

## **Annex C**

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*Updated Report of the Sewerage Impact Assessment*

# Lot 4822 in D.D. 104, Kam Pok Road and Adjoining Government Land, Mai Po, Yuen Long

## Sewerage Impact Assessment

March 2026

Date	Revision	Prepared by	Checked by	Approved by
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## 1. INTRODUCTION

- 1.1. Meinhardt Infrastructure & Environment Limited (MIEL) has been commissioned to conduct this Sewerage Impact Assessment (“SIA”) to demonstrate the proposed medium-rise residential development on the Application Site is technically feasible in the sewage disposal aspect.
- 1.2. The Application Site (hereinafter referred to as “the Site”) is located east of Kam Pok Road near Fairview Park in Yuen Long. The Site location is shown in **Appendix A**.
- 1.3. The objectives of this SIA are:
  - Identification of existing sewerage facilities serving the concerned area;
  - Description of methodology adopted in this assessment;
  - Estimation of sewage flow from the subject development;
  - Assessment of sewerage impact on the existing or/and planned sewerage facilities due to the increased sewage flow from the subject development; and
  - Recommendation of on-site sewerage facilities (on-site sewage pumping station) to cater for the development if found necessary.

## 2. GENERAL SITE DESCRIPTION

- 2.1. The Site is bounded by Fung Chuk Road, Ha Chuk Yuen Road, Kam Pok Road and Ha San Wai Road covering a total land area of about 37,870m<sup>2</sup>. It is located in the vicinity of Fairview Park, Villa Camellia, Helene Terrace, 3-storey village dwellings in Ha San Wai and Yau Pok Road Light Public Housing.
- 2.2. The existing topography of the Site is sloping gently from the north towards the south with ground level varies from +7.0mPD to +4.3mPD approximately.

## 3. DEVELOPMENT PROPOSAL

- 3.1. The proposed development comprises five medium-rise residential buildings with a domestic plot ratio of 1.5, and one 2-storey facility compound containing one club house, one 6-classroom kindergarten and an **elderly activity centre**. The building heights of these 5 residential towers are 16 residential storeys high. The total domestic GFA is about 56,805m<sup>2</sup>. Upon completion in 2031, the proposed development will provide a total of 1,303 private high-quality flats.
- 3.2. The proposed development also includes a two-storey facility compound comprising a clubhouse (of about 2,272 m<sup>2</sup> exempted GFA) which includes a restaurant, an indoor swimming pool and an outdoor swimming pool; one 6-classroom kindergarten (of about 380m<sup>2</sup> non-domestic GFA); One **elderly activity centre** with NOFA is about 303m<sup>2</sup>.
- 3.3. The proposed master layout plan is shown in **Appendix A**.

## 4. EXISTING AND PLANNED SEWERAGE SYSTEM

### Existing Sewerage System

- 4.1. Based on the sewerage records, there are no existing sewers in the vicinity of the Site. The closest existing sewerage facility is Nam Sang Wai Sewage Pumping Station (NSWSPS), which is located

west of Pok Wai South Road. The collected sewage in NSWSPS is eventually conveyed to existing Yuen Long Sewage Treatment Works (YLSTW).

- 4.2. The designed capacity and the current average daily flow in terms of ADWF (m<sup>3</sup>/day) of NSWSPS are shown in **Table 1**.

**Table 1 – Capacity of NSWSPS**

Sewerage Facility	Design Capacity (m <sup>3</sup> /day)	Current Average Daily Flow (m <sup>3</sup> /day)	Spare Capacity (m <sup>3</sup> /day)
NSWSPS	42,921	3,900	39,021

Planned Sewerage System

- 4.3. According to the approved EIA report for “Yuen Long Effluent Polishing Plant – Investigation, Design and Construction” (EIA-259/2018), DSD proposed to upgrade existing YLSTW to Yuen Long Effluent Polishing Plant (YLEPP) to enhance its capacity and treatment level. The upgrade works would be carried out in 2 stages. Under Stage 1, YLSTW will be upgraded to YLEPP with treatment capacity of 100,000m<sup>3</sup>/day. The project completion date of YLEPP – Stage 1 is mid-2027, which will be completed before the completion of the proposed development at the Site in 2031. The following Stage 2 works being planned to increase YLEPP treatment capacity to 180,000m<sup>3</sup>/day by 2030 tentatively.
- 4.4. According to the approved Planning Application No. Y/YL-NSW/7, there will be gravity sewers ranging from 375mm to 600mm diameter located along Pok Wai West Road and Pok Wai South Road connecting to NSWSPS. These sewers from manhole WKT009 to manhole WKT015 are planned to serve the potential development in the surrounded area with a minimum capacity of 15,500m<sup>3</sup>/d, which is an initial estimation by EPD, and **Application Nos. Y/YL-MP/7, Y/YL-MP/8** and proposed development at Yau Mei San Tsuen with an estimated sewage flow of **1,256.86m<sup>3</sup>/d, 1,258.27m<sup>3</sup>/d** and 962.28 m<sup>3</sup>/d respectively. Apart from the abovementioned, 2 x 675mm gravity sewers are proposed under Approved Planning Application No. Y/YL-MP/10. The estimated sewage flow from **Application Nos. Y/YL-MP/7, Y/YL-MP/8** and Yau Mei San Tsuen **are** shown in **Appendix B**. The proposed completion year of this rezoning development (Y/YL-NSW/7) is 2028.

**5. ASSESSMENT METHODOLOGY AND PARAMETERS**

- 5.1. Capacity assessment of the sewers, pumping station and sewage treatment plant is being carried out to assess the adequacy of the capacity in the existing or/and planning sewerage system. The design parameters and standards are shown in **Table 2** below.

