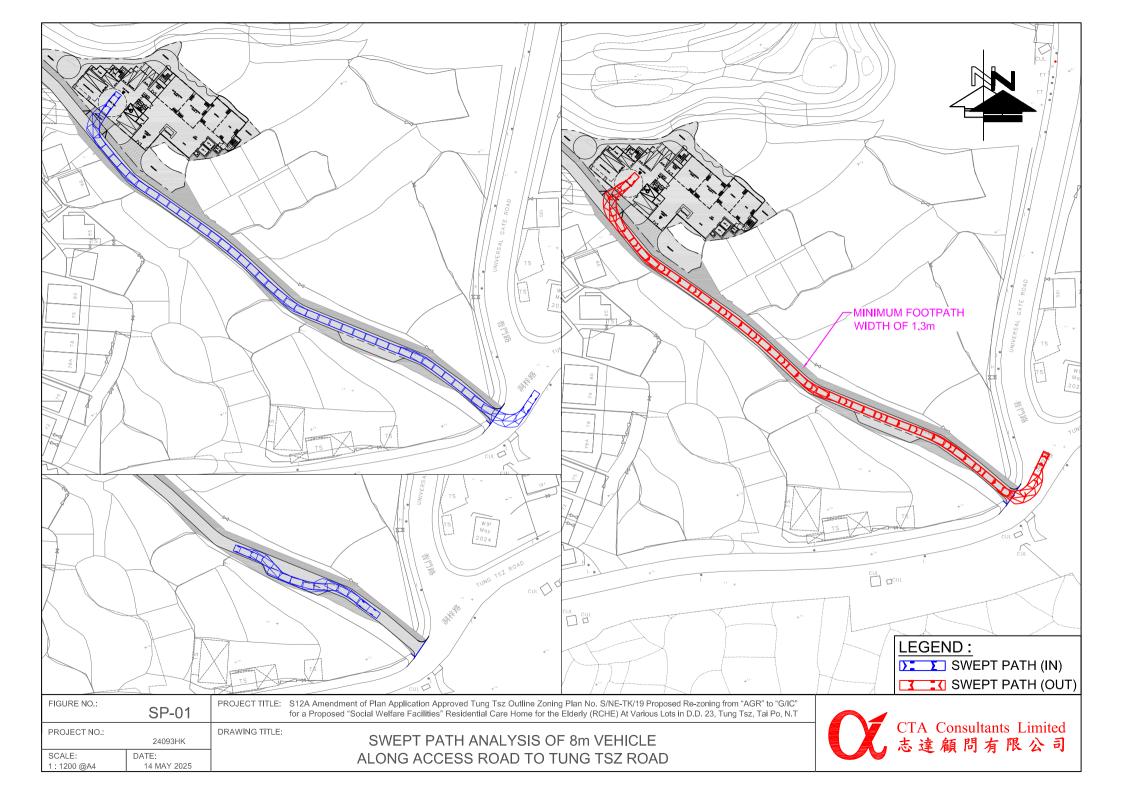
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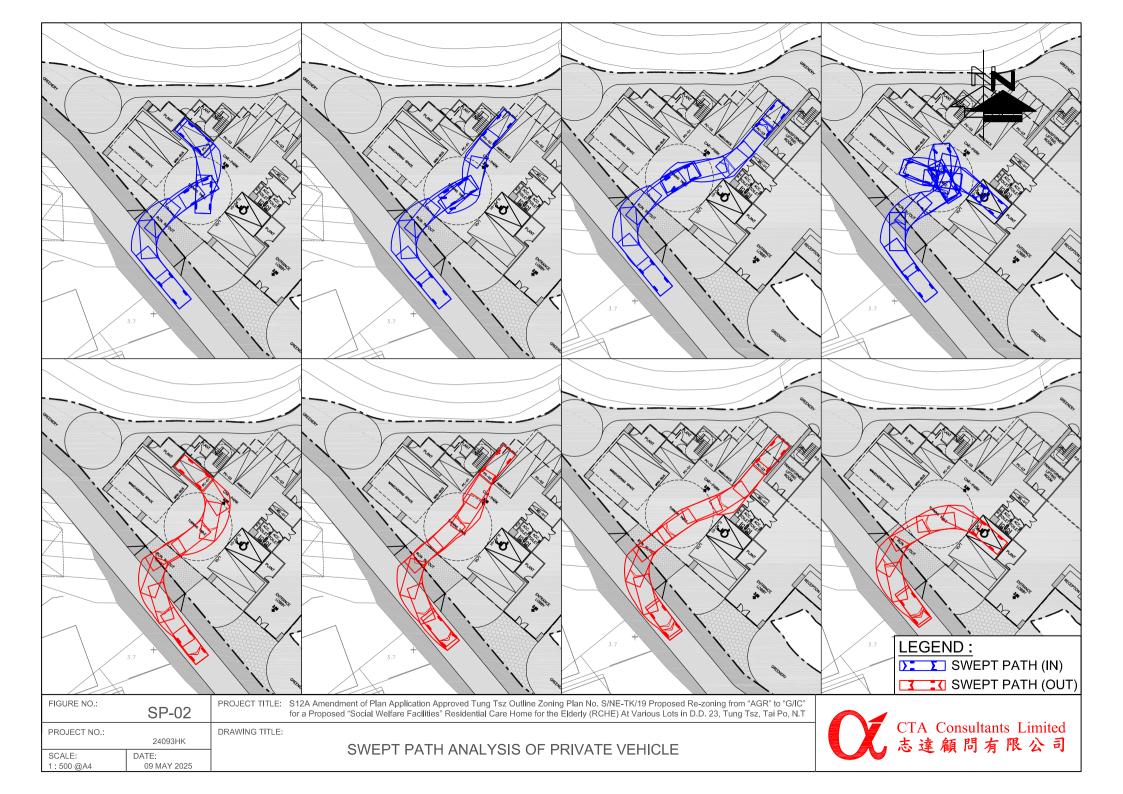
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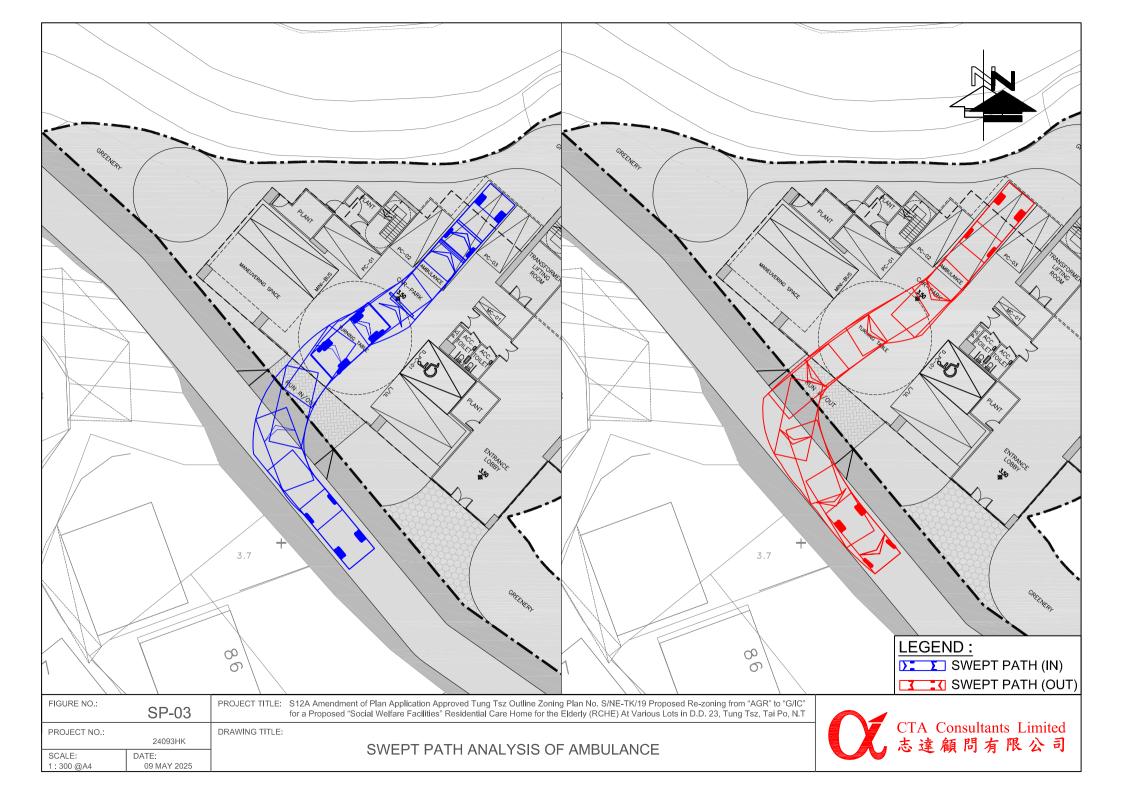
14 MAY 2025

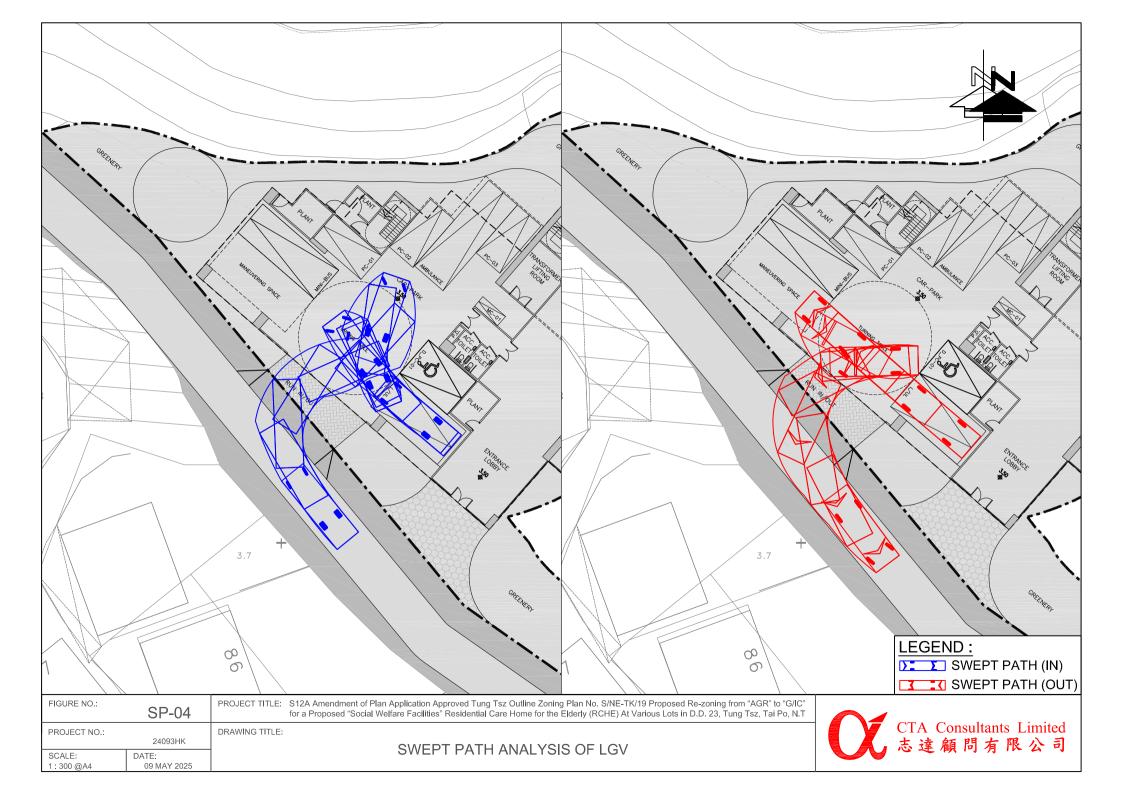
CTA Consultants Limited 志達顧問有限公司

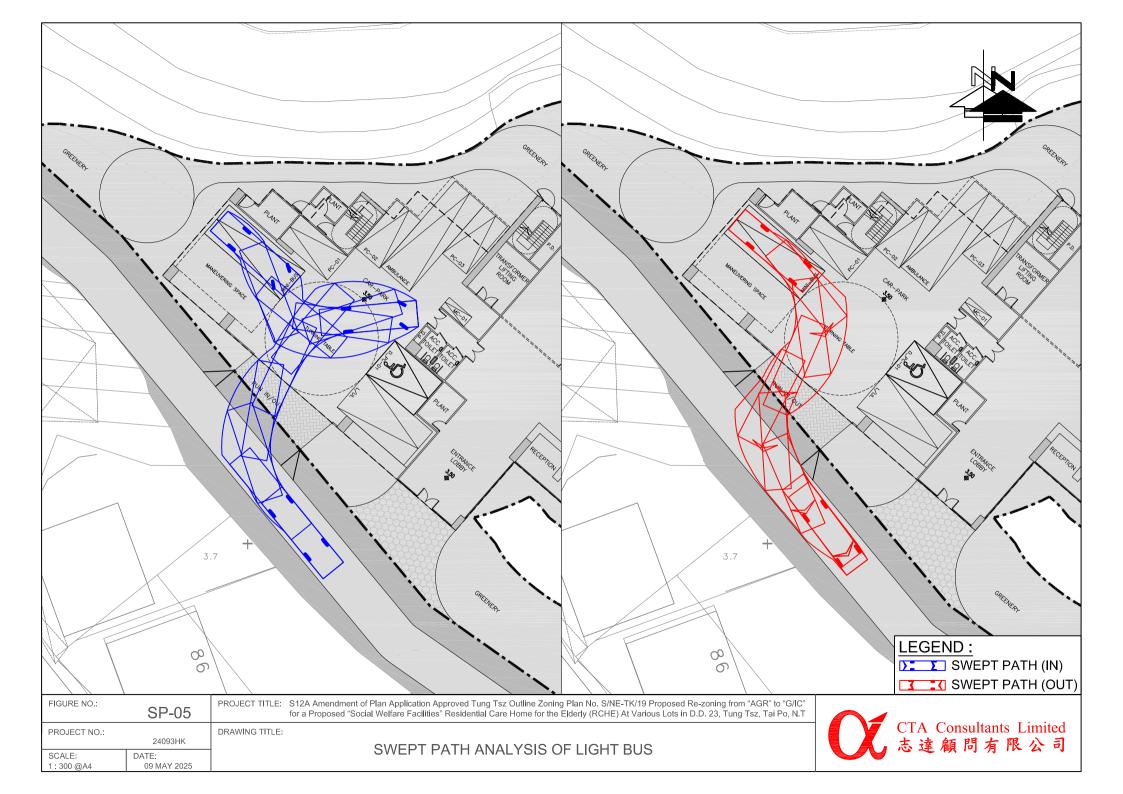
















S12A Amendment of Plan Application Approved Tung Tsz Outline Zoning Plan No. S/NE-TK/19 Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities" Residential Care Home for the Elderly (RCHE) At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

Revised TIA Report
We commit We deliver

APPENDIX A Junction Calculation Sheets

24093HK (May 2025) 32



Junctions 8

PICADY 8 - Priority Intersection Module

Version: 8.0.5.523 [19102,19/06/2015] © Copyright TRL Limited, 2025

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Filename: 24093 JnA.arc8

Path: \CTA_NAS01\Project\CTA Consultants Limited\CTA - Project\24093HK (knc) - S12A Re-zoning from AGR to GIC for a

Prop Social Welfare Facilities (RCHE) at Tung Tsz, Tai Po\Calculation\2025-05-07

Report generation date: 13/5/2025 16:50:19

- » Jn A Existing 2024, AM
- » Jn A Existing 2024, PM
- » Jn A Reference 2033, AM
- » Jn A Reference 2033, PM
- » Jn A Design 2033, AM
- » Jn A Design 2033, PM
- » Jn A SEN Design 2033, AM
- » Jn A SEN Design 2033, PM



Summary of junction performance

		AM				PM		
	Queue (PCU)	Delay (s)	RFC	LOS	Queue (PCU)	Delay (s)	RFC	LOS
			Jn A	- Des	sign 2033			
Stream B-AC	0.07	5.30	0.07	Α	0.04	5.04	0.04	Α
Stream C-AB	0.06	6.64	0.05	Α	0.10	6.71	0.09	Α
Stream C-A	-	-	-	-	_	-	-	-
Stream A-B	-	-	-	-		-	-	-
Stream A-C			-	-	-	-	-	-
			Jn A	- Exis	sting 2024			
Stream B-AC	0.03	5.05	0.03	Α	0.01	4.88	0.01	Α
Stream C-AB	0.02	6.41	0.02	Α	0.04	6.37	0.03	Α
Stream C-A	-	-	-	-	_	-	-	-
Stream A-B	-	-	-	-		-	-	-
Stream A-C	-	-	-	-	-	-	-	-
		J	n A -	Refe	rence 2033			
Stream B-AC	0.03	5.07	0.03	Α	0.01	4.90	0.01	Α
Stream C-AB	0.02	6.43	0.02	Α	0.06	6.50	0.05	Α
Stream C-A	-	-	-	-		-	-	-
Stream A-B	-	-	-	-		-	-	-
Stream A-C	-	-	-	-	-	-	-	-
		Jr	n A - S	SEN [Design 2033			
Stream B-AC	0.07	5.30	0.07	Α	0.04	5.04	0.04	Α
Stream C-AB	0.06	6.64	0.05	Α	0.10	6.71	0.09	Α
Stream C-A	-	-	-	-	-	-	-	-
Stream A-B	-	-	-	-	-	-	-	-
Stream A-C	-	-	-	-	-	-	-	-

Values shown are the maximum values over all time segments. Delay is the maximum value of average delay per arriving vehicle.