**Table 2 - Design Parameters and Standards**

<b>Design Standards/ Guidelines</b>	<ul style="list-style-type: none"> <li>• DSD Sewerage Design Manual, Part 1 &amp; 2 (SDM)</li> <li>• EPD Guideline for Estimating Sewage Flows for Sewage Infrastructure Planning (GESF)</li> <li>• Data published by Census and Statistics Department (C&amp;SD)</li> <li>• Commercial and Industrial Floor Space Utilization Survey (CIFSUS) by Planning Department</li> </ul>
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<b>Flow Formula</b>	Colebrook White Formula
<b>Sewer Roughness Coefficient, Ks</b>	1.5 mm
<b>Unit Flow Factor</b>	<ul style="list-style-type: none"> <li>• 0.27m<sup>3</sup>/d/head (Domestic, Private R2)</li> <li>• 0.28m<sup>3</sup>/d/head (Commercial Employee plus “Community, Social &amp; Personal Services”, J11)</li> <li>• 1.58m<sup>3</sup>/d/head (Commercial Employee plus “Restaurants &amp; Hotels”, J10)</li> <li>• 0.04m<sup>3</sup>/d/head (School Student)</li> </ul>
<b>Contributing Population</b>	Contributing Population = Calculated ADWF/0.27
<b>Peaking Factors</b>	Table T-5 of GESF
<b>Catchment Inflow Factor</b>	Table T-4 of GESF Catchment Inflow Factors (PCIF): Yuen Long =1.00
<b>Average Household Size</b>	2.7(reference to Table 150-12001: Projected number of domestic households and average household size of 2026 and 2031 whichever larger, by the Census and Statistics Department)
<b>Staff Density of Club House</b>	Table 8 of CIFSUS Community, Social & Personal Services: 2.3 workers per 100m <sup>2</sup> GFA
<b>No. of Staff in Restaurant</b>	15
<b>No. of Student in Kindergarten</b>	180 (6 classes of 30 students)
<b>No. of Teacher in Kindergarten</b>	23 (Pupil-Teacher Ratio is 8:1 under Key Statistics on Kindergarten Education)
<b>Elderly Activity Centre Population</b>	137.5 (4m <sup>2</sup> /person) (with GFA of 550 m <sup>2</sup> )

## 6. ESTIMATION OF SEWAGE FLOW

6.1. The estimation of sewage flow of the proposed development is shown in **Appendix C** and summarized in **Table 3** below.

**Table 3 - Total Estimated Sewage Flow from the Proposed Development**

Population Type	No. of Population	Unit Flow Factor (m <sup>3</sup> /d/person)	Average Dry Weather Flow (m <sup>3</sup> /d)
Residential Population	3,519	0.27	950.13
Staff of Clubhouse	53	0.28	14.84
Staff of Restaurant	15	1.58	23.70
Students in Kindergarten	180	0.04	7.20
Teachers in Kindergarten	23	0.28	6.44
Elderly Activity Centre Population	138	0.28	38.64
Outdoor Swimming Pool	-	-	7.03

Indoor Swimming Pool	-	-	8.44
<b>Total</b>	3,928	-	<b>1,056.42</b>

6.2. The estimated Average Dry Weather Flow (ADWF) from the proposed development is approximately 1,056.42m<sup>3</sup>/day.

## 7. PROPOSED SEWERAGE SYSTEM

7.1. The sewage from the proposed development will be collected by the proposed gravity sewers within the Site and then conveyed to the proposed on-site sewage pumping station with the installed capacity not less the peak flow (3,180m<sup>3</sup>/day). The sewage will then be pumped by a twin DN150 rising mains to the gravity sewer and eventually be conveyed to NSWSPS.

7.2. Under normal circumstances, the sewage from the proposed rezoning development will be directed into NSWSPS via a proposed sewer line (PSL), which runs along the east of Kam Pok Road and Pok Wai Road South, ending at the existing manhole FSH1001886 that connects to NSWSPS, as depicted in Figure 2 in **Appendix D**.

7.3. There are five nearby up-zoning application nos. (Y/YL-NSW/7, Y/YL-MP/7, Y/YL-MP/8, Y/YL-MP/9 and Y/YL-MP/10) and a potential site development at Yau Mei San Tsuen. All six proposed developments are expected to be completed and populated by 2031 (Y/YL-NSW/7 is expected to be completed by 2028), coinciding with the timeline of this rezoning application.

7.4. To manage the cumulative impacts, there are two scenarios:

Scenario 1 – If this application is to be proceeded before all the six proposed developments (under application nos. Y/YL-NSW/7, Y/YL-MP/7, Y/YL-MP/8, Y/YL-MP/9 and Y/YL-MP/10 and the potential site development at Yau Mei San Tsuen) – The Applicant will be responsible for the construction entire section of the PSL, accounting for these six potential developments, subject to agreement with the Drainage Services Department (DSD) and Environmental Protection Department (EPD). The long-term maintenance responsibility for gravity sewers conveying sewage from multiple developers will be handed over to DSD. Figure 3 in **Appendix D** illustrates the schematic alignment and preliminary technical details of the PSL.

Scenario 2 - If application nos. Y/YL-NSW/7 and Y/YL-MP/10 are completed as planned by 2028 and 2031 respectively, or before the commencement of the current subject rezoning application, the Applicant will liaise directly with the developer involved to arrange the sharing of PSL works. Full implementation of the PSL will be proposed, subject to agreement from DSD and EPD.

Scenario 3 – Given application nos. Y/YL-MP/7, Y/YL-MP/8, Y/YL-MP/9 and the potential site development at Yau Mei San Tsuen do not have planned completion date, if any of or all the aforesaid 4 applications are completed before the commencement of this application, the approach of PSL works will be the same as that stipulated in Scenario 2. The Applicant will be responsible for the proposed upgrading works of the sewerage system to ensure adequate pipe capacity to cater for all the six proposed developments.

Figure 4 in **Appendix D** illustrates the schematic alignment and preliminary technical details of the PSL. The sewage will be pumped from NSWSPS to YLEPP eventually.

7.5. The hydraulic checking of the proposed rising mains is shown in **Appendix E**.

## 8. SEWERAGE IMPACT ASSESSMENT

8.1. The sewerage impact on NSWSPS and the gravity sewers due to the sewage flow generated by the proposed development within the Site has been assessed.

8.2. The results of the sewerage assessment on NSWSPS are summarized in **Table 4**. **Table 5-6** summarizes the implications of the gravity sewers in the two scenarios. The hydraulic calculations for the planned gravity sewers are shown in **Appendix F**.

**Table 4 – Assessment Results of NSWSPS**

Sewerage Facility	Design Capacity (m <sup>3</sup> /day)	Current Average Daily Flow (m <sup>3</sup> /day)	Estimated Future Average Daily Flow (m <sup>3</sup> /day) <sup>(1)</sup>	Total Future Average Daily Flow (m <sup>3</sup> /day)	Spare Capacity (m <sup>3</sup> /day)
NSWSPS	42,921	3,900	20,033.83	23,933.83	18,987.17

(1) The estimated future flow includes the EPD initial estimation for the gravity sewers (i.e. 15,500m<sup>3</sup>/d), the estimated sewage flow from **Application No. Y/YL-MP7, Y/YL-MP8** (i.e. 1,256.86m<sup>3</sup>/d and 1,258.27m<sup>3</sup>/d respectively) and the potential development at Yau Mei San Tsuen (i.e. 962.28m<sup>3</sup>/d) (**Appendix B** refers) and the sewage flow generated from this proposed development. (**Appendix C** refers).

**Table 5 – Assessment Results of the Gravity Sewers (Scenario 1)**

Nominal Diameter (mm)	Average Daily Flow (m <sup>3</sup> /day)	Average Daily Flow (m <sup>3</sup> /s)	Peaking Factor	Peak Daily Flow (m <sup>3</sup> /s)	Gradient (1in X)	Flow Velocity (m/s)	Flow Capacity (m <sup>3</sup> /s)
<b>Proposed</b> Gravity Sewers under this planning application							
1000	20,033.83	0.232	3.83	0.887	400	1.441	1.001

**Table 6 – Assessment Results of the Gravity Sewers (Scenario 2)**

	Nominal Diameter (mm)	Average Daily Flow (m <sup>3</sup> /day) <sup>(1)</sup>	Average Daily Flow (m <sup>3</sup> /s)	Peaking Factor	Peak Daily Flow (m <sup>3</sup> /s)	Gradient (1in X)	Flow Velocity (m/s)	Flow Capacity (m <sup>3</sup> /s)
<b>Proposed</b> Gravity Sewers under Y/YL-MP/10								
Existing Design	2 x 675	20,033.83	0.232	3.83	0.887	400	1.167	0.835
Proposed Upgrade	1000	20,033.83	0.232	3.83	0.887	241	1.858	1.113
<b>Proposed</b> Gravity Sewers under Y/YL-NSW/7								
Existing Design	600	20,033.83	0.232	3.83	0.887	300	1.241	0.342
Proposed Upgrade	1000	20,033.83	0.232	3.83	0.887	241	1.858	1.291

(1) The estimated future flow includes the EPD initial estimation for the gravity sewers (i.e. 15,500m<sup>3</sup>/d), the estimated sewage flow from **Application No. Y/YL-MP7, Y/YL-MP8** (i.e. 1,256.86m<sup>3</sup>/d and 1,258.27m<sup>3</sup>/d respectively) and the potential development at

Yau Mei San Tsuen (i.e. 962.28m<sup>3</sup>/d) (Appendix B refers) and the sewage flow generated from this proposed development (Appendix C refers).

- 8.3. For scenario 1, the proposed DN1000 sewer has sufficient capacity to cater the sewage flow generated from the proposed development within the Site and the potential new developments.
- 8.4. For scenario 2, the results of the assessment have demonstrated that NSWSPS and the proposed gravity sewers (2 x DN675) under application no. Y/YL-MP/10 almost use up the capacity to cater the sewage flow generated from the proposed development within the Site. It is suggested to upgrade the sewer to DN1000. Besides, it is found that the existing design of the planned gravity sewers under application no. Y/YL-NSW/7 in scenario 1, i.e. DN600, has insufficient capacity to cater its planned ADWF of 20,033.83m<sup>3</sup>/day. It is suggested to upgrade the sewer size to DN1000. The abovementioned upgrade will allow sufficient capacity to cater the planned sewage flow and the sewage flow generated by the proposed development within the Site. The Applicant will be responsible for the upgrading of the proposed gravity sewers.
- 8.5. After upgrade the planned gravity sewers, there will be no adverse sewerage impact on the planned gravity sewers, NSWWS and YLEPP due to this proposed development is anticipated.
- 8.6. A comprehensive study on the technical feasibility focusing on the available underground space will be undertaken. Consideration will be given to construction feasibility, maintenance access, and future expansion possibilities to ensure the proposed rising mains can be safely and effectively accommodated within the constrained underground environment. Where necessary, coordination with relevant government departments, utility providers and other developments will be conducted to optimize the design and layout of the rising mains. The detailed design will be submitted at detailed design stage.

## 9. MAINTENANCE RESPONSIBILITY

- 9.1. All proposed sewers and sewerage facilities within the proposed development will be constructed, operated and maintained by the Applicant.
- 9.2. The proposed external twin DN150 rising mains will be constructed and maintained by the Applicant.
- 9.3. For the gravity sewers, both scenario 1 and scenario 2 are technically feasible. The Applicant will be responsible for the upgrading of the proposed gravity sewers. Maintenance of the proposed gravity sewers exclusively serving the development remains the responsibility of the Applicant at their own cost. Upon the proposed sewers becoming facilities that serve multiple users, the Applicant shall hand over the relevant sewer segments located on government land to the Drainage Services Department (DSD) for long-term maintenance.

## 10. CONCLUSION

- 10.1. This report has assessed the sewerage impact due to the proposed development of Lot No. 4822 in D.D. 104 and adjoining Government Land, Kam Pok Road, Mai Po, Yuen Long.
- 10.2. The sewage flow generated from the proposed development will be collected by proposed gravity sewers within the Site and then conveyed into the proposed sewer line via an on-site underground sewage pumping station. The sewage flow will continue to be conveyed by gravity sewers in one