Run using Junctions 8.0.5.523 at 13/5/2025 16:50:14

File summary

Title	(untitled)
Location	
Site Number	
Date	7/6/2021
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	user
Description	

[&]quot;D1 - Existing 2024, AM " model duration: 8:00 - 9:30 "D2 - Existing 2024, PM" model duration: 8:00 - 9:30

[&]quot;D3 - Reference 2033, AM" model duration: 8:00 - 9:30

[&]quot;D4 - Reference 2033, PM" model duration: 8:00 - 9:30 "D5 - Design 2033, AM" model duration: 8:00 - 9:30

[&]quot;D6 - Design 2033, PM" model duration: 8:00 - 9:30

[&]quot;D7 - SEN Design 2033, AM" model duration: 8:00 - 9:30

[&]quot;D8 - SEN Design 2033, PM" model duration: 8:00 - 9:30

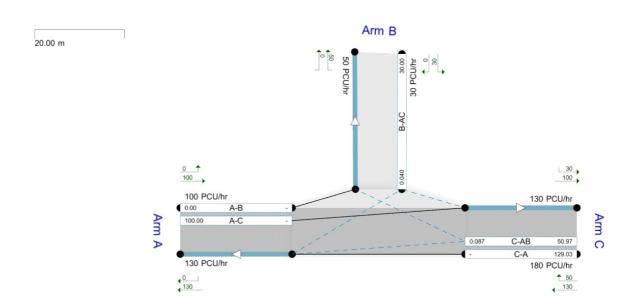


Analysis Options

Vehicle Length (m)	Do Queue Variations	Calculate Residual Capacity	Residual Capacity Criteria Type	RFC Threshold	Average Delay Threshold (s)	Queue Threshold (PCU)	
5.75			N/A	0.85	36.00	20.00	

Units

Distance Units	Speed Units	Traffic Units Input	Traffic Units Results	Flow Units	Average Delay Units	Total Delay Units	Rate Of Delay Units
m	kph	PCU	PCU	perHour	S	-Min	perMin



Steams (upstream) show Total Demand (PCUhr); Streams (downstreams) show RFC ()
Time Segment: (08:00-08:15)
Showing Analysis Set "A1 - Jn A "; Demand Set "D1 - Existing 2024, AM "

The junction diagram reflects the last run of ARCADY.



Jn A - Existing 2024, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Existing 2024, AM	Existing 2024	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type Major Road Direction		Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	5.50	А

Junction Network Options

Driving Side						
Left	Normal/unknown					

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

Ar	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central Has right reserve (m) turn bay		Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
(6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50



Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B	
1	B-A	611.947	0.109	0.274	0.173	0.392	
1	B-C	773.526	0.115	0.292	-	-	
1	С-В	602.919	0.228	0.228	-	-	

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	140.00	100.000
В	FLAT	✓	20.00	100.000
С	FLAT	✓	100.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.000	0.000	140.000		
1 10111	В	0.000	0.000	20.000		
	C	90.000	10.000	0.000		

Turning Proportions (PCU) - Junction 1 (for whole period)

				•
		1	Го	·
		Α	В	С
From	Α	0.00	0.00	1.00
FIOIII	В	0.00	0.00	1.00
	С	0.90	0.10	0.00



Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

	То				
		Α	В	С	
From	Α	1.000	1.000	1.000	
10111	В	1.000	1.000	1.000	
	С	1.000	1.000	1.000	

Heavy Vehicle Percentages - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.0	0.0	0.0				
1 10111	В	0.0	0.0	0.0				
	С	0.0	0.0	0.0				

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.03	5.05	0.03	Α
C-AB	0.02	6.41	0.02	Α
C-A	-	-	-	-
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	19.89	0.00	732.66	0.027	0.03	5.050	Α
C-AB	10.03	9.96	0.00	572.14	0.018	0.02	6.403	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	732.66	0.027	0.03	5.050	Α
C-AB	10.03	10.03	0.00	572.14	0.018	0.02	6.403	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-



Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	732.66	0.027	0.03	5.050	Α
C-AB	10.03	10.03	0.00	572.14	0.018	0.02	6.403	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	732.66	0.027	0.03	5.052	Α
C-AB	10.03	10.03	0.00	572.14	0.018	0.02	6.403	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	732.66	0.027	0.03	5.052	Α
C-AB	10.03	10.03	0.00	572.14	0.018	0.02	6.406	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	732.66	0.027	0.03	5.052	Α
C-AB	10.03	10.03	0.00	572.14	0.018	0.02	6.406	Α
C-A	89.97	89.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	140.00	140.00	0.00	-	-	-	-	-

Jn A - Existing 2024, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Na	ame	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
J	n A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Existing 2024, PM	Existing 2024	PM		FLAT	08:00	09:30	90	15		



Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	5.88	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

Ar	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
C	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	C-B	602.919	0.228	0.228		-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type Use Turning Counts		Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)		
Α	FLAT	✓	90.00	100.000		
В	FLAT ✓		10.00	100.000		
С	FLAT	✓	130.00	100.000		

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То								
		Α	В	С						
From	Α	0.000	0.000	90.000						
FIOIII	В	0.000	0.000	10.000						
	O	110.000	20.000	0.000						

Turning Proportions (PCU) - Junction 1 (for whole period)

		То							
		Α	В	С					
From	Α	0.00	0.00	1.00					
From	В	0.00	0.00	1.00					
	С	0.85	0.15	0.00					

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
From		Α	С	
	Α	1.000	1.000	1.000
	В	1.000	1.000	1.000
	С	1.000	1.000	1.000

ξ



Heavy Vehicle Percentages - Junction 1 (for whole period)

		Т	o	
From		Α	В	С
	Α	0.0	0.0	0.0
	В	0.0	0.0	0.0
	С	0.0	0.0	0.0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.01	4.88	0.01	Α
C-AB 0.03		6.37	0.04	Α
C-A	C-A -		-	-
A-B -		-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	9.95	0.00	747.26		0.01	4.882	Α
C-AB	20.13	19.99	0.00	584.99	0.034	0.04	6.369	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	747.26	0.013	0.01	4.882	Α
C-AB	20.13	20.13	0.00	584.99 0.034		0.04	6.372	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	747.26	0.013	0.01	4.882	Α
C-AB	20.13	20.13	0.00	584.99	0.034	0.04	6.374	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-



Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	747.26	0.013	0.01	4.882	Α
C-AB	20.13	20.13	0.00	584.99 0.03		0.04	6.372	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	747.26	0.013	0.01	4.882	Α
C-AB	20.13	20.13	0.00	584.99	0.034	0.04	6.374	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	747.26	0.013	0.01	4.882	Α
C-AB	20.13	20.13	0.00	584.99	0.034	0.04	6.374	Α
C-A	109.87	109.87	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	90.00	90.00	0.00	-	-	-	-	-

Jn A - Reference 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors		
Jn A	N/A			100.000			

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Reference 2033, AM	Reference 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	5.52	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
С	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	C-B	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	√	HV Percentages	2.00				√	✓



Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	150.00	100.000
В	FLAT	✓	20.00	100.000
С	FLAT	✓	110.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

	То								
		Α	В	С					
From	Α	0.000	0.000	150.000					
FIOIII	В	0.000	0.000	20.000					
	O	100.000	10.000	0.000					

Turning Proportions (PCU) - Junction 1 (for whole period)

	То							
		Α	В	С				
From	Α	0.00	0.00	1.00				
FIOIII	В	0.00	0.00	1.00				
	C	0.91	0.09	0.00				

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	1.000	1.000	1.000			
FIOIII	В	1.000	1.000	1.000			
	С	1.000	1.000	1.000			

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.0	0.0	0.0		
FIOIII	В	0.0	0.0	0.0		
	С	0.0	0.0	0.0		



Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC 0.03		5.07	0.03	Α
C-AB 0.02		6.43	0.02	Α
C-A -		-	-	-
А-В -		-	-	-
A-C -		-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	19.89	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	9.96	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	10.03	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	10.03	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	10.03	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-



Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	10.03	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	20.00	20.00	0.00	729.74	0.027	0.03	5.071	Α
C-AB	10.03	10.03	0.00	569.99	0.018	0.02	6.428	Α
C-A	99.97	99.97	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Jn A - Reference 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	ame Roundabout Capacity Model Description		Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Reference 2033, PM	Reference 2033	PM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junc	tion	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1		Ting Kok Road	T-Junction	Two-way	A,B,C	6.10	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
ပ	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	C-B	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	100.00	100.000
В	FLAT	✓	10.00	100.000
С	FLAT	✓	160.00	100.000



Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То		
		Α	В	С
From	Α	0.000	0.000	100.000
110111	В	0.000	0.000	10.000
	С	130.000	30.000	0.000

Turning Proportions (PCU) - Junction 1 (for whole period)

		Α	В	С
From	Α	0.00	0.00	1.00
10111	В	0.00	0.00	1.00
	С	0.81	0.19	0.00

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	1.000	1.000	1.000
FIOIII	В	1.000	1.000	1.000
	U	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	0.0	0.0	0.0			
From	В	0.0	0.0	0.0			
	С	0.0	0.0	0.0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.01	4.90	0.01	Α
C-AB	0.05	6.50	0.06	Α
C-A	-	-	-	-
A-B	-	-	-	-
A-C	-	-	-	-



Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	9.95	0.00	744.34	0.013	0.01	4.901	Α
C-AB	30.35	30.13	0.00	584.71	0.052	0.05	6.491	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	744.34	0.013	0.01	4.901	Α
C-AB	30.35	30.35	0.00	584.71	0.052	0.05	6.493	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	-AC 10.00 10.00		0.00 744.34		0.013	0.01	4.903	Α
C-AB	30.35	30.35	0.00	584.71	0.052	0.06	6.493	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	744.34	0.013	0.01	4.903	Α
C-AB	30.35	30.35	0.00	584.71	0.052	0.06	6.493	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	10.00	10.00	0.00	744.34	0.013	0.01	4.903	Α
C-AB	30.35	30.35	0.00	584.71	0.052	0.06	6.495	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	AC 10.00 10.00		0.00 744.34		0.013	0.01	4.903	Α
C-AB	30.35	30.35	0.00	584.71	0.052	0.06	6.495	Α
C-A	129.65	129.65	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-



Jn A - Design 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Na	ame	Roundabout Capacity Model Descrip		Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jı	n A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Design 2033, AM	Design 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Name Junction Type Major Road Direction		Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	5.80	А

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

An	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m) Has right		Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
C	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50



Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	С-В	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	150.00	100.000
В	FLAT	✓	50.00	100.000
С	FLAT	✓	130.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	0.000	0.000	150.000			
1 10111	В	0.000	0.000	50.000			
	C	100.000	30.000	0.000			

Turning Proportions (PCU) - Junction 1 (for whole period)

		1	Го	·
		Α	В	С
From	Α	0.00	0.00	1.00
1 10111	В	0.00	0.00	1.00
	С	0.77	0.23	0.00



Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	1.000	1.000	1.000			
1 10111	В	1.000	1.000	1.000			
	С	1.000	1.000	1.000			

Heavy Vehicle Percentages - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.0	0.0	0.0				
1 10111	В	0.0	0.0	0.0				
	С	0.0	0.0	0.0				

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.07	5.30	0.07	Α
C-AB	0.05	6.64	0.06	Α
C-A	-	-	-	-
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	49.71	0.00	729.74	0.069	0.07	5.291	Α
C-AB	30.28	30.06	0.00	572.39	0.053	0.06	6.634	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	ı	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-



Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.642	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.642	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Jn A - Design 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

	Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
ľ	Jn A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Design 2033, PM	Design 2033	PM		FLAT	08:00	09:30	90	15		



Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	6.09	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

	Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
ľ	ပ	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	С-В	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Traffic Flows

Demand Set Data Options

Defaul Vehicle Mix	 Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
	✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)		
Α	FLAT	✓	100.00	100.000		
В	FLAT	✓	30.00	100.000		
С	FLAT	✓	180.00	100.000		

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	0.000	0.000	100.000
FIOIII	В	0.000	0.000	30.000
	U	130.000	50.000	0.000

Turning Proportions (PCU) - Junction 1 (for whole period)

		То							
		Α	В	С					
From	Α	0.00	0.00	1.00					
FIOIII	В	0.00	0.00	1.00					
	С	0.72	0.28	0.00					

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		То							
		Α	В	С					
From	Α	1.000	1.000	1.000					
FIOIII	В	1.000	1.000	1.000					
	С	1.000	1.000	1.000					



Heavy Vehicle Percentages - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.0	0.0	0.0				
10111	В	0.0	0.0	0.0				
	С	0.0	0.0	0.0				

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	
B-AC 0.04		5.04	0.04	Α	
C-AB 0.09		6.71	0.10	Α	
C-A -		-	-	-	
A-B	-	-	-	-	
A-C	-	-	-	-	

Main Results for each time segment

Main results: (08:00-08:15)

Stream	n Total Demand (PCU/hr) Entry Flow (PCU/hr)		Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	29.83	0.00	744.34	0.040	0.04	5.037	Α
C-AB	50.97	50.58	0.00 587.71		0.087	0.10	6.698	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr) Entry Flow (PCU/hr)		Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97 50.96		0.00 587.71		0.087	0.10	6.708	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr) Entry Flow (PCU/hr)		Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97	50.97 50.97		587.71	0.087	0.10	6.706	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-



Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr) Entry Flow (PCU/hr)		Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00 30.00		0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97 50.97		0.00	587.71	0.087	0.10	6.706	Α
C-A	129.03	129.03 129.03		-	-	-	-	-
A-B	0.00 0.00		0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Total Demand (PCU/hr) Entry Flow (PCU/hr)		Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00 30.00		0.00	744.34	0.040	0.04	5.041	Α
C-AB	50.97	50.97 50.97		587.71		0.10	6.706	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr) Entry Flow (PCU/hr)		Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.041	Α
C-AB	50.97	50.97	0.00	587.71	0.087	0.10	6.708	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00 -		-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Jn A - SEN Design 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

	Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors	
ľ	Jn A	N/A			100.000		

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
SEN Design 2033, AM	SEN Design 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	5.80	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	B B Access Road			Minor
С	C C Tung Tsz Road (Major

Major Arm Geometry

	Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
ľ	С	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	C-B	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	SOURCE	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	√	HV Percentages	2.00				√	√



Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	150.00	100.000
В	FLAT	✓	50.00	100.000
С	FLAT	√	130.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	0.000	0.000	150.000
1 10111	В	0.000	0.000	50.000
	U	100.000	30.000	0.000

Turning Proportions (PCU) - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	0.00	0.00	1.00			
1 10111	В	0.00	0.00	1.00			
	C	0.77	0.23	0.00			

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	1.000	1.000	1.000			
1 10111	В	1.000	1.000	1.000			
	С	1.000	1.000	1.000			

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То					
		Α	В			
From	Α	0.0	0.0	0.0		
FIOIII	В	0.0	0.0	0.0		
	С	0.0	0.0	0.0		



Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC 0.07		5.30	0.07	Α
C-AB 0.05		6.64	0.06	Α
C-A -		-	-	-
A-B -		-	-	-
A-C -		-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	49.71	0.00	729.74	0.069	0.07	5.291	Α
C-AB	30.28	30.06	0.00	572.39	0.053	0.06	6.634	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.639	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-



Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.642	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	50.00	50.00	0.00	729.74	0.069	0.07	5.295	Α
C-AB	30.28	30.28	0.00	572.39	0.053	0.06	6.642	Α
C-A	99.72	99.72	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	150.00	150.00	0.00	-	-	-	-	-

Jn A - SEN Design 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn A	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
SEN Design 2033, PM	SEN Design 2033	PM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Jur	nction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
	1	Ting Kok Road	T-Junction	Two-way	A,B,C	6.09	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Tung Tsz Road (EB)		Major
В	В	Access Road		Minor
С	С	Tung Tsz Road (WB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central		Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
С	6.60		0.00		2.20	50.00	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	4.80										50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	611.947	0.109	0.274	0.173	0.392
1	B-C	773.526	0.115	0.292	-	-
1	С-В	602.919	0.228	0.228	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	100.00	100.000
В	FLAT	✓	30.00	100.000
С	FLAT	✓	180.00	100.000



Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.000	0.000	100.000				
1 10111	В	0.000	0.000	30.000				
	С	130.000	50.000	0.000				

Turning Proportions (PCU) - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	0.00	0.00	1.00			
FIOIII	В	0.00	0.00	1.00			
	C	0.72	0.28	0.00			

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	1.000	1.000	1.000
FIOIII	В	1.000	1.000	1.000
	U	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	0.0	0.0	0.0			
From	В	0.0	0.0	0.0			
	С	0.0	0.0	0.0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC 0.04		5.04	0.04	Α
C-AB	0.09	6.71	0.10	Α
C-A	-	-	-	-
A-B	-	-	-	-
A-C	-	-	-	-



Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	29.83	0.00	744.34	0.040	0.04	5.037	Α
C-AB	50.97	50.58	0.00	587.71	0.087	0.10	6.698	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
А-В	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97	50.96	0.00	587.71	0.087	0.10	6.708	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
А-В	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97	50.97	0.00	587.71	0.087	0.10	6.706	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.039	Α
C-AB	50.97	50.97	0.00	587.71	0.087	0.10	6.706	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.041	Α
C-AB	50.97	50.97	0.00	587.71	0.087	0.10	6.706	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	30.00	30.00	0.00	744.34	0.040	0.04	5.041	Α
C-AB	50.97	50.97	0.00	587.71	0.087	0.10	6.708	Α
C-A	129.03	129.03	0.00	-	-	-	-	-
A-B	0.00	0.00	0.00	-	-	-	-	-
A-C	100.00	100.00	0.00	-	-	-	-	-



Junctions 8

PICADY 8 - Priority Intersection Module

Version: 8.0.5.523 [19102,19/06/2015] © Copyright TRL Limited, 2025

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Filename: 24093 JnB.arc8

Path: \CTA_NAS01\Project\CTA Consultants Limited\CTA - Project\24093HK (knc) - S12A Re-zoning from AGR to GIC for a

Prop Social Welfare Facilities (RCHE) at Tung Tsz, Tai Po\Calculation\2025-05-07

Report generation date: 13/5/2025 16:59:52

- » Jn B Existing 2024, AM
- » Jn B Existing 2024, PM
- » Jn B Reference 2033, AM
- » Jn B Reference 2033, PM
- » Jn B Design 2033, AM
- » Jn B Design 2033, PM
- » Jn B SEN Design 2033, AM
- » Jn B SEN Design 2033, PM



Summary of junction performance

	AM			РМ				
	Queue (PCU)	Delay (s)	RFC	LOS	Queue (PCU)	Delay (s)	RFC	LOS
		Jn B - Design 2033						
Stream B-AC	1.74	19.72	0.64	С	0.50	10.61	0.33	В
Stream C-A	-	ı	-	-	-	-	-	-
Stream C-B	0.03	6.03	0.03	Α	0.02	6.17	0.02	Α
Stream A-B	-	-	-	-	-	-	-	-
Stream A-C	-	-	-	-	-	-	-	-
			Jn B	- Exis	sting 2024			
Stream B-AC	0.94	13.10	0.49	В	0.31	8.68	0.24	А
Stream C-A	-	1	-	-	-	-	-	-
Stream C-B	0.02	5.71	0.02	Α	0.02	5.91	0.02	Α
Stream A-B	-	-	-	-	_	-	-	-
Stream A-C	-	-	-	-	-	-	-	-
		J	n B -	Refe	rence 2033			
Stream B-AC	1.33	16.57	0.57	С	0.41	9.93	0.29	Α
Stream C-A	-	-	-	-	-	-	-	-
Stream C-B	0.03	5.96	0.03	Α	0.02	6.12	0.02	Α
Stream A-B		-	-	-		-	-	-
Stream A-C	-	-	-	-	-	-	-	-
		Jr	า B - 🤄	SEN D	Design 2033			
Stream B-AC	1.74	19.72	0.64	С	0.50	10.61	0.33	В
Stream C-A	_	-	-	-		-	-	-
Stream C-B	0.03	6.03	0.03	Α	0.02	6.17	0.02	Α
Stream A-B	-	-	-	-		-	-	-
Stream A-C	-	-	-	-	-	-	-	-

Values shown are the maximum values over all time segments. Delay is the maximum value of average delay per arriving vehicle.