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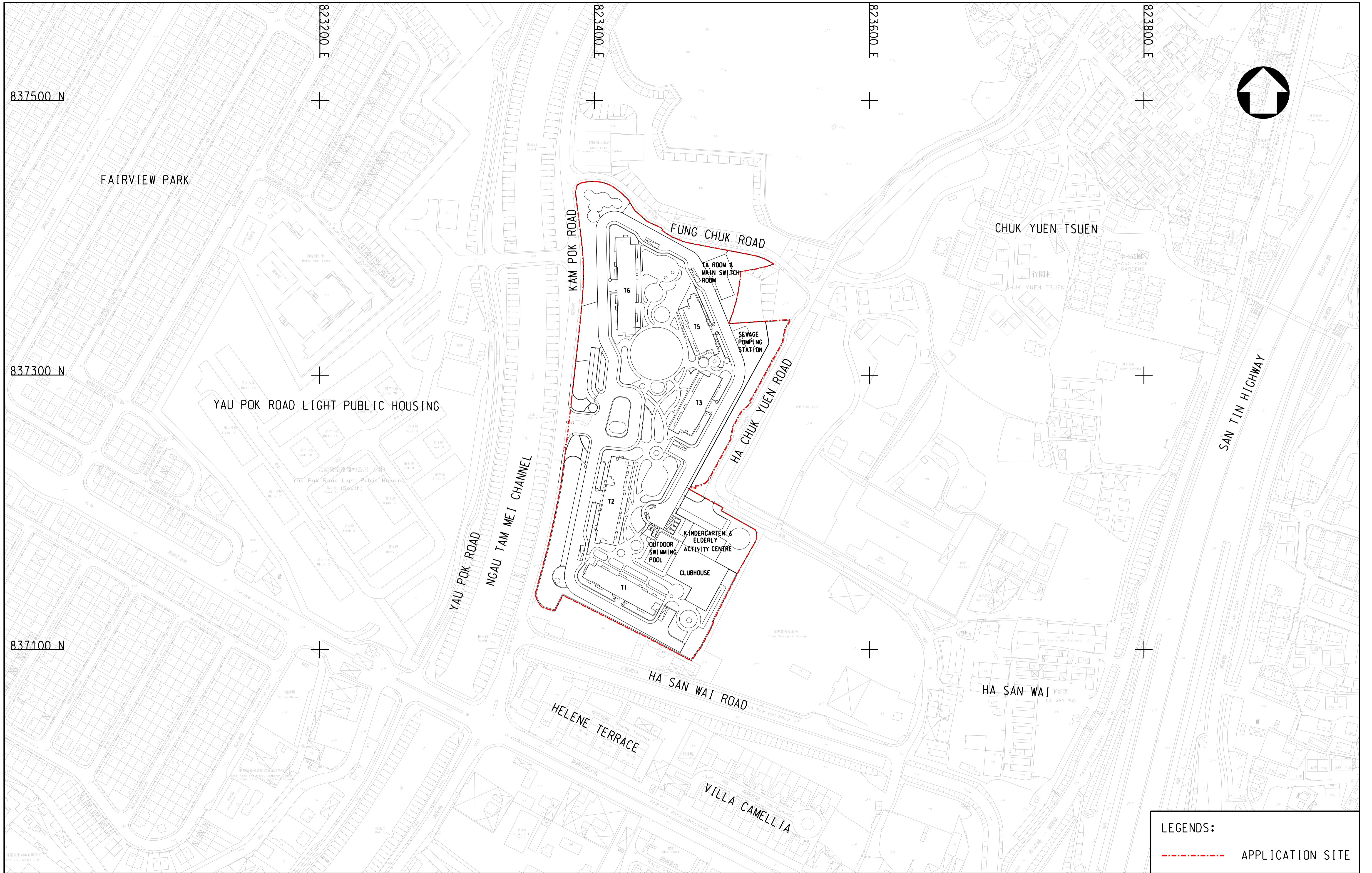
of the two scenarios: scenario 1: DN1000 sewer and scenario 2: 2 x DN675 sewer and DN600 sewer (recommended to be upgraded to sewer DN1000).

10.3. The results of the assessment have demonstrated that both proposed scenario options are technically feasible from the sewerage engineering standpoint.

**APPENDIX A –  
SITE LOCATION AND PROPOSED MASTER  
LAYOUT PLAN**

PLOTTED BY : \$USERS

PLOT DRY : \$PLTDRAWL\$



**LEGENDS:**

--- APPLICATION SITE

**APPENDIX A –  
SITE LOCATION AND PROPOSED MASTER  
LAYOUT PLAN**

Project : Lot 4822 in D.D. 104 and the adjoining Government  
 Project No. : Land, Kam Pok Road, Mai Po, Yuen Long  
 Project No. : 91469  
 Prepared by : HSC  
 Date : 10/02/2026



Checked by : CYM  
 Sheet No. : 1

**Estimation of Sewage Flow from the Y/YL-MP 7**

Residential Population

Residential Population (person)	=	3,316.00
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.37
ADWF (m <sup>3</sup> /day)	=	1,226.92
ADWF (l/s)	=	14.20

Domestic - Private R3

Community Facilities

*Total Gross Floor Area (GFA) (m <sup>2</sup> )	=	1,760.00
Assumed Worker Density (workers per 100m <sup>2</sup> GFA)	=	3.30
No. of Staff	=	58.08
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.28
ADWF (m <sup>3</sup> /day)	=	16.26
ADWF (l/s)	=	0.19

Table 8 of CIFSUS - All Types - Community, Social & Personal Services

Commercial - J11

Transport

*Total Gross Floor Area (GFA) (m <sup>2</sup> )	=	2,000.00
Assumed Worker Density (workers per 100m <sup>2</sup> GFA)	=	3.80
No. of Staff	=	76.00
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.18
ADWF (m <sup>3</sup> /day)	=	13.68
ADWF (l/s)	=	0.16

Assumed 35% of Non-Domestic GFA

Table 8 of CIFSUS - All Types - Transport

Commercial - J3

<b>Total ADWF (m<sup>3</sup>/day)</b>	=	<b>1,256.86</b>
<b>ADWF (l/s)</b>	=	<b>14.55</b>

\* Further to Development Proposal in Respect of Application No. Y/YL-MP/7 based on the further information received on 16.1.2026, 1760m<sup>2</sup> will be used 'Shop and Services', 'Eating Place', 'School' (nursery/kindergarten/language, computer, commercial or tutorial schools/technical institutes/other types of schools providing interests and hobby related courses for subjects such as arts and ballet etc.), 'Place of Entertainment' and 'Place of Recreation, Sports or Culture' uses, and transport lay-by facilities with GFA of about 2,000 m<sup>2</sup>.

Project : Lot 4822 in D.D. 104 and the adjoining Government  
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 Date : 10/02/2026



Checked by : CYM  
 Sheet No. : 1

**Estimation of Sewage Flow from the Y/YL-MP 8**

Residential Population

Residential Population (person)	=	3,373.00
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.37
ADWF (m <sup>3</sup> /day)	=	1,248.01
ADWF (l/s)	=	14.44

Domestic - Private R3

Transport

*Total Gross Floor Area (GFA) (m <sup>2</sup> )	=	1,500.00
Assumed Worker Density (workers per 100m <sup>2</sup> GFA)	=	3.80
No. of Staff	=	57.00
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.18
ADWF (m <sup>3</sup> /day)	=	10.26
ADWF (l/s)	=	0.12

Assumed 35% of Non-Domestic GFA  
 Table 8 of CIFSUS - All Types - Transport

Commercial - J3

<b>Total ADWF (m<sup>3</sup>/day)</b>	<b>=</b>	<b>1,258.27</b>
<b>ADWF (l/s)</b>	<b>=</b>	<b>14.56</b>

\* Further to Development Proposal in Respect of Application No. Y/YL-MP/8 based on the further information received on 16.1.2026, transport lay-by facilities with GFA of not less than 1,500 m<sup>2</sup>.

Project : Lot 4822 in D.D. 104 and the adjoining Government  
Project No. : 91469  
Prepared by : HSC  
Date : 10/02/2026



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Sheet No. : 2

**Estimation of Sewage Flow from the Proposed Development at Yau Mei San Tsuen**

Residential Population

Residential Population (person)	=	3,564.00
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.27
ADWF (m <sup>3</sup> /day)	=	962.28
ADWF (l/s)	=	11.14

Domestic - Private R2

<b>Total ADWF (m<sup>3</sup>/day)</b>	<b>=</b>	<b>962.28</b>
<b>ADWF (l/s)</b>	<b>=</b>	<b>11.14</b>

**APPENDIX C –  
ESTIMATION OF SEWERAGE FLOW FOR THE  
PROPOSED DEVELOPMENT**

Project : Lot 4822 in D.D. 104 and the Adjoining Government  
 Project No. : Land, Kam Pok Road, Mai Po, Yuen Long  
 Project No. : 91469  
 Prepared by : HSC  
 Date : 10/02/2026



Checked by : CYM  
 Sheet No. : 1

**Estimation of Sewage Flow from the Proposed Development**

Residential

No. of Units	=	1303
Average Household size per unit	=	2.7
Residential Population (person)	=	3519
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.27
ADWF (m <sup>3</sup> /day)	=	950.13
ADWF (l/s)	=	11.00

Domestic - Private R2

Club House

No. of Storeys	=	2
Gross Floor Area per storey	=	1136
Total Gross Floor Area (GFA) (m <sup>2</sup> )	=	2272
Assumed Worker Density (workers per 100m <sup>2</sup> GFA)	=	2.3
No. of Staff	=	53
Unit Flow Factor (m <sup>3</sup> /d/head)	=	0.28
ADWF (m <sup>3</sup> /day)	=	14.84
ADWF (l/s)	=	0.17

Table 8 of CIFSUS - Private Commercials - Community, Social & Personal Services

Commercial - J11

Restaurant

No. of Staff	=	15
Unit Flow Factor (m <sup>3</sup> /d/head)	=	1.58
ADWF (m <sup>3</sup> /day)	=	23.70
ADWF (l/s)	=	0.27

Commercial - J10

Kindergarten

Nos. of Students	=	180
Nos. of Teacher	=	23
Unit Flow Factor - Student (m <sup>3</sup> /d/head)	=	0.04
Unit Flow Factor - Teacher (m <sup>3</sup> /d/head)	=	0.28
ADWF (m <sup>3</sup> /day)	=	13.64
ADWF (l/s)	=	0.16

Assume 6 classes and 30 students in a class  
 Pupil-Teacher Ratio is around 8:1 under Key  
 Statistics on Kindergarden Education

Commercial - J11

Elderly Activity Centre

Population	=	138
Unit Flow Factor - Elderly / Staff (m <sup>3</sup> /d/head)	=	0.28
ADWF (m <sup>3</sup> /day)	=	38.64
ADWF (l/s)	=	0.45

Commercial - J11

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Checked by : CYM  
 Sheet No. : 1

Outdoor Swinning Pool

Swimming Pool Volume (m <sup>3</sup> ) (25m x 15m x 1.5m)	=	562.5
Turnover rate (hours)	=	6
Assumed Surface Loading Rate of Filter (m <sup>3</sup> /m <sup>2</sup> /hr)	=	20
Filter Area Required (m <sup>2</sup> )	=	4.69
Duration of Backwash (min/day)	=	3.00
Assumed Backwash Flowrate (m <sup>3</sup> /m <sup>2</sup> /hr)	=	30.00
Volume of Backwash (m <sup>3</sup> /day)	=	7.03
Assumed Discharge Duration (min)	=	3
Discharge Flow Rate (m <sup>3</sup> /min)	=	2.34
Discharge Flow Rate (m <sup>3</sup> /s)	=	0.04

B1.1.2 Outdoor Swimming Pool Turnver Rate -  
General Specification for Swimming Pool Water  
Treatment Installation

Indoor Swinning Pool

Swimming Pool Volume (m <sup>3</sup> ) (20m x 15m x 1.5m)	=	450
Turnover rate (hours)	=	4
Assumed Surface Loading Rate of Filter (m <sup>3</sup> /m <sup>2</sup> /hr)	=	20
Filter Area Required (m <sup>2</sup> )	=	5.63
Duration of Backwash (min/day)	=	3.00
Assumed Backwash Flowrate (m <sup>3</sup> /m <sup>2</sup> /hr)	=	30.00
Volume of Backwash (m <sup>3</sup> /day)	=	8.44
Assumed Discharge Duration (min)	=	3
Discharge Flow Rate (m <sup>3</sup> /min)	=	2.81
Discharge Flow Rate (m <sup>3</sup> /s)	=	0.05

B1.1.2 Indoor Swimming Pool Turnver Rate -  
General Specification for Swimming Pool Water  
Treatment Installation

<b>Total ADWF (m<sup>3</sup>/day)</b>	=	<b>1056.42</b>
<b>ADWF (l/s)</b>	=	<b>12.23</b>



## Table 150-12001 : Projected number of [domestic households](#) and average household size

[Full Series](#)

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### Statistics

Year	Domestic households	Average annual growth in domestic households over the past 5 years	Average household size
	Number	Number of domestic households	Number of persons
1981	1 244 700	-	3.9
1986	1 452 600	41 600	3.7
1991	1 582 200	25 900	3.4
1996	1 855 600	54 700	3.3
2001	2 053 400	39 600	3.1
2006	2 226 500	34 600	3.0
2011	2 368 800	28 500	2.9
2016	2 509 700	28 200	2.8
2021	2 674 200	32 900	2.7
2026 J	2 780 200	21 200	2.7
2031 J	2 884 800	20 900	2.6
2036 J	2 974 500	17 900	2.6
2041 J	3 042 800	13 700	2.6
2046 J	3 076 600	6 800	2.6