Run using Junctions 8.0.5.523 at 13/5/2025 16:59:47

File summary

Title	(untitled)
Location	
Site Number	
Date	7/6/2021
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	user
Description	

[&]quot;D1 - Existing 2024, AM " model duration: 8:00 - 9:30 "D2 - Existing 2024, PM" model duration: 8:00 - 9:30

[&]quot;D3 - Reference 2033, AM" model duration: 8:00 - 9:30

[&]quot;D4 - Reference 2033, PM" model duration: 8:00 - 9:30 "D5 - Design 2033, AM" model duration: 8:00 - 9:30

[&]quot;D8 - Design 2033, PM" model duration: 8:00 - 9:30

[&]quot;D9 - SEN Design 2033, AM" model duration: 8:00 - 9:30

[&]quot;D10 - SEN Design 2033, PM" model duration: 8:00 - 9:30

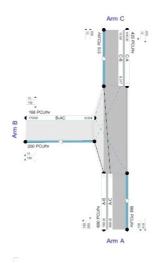


Analysis Options

Vehicle Length (m)	Do Queue Variations	Calculate Residual Capacity	Residual Capacity Criteria Type	RFC Threshold	Average Delay Threshold (s)	Queue Threshold (PCU)
5.75			N/A	0.85	36.00	20.00

Units

	Distance Units	Speed Units	Traffic Units Input	Traffic Units Results	Flow Units	Average Delay Units	Total Delay Units	Rate Of Delay Units
ſ	m	kph	PCU	PCU	perHour	S	-Min	perMin



20.00

Showing modeled flow through junction (PCUhr).

Streams (upstreams) show Total Demand (PCUhr); Streams (downstreams) show RFC ()

Time Segment: (08:00-08:15)

Showing Analysis Set "A1 - Jn B "; Demand Set "D1 - Existing 2024, AM "

The junction diagram reflects the last run of ARCADY.



Jn B - Existing 2024, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Existing 2024, AM	Existing 2024	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	12.83	В

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
A A		Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

A	rm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
(С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150



Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	С-В	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	490.00	100.000
В	FLAT	✓	260.00	100.000
С	FLAT	✓	570.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То							
		Α	В	С					
From	Α	0.000	150.000	340.000					
1 10111	В	240.000	0.000	20.000					
	C	560.000	10.000	0.000					

Turning Proportions (PCU) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.00	0.31	0.69				
FIOIII	В	0.92	0.00	0.08				
	С	0.98	0.02	0.00				

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Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
		Α	A B	
From	Α	1.000	1.000	1.000
110111	В	1.000	1.000	1.000
	С	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	0.0	0.0	0.0			
110111	В	0.0	0.0	0.0			
	U	0.0	0.0	0.0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.49	13.10	0.94	В
C-A	-	-	-	-
С-В	0.02	5.71	0.02	Α
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00	256.32	0.00	534.82	0.486	0.92	12.765	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00	9.94	0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	ı	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00	259.95	0.00	534.80	0.486	0.93	13.091	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	-	-	-



Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00 259.98		0.00	534.80	0.486	0.94	13.097	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00 259.99		0.00	534.80	0.486	0.94	13.097	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00 10.00		0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00 259.99		0.00	534.80	0.486	0.94	13.099	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00		0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	260.00 260.00		0.00	534.80	0.486	0.94	13.099	В
C-A	560.00	560.00	0.00	-	-	-	-	-
С-В	10.00 10.00		0.00	640.40	0.016	0.02	5.710	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	340.00	340.00	0.00	-	-	-	-	-

Jn B - Existing 2024, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Existing 2024, PM	Existing 2024	PM		FLAT	08:00	09:30	90	15		



Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	8.48	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arn	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	C-B	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	580.00	100.000
В	FLAT	✓	130.00	100.000
С	FLAT	✓	360.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

	То						
		Α	В	С			
From	Α	0.000	150.000	430.000			
FIOIII	В	120.000	0.000	10.000			
	O	350.000	10.000	0.000			

Turning Proportions (PCU) - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.00	0.26	0.74		
From	В	0.92	0.00	0.08		
	С	0.97	0.03	0.00		

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	1.000	1.000	1.000		
10111	В	1.000	1.000	1.000		
	С	1.000	1.000	1.000		

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Heavy Vehicle Percentages - Junction 1 (for whole period)

	То				
		Α	В	С	
From	Α	0.0	0.0	0.0	
FIOIII	В	0.0	0.0	0.0	
	С	0.0	0.0	0.0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC 0.24		8.68	0.31	Α
C-A	-	-	-	-
С-В	0.02	5.91	0.02	Α
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	128.76	0.00	544.66	0.239	0.31	8.629	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	9.93	0.00	619.47	0.016	0.02	5.906	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	129.99	0.00	544.64	0.239	0.31	8.681	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	619.47	0.016	0.02	5.906	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	130.00	0.00	544.64	0.239	0.31	8.681	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	619.47	0.016	0.02	5.906	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-



Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	130.00	0.00	544.64	0.239	0.31	8.681	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	619.47	0.016	0.02	5.908	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	130.00	0.00	544.64	0.239	0.31	8.681	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	619.47	0.016	0.02	5.908	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	130.00	130.00	0.00	544.64	0.239	0.31	8.681	Α
C-A	350.00	350.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	619.47	0.016	0.02	5.908	Α
A-B	150.00	150.00	0.00	-	-	-	-	-
A-C	430.00	430.00	0.00	-	-	-	-	-

Jn B - Reference 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Reference 2033, AM	Reference 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type Major Road Direction		Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	15.88	С

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

,	Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
	С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	C-B	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	√	HV Percentages	2.00				√	✓



Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	560.00	100.000
В	FLAT	✓	290.00	100.000
С	FLAT	√	660.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.000	170.000	390.000				
FIOIII	В	260.000	0.000	30.000				
	O	640.000	20.000	0.000				

Turning Proportions (PCU) - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	0.00	0.30	0.70			
FIOIII	В	0.90	0.00	0.10			
	С	0.97	0.03	0.00			

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	1.000	1.000	1.000			
10111	В	1.000	1.000	1.000			
	С	1.000	1.000	1.000			

Heavy Vehicle Percentages - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.0	0.0	0.0				
FIOIII	В	0.0	0.0	0.0				
	С	0.0	0.0	0.0				



Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.57	16.57	1.33	С
C-A	-	-	-	-
С-В	0.03	5.96	0.03	Α
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00	284.88	0.00	507.33	0.572	1.28	15.846	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	19.87	0.00	624.12	0.032	0.03	5.956	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00	289.90	0.00	507.28	0.572	1.31	16.539	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	624.12	0.032	0.03	5.958	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00	289.96	0.00	507.28	0.572	1.32	16.553	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	624.12	0.032	0.03	5.958	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00 289.98		0.00 507.28		0.572	1.32	16.558	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	624.12	0.032	0.03	5.958	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-



Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00	289.99	0.00	507.28	0.572	1.32	16.561	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	624.12	0.032	0.03	5.958	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	290.00	289.99	0.00	507.28	0.572	1.33	16.565	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	624.12	0.032	0.03	5.958	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Jn B - Reference 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Reference 2033, PM	Reference 2033	PM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junctio	n Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	9.69	Α

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
ပ	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	С-В	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	670.00	100.000
В	FLAT	✓	150.00	100.000
С	FLAT	✓	420.00	100.000



Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То					
		Α	В	C			
From	Α	0.000	170.000	500.000			
1 10111	В	140.000	0.000	10.000			
-	С	410.000	10.000	0.000			

Turning Proportions (PCU) - Junction 1 (for whole period)

		То				
		Α	В	С		
From	Α	0.00	0.25	0.75		
FIOIII	В	0.93	0.00	0.07		
	С	0.98	0.02	0.00		

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		То				
		Α	В	С		
From	Α	1.000	1.000	1.000		
FIOIII	В	1.000	1.000	1.000		
	O	1.000	1.000	1.000		

Heavy Vehicle Percentages - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	0.0	0.0	0.0			
FIOIII	В	0.0	0.0	0.0			
•	С	0.0	0.0	0.0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.29	9.93	0.41	Α
C-A	-	-	-	-
С-В	0.02	6.12	0.02	Α
A-B	-	-	-	-
A-C	-	-	-	-



Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	148.37	0.00	512.58	0.293	0.41	9.843	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	9.93	0.00	598.55	0.017	0.02	6.116	Α
А-В	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	149.99	0.00	512.56	0.293	0.41	9.929	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	598.55	0.017	0.02	6.116	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	150.00	0.00	512.56	0.293	0.41	9.929	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	598.55	0.017	0.02	6.116	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	150.00	0.00	512.56	0.293	0.41	9.929	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	598.55	0.017	0.02	6.118	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	150.00	0.00	512.56	0.293	0.41	9.929	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	598.55	0.017	0.02	6.118	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	150.00	150.00	0.00	512.56	0.293	0.41	9.929	Α
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	598.55	0.017	0.02	6.118	Α
A-B	170.00	170.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-



Jn B - Design 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

1	Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Γ.	Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Design 2033, AM	Design 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	18.92	С

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Aı	rm	Width of Has kerbed central carriageway (m)		Width of kerbed central reserve (m)			Visibility For Right Turn (m) Blocks?		Blocking Queue (PCU)
(С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150



Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	С-В	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	590.00	100.000
В	FLAT	✓	320.00	100.000
С	FLAT	✓	660.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То								
		Α	В	С						
From	Α	0.000	200.000	390.000						
1 10111	В	290.000	0.000	30.000						
	C	640.000	20.000	0.000						

Turning Proportions (PCU) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.00	0.34	0.66				
FIOIII	В	0.91	0.00	0.09				
	С	0.97	0.03	0.00				



Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	1.000	1.000	1.000
110111	В	1.000	1.000	1.000
	C	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		Т	o	
		Α	В	С
From	Α	0.0	0.0	0.0
110111	В	0.0	0.0	0.0
	С	0.0	0.0	0.0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	
B-AC	0.64	19.72	1.74	С	
C-A	-	-	-	-	
С-В	0.03	6.03	0.03	Α	
A-B	-	-	-	-	
A-C	-	-	-	-	

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00	313.37	0.00	502.42	0.637	1.66	18.457	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	19.87	0.00	617.15	0.032	0.03	6.025	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	ı	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00	319.81	0.00	502.37	0.637	1.70	19.666	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-



Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.93		0.00	502.37	0.637	1.72	19.701	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.97		0.00	502.37	0.637	1.73	19.714	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.98		0.00	502.37	0.637	1.73	19.720	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	3 20.00 20.00		0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.99		0.00	502.37	0.637	1.74	19.724	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Jn B - Design 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
Design 2033, PM	Design 2033	PM		FLAT	08:00	09:30	90	15		



Junction Network

Junctions

Junction	Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	10.36	В

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arr	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	C-B	754.327	0.233	0.233	-	

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Traffic Flows

Demand Set Data Options

Defaul Vehicle Mix	 Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
	✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	690.00	100.000
В	FLAT	✓	170.00	100.000
С	FLAT	✓	420.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.000	190.000	500.000		
FIOIII	В	160.000	0.000	10.000		
	U	410.000	10.000	0.000		

Turning Proportions (PCU) - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.00	0.28	0.72		
	В	0.94	0.00	0.06		
	С	0.98	0.02	0.00		

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

	То				
		Α	В	C	
From	Α	1.000	1.000	1.000	
1 10111	В	1.000	1.000	1.000	
	С	1.000	1.000	1.000	



Heavy Vehicle Percentages - Junction 1 (for whole period)

	То			
		Α	В	С
From	Α	0.0	0.0	0.0
FIOIII	В	0.0	0.0	0.0
	С	0.0	0.0	0.0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.33	10.61	0.50	В
C-A	-	-	-	-
С-В	0.02	6.17	0.02	Α
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	168.03	0.00	509.27	0.334	0.49	10.492	В
C-A	410.00	410.00	0.00	-	1	-	-	-
С-В	10.00	9.93	0.00	593.90	0.017	0.02	6.164	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	ı	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	169.98	0.00	509.24	0.334	0.50	10.609	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.164	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	169.99	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.164	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-



Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Jn B - SEN Design 2033, AM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Г	Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
	Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
SEN Design 2033, AM	SEN Design 2033	AM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junction	Name	Junction Type Major Road Direction		Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	18.92	О

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Width of kerbed central reserve (m)	Has right turn bay	Width For Right Turn (m)	Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
ပ	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	C-B	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	√	HV Percentages	2.00				✓	√



Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	590.00	100.000
В	FLAT	✓	320.00	100.000
С	FLAT	√	660.00	100.000

Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.000	200.000	390.000				
FIOIII	В	290.000	0.000	30.000				
	O	640.000	20.000	0.000				

Turning Proportions (PCU) - Junction 1 (for whole period)

		То						
		Α	В	С				
From	Α	0.00	0.34	0.66				
1 10111	В	0.91	0.00	0.09				
	C	0.97	0.03	0.00				

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		То					
		Α	В	С			
From	Α	1.000	1.000	1.000			
110111	В	1.000	1.000	1.000			
	С	1.000	1.000	1.000			

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То							
		Α	В	С				
From	Α	0.0	0.0	0.0				
From	В	0.0	0.0	0.0				
	С	0.0	0.0	0.0				



Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.64	19.72	1.74	С
C-A	-	-	-	-
С-В	0.03	6.03	0.03	Α
A-B	-	-	-	-
A-C	-	-	-	-

Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00	313.37	0.00	502.42	0.637	1.66	18.457	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	19.87	0.00	617.15	0.032	0.03	6.025	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00	319.81	0.00	502.37	0.637	1.70	19.666	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00	319.93	0.00	502.37	0.637	1.72	19.701	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.97		0.00 502.37		0.637	1.73	19.714	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-



Main results: (09:00-09:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.98		0.00	502.37	0.637	1.73	19.720	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	320.00 319.99		0.00 502.3		0.637	1.74	19.724	С
C-A	640.00	640.00	0.00	-	-	-	-	-
С-В	20.00	20.00	0.00	617.15	0.032	0.03	6.027	Α
A-B	200.00	200.00	0.00	-	-	-	-	-
A-C	390.00	390.00	0.00	-	-	-	-	-

Jn B - SEN Design 2033, PM

Data Errors and Warnings

No errors or warnings

Analysis Set Details

Name	Roundabout Capacity Model	Description	Locked	Network Flow Scaling Factor (%)	Reason For Scaling Factors
Jn B	N/A			100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Single Time Segment Only	Locked
SEN Design 2033, PM	SEN Design 2033	PM		FLAT	08:00	09:30	90	15		

Junction Network

Junctions

Junctio	n Name	Junction Type	Major Road Direction	Arm Order	Junction Delay (s)	Junction LOS
1	Ting Kok Road	T-Junction	Two-way	A,B,C	10.36	В

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown



Arms

Arms

Arm	Arm	Name	Description	Arm Type
Α	Α	Ting Kok Road (NB)		Major
В	В	Tung Tsz Road		Minor
С	С	Ting Kok Road (SB)		Major

Major Arm Geometry

Am	Width of carriageway (m)			Width of kerbed central Has right reserve (m) turn bay		Visibility For Right Turn (m)	Blocks?	Blocking Queue (PCU)
С	10.70		0.00	✓	3.50	150.00		