**APPENDIX D –  
LAYOUT PLAN FOR PROPOSED PRIVATE SEWAGE  
PUMPING STATION AND RISING MAINS**

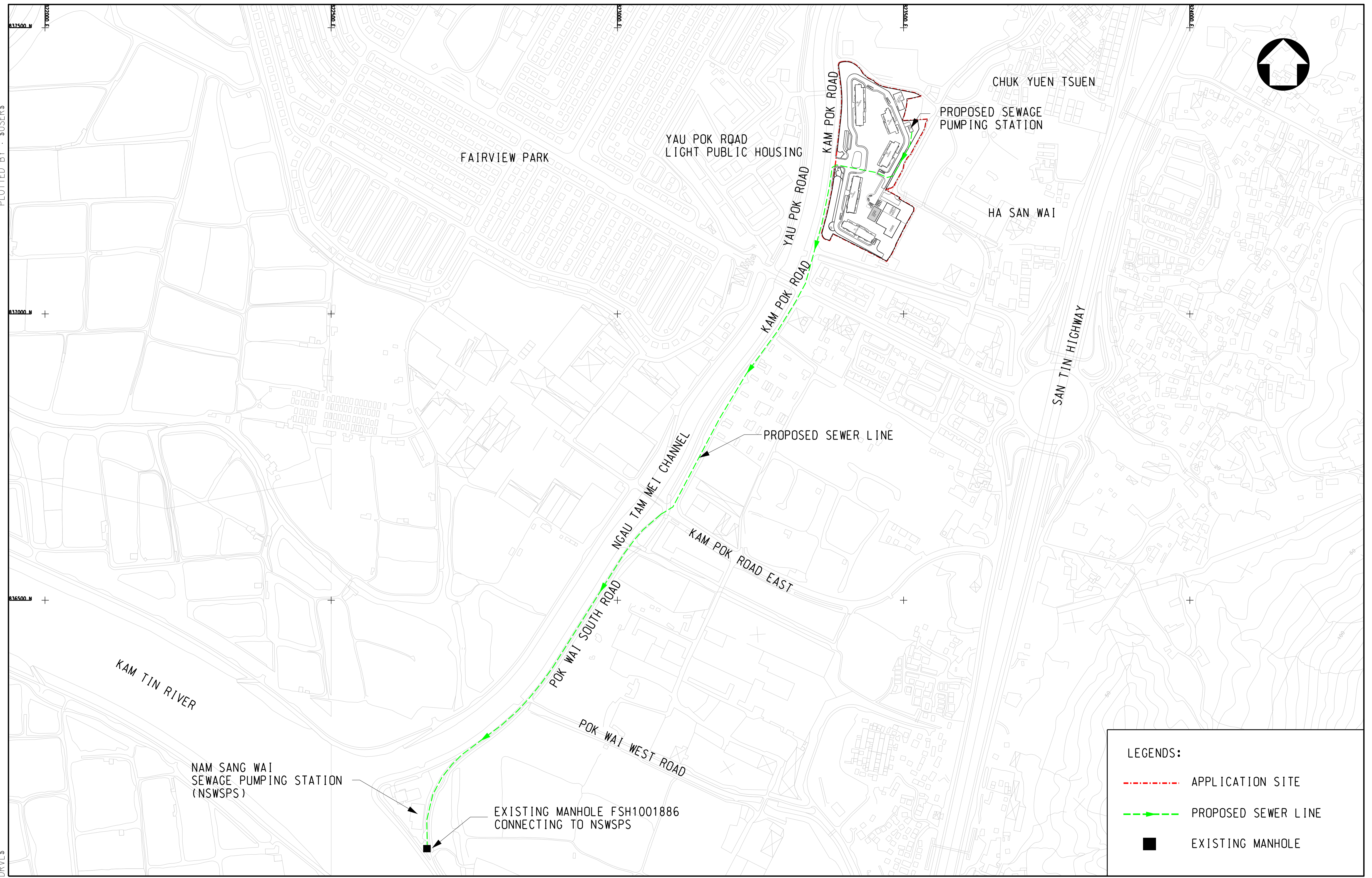
PLOTTED BY : \$USERS\$

837500.0

837000.0

PLOT.DRV : \$PLTDRAWL\$.S

CAD FILE : \$FILES\$



**LEGENDS:**

- - - APPLICATION SITE
- - - PROPOSED SEWER LINE
- EXISTING MANHOLE

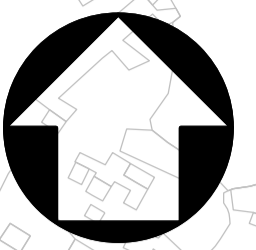
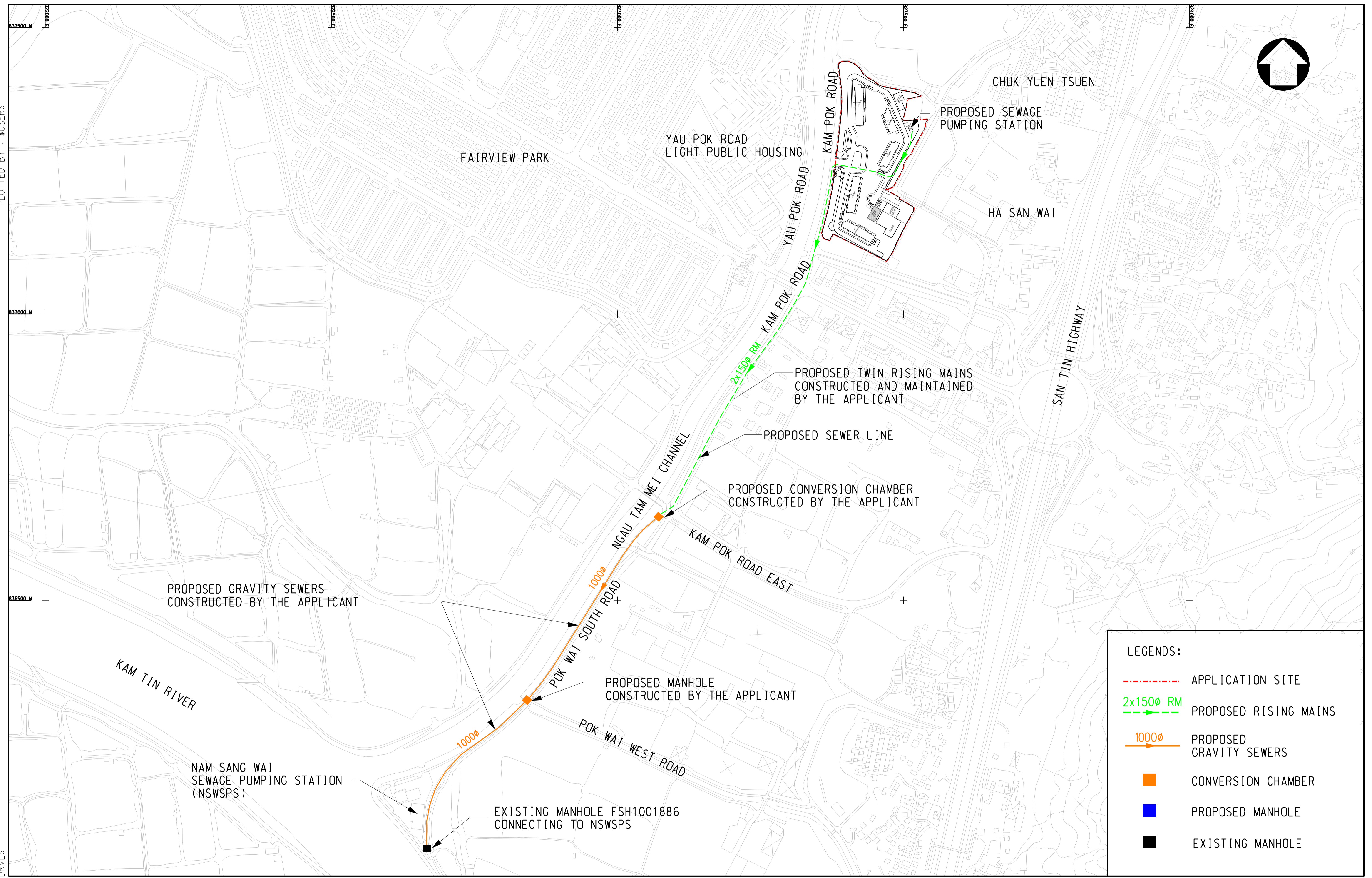
PLOTTED BY : \$USERS\$