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor Arm Type	Lane Width (m)	Lane Width (Left) (m)	Lane Width (Right) (m)	Width at give-way (m)	Width at 5m (m)	Width at 10m (m)	Width at 15m (m)	Width at 20m (m)	Estimate Flare Length	Flare Length (PCU)	Visibility To Left (m)	Visibility To Right (m)
В	One lane	5.00										150	150

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	725.644	0.105	0.266	0.167	0.380
1	B-C	862.208	0.105	0.266	-	-
1	C-B	754.327	0.233	0.233	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn		Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (PCU/hr)	Flow Scaling Factor (%)
Α	FLAT	✓	690.00	100.000
В	FLAT	✓	170.00	100.000
С	FLAT	✓	420.00	100.000



Turning Proportions

Turning Counts / Proportions (PCU/hr) - Junction 1 (for whole period)

		То				
		Α	В	C		
From	Α	0.000	190.000	500.000		
1 10111	В	160.000	0.000	10.000		
	С	410.000	10.000	0.000		

Turning Proportions (PCU) - Junction 1 (for whole period)

		То				
		Α	В	С		
From	Α	0.00	0.28	0.72		
From	В	0.94	0.00	0.06		
	С	0.98	0.02	0.00		

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

			То	
		Α	В	С
From	Α	1.000	1.000	1.000
FIOIII	В	1.000	1.000	1.000
	U	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

	То					
		Α	В	С		
From	Α	0.0	0.0	0.0		
FIOIII	В	0.0	0.0	0.0		
-	С	0.0	0.0	0.0		

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.33	10.61	0.50	В
C-A	-	-	-	-
С-В	0.02	6.17	0.02	Α
A-B	-	-	-	-
A-C	-	-	-	-



Main Results for each time segment

Main results: (08:00-08:15)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	168.03	0.00	509.27	0.334	0.49	10.492	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	9.93	0.00	593.90	0.017	0.02	6.164	Α
А-В	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:15-08:30)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	169.98	0.00	509.24	0.334	0.50	10.609	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.164	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:30-08:45)

Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	169.99	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.164	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (08:45-09:00)

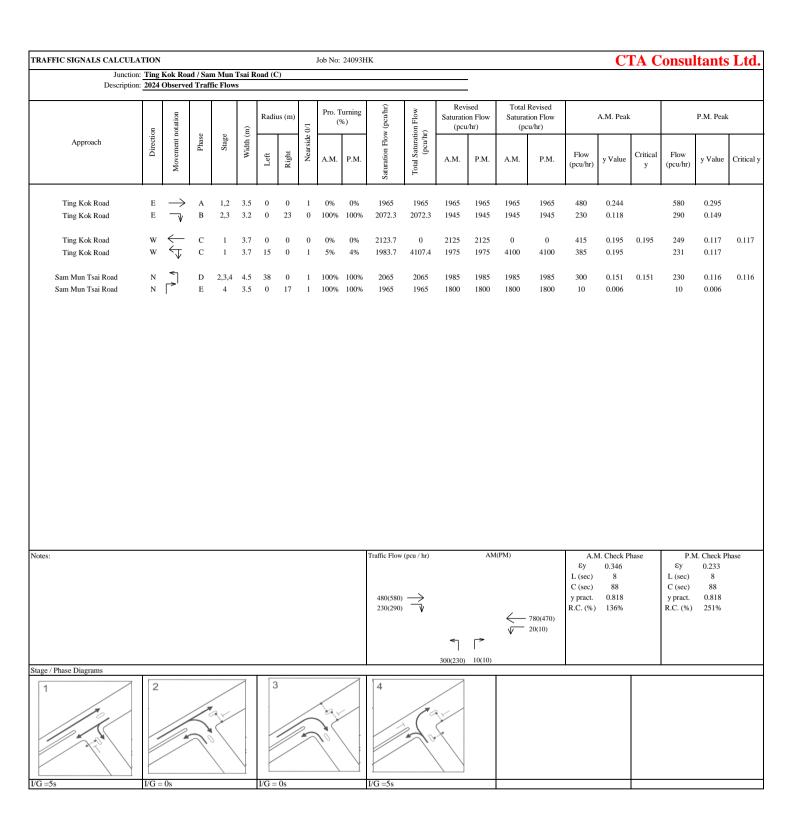
Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

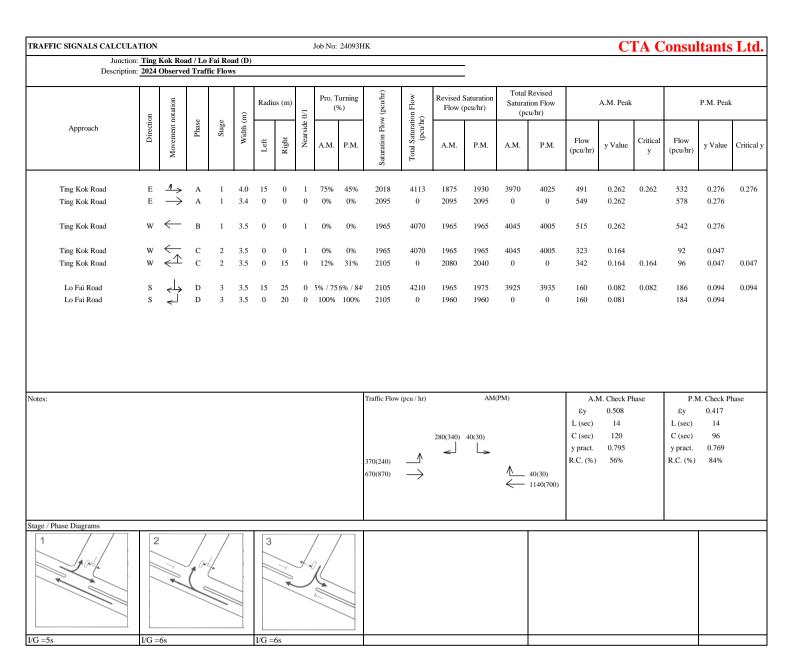
Main results: (09:00-09:15)

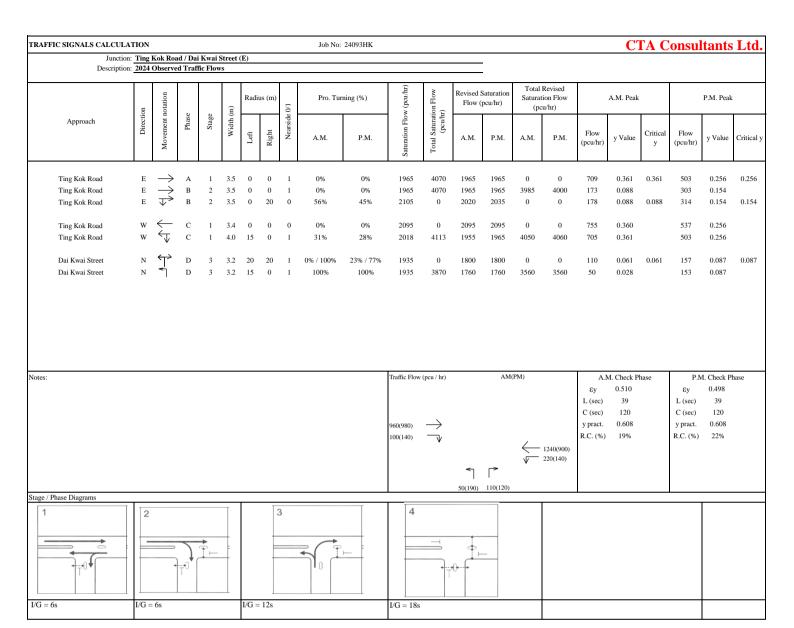
Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-

Main results: (09:15-09:30)

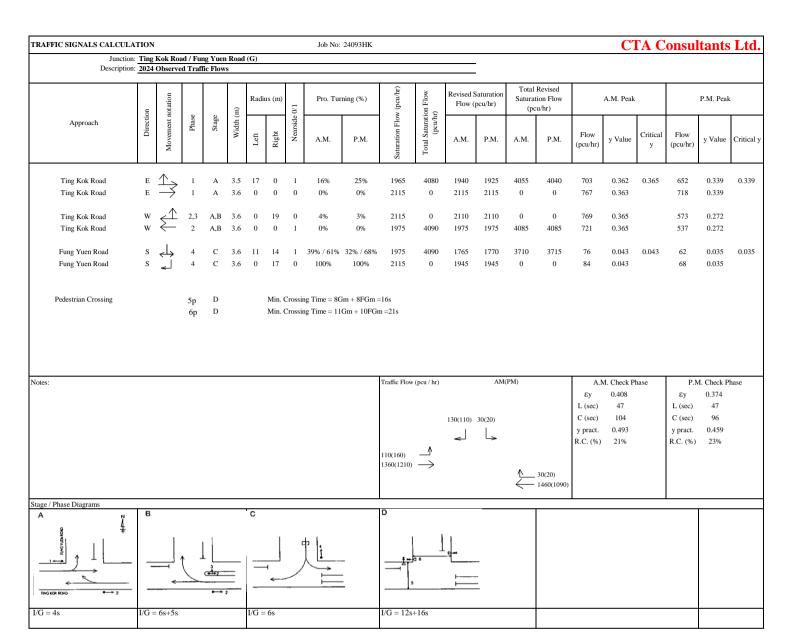
Stream	Total Demand (PCU/hr)	Entry Flow (PCU/hr)	Pedestrian Demand (Ped/hr)	Capacity (PCU/hr)	RFC	End Queue (PCU)	Delay (s)	LOS
B-AC	170.00	170.00	0.00	509.24	0.334	0.50	10.611	В
C-A	410.00	410.00	0.00	-	-	-	-	-
С-В	10.00	10.00	0.00	593.90	0.017	0.02	6.167	Α
A-B	190.00	190.00	0.00	-	-	-	-	-
A-C	500.00	500.00	0.00	-	-	-	-	-







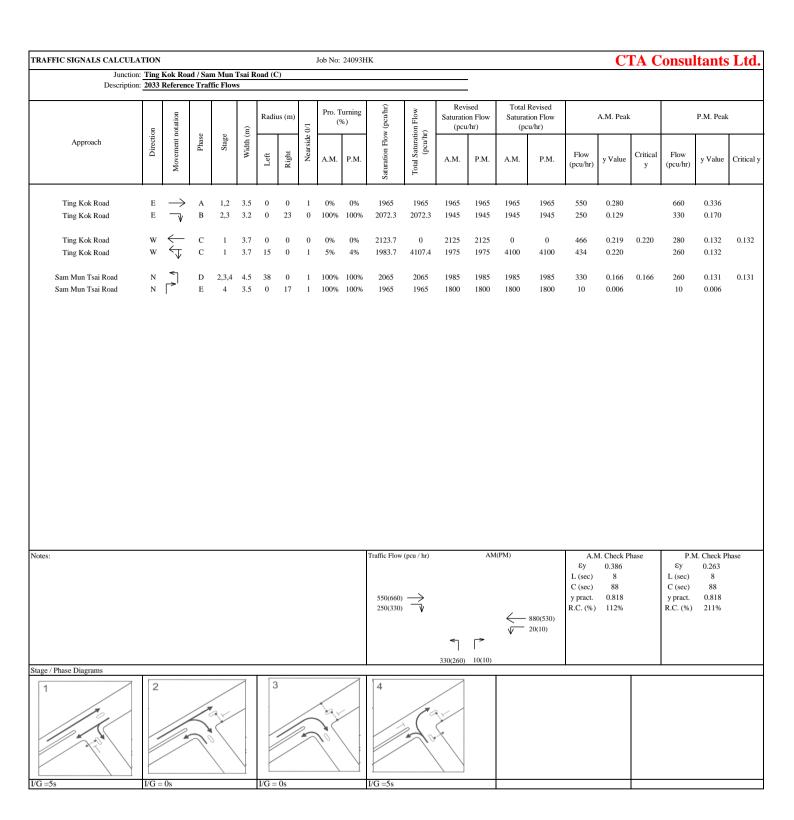
TRAFFIC SIGNALS CALCULA		7.1.5	1/5:	T . C					Job No:	24093H	K							C'.	ra c	onsul	ltants	Ltd
Junction: Description:		Kok Roa Observe)								<u>-</u>								
Approach	Direction	otation			û	Radiu	lius (m)		Pro. Turning (%)		(pcw/hr)	n Flow	Revised Saturation Flow (pcu/hr)		Total Revised Saturation Flow (pcu/hr)		A.M. Peak			P.M. Peak		
		Movement notation	Phase	Stage	Width (m)	Left	Right	Nearside 0/1	A.M.	P.M.	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Ting Kok Road Ting Kok Road	E E	$\overset{\rightarrow}{\Rightarrow}$	A A	1,2 1,2	3.5 3.5	0	0 15	1	0% 41%	0% 21%	1965 2105	4070 0	1965 2020	1965 2060	3985 0	4025 0	685 705	0.349 0.349	0.349	600 630	0.305 0.306	0.306
Ting Kok Road Ting Kok Road	W W	\rightleftharpoons	B B	3	3.5 3.8	0	0	0	0% 0%	0% 0%	2103.6 1997	4100.6 0	2103.6 1997	2103.6 1997	4100.6 0	4100.6 0	682 648	0.324 0.324	0.324	518 492	0.246 0.246	0.246
Dai Fat Street Dai Fat Street	N N		C C	1	3.5 3.5	15 20	0	0	100% 100%	100% 100%	2105 2105	4210 0	1915 1960	1915 1960	3875 0	3875 0	79 81	0.041 0.041		44 46	0.023 0.023	
Notes:											Traffic Flow	(pcu / hr)		AM(PM)		Ey L (sec) C (sec)	M. Check Pl 0.673 10 100	hase	Ey L (sec) C (sec)	1. Check P 0.552 10 100	hase
											1100(1100) 290(130)	$\overrightarrow{\neg}$			\leftarrow	1330(1010)	y pract. R.C. (%)	0.810 20%		y pract. R.C. (%)	0.810 47%	
Stage / Phase Diagrams													160(90)									
1 TING KOK ROAD	2	A CO	F	>		3	E S S S S S S S S S S S S S S S S S S S	F	E D√0)) ❖													

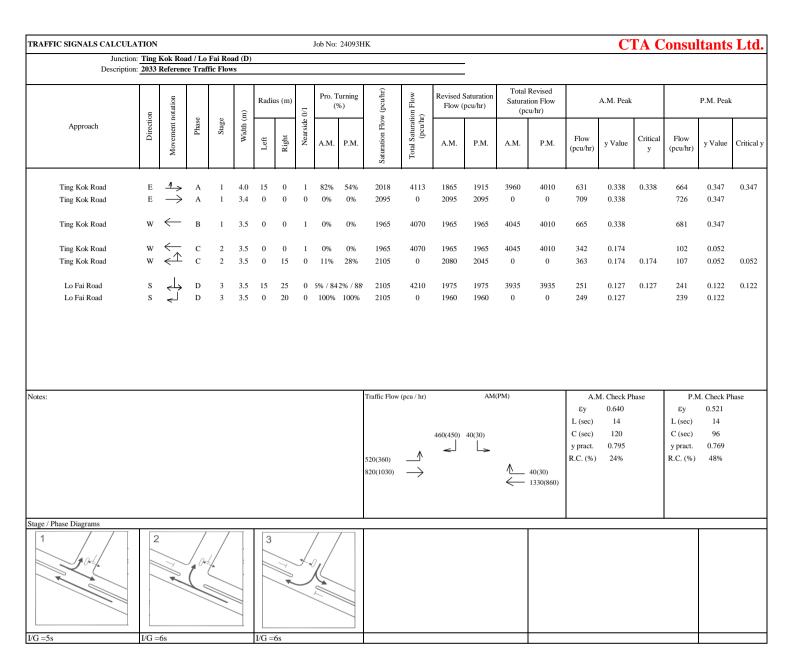


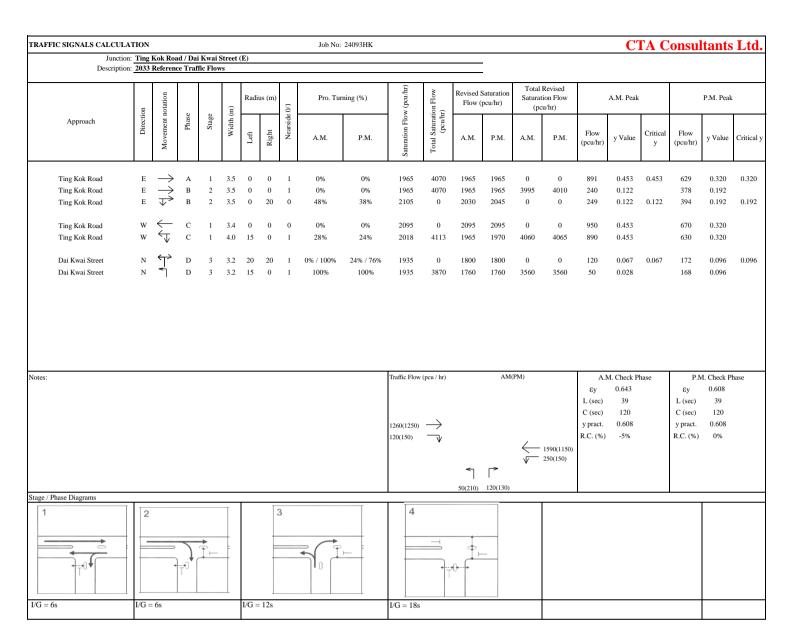
RAFFIC SIGNALS CALCULA									Job No:	24093F	łK							C'	ra c	onsu	ltants	Ltd
Junction Description		Kok Roa Observe				/ Dai F	uk Str	reet (F	H)					=								
Approach	Direction	otation	Phase		(ii	Radius (m)		0/1	Pro. Turning (%)		(pcu/hr)	on Flow	Revised Saturation Flow (pcu/hr)		Total Revised Saturation Flow (pcu/hr)		A.M. Peak			P.M. Peak		
		Movement notation		Stage	Width (m)	Left	Right	Nearside 0/1	A.M.	P.M.	Saturation Flow (pcw/hr)	Total Saturation Flow (pcu/hr)	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical
Ting Kok Road	Е		A	1,2	3.5	15	0	0	100%	100%	2105	4210	1915	1915	3875	3875	534	0.279		366	0.191	
Ting Kok Road	E		A	1,2	3.5	20	0	0	100%	100%	2105	0	1960	1960	0	0	546	0.279		374	0.191	
Ting Kok Road	E	\rightarrow	В	2	3.5	0	0	1	0%	0%	1965	1965	1965	1965	1965	1965	100	0.051		80	0.041	
Ting Kok Road	E	¬√	В	2	3.1	0	20	0	100%	100%	2067	2067	1925	1925	1925	1925	230	0.119	0.119	280	0.145	0.145
Yuen Shin Road	N	<u> </u>	C	2,3	3.5	15	0	1	100%	100%	1965	1965	1785	1785	1785	1785	230	0.129		210	0.118	
Yuen Shin Road	N	\uparrow	D	3	3.1	0	0	1	0%	0%	1927	6111	1927	1927	6111	6111	155	0.080	0.080	173	0.090	0.090
Yuen Shin Road	N	\uparrow	D	3	3.4	0	0	0	0%	0%	2095	0	2095	2095	0	0	168	0.080		189	0.090	
Yuen Shin Road	N	\uparrow	D	3	3.3	0	0	0	0%	0%	2089	0	2089	2089	0	0	168	0.080		188	0.090	
Yuen Shin Road	N	1.	D	3	3.5	0	20	0	100%	100%	2105	2105	1960	1960	1960	1960	10	0.005		10	0.005	
Dai Fuk Street	W	$\stackrel{\longleftarrow}{\leftarrow}$	E	4	3.0	17	0	1	33%	41%	1915	4000	1860	1850	3935	3925	151	0.081		146	0.079	
Dai Fuk Street	W	$\stackrel{\longleftarrow}{\leftarrow}$	Е	4	3.3	0	17	0	6%	6%	2085	0	2075	2075	0	0	169	0.081		164	0.079	
Ting Kok Road	S		F	1	3.5	0	22	1	100%	100%	1965	3930	1835	1835	3655	3655	392	0.213	0.213	276	0.150	0.159
Ting Kok Road	S	جا	F	1	3.5	0	19	1	100%	100%	1965	0	1820	1820	0	0	388	0.213		274	0.150	
Ting Kok Road	S	↓,	F	1	4.0	15	0	1	0%	6%	2018	4113	2020	2005	4115	4100	393	0.194		318	0.159	
Ting Kok Road	S	\Rightarrow	F	1	3.4	0	0	0	0%	0%	2095	0	2095	2095	0	0	407	0.195		332	0.159	
otes:											Traffic Flov	v (pcu / hr)		AM((PM)		A.M	M. Check P. 0.413	hase	P.Λ εy	1. Check P	hase
													780(550)	800(630)	0(20)		L (sec)	40		L (sec)	40	
											1080(740)	^	رک		Ĺ		C (sec)	100		C (sec)	96	
											100(80)	\rightarrow	•	•			y pract.	0.540		y pract.	0.525	
											230(280)				^	10(10)	R.C. (%)	31%		R.C. (%)	33%	
															\leftarrow	260(240)						
												~	\uparrow	>	V	50(60)						
												230(210)	490(550)	10(10)								
tage / Phase Diagrams					_						T .			_							1	
	2	G BF		н	_	3		T)		E	4 G L		E	3 ^F								
/G = 7c	I/C	- Qc	' '			I/G -	- 11e	/, /			I/G = 17a											
G = 7s	I/G	= 8s				I/G =	118				I/G = 17s											

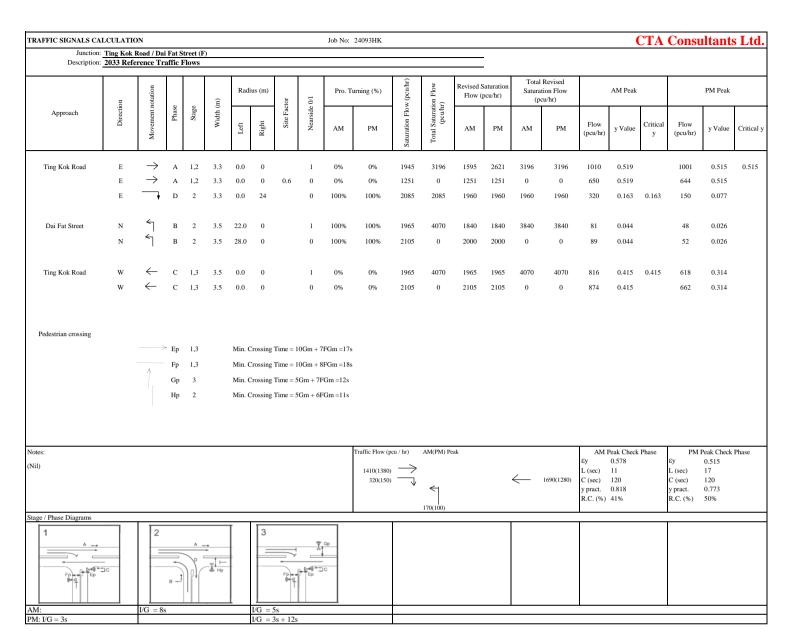
TRAFFIC SIGNALS CALCULA	TION]	lob No:	24093F	łK							C	ГА С	onsul	tants	Ltd.
Junction: Description:)								_								
						Radiu	ıs (m)	0/1	Pro. T	urning 6)	м (pcu/hr)	on Flow r)	Revised S Flow (p		Saturat	Revised ion Flow u/hr)		A.M. Peak	:		P.M. Peak	:
Approach	Direction	Movement notation	Phase	Stage	Width (m)	Left	Right	Nearside 0/1	A.M.	P.M.	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Yuen Shin Road Yuen Shin Road Yuen Shin Road	N N N	↑	A A A	1 1 1	4.0 4.0 4.0	0 0 0	0 50 45	0 0 0	0% 59% 100%	0% 11% 100%	2155 2155 2155	6465 0 0	2155 2115 2085	2155 2150 2085	6355 0 0	6390 0 0	522 513 505	0.242 0.242 0.242	0.242	408 407 395	0.189 0.189 0.189	0.189
Dai Fat Street Dai Fat Street	W W	√ √	B B	1	3.5 3.5	15 20	0	0	100% 100%	100% 100%	2105 2105	4210 0	1915 1960	1915 1960	3875 0	3875 0	257 263	0.134 0.134		282 288	0.147 0.147	
Yuen Shin Road Yuen Shin Road	s s	$\qquad \qquad \qquad \\ \downarrow \qquad \qquad \\$	C C	2	4.0 3.4	15 0	0	1	2% 0%	2% 0%	2018 2095	4113 0	2015 2095	2015 2095	4110 0	4110 0	529 551	0.263 0.263	0.263	475 495	0.236 0.236	0.236
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)		A.N	M. Check P	hase	P.M	1. Check Pl	hase
																	εy L (sec)	0.505 30		εy L (sec)	0.426 30	
													1070(960)	10(10)			C (sec)	100		C (sec)	100	
													\downarrow	L>			y pract. R.C. (%)	0.630 25%		y pract. R.C. (%)	0.630 48%	
													↑ 730(770)	810(440)	V	520(570)						
Stage / Phase Diagrams	1					ı					1					1						
	2	1 H	A SECTION OF THE PERSON OF THE			3		-07-W	F													
I/G = 7s	I/G	= 10s				I/G =	: 15s															

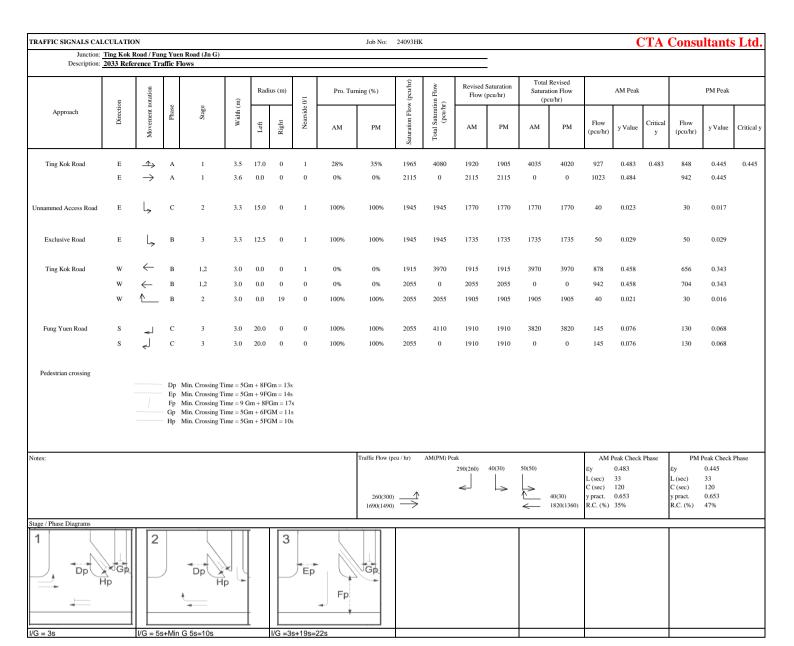
RAFFIC SIGNALS CALCUL		Po Tai V	Va Dand	/ 3/	Chi. 1	Daad (n		Job No:	240931	ıĸ							C.	IAC	onsu	itanis	Lu
Descriptio						Koad (J	1)							•								
	1	1	1		1	1		l			ଚ				Total	Revised	l					
	uo	notation	٥	0	(E)	Radiu	ıs (m)	0/1		urning %)	w (pcu/h	ion Flow rr)	Revised S Flow (Saturation pcu/hr)	Satura	tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Direction	Movement notation	Phase	Stage	Width (m)	Left	Right	Nearside 0/1	A.M.	P.M.	Saturation Flow (pcu/hr)	Total Saturation Flow (pcw/hr)	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critic
Tai Po Tai Wo Road	Е	1	A	2,3	5.8	15	0	1	100%	100%	2199	2199	2000	2000	2000	2000	490	0.245	0.214	330	0.165	0.18
Tai Po Tai Wo Road	Е	╗	В	3	3.5	0	15	0	100%	100%	2105	4210	1915	1915	3830	3830	410	0.214		350	0.183	
Tai Po Tai Wo Road	E	\neg	В	3	3.5	0	15	0	100%	100%	2105	0	1915	1915	0	0	410	0.214		350	0.183	
Yuen Shin Road	N	\uparrow	С	1	3.1	0	0	1	0%	0%	1927	6111	1927	1927	6111	6111	334	0.173	0.173	277	0.144	0.14
Yuen Shin Road	N	\uparrow	C	1	3.4	0	0	0	0%	0%	2095	0	2095	2095	0	0	363	0.173		302	0.144	
Yuen Shin Road	N	\uparrow	С	1	3.3	0	0	0	0%	0%	2089	0	2089	2089	0	0	362	0.173		301	0.144	
Yuen Shin Road	S	_1	D	2	3.1	0	20	0	100%	100%	2067	2067	1925	1925	1925	1925	440	0.229	0.229	420	0.218	0.21
Yuen Shin Road	S		E	1,2	3.5	0	0	0	0%	0%	2103.6	4100.6	2103.6	2103.6	4100.6	4100.6	595	0.283	0.22	580	0.276	0.2.
Yuen Shin Road	S	Y.	E	1,3	3.8	0	0	1	0%	0%	1997	0	1997	1997	0	0	565	0.283		550	0.276	
otes:											Traffic Flow	(pcu / hr)		AM((PM)		A.I	M. Check P	hase	P.M	1. Check P	hase
												(4)					εу	0.616		εу	0.545	
														11.00/1120			L (sec)	13		L (sec)	13	
													440(420)	1160(1130))		C (sec) y pract.	100 0.783		C (sec) y pract.	100 0.783	
													$\mathrel{\blacktriangleleft}$	\downarrow			y pract. R.C. (%)			R.C. (%)	44%	
											490(330)	^								,		
											820(700)	- V	_	•								
													1	1								
age / Phase Diagrams													520(850)	1060(880)								
ige / Filase Diagrailis																						
1	2	Dį		B	į.	3	Ρţ	•														
at Po Tai Wo Rd			3				EŢ															
G = 5s	I/G	= 5s				I/G =	6s															
						1					1					ı					l	