837500.0

837000.0

PLOT DRY : \$PLTDRAWL\$

CAD FILE : \$FILES\$



- LEGENDS:**
- - - APPLICATION SITE
  - - - 2x150ø RM PROPOSED RISING MAINS
  - 1000ø PROPOSED GRAVITY SEWERS
  - CONVERSION CHAMBER
  - PROPOSED MANHOLE
  - EXISTING MANHOLE

PLOTTED BY : \$USERS\$

837500.0

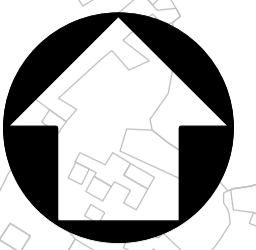
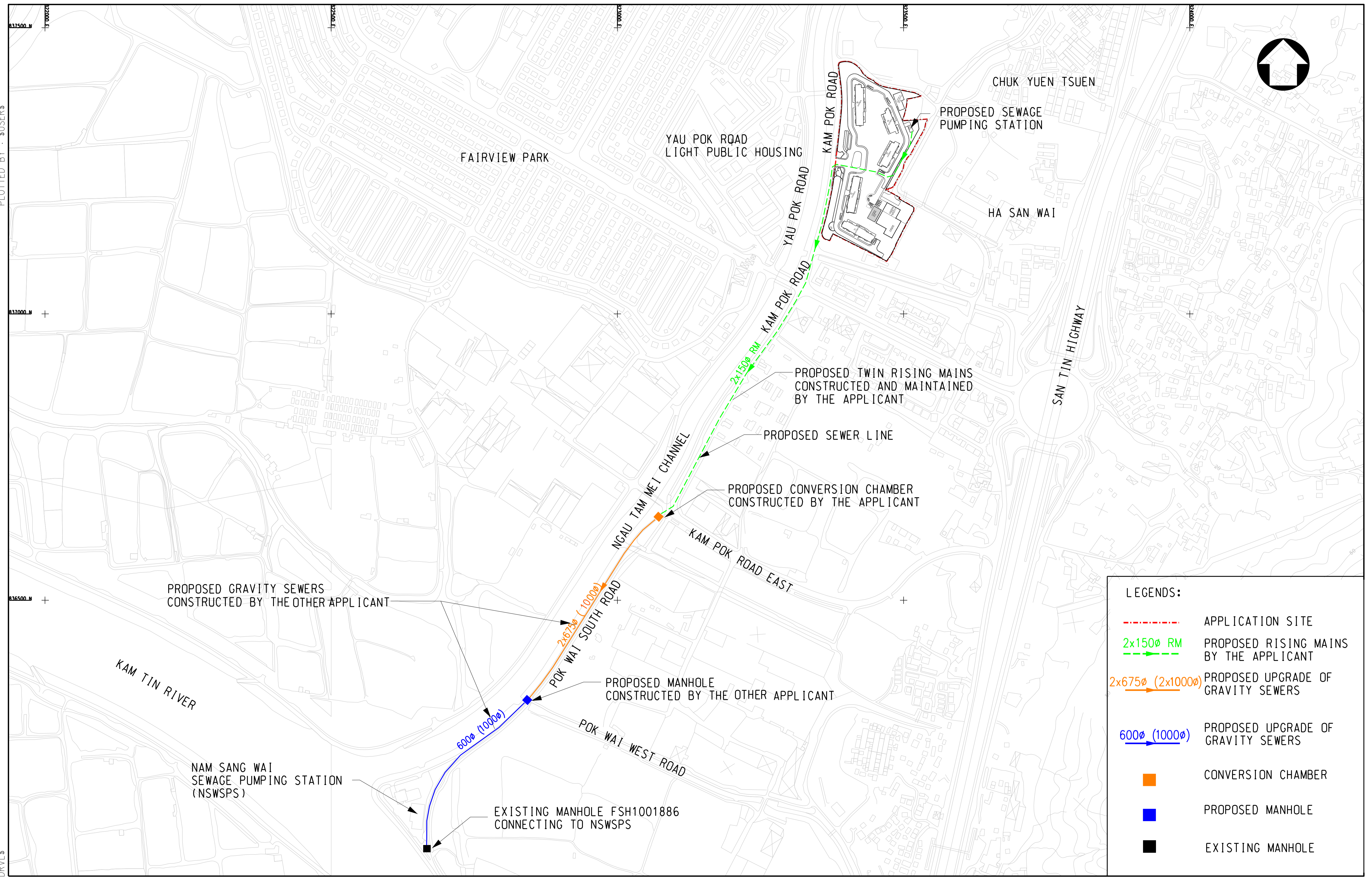
+