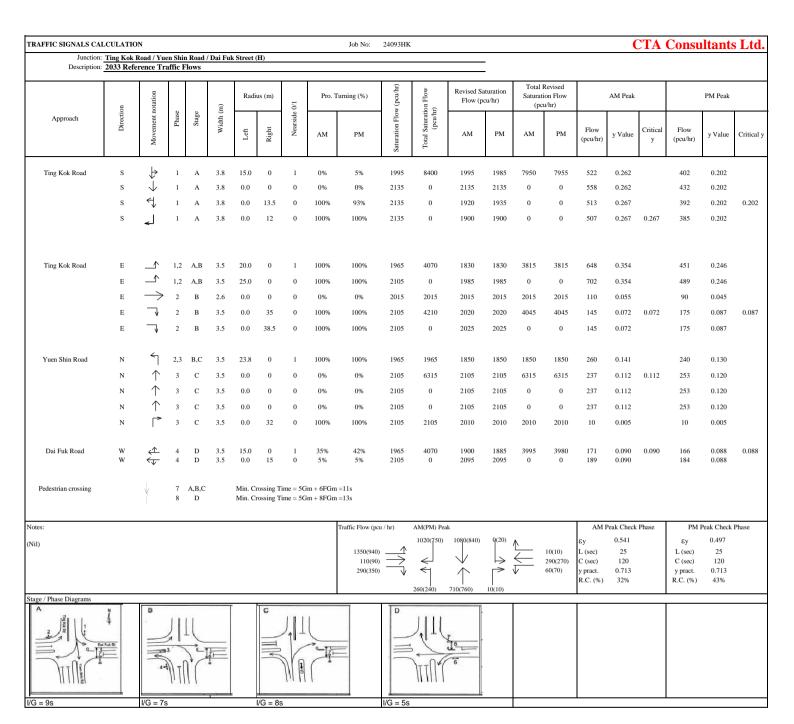


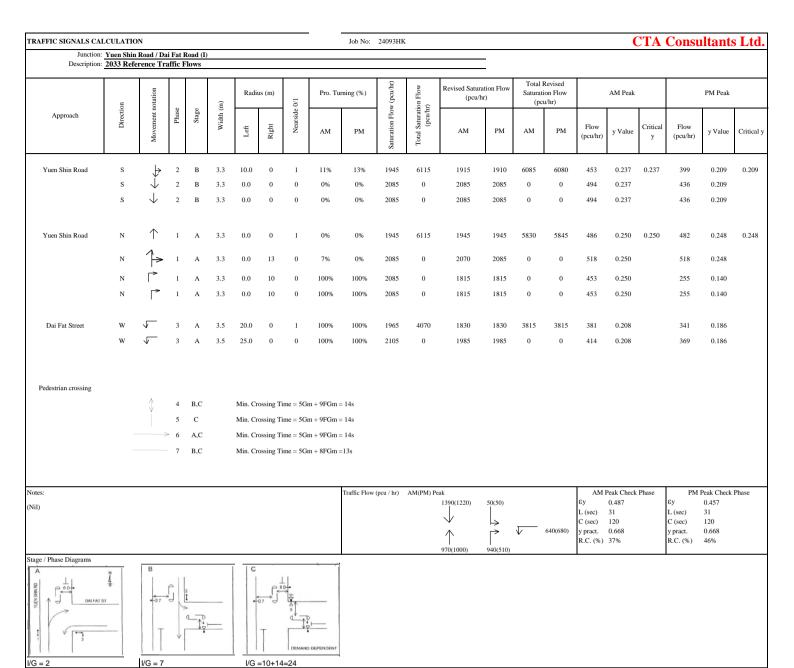




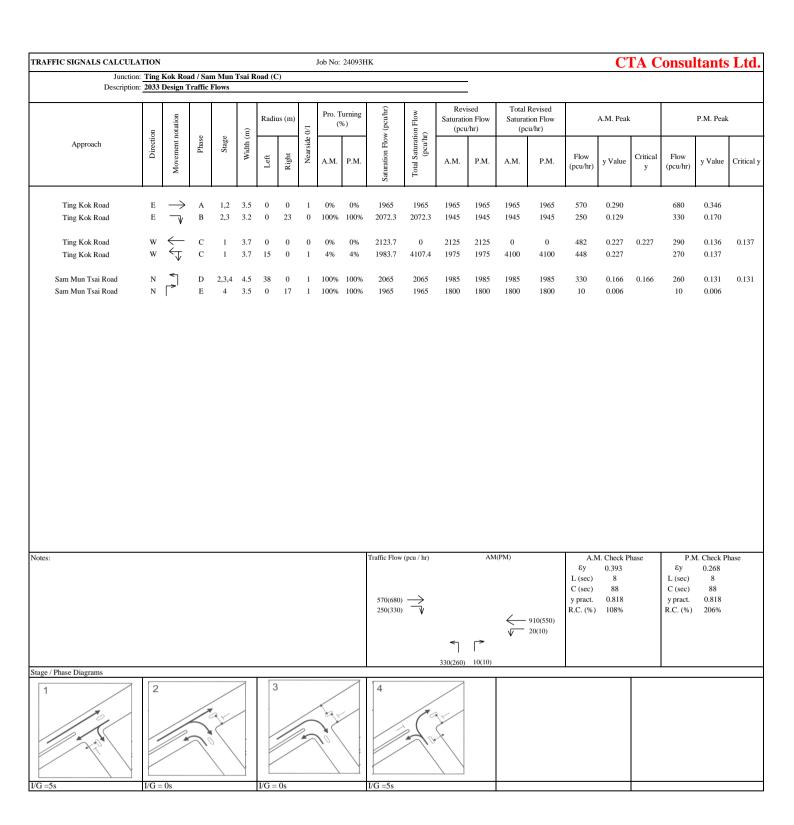


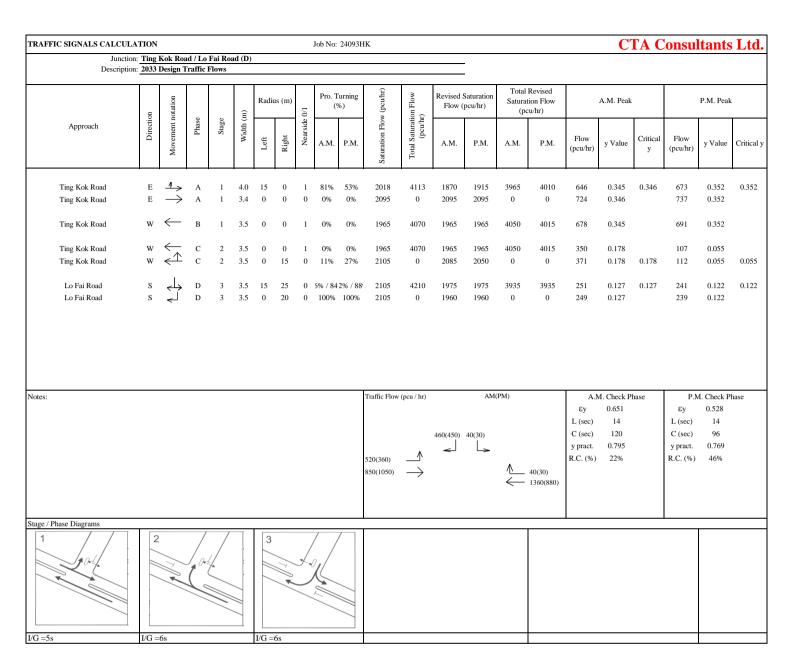


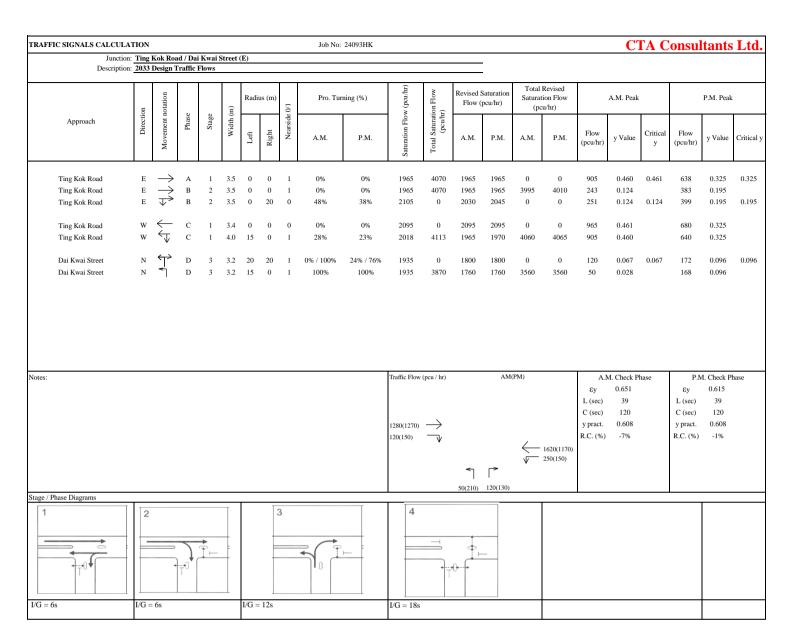


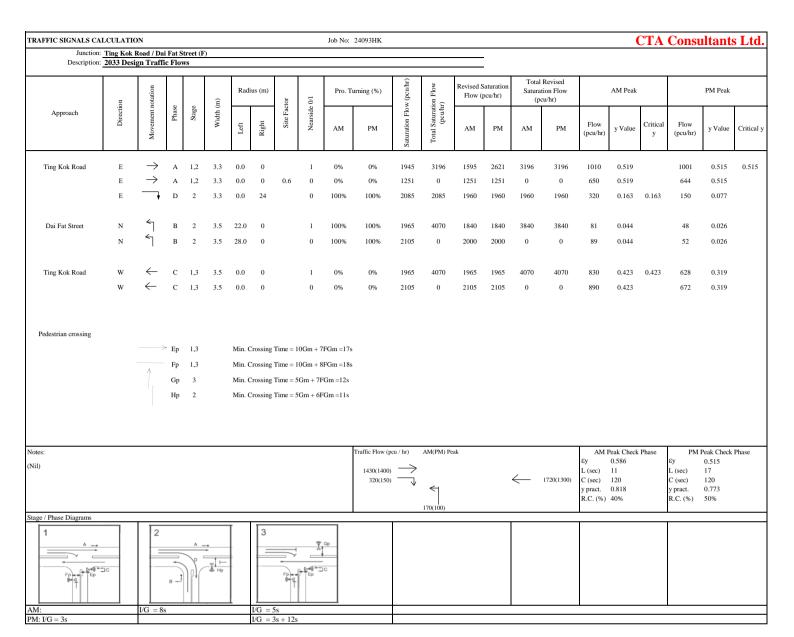


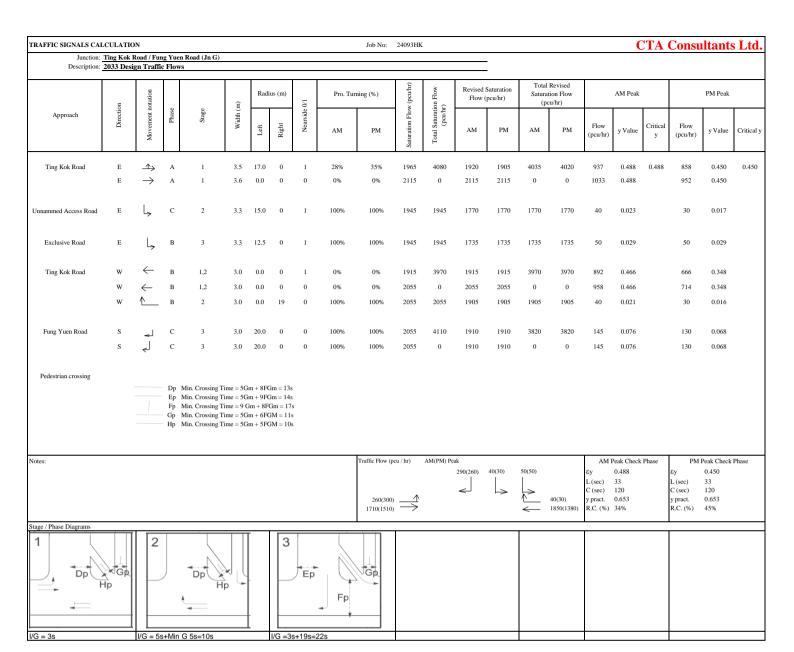
TRAFFIC SIGNALS CAI	LCULATIO	N								Job No:	24093HK	ζ						(CTA	Const	ıltant	s Ltd.
Junction: Description:	Tai Po Tai				oad (J)									_								
					(m)	Radi	ius (m)	1/0:	Pro. Tu	urning (%)	w (pcu/hr)	ion Flow r)	Revised Sa Flow (po		Saturati	Revised on Flow 1/hr)		AM Peak			PM Peak	
Approach	Direction	Movement notation	Phase	əgaS	(m) Width (m)	Left	Right	Nearside 0/1	AM	PM	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	AM	PM	AM	PM	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Yuen Shin Road	s	\downarrow	2	A,B	3.5	0	0	1	0%	0%	1965	4070	1965	1965	4070	4070	715	0.364		681	0.346	
	S	\downarrow	2	A,B	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	765	0.364		729	0.346	
	S	٦	3	В	3.5	0	32	0	100%	100%	2105	2105	2010	2010	2010	2010	520	0.259	0.259	500	0.249	0.249
Tai Po Tai Wo Road	Е	_	4	В,С	3.5	24	0	1	100%	100%	1965	1965	1850	1850	1850	1850	610	0.330		430	0.232	
	E	_	5	C	3.5	0	16	0	100%	100%	2105	6315	1925	1925	5695	5695	314	0.163	0.163	267	0.139	0.139
	E	\neg	5	C	3.5	0	14	0	100%	100%	2105	0	1900	1900	0	0	310	0.163		264	0.139	
	E	\neg	5	С	3.5	0	12	0	100%	100%	2105	0	1870	1870	0	0	305	0.163		259	0.139	
Yuen Shin Road	N	\uparrow	1	A	3.5	0	0	1	0%	0%	1965	6175	1965	1965	6175	6175	417	0.212	0.212	344	0.175	0.175
	N	\uparrow	1	Α	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	447	0.212		368	0.175	
	N	1	1	A	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	447	0.212		368	0.175	
Notes:										Traffic Flow	(pcu / hr)	AM(PM) Per 520(500)	ık 1480(1410)					Peak Check 0.634		PM εy	Peak Check 0.562	Phase
(Nil)										610(430) 930(790)			1310(1080)				L (sec) C (sec)	13 120 0.803		L (sec) C (sec) y pract. R.C. (%)	13 120 0.803 43%	
Stage / Phase Diagrams						_	Т.				1									ı		
Ion Po Ion Vo Rd	2-4+ Poor une	1/G =	5				C	4 t 6														



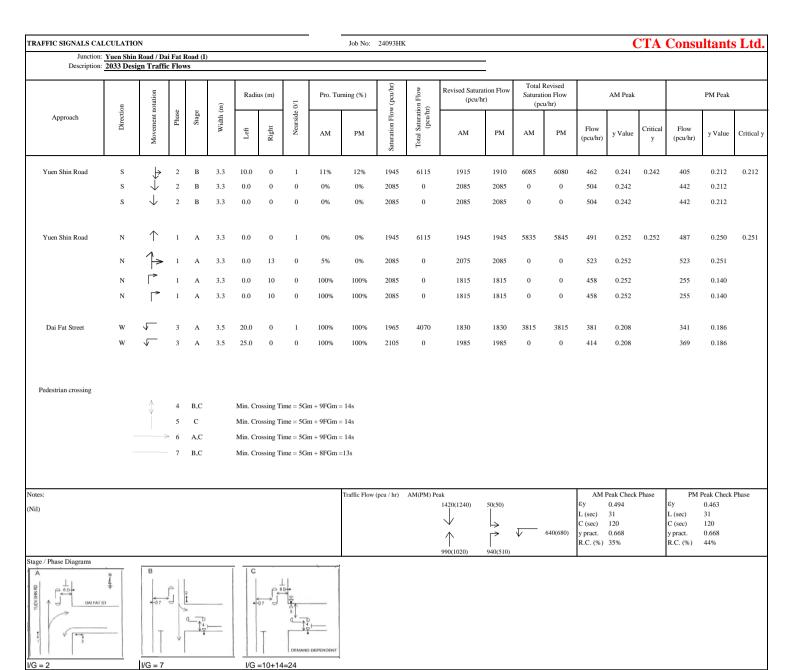




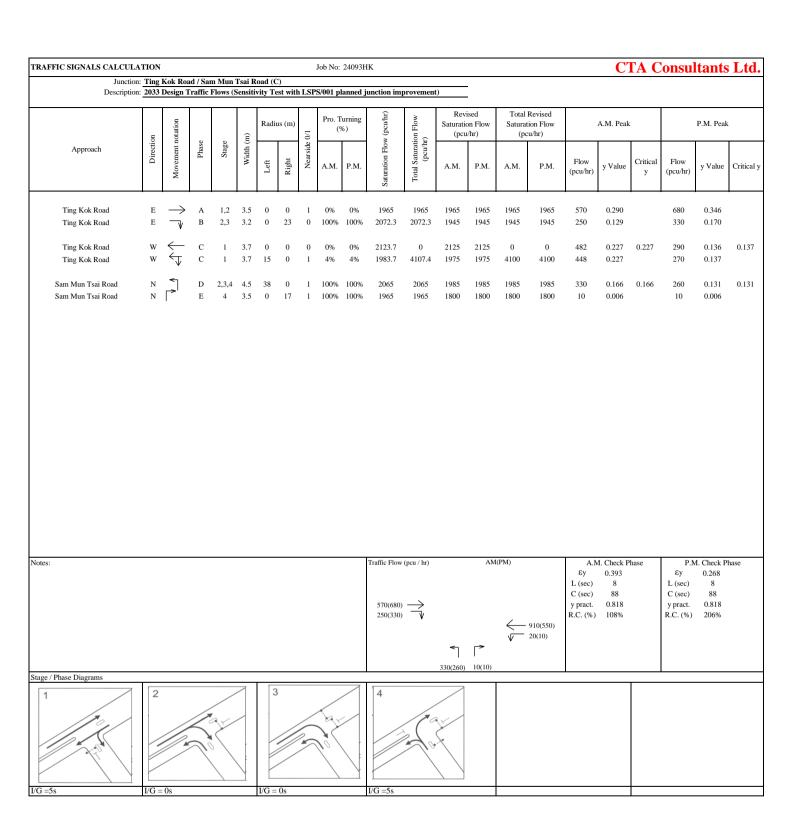


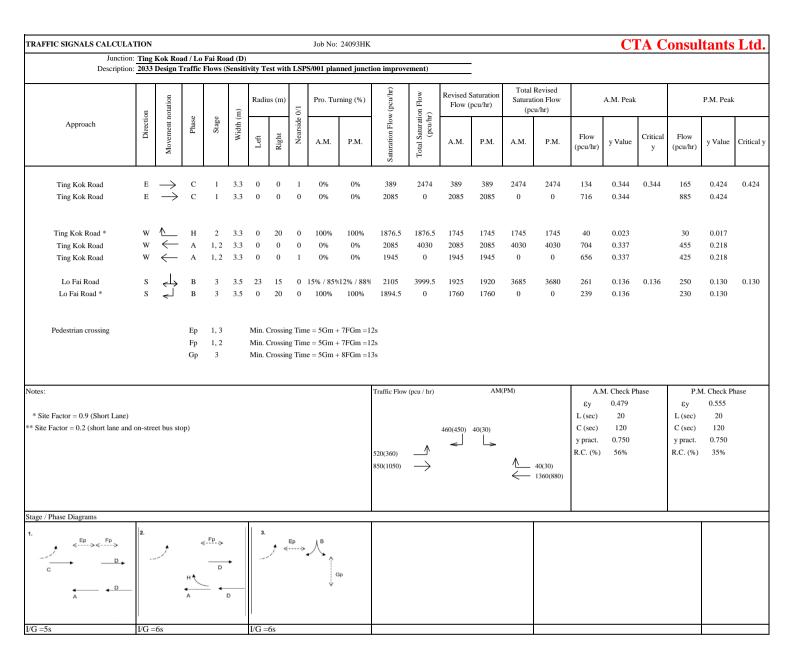


RAFFIC SIGNALS CAI										Job No:	24093HK							(CTA	Consu	ltants	s Ltd
Junction: Description:	Ting Kok 2033 Des				/ Dai Ful	k Street (H)							- -								
	uc	otation			(ii	Radio	ıs (m)	0/1	Pro. T	`urning (%)	w (pcu/hr)	on Flow r)	Revised Sa Flow (p		Saturat	Revised ion Flow u/hr)		AM Peak			PM Peak	
Approach	Direction	Movement notation	Phase	Stage	Width (m)	Left	Right	Nearside 0/1	AM	PM	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	AM	PM	AM	PM	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical
Ting Kok Road	s	\Rightarrow	1	A	3.8	15.0	0	1	0%	5%	1995	8400	1995	1985	7955	7960	534	0.268		407	0.205	
	S	\downarrow	1	A	3.8	0.0	0	0	0%	0%	2135	0	2135	2135	0	0	572	0.268		437	0.205	
	S	\Leftrightarrow	1	A	3.8	0.0	13.5	0	99%	91%	2135	0	1925	1940	0	0	515	0.268		397	0.205	0.205
	S	٦	1	A	3.8	0.0	12	0	100%	100%	2135	0	1900	1900	0	0	509	0.268	0.268	389	0.205	
Ting Kok Road	E	_	1,2	A,B	3.5	20.0	0	1	100%	100%	1965	4070	1830	1830	3815	3815	648	0.354		451	0.246	
	E	^	1,2	A,B	3.5	25.0	0	0	100%	100%	2105	0	1985	1985	0	0	702	0.354		489	0.246	
	E	\rightarrow	2	В	2.6	0.0	0	0	0%	0%	2015	2015	2015	2015	2015	2015	110	0.055		90	0.045	
	E	\neg	2	В	3.5	0.0	35	0	100%	100%	2105	4210	2020	2020	4045	4045	145	0.072	0.072	175	0.087	0.087
	Е	\neg	2	В	3.5	0.0	38.5	0	100%	100%	2105	0	2025	2025	0	0	145	0.072		175	0.087	
Yuen Shin Road	N		2,3	В,С	3.5	23.8	0	1	100%	100%	1965	1965	1850	1850	1850	1850	260	0.141		240	0.130	
	N	\uparrow	3	C	3.5	0.0	0	0	0%	0%	2105	6315	2105	2105	6315	6315	247	0.117	0.117	260	0.124	
	N	\uparrow	3	C	3.5	0.0	0	0	0%	0%	2105	0	2105	2105	0	0	247	0.117		260	0.124	
	N	\uparrow	3	C	3.5	0.0	0	0	0%	0%	2105	0	2105	2105	0	0	247	0.117		260	0.124	
	N		3	С	3.5	0.0	32	0	100%	100%	2105	2105	2010	2010	2010	2010	10	0.005		10	0.005	
Dai Fuk Road	W W	←	4 4	D D	3.5 3.5	15.0 0.0	0 15	1 0	35% 5%	42% 5%	1965 2105	4070 0	1900 2095	1885 2095	3995 0	3980 0	171 189	0.090 0.090	0.090	166 184	0.088 0.088	0.088
Pedestrian crossing		¥	7 8	A,B,C D					im + 6FGm im + 8FGm													
tes:										Traffic Flow (pcu	/ hr)	AM(PM) Pea	ık				AM I	Peak Check	Phase	PM I	eak Check	Phase
1)											A	1020(750)	1110(860)	0(20)	<u></u>	10/16:	εy	0.547		εy	0.503	
										1350(940) 110(90)	\Rightarrow	\leftarrow	\downarrow	\rightarrow	$\overline{\leftarrow}$	10(10) 290(270)	L (sec) C (sec)	25 120		L (sec) C (sec)	25 120	
										290(350)	\neg	\leq	\uparrow	>	$\sqrt{}$	60(70)	y pract. R.C. (%)	0.713 30%		y pract.	0.713 42%	
												260(240)	740(780)	10(10)			K.C. (%)	30%		R.C. (%)	4270	
ge / Phase Diagrams	4	В		Ĥ		!	С		1		D	1111										
	Fulk Sit	4.	3 - [7 1				F) TT [] *	18 <u> </u>									
as Sido Nd			1111	11				11 5				111111										
= 9s	_	I/G = 7s	3				/G = 8s			_	I/G = 5s		- It									