837000.0

+

PLOT DRY : \$PLTDRAWL\$

CAD FILE : \$FILES\$



**LEGENDS:**

	APPLICATION SITE
	PROPOSED RISING MAINS BY THE APPLICANT
	PROPOSED UPGRADE OF GRAVITY SEWERS
	PROPOSED UPGRADE OF GRAVITY SEWERS
	CONVERSION CHAMBER
	PROPOSED MANHOLE
	EXISTING MANHOLE

## **APPENDIX E – HYDRAULIC ASSESSMENT FOR RISING MAINS**

Project : Lot 4822 in D.D. 104 and Adjoining Government Land,  
Project No. : Kam Pok Road, Mai Po, Yuen Long  
Prepared by : 91469  
Date : HSC  
Date : 11/12/2025



Checked by : CYM  
Sheet No. : 1

**Hydraulic Assessment of Proposed Pump Rate**

Total ADWF (m <sup>3</sup> /d)	=	1,056.42
Contributing Population	=	3,912.67
Peaking Factor	=	4.00
Total PDWF (m <sup>3</sup> /d)	=	4,225.68
Total PDWF (l/s)	=	48.91
Design Pump Capacity (l/s)	=	50.00

**Design of Proposed Twin Rising Mains**

Diameter (mm)	=	140.40
Flow Area of one Rising Main (m <sup>2</sup> )	=	0.02
Flow in one Rising Main (m <sup>3</sup> /s)	=	0.03
Flow Velocity in one Rising Main (m/s)	=	1.61

## **APPENDIX F – HYDRAULIC ASSESSMENT FOR SEWERS**

Hydraulic Checking for Gravity Sewers (Scenario 1)													INITIAL	DATE	REV.				
													HSC	3/20/2026	-				
PROJECT :	Lot 4822 in D.D. 104 and Adjoining Government Land, Kam Pok Road, Yuen Long											Roughness Coefficient ks (mm)		1.5					
Manholes		Pipes Details							Pipe Capacity		Check for Capacity						Check for Self-cleansing Velocity		
From	To	Internal Pipe Diameter (mm)	Upper Ground Level (mPD)	Lower Ground Level (mPD)	Upper Invert Level <sup>#</sup> (mPD)	Lower Invert Level <sup>#</sup> (mPD)	Pipe Length (m)	Existing Pipe (Y/N)	Minimum Hydraulic Gradient (1 in _)	Velocity ( m/s )	Capacity ( m3/s )	ADWF(m <sup>3</sup> /s)	Contribution Population	Peaking Factor*	Peak Flow (m <sup>3</sup> /s)	Ratio of the Peak flow to the Full Bore Capacity (%)	Pipe Capacity > Peak Flow?	Velocity in Full Pipe Condition (m/s)	Velocity >1 m/s?
Conversion Chamber	NSWSPS	940.7	4.50	4.20	-2.00	-3.85	740.00	N	400	1.441	1.001	0.231873	74199.37	3.83	0.887183	88.61%	YES	1.441	OK

Remarks

DN 1000 Pipe

**Notes**

\* Peaking factors taken including stormwater allowance

<sup>#</sup> Invert levels of the proposed sewerage system are subject to change during detailed design stage

The contribution population, peaking factors are determined according to GESF published by EPD

Roughness Coefficient ks of 1.5mm is adopted in the assessment.

All proposed sewers will be HDPE pipe

Hydraulic Checking for Gravity Sewers (Scenario 2)																	INITIAL	DATE	REV.
																	HSC	3/20/2026	-
PROJECT :	Lot 4822 in D.D. 104 and Adjoining Government Land, Kam Pok Road, Yuen Long																Roughness Coefficient ks (mm)	1.5	
Manholes		Pipes Details								Pipe Capacity		Check for Capacity						Check for Self-cleansing Velocity	
From	To	Internal Pipe Diameter (mm)	Upper Ground Level (mPD)	Lower Ground Level (mPD)	Upper Invert Level <sup>#</sup> (mPD)	Lower Invert Level <sup>#</sup> (mPD)	Pipe Length (m)	Existing Pipe (Y/N)	Minimum Hydraulic Gradient (1 in _)	Velocity ( m/s)	Capacity ( m3/s)	ADWF(m <sup>3</sup> /s)	Contribution Population	Peaking Factor*	Peak Flow (m <sup>3</sup> /s)	Ratio of the Peak flow to the Full Bore Capacity (%)	Pipe Capacity > Peak Flow?	Velocity in Full Pipe Condition (m/s)	Velocity >1 m/s?
Proposed Sewerage Works by Other Applicant( Including all planned sewage flow as mentioned in this report)																			
Conversion Chamber	WKT009	675	4.50	4.20	-0.78	-1.85	430.00	N	400	1.167	0.835	0.231873	74199.37	3.83	0.887183	106.22%	NO	1.167	OK
WKT009	WKT010	592.6	4.20	4.00	-1.85	-2.11	80.00	N	300	1.241	0.342	0.231873	74199.37	3.83	0.887183	259.23%	NO	1.241	OK
Proposed Sewerage Works by This Applicant (Upgrade to DN1000 Pipe)																			
Conversion Chamber	WKT009	940.7	4.50	4.20	-0.78	-2.56	430.00	N	241	1.858	2.582	0.231873	74199.37	3.83	0.887183	34.36%	YES	1.858	OK
WKT009	WKT010	940.7	4.20	4.00	-2.56	-2.90	80.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT010	WKT011	940.7	4.00	4.15	-2.90	-3.23	80.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT011	WKT012	940.7	4.15	4.20	-3.23	-3.42	45.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT012	WKT013	940.7	4.20	4.15	-3.42	-3.56	35.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT013	WKT014	940.7	4.15	4.10	-3.56	-3.69	30.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT014	WKT015	940.7	4.10	4.05	-3.69	-3.79	25.00	N	241	1.858	1.291	0.231873	74199.37	3.83	0.887183	68.72%	YES	1.858	OK
WKT015	FSH1001886	940.7	4.05	5.35	-3.79	-3.85	15.00	N	245	1.843	1.281	0.231873	74199.37	3.83	0.887183	69.28%	YES	1.843	OK

Remarks

(2xDN675)

Existing Design

Proposed Upgrade to DN 1000 pipe  
Proposed Upgrade to DN 1000 pipe

**Notes**

\* Peaking factors taken including stormwater allowance

<sup>#</sup> Invert levels of the proposed sewerage system are subject to change during detailed design stage

The contribution population, peaking factors are determined according to GESF published by EPD

Roughness Coefficient ks of 1.5mm is adopted in the assessment.

All proposed sewers will be HDPE pipe