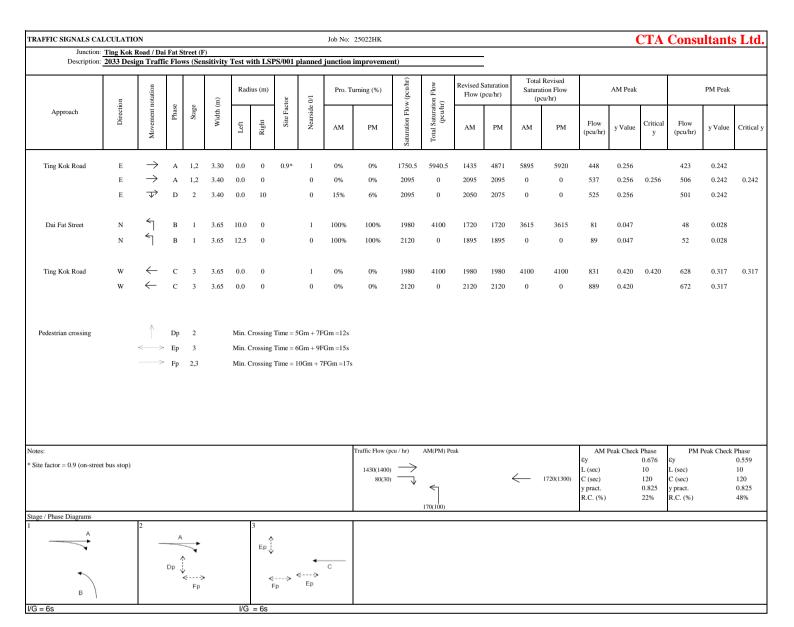


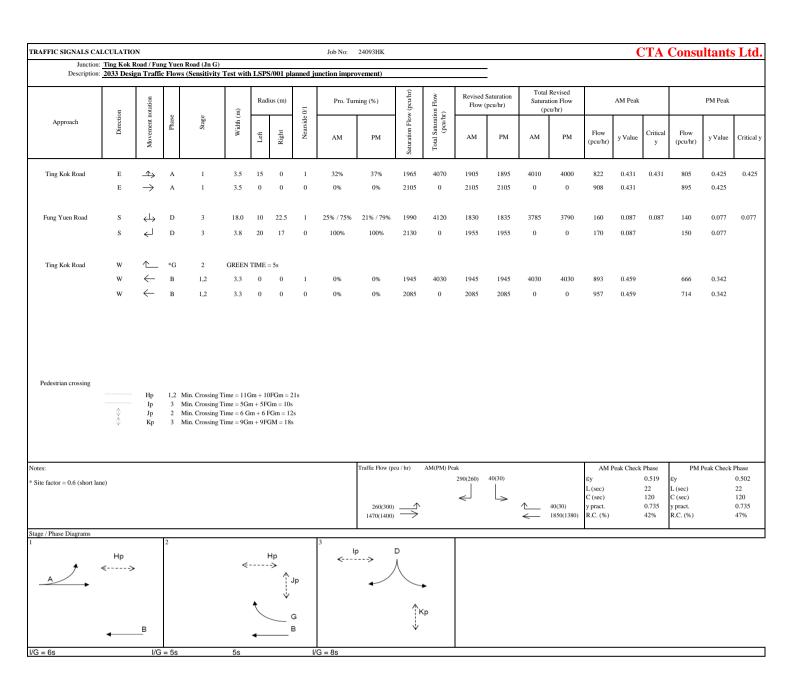
TRAFFIC SIGNALS CAI	LCULATIO	N								Job No:	24093НК	ζ						(CTA	Consu	ıltants	s Ltd.
Junction: Description:	Tai Po Tai				oad (J)									_								
					(m)	Radi	ius (m)	1/0	Pro. Tu	urning (%)	w (pcu/hr)	ion Flow r)	Revised Sa Flow (po		Saturati	Revised on Flow 1/hr)		AM Peak			PM Peak	
Approach	Direction	Movement notation	Phase	əgaS	(m) Width (m)	Left	Right	Nearside 0/1	AM	PM	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	AM	PM	AM	PM	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Yuen Shin Road	s	\downarrow	2	A,B	3.5	0	0	1	0%	0%	1965	4070	1965	1965	4070	4070	729	0.371		690	0.351	
	S	\downarrow	2	A,B	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	781	0.371		740	0.351	
	S	٦	3	В	3.5	0	32	0	100%	100%	2105	2105	2010	2010	2010	2010	520	0.259	0.259	500	0.249	0.249
Tai Po Tai Wo Road	Е	_	4	В,С	3.5	24	0	1	100%	100%	1965	1965	1850	1850	1850	1850	610	0.330		430	0.232	
	E	_	5	C	3.5	0	16	0	100%	100%	2105	6315	1925	1925	5695	5695	314	0.163	0.163	267	0.139	0.139
	E	\neg	5	C	3.5	0	14	0	100%	100%	2105	0	1900	1900	0	0	310	0.163		264	0.139	
	E	\neg	5	С	3.5	0	12	0	100%	100%	2105	0	1870	1870	0	0	305	0.163		259	0.139	
Yuen Shin Road	N	\uparrow	1	A	3.5	0	0	1	0%	0%	1965	6175	1965	1965	6175	6175	426	0.217	0.217	350	0.178	0.178
	N	\uparrow	1	A	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	457	0.217		375	0.178	
	N		1	A	3.5	0	0	0	0%	0%	2105	0	2105	2105	0	0	457	0.217		375	0.178	
Notes:										Traffic Flow	(pcu / hr)	AM(PM) Per 520(500)	ak 1510(1430)					Peak Check 0.639		PM εy	Peak Check 0.566	Phase
(Nil)										610(430) 930(790)		590(960)	1340(1100)				L (sec) C (sec)	13 120 0.803		L (sec) C (sec) y pract. R.C. (%)	13 120 0.803 42%	
Stage / Phase Diagrams						_	Т.				1											
Ion Po Ion Wo Rd	2-d+	1/G =	5				C	5.5														

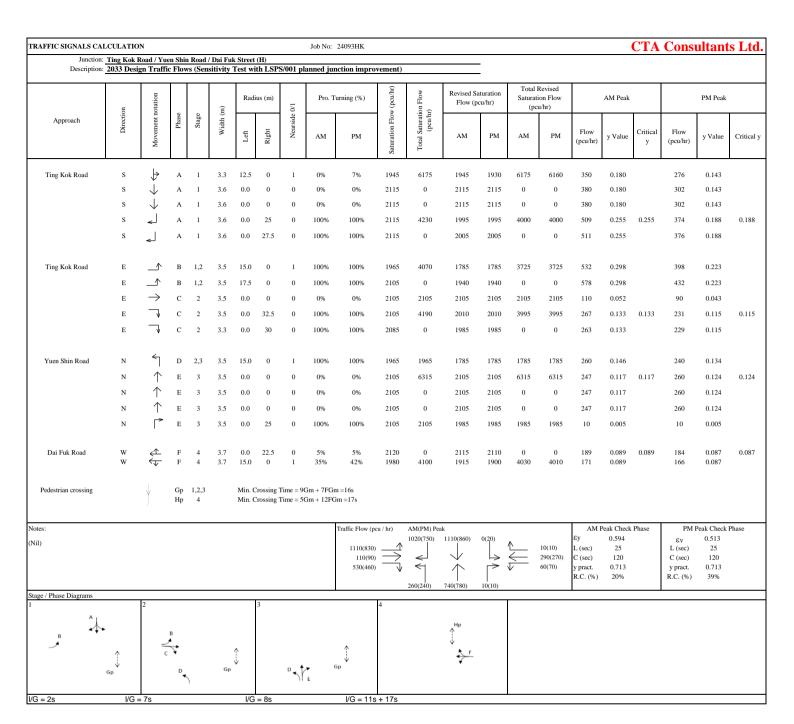


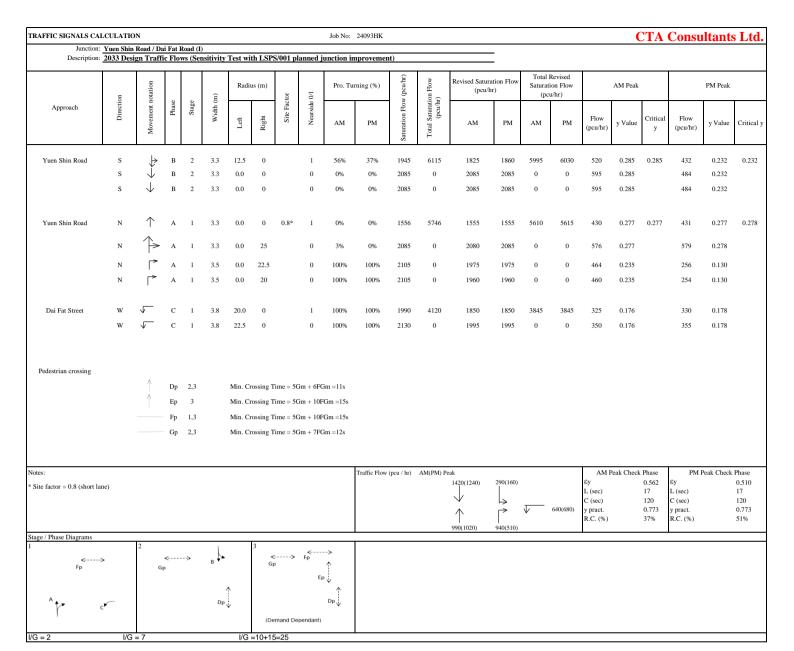


Innotice	ATION	Kok Roa	d / De	Kwa:	Street	Œ)			-55110.	24093HK								U.	лс	onsul	tants	Lu
							st with	LSPS	5/001 planned j	junction impro	vement)											
	$\overline{}$			ı		ı	ı				Ē		1		Total	Revised				1		
	ion	notation	ě	e e	(m)	Radiu	ıs (m)	e 0/1	Pro. Tur	ning (%)	ow (pcu/h	tion Flow hr)		Saturation pcu/hr)	Saturat	ion Flow u/hr)		A.M. Peak	1		P.M. Peak	
Approach	Direction	Movement notation	Phase	Stage	Width (m)	Left	Right	Nearside 0/1	A.M.	P.M.	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critic
Ting Kok Road	Е	\rightarrow	В	1, 2	3.4	0	0	1	0%	0%	1955	0	1955	1955	0	0	618	0.316		613	0.314	0.3
Ting Kok Road	E	\rightarrow	В	1, 2	3.4	0	0	0	0%	0%	2095	4050	2095	2095	4050	4050	662	0.316		657	0.314	
Ting Kok Road	E	\neg	D	2	3.4	0	15	0	100%	100%	2095	2095	1905	1905	1905	1905	120	0.063	0.063	150	0.079	
Ting Kok Road	W	\leftarrow	A	1	3.4	0	0	0	0%	0%	2095	4190	2095	2095	4190	4190	810	0.387	0.387	585	0.279	
Ting Kok Road	W	\leftarrow	A	1	3.4	0	0	0	0%	0%	2095	0	2095	2095	0	0	810	0.387		585	0.279	
Ting Kok Road *	W	$\sqrt{}$	A	1	3.4	30	0	1	100%	100%	977.5	977.5	930	930	930	930	250	0.269		150	0.161	
Dai Kwai Street	N	↔	Е	3	3.5	18	15	0	0% / 100%	26% / 74%	2105	0	1915	1920	0	0	120	0.063	0.063	176	0.092	0.09
Dai Kwai Street	N	4	E	3	3.5	15	0	1	100%	100%	1965	4070	1785	1785	3700	3705	50	0.028		164	0.092	
			Fp Gp	4 4			_		= 12Gm + 8FC = 5Gm + 10FC		Traffic Flow	(pcu / hr)		AMı	(PM)		A.l	M. Check P	hase	P.M	A. Check P	hase
Pedestrian crossing tes: te Factor = 0.5 (Short Lane)							_				Traffic Flow 1280(1270) 120(150)	(pcu / hr) →		AMé	\leftarrow	1620(1170) 250(150)	A.l. Ey L (sec) C (sec) y pract. R.C. (%)	0.512 30 120 0.675	hase	P.M. 8y L (sec) C (sec) y pract. R.C. (%)	1. Check P 0.405 36 120 0.630 55%	hase
tes:							_				1280(1270)	(pcu / hr) →	•	 *	\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase
tes:							_				1280(1270)	(pcu/hr)	50(210)	AM(\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase
tes: te Factor = 0.5 (Short Lane)	2.						_				1280(1270)	(pcu/hr) →	50(210)	 *	\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase
tes: te Factor = 0.5 (Short Lane)	- 2	8				Min. (_				1280(1270) 120(150)	(pcu / hr) →	*\]	 *	\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase
tes: te Factor = 0.5 (Short Lane)	- 2.	В				Min. (_				1280(1270) 120(150)	(pcu / hr) → → Fp	50(210)	 *	\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase
tes: te Factor = 0.5 (Short Lane)	- 2 	B D				Min. (*E				1280(1270) 120(150)	$\overrightarrow{\rightarrow}$	₹] 50(210)	 *	\leftarrow		Ey L (sec) C (sec) y pract.	0.512 30 120 0.675	hase	εy L (sec) C (sec) y pract.	0.405 36 120 0.630	hase









RAFFIC SIGNALS CAL	CULATIO	N								Job No:	24093HK									CTA	Consu	ltant	s Ltd
Junction: Description:	Tai Po Tai 2033 Desi					Test w	ith LSI	PS/001 p	olanned	junction	improven	nent)			-								
		1	1									TI.		ı	_						1		
	uo	notation			(m)	Radi	us (m)	tor	0/1	Pro. Tu	ırning (%)	w (pcu/hr)	ion How r)	Revised Sa Flow (po		Saturat	Revised ion Flow u/hr)		AM Peal	k		PM Peak	
Approach	Direction	Movement notation	Phase	Stage	Width (m)	Left	Right	Site Factor	Nearside 0/1	AM	PM	Saturation Flow (pcu/hr)	Total Saturation Flow (pcu/hr)	AM	PM	AM	PM	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical
Yuen Shin Road	S	\downarrow	В	1,2	3.3	0.0	0	0.9*	0	0%	0%	1876.5	6046.5	1876.5	1876.5	6046.5	6046.5	469	0.250		444	0.237	
	S	Ţ	В	1,2	3.3	0.0	0		0	0%	0%	2085	0	2085	2085	0	0	521	0.250		493	0.237	
	S	\downarrow	В	1,2	3.3	0.0	0		0	0%	0%	2085	0	2085	2085	0	0	521	0.250		493	0.237	
	S	لے	C	2	3.3	0.0	17.5		0	100%	100%	2085	3961.5	1920	1920	3625	3625	275	0.143	0.143	265	0.138	0.138
	S	ل	C	2	3.3	0.0	15	0.9*	0	100%	100%	1876.5	0	1705	1705	0	0	245	0.143		235	0.138	
Tai Po Tai Wo Road	E	¬,	Е	3	3.3	0.0	22.5	0.9*	0	100%	100%	1876.5	6046.5	1760	1760	5595	5595	293	0.166	0.166	249	0.141	0.14
	Е	¬,	Е	3	3.3	0.0	20		0	100%	100%	2085	0	1940	1940	0	0	322	0.166		274	0.141	
	E	\neg	E	3	3.3	0.0	15		0	100%	100%	2085	0	1895	1895	0	0	315	0.166		268	0.141	
		_																					
Yuen Shin Road	N	1,	A	1	3.3	0.0	0	0.9*	1	0%	0%	1750.5	5920.5	1750.5	1750.5	5920.5	5920.5	396	0.226	0.226	325	0.186	0.18
	N	1	A	1	3.3	0.0	0		0	0%	0%	2085	0	2085	2085	0	0	472	0.226		387	0.186	
	N	\uparrow	A	1	3.3	0.0	0		0	0%	0%	2085	0	2085	2085	0	0	472	0.226		387	0.186	
							*																
otes:											Traffic Flow	(pcu / hr)	AM(PM) Pe					AM I	Peak Check			Peak Check	
Site factor = 0.9 (short lan	ne)										610(430) 930(790))	520(500)	1510(1430)				ey L (sec) C (sec) y pract. R.C. (%)			ey L (sec) C (sec) y pract. R.C. (%)		0.465 15 120 0.788 69%
age / Phase Diagrams													590(960)	1340(1100)									
	В			<i>†</i>	c \delta	В	3	~	D E														
G = 5	I/G	= 8				I/G	G = 8																





Revised TIA Report

We commit We deliver

APPENDIX B Fu Tip Estate (A/TP/672) Population Intake as of December 2024

24093HK (May 2025) 33



Text Size 繁 简 Other Languages 🗸

At A **PRH Public** Home **Commercial Business About** Home **Glance Application** Housing **Ownership Properties Partnerships** Us Hot Topics: HOS 2024 Estate Locator Enter Estate Name Job Vacancies **Tenders** Forms More ∨ **Global Elements Estate Locator**

Estate Locator

Property Type	PRH/TPS Estates 💙	Go
Search Property	Enter Keyword	Search
Fu Tip Estate, Tai	Po, New Territories	



Type of Estate: Public Rental Housing

Year of Intake: 2021/2024

Type(s) of Block(s): Non-Standard Block

No. of Blocks:	9
Name of Block(s):	Ban Tip House Chun Tip House Fan Tip House Fung Tip House Gaap Tip House Hei Tip House Hin Tip House Tsz Tip House Wong Tip House
No. of Rental Flats#:	7 400 As at 31.12.2024
Flat Size (m ²):	14.05-30.73
No. of Households#:	7 300 As at 31.12.2024
Authorised Population#:	17 400 As at 31.12.2024
District Tenancy Management Office/Estate Office:	Tai Po, North & Shatin District Tenancy Management Office (12) Unit 221 - 240, 2/F, Shek Yuk House, Chun Shek Estate, Shatin, N.T. Telephone: 2694 4408 Fax: 2647 1930
Property Management:	Pioneer Management Limited G/F, Tsz Tip House, Fu Tip Estate, 11 Choi Tip Street, Tai Po, N.T. Telephone: 2617 8033 Fax: 2617 8183
Carpark Management:	Pioneer Management Limited (for Ban Tip House Carpark) G/F, Tsz Tip House, Fu Tip Estate, 11 Choi Tip Street, Tai Po, N.T. Telephone: 2617 8033 Fax: 2617 8183 Yue Xiu APT Parking Limited (Excluding Ban Tip House Carpark) 27/F, Seabright Plaza, 9-23 Shell Street, North Point, H.K.

Telephone: 2512 9611

Fax: 2512 9617

Estate Website:		
Further Information:		

Quick Links

Learn More About

#Rounded to the nearest hundred

Typical floor plans

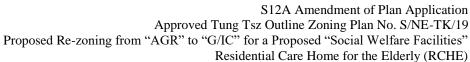
Other estates in the same district

Other districts

Access Co-ordinator and Access Officer Scheme

SITEMAP ^







Revised TIA Report
We commit We deliver

At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T

APPENDIX C

Email reply from Planning Department on Potential/Committed Developments in the Vicinity of the Proposed Development

24093HK (May 2025) 34

Catherina Chu

From: Charlotte Tsz Wing WUN/PLAND <ctwwun@pland.gov.hk>

Sent: 17 March 2025 5:31 pm

To: Catherina Chu

Cc: Ka Fai CHAN/TD; 'Horace Mak'; edmundyip@ctaconsultants.com; rlee(01); Ching Hoi

Ching NG/PLAND; Shing Fung CHAIR/PLAND

Subject: Re: Planning Application No. Y/NE-TK/19 - Departmental Comments

Attachments: Y_NE-TK_19_Departmental Comments_TIA Assumptions.pdf; Y_NE-TK_19_Departmental

Comments_TIA Assumptions_Attachments 1 & 2.pdf

Dear Ms. CHU,

I refer to the planning application (No. Y/NE-TK/19) for rezoning the application site at various lots in D.D. 23, Tung Tsz, Tai Po, New Territories from "Agriculture" and "Green Belt" to "Government, Institution or Community".

As per your enquiries on the planned/committed developments in the Traffic Impact Assessment, please find the comments from this office attached for your consideration.

[See attachment "Y NE-TK 19 Departmental Comments TIA Assumptions.pdf"][See attachment

[See attachment "Y_NE-TK_19_Deparmental Comments_TTA Assumptions.pdf"][See attachment "Y_NE-TK_19_Deparmental Comments_TTA Assumptions_Attachments 1 & 2.pdf"]

Should you have any questions related to the comments, please feel free contact the undersigned.

Thank you.

Regards, Charlotte WUN ATP/TP5 Sha Tin, Tai Po and North District Planning Office Planning Department

Tel: 2158 6018

From: Catherina Chu [mailto:catherinachu@ctaconsultants.com]

Sent: 26 February 2025 5:00 pm **To:** 'ctwwun@pland.gov.hk'

Cc: 'Ka Fai CHAN'; 'Horace Mak'; 'edmundyip@ctaconsultants.com'

Subject: RE: Planning Application No. Y/NE-TK/19 - Departmental Comments

Dear Charlotte,

We, CTA Consultants Limited, are commissioned as the traffic consultant of the captioned project.

As per comments (#v(ix) and (xi)) from TD, confirmation from PlanD regarding the adjacent committed and planned developments in the vicinity of the proposed development should be sought, and Land Sharing Pilot Scheme No. 001 at Lo Fai Road should also be considered. We shall be grateful if you could assist to provide information of the adjacent committed and planned developments in the vicinity of the site, and Land Sharing Pilot Scheme No. 001 at Lo Fai Road.

Should you have any queries or require further information, please do not hesitate to contact Mr. Horace Mak or the undersigned at 2214 0849.

Thank you very much for your kind attention and we are looking forward to your favourable reply at your earliest convenience.

Best Regards,

Catherina Chu

Chief Transport Planner

CTA Consultants Limited

Unit 2108, 21/F, Westlands Centre, 20 Westlands Road, Quarry Bay, Hong Kong Tel: (852) 2214 0849 Fax: (852) 2214 0817





Revised TIA Report
We commit We deliver

APPENDIX D Summary of 'Responses to Comments' (May 2025)

24093HK (May 2025) 35





"Summary of "Responses to Comments"

We commit We deliver

S12A Amendment of Plan Application
Approved Tung Tsz Outline Zoning Plan No. S/NE-TK/19
Proposed Re-zoning from "AGR" to "G/IC" for a Proposed "Social Welfare Facilities"
Residential Care Home for the Elderly (RCHE)
At Various Lots in D.D. 23, Tung Tsz, Tai Po, N.T
TIA Report
Summary of 'Responses to Comments' (May 2025)

Comments of Traffic Engineering (NTE) Division, Transport Department dated 25 February 2025 (Mr. CHAN Ka Fai, Issac, 2399 2406)	Responses
(v) Para. 2.4.1 – The provision of parking spaces and loading/unloading spaces shall meet the operation need, including the parking need of their staff and visitors – – please justify the sufficiency of parking spaces and loading/unloading spaces, including but not limited to	Noted and please refer to Table 2.2 of the revised TIA report for similar developments to justify the sufficiency of parking spaces and loading/unloading spaces.
making reference to similar developments; - similar to hospitals / clinics, please consider providing loading/unloading spaces for MGV/HGV as well as ambulance;	Please note that the proposed parking provision has been reference to other existing RCHE, please refer to Section 2.4 of the revised TIA report for details.
 there are 40 suites provided within the development where their parking needs should be reasonably estimated. 	Please be clarified that there are 29 nos. of suites. Please note that the proposed parking provision has been reference to other existing RCHE, please refer to Section 2.4 of the revised TIA report for details.
(vi) Para. 2.4 – Illegal parking were found at Tung Tsz Road and the adjacent village roads. Please explore the possibility to provide suitable number of public parking spaces to address the potential shortfall of parking spaces.	Taking into consideration the site constraints of the site, it is revealed that the size of site is very small (~1,500m²). As shown in Figure 2.2 , besides the essential space for fulfilling all necessary requirements of E&M and fire regulations (provision of sufficient fire escape staircases), etc., there are various structural columns inside the proposed development, very limited space is left for manoeuvring, parking / loading and unloading of motor



Divis date	nments of Traffic Engineering (NTE) sion, Transport Department d 25 February 2025 CHAN Ka Fai, Issac, 2399 2406)	Responses
		vehicles inside the site, not to mention the turnaround space. Turntable has been added for manoeuvring of vehicles to the parking spaces.
		In addition, the access road between the site and Tung Tsz Road is a single track for two-way traffic. In view of the RCHE nature where emergency vehicles will use the proposed access road, therefore public parking spaces is not proposed at the site.
(vii)	Para. 3&4 – The assessment for road links shall be included.	Noted and please refer to Table 3.3 and Table 5.2 of the revised TIA report for the assessment for road links.
(viii)	Para. 4.2.1 – Please also consider other historical traffic data, e.g. Annual Traffic Census (ATC) to determine the growth factor.	Please refer to Table 4.1 of the revised TIA report for ATC to determine the growth factor.
(ix)	Para. 4.3.1 – Please seek PlanD's confirmation on the list of committed and planned developments.	Please refer to Appendix C of the revised TIA report for the reply from PlanD regarding the list of committed and planned developments for your record.
(x)	Table 4.2 – Population intake for Planning Application Nos. A/TP/672 and A/NE-TK/702 had been commenced at the time of survey, the applicant shall consider the intake progress and justify the adoption of the trips generated and attracted by this development.	Please refer to the revised Table 4.3 and the footnotes, and Table 4.4 of the revised TIA report for the revised assumption for population intake for Planning Application Nos. A/TP/672 and A/NE-TK/702.
(xi)	Para. 4.4 – The Land Sharing Pilot Scheme No. 001 at Lo Fai Road is also undergoing planning application. Please review the assessment accordingly.	Please note that the assessment for the Land Sharing Pilot Scheme No. 001 at Lo Fai Road has been reviewed, please refer to Section 4.3 and Section 4.4 of the revised TIA report for details.



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	ments of Traffic Engineering (NTE)	
	sion, Transport Department	Responses
	d 25 February 2025	•
	. CHAN Ka Fai, Issac, 2399 2406)	
(X11)	Para. 4.6 – We notice that reference	
	was made to the trip generations of the	
	existing Tung Wah Group of Hospitals	
	Shuen Wan Complex for the Elderly –	
	 it appears that the trip generation 	Please refer to Table 4.7 of the revised TIA
	was on the low side. Please advise	report for the derivation of the trip rates of
	how the trip rates of Tung Wah	Tung Wah Group of Hospitals Shuen Wan
	Group of Hospitals Shen Wan	Complex for the Elderly.
	Complex for the Elderly was	
	obtained;	
	 please include one more reference 	Please refer to Table 4.7 of the revised TIA
	to substantiate the current rates are	report for additional reference sites to
	reasonable.	substantiate the adopted rates.
(v;;;)	Figure 2.1 – The width of run-ins	Noted and please note that the proposed width
(XIII)	should be kept to the minimum	of run-ins is ~8m, please refer to Figure 2.2
	compatible with satisfactory operation	of the revised TIA report and Figure SP-02 to
	of vehicles using the run-in. Please	Figure SP-05 for the revised swept paths.
	justify the proposed 7.5m wide run-	rigure SI -03 for the revised swept paths.
	in/out.	
	in out	
(xiv)	Figure 2.1 – Please indicate the area	Please refer to Figure 2.3 of the revised TIA
	on two sides of run-in/out and	report for the proposed run-in/out and its
	demonstrate the sightline at the run-	sightline assessment.
	in/out comply with TPDM	
	requirements.	Since the visibility splay for southbound
		vehicles does not fulfil TPDM requirement of
		60m sight distance, it is proposed to add
		flashing alarm lights as safety measure to alert
		pedestrians and drivers that vehicle is going
		out. Please refer to Section 2.3 of the revised
		TIA report for details.
(xv)	Figure 4.4 – The increase of traffic	Noted and rectified, please refer to Figure
	flow along the route are not consistent.	4.16 of the revised TIA report for details.
	Please review.	
(·)	Other	
(XV1)	Others –	
	(a) The applicant shall review if there	Please note that a footpath is proposed
	is a proper pedestrian route from the	between the Site and Tung Tsz Road, please
	is a proper pedesarian route from the	octween the one and rung 152 Road, please



Comments of Traffic Engineering (NTE) Division, Transport Department	
dated 25 February 2025	Responses
(Mr. CHAN Ka Fai, Issac, 2399 2406)	
development to public road, in particular on the part of the village access where there is a lack of separation of pedestrian and the vehicles.	
(b) We notice that minibus and coach loading / unloading are proposed. Please advise if there are shuttle services and if positive, provide details of the services including but not limited to the routes, assessment on the sufficiency of the loading / unloading provision at the pick-up points etc.	1

Comments of Traffic Engineering (NTE)	
Division, Transport Department	Dognongog
dated 24 March 2025	Responses
(Mr. CHAN Ka Fai, Issac, 2399 2406)	
TD's comments via PlanD's email	Noted.
dated 25.2.2025 remain valid.	
We have the following comments on	
the RtC to SWD's comments from	
traffic engineering perspective:	
<u>RtC 13</u>	
The provision of parking spaces and loading/unloading spaces shall meet the operation need, including the parking need for their staff and visitors. The operation of the shared-use parking space for mini-bus and emergency vehicles shall be justified.	provision has been revised and made reference to other existing RCHE, and should be sufficient for the daily operation needs of the proposed development, please refer to



Comments of Traffic Engineering (NTE) Division, Transport Department dated 24 March 2025 (Mr. CHAN Ka Fai, Issac, 2399 2406)	Responses
Access road to subject site	
The access road connecting the Site is not managed by TD. Comments from the management and maintenance departments shall be obtained. The proposed new access road and footpath shall be designed and constructed by the applicant and the management and maintenance agents shall be identified. TD will not take up the management role of a village access in village area.	Noted and please note that the proposed new access road and footpath shall be designed and constructed by the Applicant and the management and maintenance agents shall be Lands Department.
Please find the below comments from Transport Operation (NT) Division:	
Please note there is no direct public transportation to the subject site. The public could access to Tung Tsz Road by NT GMB Route Nos. 20B, 20T and Resident's service NR532, and to Ting Kok Road by NT GMB Route Nos. 20C, 20E, 20R and Bus Route Nos. 73P, 74E, 75K and 275R. The applicant could consider to provide transport services for the staff, residents and the public if necessary subject to the detailed submission by the applicant and the transport service provider.	Noted and please refer to Section 2.6 of the revised TIA report for details.



	nments of Planning Department WUN Tsz Wing, Charlotte, 2158	Responses
	Appendix 2 – Traffic Impact Assessment	
31.	The comments on the planned and committed developments incorporated in the Traffic Impact Assessment have been provided separately.	Noted and please note that comments on the planned and committed developments have been incorporated, please refer to Section 4.3 of the revised TIA report for details.
32.	Section 2.2.1 – The current Traffic Impact Assessment has not taken into account (i) the number of beds within the proposed isolated rooms and staff quarters and (ii) the estimated number of employees and visitors on daily basis. Please liaise with the TD to confirm whether they should be included for assessment purpose.	Please note that the revised TIA report has taken into account (i) the number of beds within the proposed isolated rooms and staff quarters and (ii) the estimated number of employees and visitors on daily basis for assessment purpose please refer to Section 2.2 of the revised TIA report for details.
33.	Please beef up the key findings and any proposed mitigation measures in Section 3.4 of the Supporting Planning Statement.	The peak traffic generated by the proposed development is small and would induce insignificant impact on the surrounding road network.
		The proposed mitigation measures are as follow:
		With consideration of existing road configuration, no proper footpath from the proposed development to Tung Tsz Road, minor road improvement of 3.5m wide single track for two-way traffic with widening at turning area together with a minimum 1.3m wide footpath is proposed. Please refer to Section 2.3 of the revised TIA report for details.
		To avoid overload the public transport by visitors coming/leaving the proposed development, it is proposed to provide light bus by the Applicant for the visitors with 1 veh/bound/hr during operation hour of 9am to 5pm so as to minimize traffic impact to the



Comments of Planning Department (Ms. WUN Tsz Wing, Charlotte, 2158 6018)	Responses
	surrounding road network especially Ting Kok Road while maintaining shuttle service to visitors. The proposed routing of shuttle service (light bus) will be between the proposed development and Tai Po Market Station. Please refer to Section 2.6 of the revised TIA report for details.

Comments of Drainage Services Department dated 8 April 2025 (Ms. LO Chau Ling, Yvonne, 2300 1145)	Responses
Our previous comments remain valid.	
It is noted that the TIA report and the conclusion made therein had been commented by TD and the resubmission of TIA from the applicant is required. In this connection, any proposed road / traffic improvement works on public roads shall be carried out by the applicant at their own expenses according to the latest TPDM and HyD standards.	Noted and please note that any proposed road / traffic improvement works on public roads shall be carried out by the applicant at their own expenses according to the latest TPDM and HyD standards.